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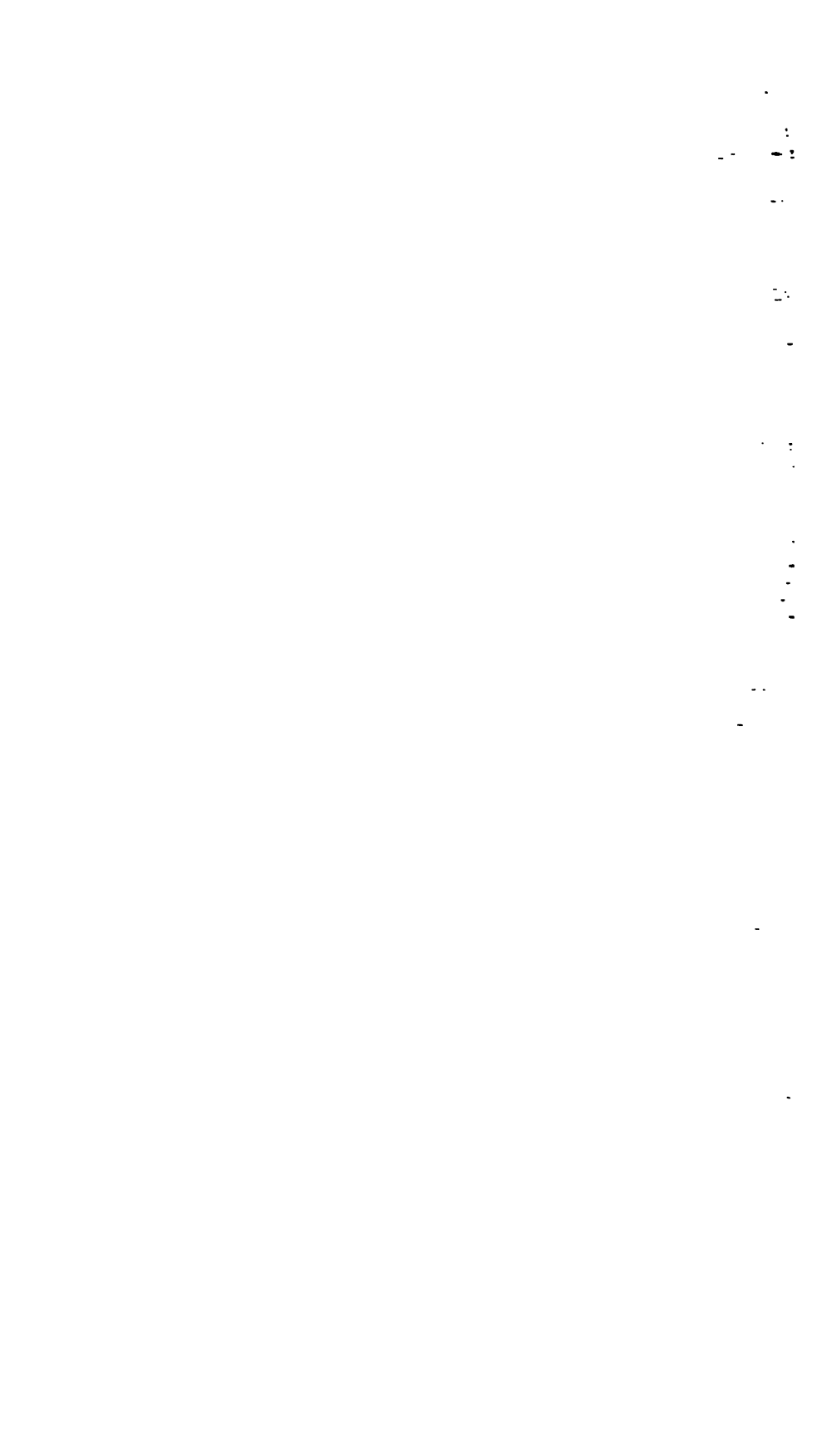
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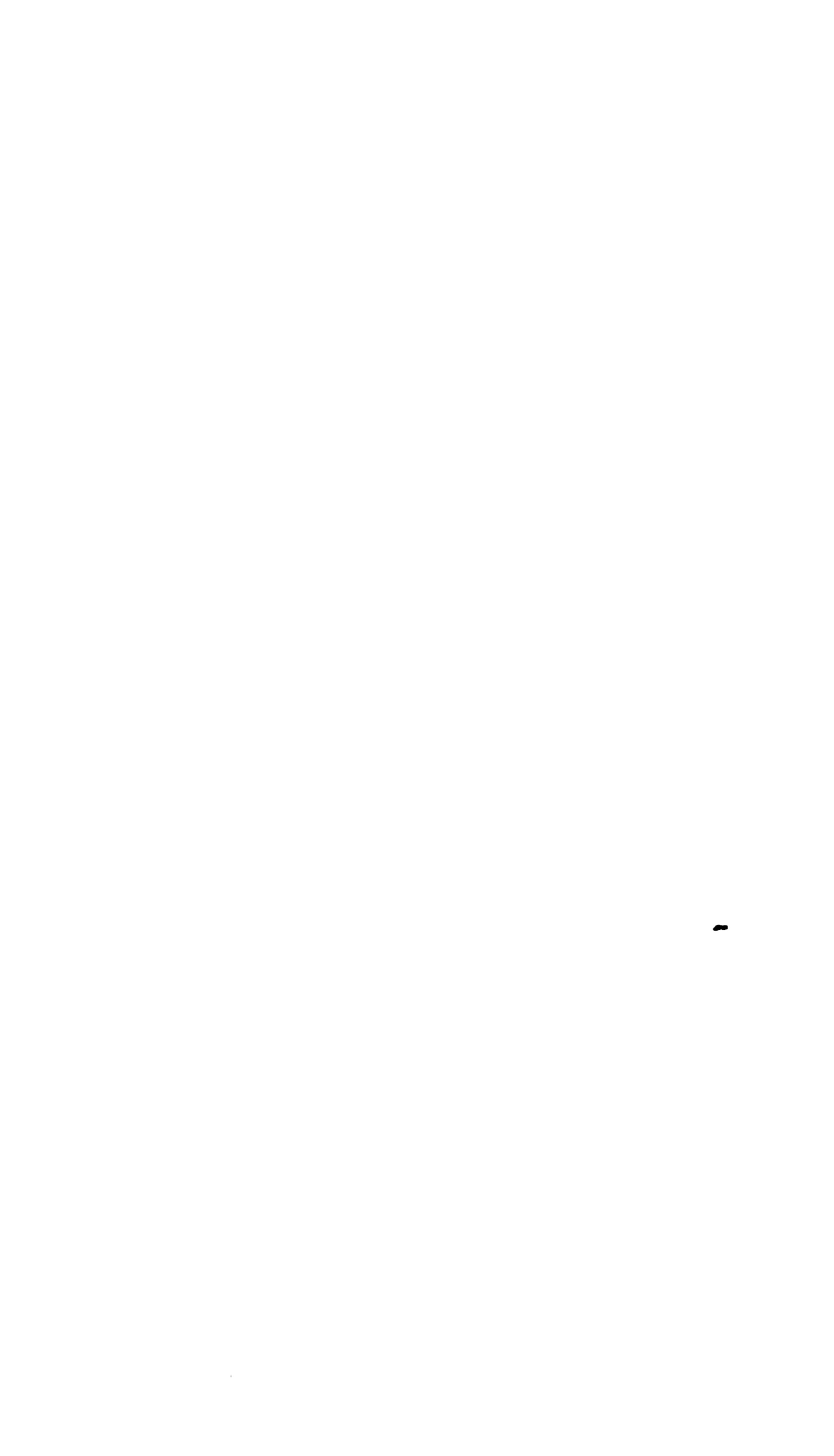
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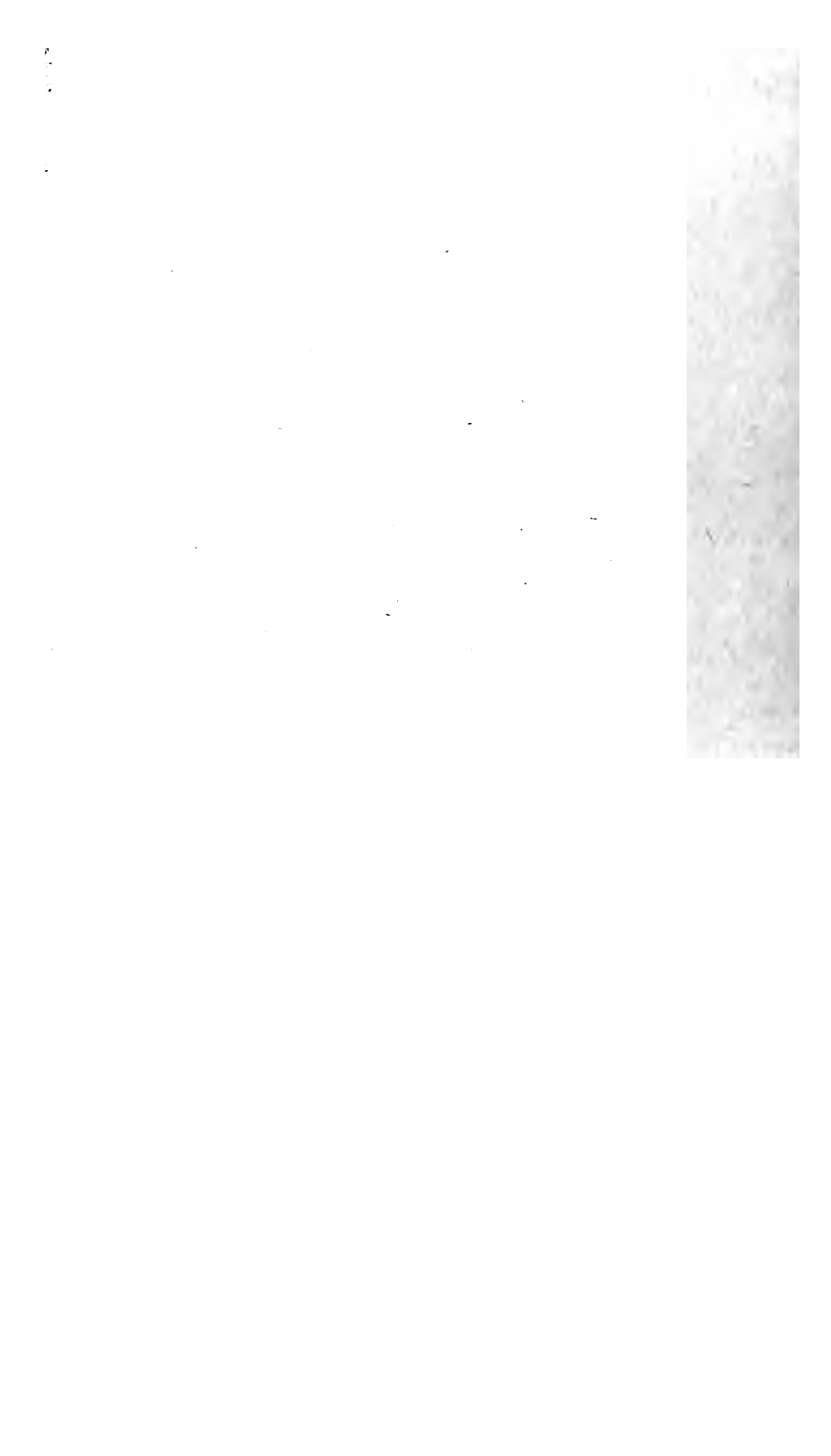
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APPENDIXES

TO THE

REPORT OF THE CHIEF OF ENGINEERS,

UNITED STATES ARMY.

(CONTINUED.)



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APPENDIX D D.

MOVEMENT OF OHIO, MONONGAHELA, CHEAT, ALLEGHENY, AND MUSKINGUM RIVERS.

REPORT OF MAJOR AMOS STICKNEY, CORPS OF ENGINEERS, OFFICER IN CHARGE, FOR THE FISCAL YEAR ENDING JUNE 30, 1892, WITH OTHER DOCUMENTS RELATING TO THE WORKS.

IMPROVEMENTS.

- | | |
|---|---|
| <p>Ohio River.</p> <p>Operating snag boats on Ohio River.</p> <p>Operating and care of Davis Island Dam, Ohio River, near Pittsburg, Pennsylvania.</p> <p>Movable dam in Ohio River below mouth of Beaver River, Pennsylvania.</p> <p>Monongahela River, West Virginia and Pennsylvania.</p> <p>Operating and care of Locks and Dams Nos. 8 and 9, Monongahela River.</p> <p>Purchase of Lock and Dam No. 7, Monongahela River.</p> | <p>8. Purchase of Lock and Dam No. 6, Monongahela River.</p> <p>9. Cheat River, West Virginia.</p> <p>10. Allegheny River, Pennsylvania.</p> <p>11. Dam at Herr Island, Allegheny River, near Pittsburg, Pennsylvania.</p> <p>12. Ice harbor at mouth of Muskingum River, Ohio.</p> <p>13. Operating and care of ice-harbor lock at mouth of Muskingum River, Ohio.</p> <p>14. Muskingum River, Ohio.</p> <p>15. Operating and care of locks and dams on Muskingum River, Ohio.</p> |
|---|---|

UNITED STATES ENGINEER OFFICE,
Cincinnati, Ohio, July 16, 1892.

GENERAL: I have the honor to transmit herewith the annual reports of the works under my charge for the fiscal year ending June 30, 1892.

Very respectfully, your obedient servant,

AMOS STICKNEY,
Major of Engineers.

Brig. Gen. THOMAS L. CASEY,
Chief of Engineers, U. S. A.

1950

1951

1952

1953

1954

1955

1956

1957

1958

1959

1960

1961

1962

1963

1964

1965

1966

1967



plotted, and various incidental expenses which were not counted at the time the allotments were made.

Dredging works were in progress during the year:

Between Davis and Neville islands, 5 miles below Pittsburg.—At the beginning of the fiscal year some minor repairs, consisting of repavement of the slopes of the dam, were being made. These repairs were completed August 15, 1891.

At Marietta Island, 168 miles below Pittsburg.—This dam was completed in December, 1890. During the winter of 1890-'91 some repairs were done to the paving and filling of the downstream slope. At the beginning of the fiscal year this damage was being repaired. The work was done by hired labor and was completed October 21, 1891.

At the foot of Marietta Island, 171 miles below Pittsburg.—Under contract dated February 14, 1891, with J. C. Graham. The object of the work is to hold the water coming down the right-hand chute of Marietta Island and prevent it from spreading until after it has passed Marietta Wharf. It was originally built in 1844, and was repaired and strengthened in 1875. It was in good condition, but owing to the increase in weight given to the dam at the head of the island it was necessary to raise the dike to the same height in order to cut off the cross channel.

Work was commenced July 6, 1891, and completed October 15, 1891.

At the head of Blennerhassett Island, 185 miles below Pittsburg.—Under contract dated February 14, 1891, with Richardson & Monroe. The object of rebuilding this dam (originally constructed in 1844), which had been gradually worn down, was to send more water down the main channel on the Ohio side of the island and to improve the foot. Work was progressing at the beginning of the fiscal year and the dam was completed December 28, 1891.

At Eight Mile Island, 256 miles below Pittsburg.—Under contract dated February 9, 1889, with John J. Shipman. The object of the dike was to open the water on the bar at the foot of Eight Mile Island. At high water the contractor failed to complete the dike within the time specified, and on the recommendation of the officer in charge the contract was extended to December 31, 1891. Work commenced February 15, and the dike was completed November 19, 1891.

At Cullums, 471 miles below Pittsburg.—Under contract dated February 9, 1891, with John J. Shipman. The object of this dike is to deepen and widen the channel by cutting off the narrow and crooked channel down the right-hand shore, and thus compel the river to make a more commodious channel across the present bar. Work on this dike was progressing at the beginning of the fiscal year and was completed December 9, 1891.

At lower bar at Rising Sun, 502 miles below Pittsburg.—Under contract dated February 9, 1891, with John J. Shipman. The object of the work is to improve the lower bar at Rising Sun, which, since the completion of the upper bar, has been the chief sticking place between Cincinnati and Louisville. Work on this dike was progressing at the beginning of the fiscal year and was completed November 25, 1891.

At Madison, Indiana, 552 miles below Pittsburg.—Under contracts dated February 9, 1889, and February 14, 1891, with William Kirk. The construction of this dike was ordered by Congress, and its object is to improve the harbor of Madison by deepening the water on the flat bar that lies along the whole front of the city. Work on this dike was progressing at the beginning of the fiscal year and continued

g. Under contract No. 10, pier 18 located at the mouth of the river, commenced November 1, 1892, when the contractor received 10 per cent of the contract price in advance, and to June 1, 1892, was

ampt.—Three sections of 100 ft. each, for a total length of 300 ft., completed December 1, 1891.

Under contract No. 11, commenced July 6, 1891,

Pittsburg.—The river and subject of this work is to be in great floods, and the river, now forming a bay. The extension of the river, with John J. Johnson, contractor, by this contract, is to be of material put in place by Chief of Engineers, dated November 1, 1891, with the Cleveland, C. & C. Co., for a further extension of funds. Under this contract,

channel a sunken coal flat, which the owners afterwards raised and removed.
Marietta Dike, 171 miles below Pittsburg.—After the dam across the West Virginia
at the head of Marietta Island was rebuilt, it was found that the increased
of water on the Ohio side caused a dangerous cross current over the lower
he riprap dike at the foot of the island, particularly during a rise of the
gum. About 500 feet of the lower end of the old dike was therefore removed
ledges, which at the same time dug off a part of the gravel bar at the dike.
tion made July 17 to 24 and July 27 to 30: loose rock, 4808 cubic yards, gravel,
cubic yards. The dredges also removed 171 piles, 1 coal-boat wreck, and 10
l snags weighing 26.1 tons.

Marietta, Ohio, 170½ miles below Pittsburg.—During a rise of the river, which stopped
at Marietta Dike, the dredges were employed to remove the shore bar at
broad incline just above the Fourth Street Wharf at Marietta. Material
l from this bar was dumped at the foot of the island to stop leaks at the new
in process of construction. Excavation made July 25 to 29: hard clay and
3,333 cubic yards.

Blennerhassett Island, 185 miles below Pittsburg.—July 30 the dredges stopped
traveling to tear up and remove from the channel one wreck of coal barge.

Blennerhassett Island, 188½ miles below Pittsburg.—A bar consisting of mud-
stone and hard gravel, on the Ohio shore above the location of the old dike,
dged off. The removal of this bar, which deflected the current toward the
d below the island, has materially facilitated the passage of tows in both
ns, and by permitting the current to follow the axis of the river will tend
open a good low-water channel. Excavation made July 31 to August 20:
g stone, 5,703 cubic yards; hard gravel, 17,107 cubic yards. There was also
1 from the channel 1 snag weighing 3.1 tons.

Blennerhassett Island, 188½ miles below Pittsburg.—The dredges completed the
l of the old dike on the Ohio shore, on which they had been previously em-
in 1889 and 1890, until stopped by high water. Excavation made July 31 to
3: loose rock 1851.0 cubic yards.

Bar, 212½ miles below Pittsburg.—While traveling the dredges removed from
annel at this place, August 20, 1 snag weighing 12 tons.

St. Ripple, 240½ miles below Pittsburg.—A wrecked coal barge was broken up
scooped from the channel at this place August 21.

Indian Creek, above New Richmond, Ohio, 445½ miles below Pittsburg.—During
ster, which prevented the dredges from working at Nine Mile Bar, they re-
the creek bar at this point, which had been dredged off in 1880, but had again
it into the river. The work straightened the channel and widened it 145 feet.
tion made August 25 to September 16: gravel, etc., 16,119.1 cubic yards;
removed, 1 coal flat; snag removed, 1, weighing 2.5 tons.

Mile Creek Bar, 451½ miles below Pittsburg.—Ninemile and Tennmile creeks, on
side, come together a short distance from the river and formerly ran parallel

APPENDIX D D—REPORT OF MAJOR STICKNEY.

1967

DREDGES OUT OF COMMISSION.

inary	Days.
al repairs	143
al	80
tal	223

COST.

in ordinary	\$2,036.48	
etc	120.00	
total in ordinary		\$2,156.48
during annual repairs	5,002.86	
repairs (shop bills, materials, etc.)	4,639.78	
total cost of repairs		9,642.64
total out of commission		11,799.12
day in ordinary (including superintendent's pay)		15.08

COST OF WORK, INCLUDING ALL EXPENDITURES IN 1891.

g gravel, sand, etc	\$15,324.99
g loose rock, pudding stone, etc	8,757.13
g logs, piles, and snags	875.71
g wrecks	1,313.57
total expenditures in 1891	26,271.40

COST PER UNIT.

c yard of gravel, etc., excavated	\$0.188
c yard of loose rock and pudding stone377
in commission	101.92
of work	291.90

Location and description of work.

Place.	Miles below Pittsburg.	Date, 1891.	Days of work.	Snags and logs removed.	Tons weight.	Piles re- moved.	Wrecks re- moved.
and Lock at lower guiding crib	5½	July	3				
others Islands	160	do	4				1
ot of Marietta Island	171	do	4½				
.....	171	do	4½				
(Ohio) Bar, outside of Fourth Street	170½	do	2	10	26.1	171	1
.....		do	½				1
.....	185	do	½				1
.....	188½	July-Aug ..	24				
.....	188½	do	10	1	3.1		
.....	188½	do	8				
.....	212½	August	1	1	12		
.....	240½	do	1				1
.....	445½	Aug.-Sept ..	13				
.....	445½	do	1	1	2.5		1
.....	451½	Sept.-Oct ..	3	11	9		
.....	452	do	10				
.....	451½	Oct.-Nov ..	12				
.....	451½	do	10				
.....	453	Sept.-Oct ..	3				1
.....	471½	November ..	2				1
.....			90	24	52.7	171	6

1. The first part of the document is a list of names and addresses, which appears to be a directory or a list of subscribers. The names are listed in a column, and the addresses are listed in a column to the right.

Expenditures

Date	Particulars	Amount
1894	Jan 1	100.00
	Feb 1	50.00
	Mar 1	75.00
	Apr 1	60.00
	May 1	80.00
	Jun 1	90.00
	Jul 1	100.00
	Aug 1	110.00
	Sep 1	120.00
	Oct 1	130.00
	Nov 1	140.00
	Dec 1	150.00
	Total	1500.00

The second part of the document is a list of expenditures, which appears to be a ledger or a record of expenses. The entries are listed in a column, and the amounts are listed in a column to the right.

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Illis Island, 40 cubic yards of rock.
 ne Island, 80 cubic yards of rock and some snags.
 bbs Island, wreck of coal boat.

ENCROACHMENTS.

any places along the banks of the river where navigation of encroachments, examinations were made, were stopped from depositing waste material in the firm, who have been engaged in the business of dredging the bed of the river, have been in the habit of leaving the refuse of their digging operations, lying in the vicinity of Brunot Island. In November their operations were so dangerous to navigation that it was necessary to have them stopped. When the case came to trial the members of the court pleaded guilty. The court imposed a fine of \$100 on the principals, and suspended sentence on the others.

Information has been received at various times that certain parties on the Ohio River were dumping material into the river.

Investigations have been made, and wherever they have been made, the river interfered with navigation, or where it has been in the channel of the river, the matter has been referred to the States district attorney for his action.

HARBOR LINES.

Under the provisions of section 12 of the river and harbor act of August 19, 1890, three boards of engineer officers were convened by the Secretary of Engineers, under the authority of the Secretary of War, to command and report upon the subject of harbor lines at Pittsburg, Pa., Wheeling, W. Va., and Cincinnati, Ohio. These boards had meetings and recommended the making of careful surveys and maps of the three harbors, and the officer in charge of the Ohio River improvement recommended that the work be done and paid for from the appropriation for the Ohio River. The field work has been finished and a part of the work on the maps, but this latter work was suspended on account of the lack of funds for completion.

BRIDGES OVER THE OHIO RIVER.

During the past fiscal year this office has had supervision of the construction of the following bridges, viz:

Bridge of Wheeling Bridge Company, at Wheeling, W. Va., 90 miles above Pittsburg. This bridge was completed in October, 1891, in full accordance with the plans approved by the Secretary of War.

Bridge of West Virginia and Ironton Railroad Company, at Kenova, W. Va., 313 miles below Pittsburg. It is believed that this bridge was completed within the last few days, but this office has not been notified of its completion by the builders of the bridge.

Bridge of the Central Bridge Company, between Newport and Cincinnati, 66 miles below Pittsburg. This bridge was completed in November, 1891, in full accordance with the plans approved by the Secretary of War.

1900 REPORT OF THE CHIEF OF ENGINEERS, U. S. ARMY.

STAGES OF THE OHIO RIVER DURING 1891-'92.

The following are the records of the gauges at Pittsburg, Cincinnati and Evansville, which may be taken to represent the navigability of the Upper, Middle, and Lower Ohio.

Group of Davis Island Dam, near Pittsburg.—When the dam is up water readings must be obtained from the gauge at the lower end lock. On this gauge 3 feet 2 inches corresponds to a navigable of 3 feet, and 6 feet corresponds to the same depth in the river.

Davis Island Dam	Depth in channel.			Gauge read	
	Under 2 feet.	3 feet and over.	6 feet and over.	Highest.	Lowest.
1891.					
July	Days	Days	Days	Feet.	
August	0	21	13	11.9	
September	0	21	12	10.7	
October	16	20	4	6.8	
November	20	6	0	3.6	
December	12	14	7	12.8	
	0	21	29	15.10	
1892.					
January	0	21	22	21.4	
February	0	20	17	13.1	
March	0	21	20	14.9	
April	0	20	20	12.7	
May	0	21	20	12.7	
June	0	20	22	15.3	
Total	47	229	224		

Cincinnati gauge.—The zero of this gauge is about 2 feet below water; readings of about 4 feet correspond to about 3 feet in the channel and those of 7 feet to about 6 feet in the channel.

Cincinnati.	Depth in channel.			Gauge read	
	Under 2 feet.	3 feet and over.	6 feet and over.	Highest.	Lowest.
1891.					
July	Days	Days	Days	Feet.	
August	0	21	21	18.2	
September	0	21	21	20.1	
October	0	20	22	17.8	
November	0	21	0	6.2	
December	0	20	17	24.0	
	0	21	21	27.2	
1892.					
January	0	21	21	41.6	
February	0	20	20	29.0	
March	0	21	21	34.5	
April	0	20	20	41.8	
May	0	21	21	32.2	
June	0	20	20	31.8	
Total	4	200	220		

ille gauge.—The zero of this gauge is at the low-water line. of 2 feet correspond to about 3 feet in the channel, and read- feet correspond to about 6 feet in the channel.

Evansville.	Depth in channel.			Gauge readings.	
	Under 3 feet.	3 feet and over.	6 feet and over.	Highest.	Lowest.
1891.	<i>Days.</i>	<i>Days.</i>	<i>Days.</i>	<i>Feet.</i>	<i>Feet.</i>
	0	31	31	16.0	6.8
	0	31	31	9.4	6.6
	0	30	21	13.5	3.9
	0	31	0	3.7	2.1
	0	30	7	15.9	2.0
	0	31	31	18.9	9.3
1892.					
	0	31	31	33.4	12.0
	0	29	20	55.9	12.9
	0	31	31	29.8	17.0
	0	30	30	38.2	20.6
	0	31	31	34.0	16.0
	0	30	30	24.1	12.1
Total.		306	303		

OPERATION OF DRAWS IN HIGH BRIDGES.

ere are no low drawbridges on the Ohio River; the general Ohio r bridge law requires that all bridges shall have a height of at 40 feet above high water, and, since large steamboats require more 40 feet, it is also provided that all bridges below the Cincinnati ension Bridge shall have a draw for use in high water, unless they a clearance of at least 53 feet above high water. There are three -water draws on the Ohio River, but the draw in the Ohio Falls ge, at Louisville, is exceptionally located, and no record of its ation is kept.

ports from the two other draws have been received. Owing to the rable stage of water in the river these draws were not operated ng the fiscal year.

STOPPAGE OF NAVIGATION BY ICE.

he past winter was a very mild one, and assuming, as heretofore, the condition of affairs at Cincinnati is a fair average for the whole r, I have to report that the navigation of the Ohio River was not ted by ice during the past fiscal year.

LOSSES BY COLLISION WITH BRIDGES.

he following tables show the losses sustained by the commerce of Ohio River during the past fiscal year by collision with the piers of ges crossing the Ohio River.

KENOVA BRIDGE.*

Date.	Owner.	Steamboat.	Loss.	Amount.
2, 1892	Previously reported			\$8,700.00
	S. S. Crump & Co	J. C. Risher	4 coal boats	10,000.00
	Total			18,700.00

* Unfinished.

1972 REPORT OF THE CHIEF OF ENGINEERS, U. S. ARMY.

COVINGTON AND CINCINNATI RAILROAD BRIDGE.*

Date.	Owner.	Steamboat.	Loss.	Amount
May 22, 1892	Previously reported W. H. Brown Sons.....	Charles Brown.....	Barge damaged.....	\$43, 15 15
	Total.....			43, 30

HENDERSON BRIDGE.†

Dec. 7, 1891	Previously reported W. H. Brown Sons.....	Alice Brown.....	1 coal boat.....	\$18. 48 2. 08
	Total.....			20, 56

CAIRO BRIDGE.*

Jan. 26, 1892	Previously reported S. S. Crump & Co.....	Smoky City.....	1 coal boat, 1 barge, 1 flat.....	\$9, 38 4, 30
Feb. 18, 1892	W. H. Brown Sons.....	Mariner.....	1 barge.....	2, 45
	Total.....			15, 99

Losses by collision with Ohio River bridges to June 30, 1892.

Beaver Bridge.....	\$56, 040. 00	Covington and Cincinnati railroad Bridge.....	\$13, 300
Steubenville Bridge.....	90, 308. 00	Cincinnati Southern Railroad Bridge.....	9, 812
Wheeling and Martins Ferry Bridge.....	9, 800. 00	Ohio Falls Bridge.....	80, 350
Bellaire Bridge.....	126, 576. 00	Kentucky and Indiana Bridge.....	27, 767.
Parkersburg Bridge.....	72, 677. 00	Cairo Bridge.....	15, 996
Point Pleasant Bridge.....	9, 600. 00	Henderson Bridge.....	20, 520
Kenova Bridge.....	18, 700. 00		
Newport and Cincinnati Rail- road Bridge.....	11, 107. 00	Total.....	625, 553

ESTIMATE.

The following estimate made by my predecessor, Lieut. Col. William E. Merrill, in his last annual report, of the amount of money that should be appropriated to a river of the magnitude of the Ohio and with large and valuable a commerce, is repeated:

Low dams and dikes.....	\$600, 000
Davis Island Dam.....	11, 000
Rock bar at mouth of Locking River.....	20, 000
Dredging.....	20, 000
Minor improvements.....	20, 000
Contingencies.....	35, 000
	706, 000

The \$11,000 set aside for the Davis Island Dam is composed of \$8,000 for the construction of the dam and \$3,000 for the extension of the lock. Experience has shown that the men were required to make the dam was originally estimated for, and it became necessary to increase the estimate for the Davis Island. They were also required to be employed by the laboring force during the construction of these buildings are no longer required. The extension below the lock is urgent and necessary. At several stages of the river the water is so low that the boats are sometimes lost before they can be taken down the river.

Money statement.

1, balance unexpended	\$317,057.10
Property.....	243.71
	<hr/>
892, amount expended during fiscal year.....	317,300.81
	<hr/>
92, balance unexpended.....	58,186.44
92, outstanding liabilities.....	\$8,159.99
92, amount covered by uncompleted contracts.....	16,331.13
	<hr/>
	24,491.12
	<hr/>
92, balance available.....	33,695.32
appropriated by act approved July 13, 1892.....	360,000.00
	<hr/>
available for fiscal year ending June 30, 1893	393,695.32
	<hr/>
that can be profitably expended in fiscal year ending June 30, 1894	500,000.00
needed in compliance with requirements of sections 2 of river and or acts of 1866 and 1867.	

of proposals for riprapping levee at Shawneetown, Ill., opened September 1, 1891.

Name and address of bidder.	Price per cubic yard.	Aggregate.
James Short, St. Charles, Mo.....	\$1.50	\$6,000.00
W. H. Kirk, Madison, Ind.....	1.95	8,970.00
Rocky Stone Ballast and Construction Company, Frankfort, Ky ..	2.15	9,890.00
L. Harsha & Co., Shawneetown, Ill.....	2.48	11,408.00

Contract awarded to James Short and executed under date of September 23, 1891.

of contracts for improving Ohio River made during the fiscal year ending June 30, 1892.

Contractor.	Work of improvement.	Date of contract.	To expire.
Short.....	Riprapping southern levee at Shawneetown, Ill..	Sept. 23, 1891	April 19, 1892

OHIO RIVER COMMERCE.

connection with the commercial statistics, which are given farther on in this report, it is deemed pertinent, interesting, and valuable to give the following extracts from the last annual report of the superintendent of the Cincinnati Chamber of Commerce:

The records for 1889-'90 showed a large gain in river business over previous years for a period, and this increase was maintained during 1890-'91 in both freight and messenger traffic. It is to be noted that this enlargement of volume of river commerce was due more to an increased movement of manufactures than of produce being some deficiencies in crops in the valley region. Southern products, such as sugar, molasses, cotton, etc., contributed a liberal volume of freight.

The revival of river commerce necessitated enlargement of transportation facilities during the year two steamers were rebuilt for the Cincinnati, Portsmouth and Meroy trade, two for the Cincinnati and Louisville Mail Line, and one new steamer built and one purchased for the Memphis and Cincinnati Packet Company; the three lines being practically under one management.

In the additions mentioned, the equipment in these lines was better than previous during a period of six years, embracing seventeen boats, with a capacity of 10,000 tons, and representing about half of the tonnage engaged in the Cincinnati trade. There were daily departures (excepting Sundays) to Pomeroy, Portsmouth, Paducah, and Louisville, and intermediate points, and two departures weekly to Paducah.

1974 REPORT OF THE CHIEF OF ENGINEERS, U. S. ARMY.

"The Pittsburg and Cincinnati Packet Company, with an equipment of six steamers, made daily departures, except Wednesdays. The tonnage in this trade was 3,370 tons, and was fully employed.

"The Southern Transportation line had five steamers in service, with a tonnage of 5,606 tons, making three departures every two weeks. The line had a good business up to the close of the season. The traffic of this line largely represents reshipment of goods from Pittsburg and lower Ohio business; also the movement of sugar, molasses, etc., from the South, these products showing the largest volume in any year since 1882.

"The lines mentioned embrace the most important part of river transportation interests at Cincinnati. The Kanawha Packet Line made weekly trips to Charleston. The Chilo Packet made daily trips. There were tri-weekly departures to Peterburg.

"The year's business has been so satisfactory, following two years of improving returns for river transportation interests, that expectations have been greatly encouraged, and a further revival in river commerce is confidently looked for. The large increase in passenger traffic the past two years has occasioned decided improvements to be made in the building of new steamers for the accommodation and comfort of the traveling public. The equipment in the Cincinnati trade now compares favorably with river navigation elsewhere.

"With 1890-91 there has been a movement of steamers in the Cincinnati trade, which, in respect being quite exceptional. Communication by the largest steamers, with short interruptions in the Pittsburg trade, recorded at Cincinnati during interruptions to steamers from the only slight delays in the upper Cincinnati during the year was 5 few days' interruption to traffic, or loss of river transportation."

Coal shipped from Pittsburg, Pa.

[Furnished by J. M. Evans.]

Months.	To Cincinnati.					To Louisville.				
	Trips.	Coal boats.	Coal barges.	Fuel flats.	Tons.	Trips.	Coal boats.	Coal barges.	Fuel flats.	Tons.
January	7	92	92	43,362	13	63	77	2	60
February	2	2	7	39,873	1	12	29
March	10	121	5	43,552	10	88	23	10	237
April	16	5	294	105,093	18	82	133	9	162
May
June	65	55	669	1	359,547	71	287	623	37	655
July	20	12	233	123,906	21	36	154	14	151
August	12	2	144	1	70,611	12	15	173	7	71
September
October
November	24	32	252	10	153,018	40	314	221	21	321
December	20	50	163	7	126,743	25	149	111	8	195
Total	176	250	1885	26	1,065,755	211	934	1527	108	1,828
Grand total	2,898

Comparative table showing coal shipments from Pittsburg during the eight years ending December 31, 1891.

Year.	To Cincinnati.	To Louisville.	Total.
1884	946,124	1,160,292	2,106,416
1885	1,733,104	1,114,908	2,848,012
1886	1,262,702	2,220,530	3,483,232
1887	788,690	1,367,012	2,155,702
1888	1,950,882	2,223,494	4,174,376
1889	1,149,892	1,439,747	2,589,639
1890	1,324,641	2,095,716	3,420,357
1891	1,065,705	1,828,047	2,893,752

Commerce passing Davis Island Dam in 1891.

Articles.	Ascending.				Descending.				Total freight.
	Through lock.	Freight.	Through pass.	Freight.	Through lock.	Freight.	Through pass.	Freight.	
	No.	Tons.	No.	Tons.	No.	Tons.	No.	Tons.	
.....	141	15,560	415	46,689	140	15,630	423	46,890	124,760
.....	406	1,278	303	1,380	1,200
.....	15	43	3	62	58,900	58,000
.....	102	443	21	16,000	1,398	1,273,976	1,289,976
.....	879	2,800	3,052	8,180	128	68,096	3,660	1,947,120	2,026,176
.....	495	1,300	1,127	4,560	367	55,784	1,600	243,200	304,784
.....	33	54	25	17,300	59	52,109	69,409
.....	50	70
.....	2,071	19,660	6,412	50,340	1,037	172,810	8,656	3,623,395	3,875,205

River commerce of Cincinnati for the year ending August 31, 1891.

Articles.	Re-ceipts.	Ship-ments.	Total.	Articles.	Re-ceipts.	Ship-ments.	Total.
	Tons.	Tons.	Tons.		Tons.	Tons.	Tons.
.....	74	74	Lead:
.....	1,617	1,617	Pig	6	6
.....	2,075	175	2,250	White	295	295
.....	14	14	Leather	299	92	391
.....	36	2	38	Lemons	48	48
.....	133	67	200	Lime	795	730	1,525
.....	10	10	Lumber	6,620	2,129	8,749
shoes	18	500	518	Malt	22	46	68
.....	248	733	981	Manufactures	180	180
B.....	3	3	Merchandise	28,175	24,782	52,957
.....	109	21	130	Molasses	7,207	4,094	11,301
.....	31	31	Nails	33,478	18,908	52,386
.....	1,744	1,924	3,668	Oats	5	320	325
.....	4,395	106	4,501	Oil	1,186	1,290	2,476
.....	8,388	3,490	11,878	Onions	61	158	219
.....	2	66	68	Oranges	3	183	186
.....	410	400	810	Peanuts	1,815	541	2,356
.....	57	1,347	1,404	Petroleum	865	1,412	2,277
.....	674	392	1,066	Potatoes	951	882	1,833
.....	584	972	1,556	Rice	204	138	342
.....	708	711	Rope	879	1,327	2,206
.....	13,345	557	13,902	Rosin	5	99	104
.....	622	588	1,210	Rye	309	62	371
.....	748	29	777	Salt	16,028	8,147	24,175
.....	65	5	70	Seed, clover and timothy	93	867	960
.....	7	944	951	Sheep	1,196	65	1,261
.....	614	3,768	4,382	Shot	256	256
.....	59	144	Soap	23	666	689
.....	85	67	152	Sorghum	987	13	1,000
.....	1,611	85	1,696	Spices	7	7
.....	1,021	1,021	Starch	285	202	487
dow	192	85	277	Stearine	19	6	25
.....	4,354	3,541	7,895	Sugar	1,025	2,693	3,719
.....	299	219	518	Tallow	590	15	605
.....	2,510	2,988	5,498	Tea	8	8
.....	3,046	516	3,562	Tobacco	19,309	2,765	22,074
.....	315	106	421	Turpentine	2	18	20
.....	7,788	58	7,846	Vegetables, green	443	128	571
ref.	16	2,657	2,673	Vinegar	1,260	1,260
.....	8	8	Wheat	4,334	825	5,159
.....	878	668	1,546	Whisky	8,861	6,483	15,344
.....	14,150	10,296	24,446	Wines and liquors	190	366	556
.....	4,372	2,465	6,737	Wool	91	11	152
.....	361	361	Yarn, cotton	446	66	446

Recapitulation.

.....	Tons.
.....	210,956
.....	25,831
.....	336,787

APPENDIX D D—REPORT OF MAJOR STICKNEY. 1975

Commerce passing Davis Island Dam in 1891.

Vessels.	Ascending.				Descending.				Total freight.
	Through lock.	Freight.	Through pass.	Freight.	Through lock.	Freight.	Through pass.	Freight.	
	No.	Tons.	No.	Tons.	No.	Tons.	No.	Tons.	
	141	15,560	415	46,680	140	15,630	423	46,890	124,760
	406		1,278		303		1,380	1,200	1,200
	15		43		3		62	58,900	58,900
	102		443		21	16,000	1,398	1,273,976	1,289,976
	879	2,800	3,052	8,160	128	68,096	3,660	1,947,120	2,026,176
	495	1,300	1,127	4,500	367	55,784	1,600	243,200	304,784
	33		54		25	17,300	59	52,109	69,409
	2,071	10,660	6,412	59,340	1,037	172,810	8,656	3,623,395	3,875,205

River commerce of Cincinnati for the year ending August 31, 1891.

Vessels.	Receipts.	Shipments.	Total.	Articles.	Receipts.	Shipments.	Total.
	Tons.	Tons.	Tons.		Tons.	Tons.	Tons.
	74	74	148	Lead:			
	1,617	1,617	3,234	Pig	6	6	12
	2,075	175	2,250	White	295	295	590
	14	14	28	Leather	299	92	391
	36	2	38	Lemons		48	48
	193	67	260	Lime	795	730	1,525
	10	10	20	Lumber	6,620	2,129	8,749
	18	500	518	Malt	22	46	68
	248	733	981	Manufactures		180	180
	3	3	6	Merchandise	28,175	24,782	52,957
	109	21	130	Molasses	7,207	4,094	11,301
	31	31	62	Nails	33,478	18,908	52,386
	1,744	3,668	5,412	Oats	5	320	325
	4,395	106	4,501	Oil	1,186	1,290	2,476
	8,388	3,490	11,878	Onions	61	158	219
	2	66	68	Oranges	3	189	192
	410	400	810	Peanuts	1,815	541	2,356
	57	1,347	1,404	Petroleum	865	1,412	2,277
	674	392	1,066	Potatoes	951	882	1,833
	584	972	1,556	Rice	204	138	342
	3	708	711	Rope	879	1,327	2,206
	13,345	557	13,902	Rosin	5	99	104
	622	588	1,210	Rye	309	62	371
	748	29	777	Salt	16,028	8,147	24,175
	65	5	70	Seed, clover and timothy	93	867	960
	7	944	951	Sheep	1,196	65	1,261
	614	3,768	4,382	Shot		256	256
	85	59	144	Soap	23	666	689
	1,611	67	1,678	Sorghum	987	13	1,000
	192	85	277	Spices		7	7
	4,354	3,541	7,895	Starch	285	202	487
	299	219	518	Stearine	19	6	25
	2,510	2,988	5,498	Sugar	1,026	2,693	3,719
	3,046	516	3,562	Tallow	590	15	605
	315	106	421	Tea		8	8
	7,788	58	7,846	Tobacco	19,309	2,765	22,074
	16	2,657	2,673	Turpentine	2	18	20
	8	8	16	Vegetables, green	443	128	571
	878	668	1,546	Vinegar	1,269	1,260	2,529
	14,150	10,266	24,416	Wheat	4,334	825	5,159
	4,272	2,465	6,737	Whisky	8,867	6,483	15,344
	361		361	Wines and liquors	190	266	456
				Wool	91	11	102
				Yarn, cotton	446	66	512

Receipts

Tons.
210,956
225,831
336,787

1976 REPORT OF THE CHIEF OF ENGINEERS, U. S. ARMY.

Arrivals and departures of steamboats at the port of Cincinnati, Ohio.

Ports.	1885-'86.		1886-'87.		1887-'88.		1888-'89.		1889-'90.		1890-'
	Arri-vals.	De-part-ures.	Arri-vals.	De-part-ures.	Arri-vals.	De-part-ures.	Arri-vals.	De-part-ures.	Arri-vals.	De-part-ures.	Arri-vals.
New Orleans.....	63	66	34	38	35	37	51	51	38	37	31
Pittsburg.....	103	103	69	70	156	153	237	237	197	194	208
Other ports.....	2,323	2,314	2,169	2,173	1,389	1,385	2,233	2,234	2,022	2,021	1,995
Total.....	2,489	2,483	2,273	2,281	1,580	1,575	2,521	2,522	2,257	2,252	2,234

Schedule of rates on flour per barrel by rail and river from Cincinnati for six years ending August 31, 1891.

Destination.	1886.		1887.		1888.	
	By rail.	By river.	By rail.	By river.	By rail.	By river.
	Cents.	Cents.	Cents.	Cents.	Cents.	Cents.
Pittsburg.....	26 to 31	15 to 20	24 to 31	15	22 to 24	
Louisville.....	15	15	15	15	15	15
New Orleans.....	44	35 to 39	44 to 49	35	40 to 49	

Destination.	1889.		1890.		1891.	
	By rail.	By river.	By rail.	By river.	By rail.	By river.
	Cents.	Cents.	Cents.	Cents.	Cents.	Cents.
Pittsburg.....	23	15	23 to 24	15	24	
Louisville.....	15	15	15	15	15	15
New Orleans.....	54	35	49 to 54	35	49	

Comparative statement of commerce passing the falls of the Ohio River by canal and river.

Years.	Canal.		Open river.		Total.	
	Vessels.	Tons.	Vessels.	Tons.	Vessels.	Tons.
1882.....	3,988	935,628	2,305	608,866	6,293	1,544,494
1883.....	5,231	1,349,115	1,484	483,281	6,715	1,832,396
1884.....	3,485	785,192	1,548	484,124	5,033	1,269,316
1885.....	5,678	1,443,181	822	262,862	6,500	1,706,043
1886.....	4,964	1,240,112	1,796	577,547	6,760	1,817,659
1887.....	4,069	867,807	2,297	1,204,518	6,366	2,072,325
1888.....	6,963	1,915,365	2,252	1,162,305	9,215	3,077,670
1889.....	5,534	1,404,573	1,984	957,829	7,518	2,362,402
1890.....	4,386	998,062	3,828	1,717,158	8,214	2,715,220
1891.....	4,739	1,302,471	2,137	1,062,428	6,876	2,364,899

Commerce of Ohio River in 1891.

Boat or company	Between what points on the Ohio River.	Distance on Ohio River.	Number of months in commission.	Freight.	Number of passengers.
		Miles.		Tons.	
Boats from Pittsburg	Pittsburg to Cairo	965	9	2,893,752	
River Navigation Co	do	965	8	24,104	
and St. Louis Tow- er	do	965	22	166,896	
Waters & Co.	do	965	10	223,740	
Wet Co	Pittsburg to Louisville	599	10	139,106	
Hawk	do	599	6	58,880	
Hudson	Pittsburg to Cincinnati	467	9	30,157	9,157
Keystone State	do	467	9	29,221	8,360
Scotia	do	467	8	14,880	5,920
Andes	do	467	6	13,250	3,382
C. W. Batchelor	do	467	8	14,850	3,300
Congo	do	467	8	17,846	6,282
Louise	Pittsburg to Portsmouth	354	4	1,692	5,842
Lizzie Bay	Pittsburg to Gallipolis	267	10	12,600	5,600
New Courier	Pittsburg to Parkersburg	183	10	2,813	11,025
Ben Hur	do	183	7	7,801	9,511
H. K. Bedford	Pittsburg to Wheeling	90	10 1/2	7,500	4,569
Leni Leoti	Pittsburg to McKees Rocks	34	12	735	79,382
Venice	do	34	10	470	63,192
Venus	do	34	12	820	86,357
Olivette	East Liverpool to Steubenville	23	6	1,900	5,843
R. E. Phillips	Wheeling to Bellaire	4	10	3,000	50,000
Liberty	Wheeling to Clarington	28	12	14,100	8,156
Eliza H.	Wheeling to Parkersburg	93	12	36,608	
Sand and Gravel Co	Wheeling to Cincinnati	376	12	65,500	
T. N. Barnsdall	Raven Rock to Marietta	23	10	2,600	9,600
T. M. Bayne	Marietta to Parkersburg	13	10	2,945	15,560
Rob Ballard	Marietta to Gallipolis	96	3 1/2	2,080	1,325
Matt F. Allen	do	96	1	675	500
Mary E.	Parkersburg and vicinity		3	5,022	
Jessie	Parkersburg to Louisville	415	12	5,760	
Valley Belle	Ravenswood to Middleport	34	8	8,320	6,241
	Ravenswood to Warsaw	304	12	29,021	
Capitol City	Letart to Gallipolis	33	10	9,000	4,500
Coal Co	Syracuse to Cincinnati	224	12	35,947	
Stt & Son	Pomeroy to Cairo	718	10	377,000	
Cincinnati & Louisville line	Pomeroy to Louisville	351	8	160,000	
Freestone Co	Pomeroy to Cincinnati	219	12	20,100	
Coal Co	do	219	12	20,000	
and Pomeroy Packet	do	219	10	132,289	51,329
Illins	do	219	8	133,250	
Bros	Point Pleasant to Louisville	336	10	80,000	
Adwa Towboat Co	Point Pleasant to Cairo	703	6	28,800	
dd & Co	Point Pleasant to Cincinnati	203	12	31,104	
and Ohio Transporta- tion	do	203	10	181,336	
an Towboat Co	do	203	10	287,500	
Henry M. Stanley	do	203	10	15,620	10,200
all	Gallipolis to Louisville	332	12	18,700	
Ida Smith	Crown City to Ironton	37	7	1,120	3,920
B. T. Enos	Proctorville to Portsmouth	53	10	7,500	6,000
F. J. O'Connell	Guyardotte to Coal Grove	18	10	35,000	
Chevalier	Huntington to Ironton	19	12	100	17,306
Coal and Iron Rwy. Co.	Ashland to Portsmouth	33	10	54,640	
Mink No. 2	Ashland to Cincinnati	147	10	10,000	
Reliance	Portsmouth to Rome	27	11	3,000	8,935
Silver Wave	Vanceburg to Maysville	30	11	3,500	16,500
Illins	Manchester to Cincinnati	72	12	65,196	
M. P. Wells	Maysville to Augusta	20	10	1,500	18,000
sp	Charleston Bar to Covington	55	10	14,000	
Tacoma	Chilo to Cincinnati	35	10 1/2	13,800	18,768
Panline	Blairsville to Desiccating Factory	24	12	150,000	
Island Co	Coney Island Landing to Cincinnati	10	3	525	287,521
He & Son	Dayton to Cincinnati	3	8	20,000	
Bellevue	Dayton to Cincinnati	3	12		309,329
Levi J. Workum	Cincinnati to Petersburg	25	12	9,540	886
ding	Cincinnati and vicinity		12	20,418	
Line Co.	Cincinnati to Louisville	132	12	158,510	127,206
R. F. Young	Cincinnati to Paducah	455	1	800	

at to Portsmouth ten months.
 ets from Cincinnati to Madison, one boat from Warsaw to Madison, and two boats from
 to Louisville.

Commerce of Ohio River in 1897.—Continued.

Name of boat or company.	Between what points on the Ohio River.	Ohio River.		Freight.
		Distance in miles.	Number of months in commission.	
Memphis and Cincinnati Pk. Co.	Cincinnati to Cairo	336	10	72,902
Steamboat C. P. Schrock	do	336	10	9,425
Steamboat State of Missouri	do	336	10	11,000
Steamboat State of Kansas	do	336	10	1,900
Steamboat New Wave Houston	do	336	10	14,700
Steamboat Guiding Star	do	336	10	11,750
Steamboat Golden Rule* (estimated).	do	336	10	14,000
Steamboat Falls City	Cincinnati to Louisville	56	12	21,654
Steamboat City of Clarksville	do	56	10	5,220
Steamboat Bellows	Madison to Louisville	47	8	4,500
Louisville and Evansville Mail Co.	Louisville to Henderson	187	12	105,453
Steamboat Jubilee	Cincinnati to Evansville	32	12	6,260
Steamboat Royal	do	40	8	2,878
John Ingle & Co.	do to Evansville	8	12	350,000
Steamboat T. C. Woodward	do	8	10	700
Steamboat Fannie	do	8	1	175
Steamboat Frank Stein	Evansville to Henderson	12	12	2,100
Steamboat John S. Hopkins	Evansville to Smith	107	12	17,000
Steamboat Joe Fowler	do	107	12	18,400
Steamboat W. F. Nibbet	do	107	12	25,500
Steamboat D. A. Brooks	do	100	12	21,216
Steamboat City of Metropolis	Metropolis to Paducah	9	12	3,250
Steamboat Geo. Fowler	do	45	12	67,600
Steamboat Louis Brown	do	45	10	520
Steamboat City of Sheffield	do	47	10	21,670
Steamboat City of Serranah	do	45	12	21,000
Steamboat City of Paducah	do	45	6	10,878
Southern Transportation and Lumber Co.	Brooklyn to Cairo	42	1	62
Sea Lion Tugboat Co.	Caldwellburg to Louisville	284	10	485,150
Total				7,276,516

* Boat fitted with books and papers.

Tonnage from which no report can be obtained (estimated).

Logs, lumber, and railroad ties towed and rafted	1,00
Flatboats: corn, oats, hay, apples, potatoes, cabbage, cider, tan bark, hoop poles, etc	5
Trading boats: glass and stone ware, dry goods, groceries, oil, hides, pelts, oil barrels, scrap iron, old rope, etc	1
Total	1,00

D D 2.

OPERATING SNAG BOATS ON OHIO RIVER.

The river and harbor act of September 19, 1890, contained the following item:

Sec. 13. That for the purpose of securing the uninterrupted work of operating snag boats on the Ohio River, and removing snags, wrecks, and other obstructions said river, the Secretary of War, upon the application of the Chief of Engineers hereby authorized to draw his warrant or requisition from time to time upon the Secretary of the Treasury for such sums as may be necessary to do such work, to exceed in the aggregate for each year the sum of twenty-five thousand dollars: *Provided, however,* That an itemized statement of said expense shall accompany the annual Report of the Chief of Engineers.

the purpose of the above law was frustrated during the latter part of the calendar year 1891 and the first half of the calendar year 1892, to the fact that, in addition to the usual annual repairs, a battery of new boilers and some betterments to the snag boat *E. A. Woodruff* consumed two-thirds of the money allowed for operating expenses of the boat for a whole year.

The above-mentioned work was contracted for in May, 1891, and was not yet completed and paid for out of the funds available for the fiscal year ending June 30, 1891, but in consequence of unavoidable delays in the delivery of material to the contractors, these several jobs were not completed until the latter part of August, 1891, necessitating extra payments for the work which, including pay of crew and amounting to \$73.70, had to be taken from funds available for the fiscal year ending June 30, 1892, which left only \$9,226.30 to pay the current expenses of the snag boat and the outside expenditures for the year.

After deducting the outside expenditures and the cost of taking care of the snag boat during the winter months from the balance on hand, it became evident that the boat could not be kept in commission later than November 30, 1891. The snag boat was therefore ordered put in winter quarters, in the mouth of the Kentucky River, not later than September 1, 1891, where she was compelled to remain until the appropriation for the next fiscal year should become available.

In submitting the annual report of the Ohio River snag boat *E. A. Woodruff*, it has been deemed best to continue the practice of reporting the operations of the boat by calendar years, as by this means the character of each season's work is presented by itself, while if the report were made for the fiscal year it must necessarily include parts of two seasons' work, which is less satisfactory. It is evidently necessary, however, in view of the limit to annual expenditures, to report the latter part of the fiscal years. The present report of operations is for the calendar year 1891, and the report of expenditures covers the fiscal year from July 1, 1891, to June 30, 1892.

The work of the United States snag boat *E. A. Woodruff* has been under the supervision of Capt. W. H. Christian.

OPERATIONS.

(EXTRACT FROM THE REPORT OF CAPTAIN W. H. CHRISTIAN.)

After undergoing her annual repairs at Cincinnati, the United States snag boat *E. A. Woodruff* began her season's work on the Ohio River on the 8th day of September, 1891; she descended the river as far as Cullum Bar and removed part of a steamboat and three coal-barge wrecks and seven snags from the new channel made on the rocky side of the river by the dike recently built at that point. On the 11th of September she ascended the river as far as Wheeling, W. Va., and then, being prevented by low water and a falling river from proceeding to Pittsburg, worked her way down stream to Flint Island, 681 miles below Pittsburg, arriving at that place on November 22. Returning she ran up stream to winter quarters in the mouth of the Kentucky River, where she went out of commission on the 27th of November. During the short working season the *Woodruff* removed 473 snags, 9 steamboat wrecks, 24 coal boats, barges, and flats, 1 brick boat, 1 model barge, 1 ferry-boat, 11 rocks, 1 mass of pudding stone, 3 large pieces of iron wreckage of the Chesapeake and Ohio Railway bridge at Cincinnati. Considerable time was spent in dragging with hook and chain over wrecks nearly covered with earth and in breaking crust of sand and gravel bars to assist scouring. Total distance traveled during the season was 1,396 miles.

Among the most dangerous obstructions removed may be mentioned two large snags at the foot of Eightmile Island; two in front of Proctorville, Ohio; three in front of upper end of Ironton, Ohio; the steamboat *Nail City* in front of Catletts-

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burg, Ky.; abundant stone in front of upper end of channel, Ohio; abundant E. at the foot of Brush Creek Island; one large snag from the new channel; one snag at Flat Island; one large snag from channel at upper end of Greenupburg, Ky., which had sunk two boats and damaged one snag from channel at Mammoth Bar, which sunk abundant boats.

REMOVAL OF SNAGS, WRECK, AND OTHER OBSTRUCTIONS OHIO RIVER.

In addition to the work done by the snag boat E. A. Wood a large number of rocks that obstructed the navigation of the River, near Catlettsburg, Ky., were removed by a gang of men for that purpose.

A steam dredge was also hired, and worked fourteen and a half days at Cullums Ripple, removing obstructions from the channel river at that place.

Detailed statement of expenses for operating snag boats on the Ohio River, and snags, wrecks, and other obstructions in said river.

EXPENSES OF U. S. SNAG BOAT E. A. WOODRUFF.

Month	Ton of coal	Subsistence	Fuel	Months wages supplies	Yards and hire	General supplies	Contingent expenses
June 1901	\$1,174.22	\$1.35	\$21.22	\$21.00		\$22.25	
August 1901	1,427.35	21.00	31.00	21.00	\$22.00	1,022.25	
September 1901	1,072.00	21.00	21.00	1.00		21.25	\$21.25
October 1901	1,021.00	21.00	21.25	21.00	1.00		21.00
November 1901	1,021.00	21.00	21.25	21.00	1.00		21.00
December 1901	1,021.00	21.00	21.25	21.00	1.00		21.00
January 1902	1,021.00	21.00	21.25	21.00	1.00		21.00
February 1902	1,021.00	21.00	21.25	21.00	1.00		21.00
March 1902	1,021.00	21.00	21.25	21.00	1.00		21.00
April 1902	1,021.00	21.00	21.25	21.00	1.00		21.00
May 1902	1,021.00	21.00	21.25	21.00	1.00		21.00
June 1902	1,021.00	21.00	21.25	21.00	1.00		21.00
Total	12,121.22	210.00	210.00	210.00	210.00	1,022.25	121.00
(Other expenditures)							
Removal of snags, wrecks and other obstructions from Ohio River near Catlettsburg, Ky.							571.00
Removal of snags, wrecks and other obstructions from Ohio River at Cullums Bar.							322.00
Removal of obstructions by dredge past during fiscal year							1,287.15
Total expenditures for removal of snags, wrecks, and other obstructions							2,180.15

DD3.

OPERATING AND CARE OF DAVIS ISLAND DAM, OHIO RIVER, PITTSBURGH, PENNSYLVANIA.

This work has been under the immediate supervision of Mr. William H. ...

The following is a statement of the amount and date of all allotments...

...	May 1901	\$2.00
...	June 1901	14.71
...	July 1901	14.81
...	August 1901	10.31
Total		112.41

the close of the last fiscal year the dam was up, having been raised on the 30, 1891. The maneuvers during the year were as follows:

- Lowered the dam, July 8.
- Raised the dam, July 15.
- Lowered the dam, July 24.
- Raised the dam, August 10.
- Lowered the dam, August 24.
- Raised the dam, September 2.
- Lowered the dam, November 24.
- Raised the dam, June 30.

At the end of the year the dam was up.

OPERATIONS DURING THE FISCAL YEAR.

(EXTRACT FROM THE REPORT OF MR. WILLIAM MARTIN.)

At the close of the last fiscal year, in the recess of the lower lock gate, which showed evidence of weakening at the close of the last fiscal year, came off entirely in September, and a month of October, when the stage of water was at its lowest, it was renewed with a new rail of heavier section. To do this required closing the recess by a coffer-dam and pumping out the inclosure.

An old rail fit into a rabbet in a 3½-inch timber which rested on the masonry. This rail, which was three-quarter by 6 inches, came off, the lock gate wheels ran into and through the timber and were soon running on the stone masonry, which was 2½ inches lower than the track level strained the gate and made it run very tight. A new steel rail, 3 by 11 inches, was substituted for the old rail and the 3½-inch timber, and was raised above the rough masonry surface one-quarter of an inch higher, thus bringing the new rail to the proper level.

During the period of putting in the new rail, between the 12th and 19th of October the stage of water was so low that the boats could not run, so no interference with navigation was caused by having the lock closed for the period stated. At 12:50 p. m. on the morning of the 13th of September, the passenger steamer *Courier* de-arrived ran into the lock under full head and, crashing into the lower lock gate, struck the upper chord of the gate truss, the top rail, one of the vertical posts, and large oak timber to which the lock gate chain is attached. The captain and crew of the boat were arrested and the boat libeled. At the hearing before the United States Commissioners the officers of the boat were charged with wilful carelessness, but for lack of sufficient testimony to prove the charge they were dismissed. The libel against the boat has not yet been disposed of.

Service pump.—The contract for building a new service pump to replace the old one, for use in supplying water for the tanks for operating the lock-filling and discharging valves, was let August 8 to The Hall Steam Pump Company, of Allegheny City, Pa. The design of the pump was prepared in this office, and dated May 19, 1891.

The old pump was of the vertical piston class, and was poorly adapted to the work required to perform, having to work against a head of 65 feet. The new pump is of the plunger class, of more substantial build, and better adapted to the work. It is completed but not yet erected, as the high water interfered before the work was completed.

Drift chute.—The drift chute constructed through the lower wall of the lower back access performs a great service in freeing the recess of all drift which is naturally drawn into it by the operation of the lock-discharging valves. By its use the drift is quickly passed below the lock and out of the way. I hope next year to have a similar plan carried out in respect to the recess of the upper gate, as at the latter place the greatest annoyance is experienced from this cause.

Stone in land wall inclosure.—About 25 cubic yards of large stone, left over from the construction of the bear-trap piers and which had been stored in the land lock inclosure, were removed and deposited in the river bed below the bear trap, adding to the protection of the river bed at this point.

Lowerable dam.—On the evening of the 8th of August, 1891, we began to raise the dam. Fourteen wickets next to the river wall were raised, when operations were suspended until the next morning. On the outer wicket a red signal light was placed to warn navigators. The towboat *Nellie Walton*, ascending at 10:15 p. m., with a tow of empty barges, ran into these wickets and knocked four of them down. Examination showed that four wicket boxes, three cross heads, three quoins, and six wicket-box bolts were broken and the horses badly bent. The excuse the officers of the boat gave was that they did not see the signal light. This accident delayed raising of the dam, as the repairs of the damage done required two days' time.

The following information is being furnished to you for your information only. It is not intended to constitute an offer of insurance or any other financial product. The information is provided for your general information only and should not be relied upon as a basis for any investment decision. The information is provided for your general information only and should not be relied upon as a basis for any investment decision.

Item	Amount
1. Cash	100.00
2. Bonds	200.00
3. Stocks	300.00
4. Other	400.00
Total	1000.00

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In October the river was at a very low stage and many of the levels of the bed were retaken on dry land, the stage of the water permitting the wading river. These levels being very accurate were substituted for the soundings sent by the map of March 24 above mentioned.

The land necessary for the construction of the lock and abutment was obtained by purchase after condemnation. The tract on the north side of the river, construction of the lock and its approaches, containing six acres, was purchased from Mr. James McTaggart; that on the south side, containing 3.46 acres, belonged to the Beaver County Infirmary, and was controlled by a board of directors. A board was created by act of assembly of March 29, 1851, and by its terms authority is given the board to sell any of the property acquired for the purposes of the act. Accordingly a board of appraisers was appointed by the court for the purpose of condemning the necessary land. By the appraisers report \$166 2/3 per acre was agreed upon, being the same price per acre as paid to Mr. McTaggart for the north side of the river. At this price the total cost to the United States land needed for the lock and dam was \$1,576.67. This has been paid and invested in the United States.

The reference of the sill of the Beaver Dam, as fixed by the diagram of the of the Ohio River from Davis to Montgomerys Island, dated February 24, 1855, 871 feet above mean tide.

Borings to determine the character of the foundation were made on the site location. No rock was encountered except at a few points, the strata in all being from two to three feet in thickness. The material passed through consisted of clay, gravel, and sand, sandstone, a dark shale resembling coal, and some indications of quicksand.

The preliminary work of construction was begun with a small force on the June, 1892, by clearing and cross-sectioning the land of the site and erecting gauges for hydrographic purposes.

It is expected that the construction of the lock will be well advanced during the present season.

Money statement.

July 1, 1891, balance unexpended	\$248
June 30, 1892, amount expended during fiscal year.....	5
July 1, 1892, balance unexpended	243
July 1, 1892, outstanding liabilities	
July 1, 1892, balance available.....	243
Amount appropriated by act approved July 13, 1892.....	100
Amount available for fiscal year ending June 30, 1893	343
{ Amount (estimated) required for completion of existing project	550
{ Amount that can be profitably expended in fiscal year ending June 30, 1894	250
{ Submitted in compliance with requirements of sections 2 of river and harbor acts of 1866 and 1867.	

D D 5.

IMPROVEMENT OF MONONGAHELA RIVER, WEST VIRGINIA AND PENNSYLVANIA.

Statement of the amount and date of all appropriations.

June 10, 1872	\$25,000	March 3, 1881	
March 3, 1873	66,000	August 2, 1882	
June 23, 1874	25,000	July 5, 1884	
March 3, 1875	22,000	August 5, 1886	
June 18, 1878	25,000	August 11, 1888	
March 3, 1879	24,000		
June 14, 1880	25,000	Total	

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The following table shows the commerce that passed through United States No. 9 during 1891:

Month.	Ascending.		Descending.	
	Passengers.	Freight.	Passengers.	Freight.
January.....	368	Tons. 451.70	320	370
February.....	363	235.12	307	340
March.....	393	379.30	400	349
April.....	357	397.16	393	393
May.....	431	397.60	600	600
June.....	618	525.25	1,361	1,733
July.....	1,599	690.85	863	541
August.....	1,991	474.00	427	359
September.....	890	458.20		
October.....	600	706.95		
November.....	473	488.40		
December.....	389	388.80		
Total.....	8,477	5,593.33	7,662	9,600

Taking the average of the totals given in the above tables we find that the commerce during 1891 on the 14 miles of the Monongahela River which is controlled by the United States is as follows:

Tons of freight..... 11
 Passengers..... 11

The following table compiled from the annual reports of the Monongahela Navigation Company shows the commercial movement on the lower part of this river during the past eight years.

Articles.	1884.	1885.	1886.	1887.	1888.	1889.	1890.
	Tons.	Tons.	Tons.	Tons.	Tons.	Tons.	Tons.
Brick.....	1,467	1,325	8,907	5,279	4,504	6,333	7,856
Cattle and horses..	108	374	386	501	565	729	648
Classified freight..	13,890	13,683	13,086	16,486	17,450	18,334	23,169
Coal and coke.....	3,104,800	3,265,078	4,297,768	2,998,690	4,400,996	3,084,175	4,486,322
Fire clay.....	2,140	3,463	3,295	2,383	3,476	5,982	5,021
Flour.....	254	198	83	259	379	266	340
Iron.....							
Ore.....	20,840	16,486	17,823	17,683	8,079	3,371	2,335
In pigs.....	17	11	5	7	10	15	5
Lumber.....	10,714	9,622	10,231	11,144	13,778	10,095	11,615
Oil.....	9	1,073		55			
Pipe.....		595		2,285			2,394
Posts.....	5	256	437	258	489	700	1,089
Posts pit.....	35,072	32,166	18,392	30,472	22,432	18,797	29,336
Railroad ties.....	1,256	135	161	30	555	3,165	232
Sand.....	37,345	51,771	59,229	89,230	63,065	73,590	105,600
Sheep.....	170	194	108	233	193	164	60
Staves.....		9	9		7	26	140
Steel rails.....	23,844	22,428	45,795	57,490	10,688	32,177	34,609
Stone.....	56,116	7,612	4,621	910	3,636	9,589	811
Timber.....	47,064	46,172	52,608	53,561	84,191	54,696	43,970
Whisky.....	254	198	764	49	88	134	180
Wood.....		230	196		184		58
Total.....	3,855,425	3,472,659	4,533,484	3,287,005	4,634,785	3,322,239	4,755,790
Passengers.....	26,871	24,608	26,885	50,584	44,714	45,811	31,828

D D 6.

OPERATING AND CARE OF LOCKS AND DAMS NOS. 8 AND 9, MONONGAHELA RIVER.

The Monongahela River, from Morgantown, W. Va., to its mouth at Pittsburgh, is canalized by nine locks and dams, of which the first six counting from Pittsburgh, are the property of the Monongahela Navigation Company.

company, and the eighth and ninth are the property of the United

The eighth dam is 88.15 miles above Pittsburg, and Morganhead the head of the slackwater, is 102.16 miles from the same. It therefore, be stated that 14 miles of this river is under the direct control of the United States and 88 miles is under a private corpora-

following is a statement of date and amount of all allotments for such work:

May 5, 1884	\$1,000
September 1, 1884	4,000
September 3, 1884	4,500
September 20, 1884	500
1885	100
1885	1,000
1886	1,200
1887	1,100
1887	11,000
1888	6,270
September 27, 1888	3,000
1889	1,480
September 7, 1889	1,500
September 8, 1889	1,200
1890	12,300
1891	24,250
September 25, 1889, from appropriation for examinations, surveys, and contingencies of rivers and harbors	4,000
	78,400

The only stoppage to navigation throughout the year was on account of high water during a few days in midwinter.

All work has been under the immediate supervision of Mr. Philip G. Stickney, assistant engineer, and the information relating to the operations of the year is contained in his report.

OPERATIONS DURING THE FISCAL YEAR.

(EXTRACT FROM REPORT OF MR. PHILIP GOLAY.)

In the spring of 1890 the aprons of both dams were injured by floods, but on account of continued high water it was impossible to repair them during that year. The past year has been a little more favorable for such work, and repairs were made on both dams.

LOCK AND DAM NO. 8, MOUTH OF DUNKARD CREEK.

Repair of dam.—The injury sustained by Dam No. 8 was confined to the middle section. Two sections of the dam, one 64 feet long next to the lock, and one 58 feet long next to the abutment, showed no signs of injury. On the intervening section, however, the damage, beginning with the lifting of a small portion of the apron sheeting, increased with subsequent rises of the river until it extended across the entire length of the dam, the sheeting being separated from the purlins except the one on the up-side which was held in place by the sheet piling spiked to it. Also a few sections of the apron substructure were carried away. Additional anchorage was given to the toe of the apron of the uninjured parts; the apron sheeting of the middle part was removed and the apron repaired to a height of 20 inches higher. The apron sheeting was then replaced, and, together with the adjacent course of sheeting, was securely bolted down. On account of the short duration of the low-water season and its short duration, further repairs could not be made; and the work of replacing and bolting down the upper courses of sheeting and purlins beneath was postponed until a more favorable time. The face of the dam was thus left, and still remains, elevated to such a height as to enable Dam No. 8, during low water, to stand about 1 foot above normal pool level. In order to make close repairs, and as a protection to the apron of the dam against the reverse current, 976 cubic yards of riprap stone were piled against the lower face of the dam up to the sheeting of the apron. Upon recent examination, these stones

persons encroaching on the river have had the effect of causing almost a suspension of encroachments in general. Notwithstanding this, however, a vigilance is necessary in order to maintain a permanent obedience to the

Estimate for fiscal year ending June 30, 1893.

ing spoil bank to destroy a dangerous eddy and to back Dam No. 9.....	
g material below lock to back Dam No. 8.....	
and wages.....	
g expenses.....	
el, light, and stationery.....	
g snags and other obstructions.....	
s 8, repairs, etc.:	
res in turbine races.....	
stering turbine races.....	
k protection below lock.....	
uilding storage shed.....	
iting lock gates and buildings.....	
eral repairs to operating machinery.....	
s 9, repairs, etc.:	
uilding lock-keeper's dwellings, carried away by flood, 1888.....	
on apron of dam.....	
sting guide wall.....	
ing dam.....	
uilding storage shed.....	
tingencies.....	
total.....	

*statement of expenses incurred in operating and care of lo
Monongahela River during the fiscal year ending June 30*

Operating expenses.				Repairs.				Tools and appliances.	Grand total.
Salaries.	Supplies.	Miscellaneous expenses.	Total.	Hire of boats and barges.	Labor.	Material.	Total.		
\$565.04		\$11.00	\$576.04			\$488.91	\$488.91		\$1,064.95
622.91	\$21.00	84.80	728.71	\$416.00	\$538.67	595.34	1,550.01	\$858.42	3,137.14
540.00	31.65	31.40	603.05	410.00	1,830.42	1,279.51	3,519.93	113.60	4,230.58
395.00	13.17	20.89	429.06	322.00	2,881.57	2,649.98	5,853.55	174.77	6,457.38
552.95		32.45	585.40		333.34	1,484.34	1,817.68	29.98	2,433.06
488.50			488.50						488.50
496.15		8.00	504.15						504.15
486.89		28.52	515.41						515.41
415.00	1.55	16.00	432.55	10.00			10.00		442.55
488.50	2.10	17.94	507.64			.47	.47		508.11
673.90	4.78		678.68			45.62	45.62	11.00	735.30
723.78	1.60		725.38			251.21	251.21	2.90	979.49
6,448.53	75.85	249.90	6,774.28	1,158.00	5,584.00	6,795.38	13,537.38	1,190.67	21,502.33

Commercial statistics are attached to the report for the improvement of Monongahela River.

D D 7.

PURCHASE OF LOCK AND DAM NO. 7, MONONGAHELA RIVER.

close of the last fiscal year the case of the United States vs. Monongahela Navigation Company, in the matter of the condemnation of Dam No. 7, was set to be heard on appeal at the National Circuit court of the United States for the western district of Virginia.

Money statement.

91, balance unexpended	\$167,000.00
92, balance unexpended	167,000.00

D D 9.

IMPROVEMENT OF CHEAT RIVER, WEST VIRGINIA.

Act of September 19, 1890, contained an appropriation of \$13,000 for the improvement of Cheat River. This is the largest appropriation ever made for this river.

The object of the improvement was to make this river navigable for rafts, so as to bring out the immense supplies of timber found in the forests on the Cheat River and its tributaries. In its natural state it was impossible to bring down rafts, and a large percentage of these logs was stranded among the rocks and left there to decay. A large part of the river commences about 3 miles below Rowlesburg and terminates at Beaver Hole, 16 miles above the mouth, where it emerges from Laurel Ridge. The total length of difficult navigation is 35 miles.

The original project for the improvement of the river was the removal of the rock obstructions as interfered with the free passage of logs on the upper part of the river. At the suggestion of lumbermen and other parties interested in the improvement of this river, an additional project was adopted of making a low-water channel from near Rowlesburg to the river in which logs could be floated on a rise of from 2 to 3

feet. This work has been under the immediate supervision of Mr. Philip G. Assistant Engineer.

REPORT OF OPERATIONS FOR THE FISCAL YEAR.

(EXTRACT FROM REPORT OF MR. PHILIP GOLAY.)

Work was begun in November, 1890, suspended in January, 1891, on account of a heavy snow, resumed in the following April, and closed January 9, 1892, at which time the amount of the appropriation (\$13,000) was found to be exhausted.

On the latter part of September, 1891, the work was prosecuted with a view to the removal of such rock obstructions as interfered with the passage of loose logs when the water was from 6 to 8 feet occur. To this end 10,058 cubic yards of rock obstruction were removed and the river was put in fair condition as far down as Green Island, 20½ miles below Rowlesburg.

In the month of September, 1891, lumbermen urged that a low-water channel be made. About this time, also, Col. Merrill paid a visit to the Cheat River and Morgantown office, and after discussing the subject with the parties interested in the improvement of Cheat River, he directed that the plan suggested by the lumbermen be adopted. Accordingly about the latter part of September, a force of workmen with tools and material were sent again to the upper end of the river, about 2 miles below Rowlesburg, and working in the low-water channel 3,789 cubic yards of rock, clearing the channel so that logs can be driven on from 2 to 3 feet rises down to a point 8 miles below Rowlesburg. The total number of cubic yards of rock obstruction removed, or leveled down, is 13,847.

An additional sum could be advantageously expended in removing more rock obstructions, about 3,000 cubic yards between Pringle Run and Green Island, mostly in low-water, and 7,000 cubic yards of channel and shore rocks, between Green Island and the mouth, making a total of 10,000 cubic yards of rock obstruction yet to be blasted. Guide cribs should also be constructed at the heads of

REPORT OF THE BOARD OF ENGINEERS, U. S. ARMY.

For the purpose of the building of these guide locks the additional sum of \$20,000 is needed to purchase the West River property was transferred to the West River storage room.

Money Statement.

.....	\$7,000
.....	7,000
.....	20,000
.....	20,000

Statement of Receipts and Disbursements for the year ending December 31, 1891.

Date	Received by Mills at Mouth of River	Passed on to points below.	Total
.....	142.78
.....	305.86
.....	180.28
.....	1,846.94
.....	1,509.17
.....	2,011.94
.....	1,204.17
.....	1,873.00
.....	1,247.08
.....	665.51	12,000

STATE OF PENNSYLVANIA.

Statement of Receipts and Disbursements for the year ending December 31, 1891.

.....	\$30,000
.....	25,000
.....	20,000
.....	180,000

..... this river was the result of the construction of low water locks. In the contract the water was to be raised on this project.

..... year \$18,599.88
 Mr. J. W.

APPENDIX.

..... improve
 at
 supply was

LOW DAMS AND DIKES.

chute at Corydon, Pa., 209 miles above Pittsburg.—During the past winter an ice damaged part of the mill dam across the river at Corydon, thereby injuring the sheeting of the log chute, which is built at the lower side of it. It is estimated that the repairs can readily be made at a cost not exceeding \$70.

at Cornplanter Island, 204 miles above Pittsburg.—This dam, which at the close of the last fiscal year had been nearly finished, was completed. Its length is 334 feet, at base 18 feet, and height above low water 3 feet. An abutment for the protection of the roots of the dam was built at each end to the height of the bank. The total quantities of material used in the construction of the dam and abutments are as follows:

umber.....	feet, B. M.	34, 636
ick lumber.....	do.....	23, 632
olts.....	pounds..	2, 991
.....	cubic yards..	895
tion made.....	do.....	240

The cost of labor and materials in the entire work was \$2,503.05.

The dam was designed to confine the water theretofore wasting down the island thereby reducing the navigable depth of the water on the very shallow bar opposite to the channel on the right. No actual soundings were made, but the observations and experience of timber and lumbermen show beyond a doubt that there has been an increase of more than a foot in the depth of the water at a 3-foot stage. The reports of the lumbermen speak in terms of highest praise of this improvement, and point to the fact that since its completion the shipments from above that point have doubled. The dam at present is in good condition.

at Hickory, 157 miles above Pittsburg.—The dam designed to improve Hickory was originally located across the left or main channel, at the head of Greens Island, the intention having been to make the right chute the channel for navigation.

When everything was in readiness for commencing its construction serious objections were offered by some local timber men, who claimed that the erection of a dam at the left chute at the head of Greens Island would greatly impair the usefulness of their timber landings on that side of the river opposite the foot of the island. To obviate the possibility of damaging the landings a survey above and below the head of Greens Island was made, and the location of the dam changed to a point about a mile farther down, and so as to cross the right chute at the head of the lower bar.

The dam here was designed to confine the water below a 3-foot stage, about 200 yards of which drifted into the right chute at this point, into the left or main channel, and to cut out the channel bar opposite. Work was commenced on the construction about August 1, and the structure completed on October 14. The dam is built on a sharp curve, running from the right bank around the head of the island.

Its length is 730 feet, width at base 18 feet, and height from 6 to 7 feet; it is built on a 3-foot stage of water. An abutment 40 feet long at the root and one at the head of the island 50 feet long were built, and the outer side of the island protected with heavy riprap stone. The structure is reported as being in good condition.

In the construction of this work the following quantities of material were expended,

umber.....	feet, B. M.	68, 526
ick lumber.....	do.....	56, 684
olts.....	pounds..	8, 022
.....	do.....	285
tion made.....	cubic yards..	2, 170
.....	do.....	180

The cost of material and labor was \$5,623.49.

This improvement also has been productive of excellent results, and whereas formerly the difficulty of navigation had always been dreaded on account of its shallowness, caused by the ice spreading over so much territory, now, according to the reports of navigators, the difficulties and dangers heretofore existing have entirely disappeared.

at Pithole, 143 miles above Pittsburg.—The construction of this dam had been commenced at the close of the last fiscal year. Under favorable conditions the work proceeded along rapidly and the structure finished in September. It extends from the left bank down to the high bar, formerly Pithole Island, a distance of 1,132 feet. The dam at base is 18 feet, and its height the level of a 3-foot stage. An abutment 40 feet long and 10 feet high was built at the root to protect the bank.

The object of this improvement was to stop the waste of water down the left bank which almost equaled the flow down the right, and thus increase the navigable depth of the water in the channel. This has been accomplished, fully a foot

in depth having been added to the channel at a 3-foot stage. No damage has occurred to this work since its completion.

The following is a statement of the quantities of material expended in the construction of the dam and abutment:

Oak lumber	feet, B. M.	104,554
Hemlock lumber	do	84,090
Drift bolts	pounds	11,300
Spikes	do	325
Stone	cubic yards	2,490
Excavation made	do	860

The cost of materials and labor was \$8,075.76.

Dike at Red Bank, 64 miles above Pittsburg.—Although one of the most extensive ice gorges seen in many years in the Allegheny swept over this dike in February of the past winter, yet the damage to it is slight, not exceeding the displacement of a few squares of paving. It is estimated that \$40 will make the repairs.

Dam at Nicholson Island, 37 miles above Pittsburg.—This work was put in good repair in the season of 1891 by replacing about a square of paving which had washed out. During the past winter the Parker ice gorge damaged some of the paving. The necessary repairs, it is estimated, can be made for \$30.

Dam at Sixmile Island.—This structure has remained in good condition.

REMOVAL OF OBSTRUCTIONS.

Removal of bar at foot of Pithole Ripple.—This work was commenced but not carried to completion, owing to the lateness of the season; 500 cubic yards of rock were removed.

Removal of rock and other obstructions below Kittanning.—A number of dangerous rocks and snags had made their appearance on the lower portion of the river, and in October a small force was put to work removing them. The party operated from Kittanning to Pittsburg, a distance of 45 miles, making the following removals of obstructions, viz:

Locality.	Rock.	Snags.
	<i>Cu. yds.</i>	
Kittanning	40	
Cogleys Island	50	
Head of Ross Island	125	3
Nicholsons Island	205	
White Rock	10	
Murphys Island		6
Tarentum Island	50	
Bull Creek	75	1
Logans Landing	6	
Fourteenmile Island	60	1
Total	621	11

ENCROACHMENTS.

At the close of the last fiscal year most of the leading parties encroaching on the navigable capacity of the river had been restrained, either by injunction, or by stipulations in which they agreed to stop depositing waste material over the banks, pending the establishment of harbor lines. During the year, however, navigators had frequent occasion to report parties engaged in filling material into the stream. These cases were promptly investigated, and the guilty parties were notified to stop their unlawful practices, which in every case they did, thus obviating the necessity of taking legal action.

Navigators have heretofore frequently experienced great inconvenience on account of huge piles of coarse gravel formed in the harbor by parties digging sand from the river bed, and last season the operations of one company became so objectionable that an information was made against them and their employés. At the hearing they were indicted and held for trial at court. Subsequently they removed the piles of gravel. Members and employés of a dredge company were prosecuted for obstructing the channel at the Union Bridge. After being indicted they removed all the material put in. These cases have not yet been tried.

The removal of large quantities of rocks, snags, and other obstructions from the bed of the river, and the erection of low dams and dikes, shutting off duplicate chutes and contracting the channel where spread over large areas, have resulted in great good to the general trade, and much more work of the same nature remains to be

done for the relief and safety of navigation. The most important improvement that can be made on the Allegheny, however, and which should receive serious consideration, is the extension of slack water, at least as far up as Soda Works, 20 miles above Pittsburg. This part of the Allegheny Valley is rapidly building up with mills and industrial establishments of other kinds, which demand and deserve facilities for cheap and permanent transportation.

- Money statement.

July 1, 1891, balance unexpended	\$18,589.88
June 30, 1892, amount expended during fiscal year.....	16,977.73
July 1, 1892, balance unexpended	1,632.15
July 1, 1892, outstanding liabilities	163.00
July 1, 1892, balance available.....	1,459.15
Amount appropriated by act approved July 13, 1892.....	25,000.00
Amount available for fiscal year ending June 30, 1893	26,459.15
(Amount that can be profitably expended in fiscal year ending June 30, 1894 (Submitted in compliance with requirements of sections 2 of river and (harbor acts of 1866 and 1867.	25,000.00

COMMERCE.

It has never been possible to procure complete statistics of the commerce of the Allegheny River, owing to its scattered condition and the fact that there is no record of it kept anywhere. It is, therefore, impossible to state whether there has or has not been an increase in the tonnage over that of the previous year. The local harbor tonnage exceeds that of 1890 by 361,023 tons, most of which is attributable to an increase in the trade.

The following is a statement of the principal commercial movements on the river during the calendar year 1891:

Articles.	Quantity.	Articles.	Quantity.
	<i>Tons.</i>		<i>Tons.</i>
Barges.....	2,700	Lath.....	286
Bark.....	2,624	Manure.....	800
Barrels.....	900	Nitroglycerin.....	12
Boat bottoms.....	29,412	Piles.....	1,896
Brass poles.....	813	Pit posts.....	6,223
Cattle and horses.....	926	Rough lumber.....	105,959
Chick posts.....	1,510	Sand.....	84,439
Coal.....	1,400	Sheep.....	30
Curbstone.....	406	Shingles.....	270
Fire clay.....	2,700	Stone.....	36,947
General freight.....	2,625	Timber.....	105,245
Gravel.....	12,453	Total.....	400,726
Hay.....	150		

Passengers carried, 5,650.

In addition to the above, the following is a statement of the principal commercial movements in the Allegheny Harbor at Pittsburg:

Articles.	Quantity.	Articles.	Quantity.
	<i>Tons.</i>		<i>Tons.</i>
Cars, loaded.....	862,342	Oil.....	11,040
Cars, empty.....	77,916	Railroad ties.....	14,825
Chimney.....	1,500	Sand.....	172,040
Coal.....	189,282	Staves.....	9,378
Gas pipe.....	5,610	Vegetables.....	360
Gravel.....	33,845	Total.....	896,005
Fire brick.....	1,059		
Manure.....	16,870		

Passengers carried, 59,000.

D D II.

DAM AT HERR ISLAND, ALLEGHENY RIVER, NEAR PITTSBURG, PENNSYLVANIA.

The following is a statement of the amount and date of all appropriations for this work:

August 5, 1888.....	\$37, 50
August 11, 1888.....	35, 00
September 19, 1890.....	35, 00
Total.....	107, 50

The land required on both banks of the river was purchased. The authorities of Allegheny City, to whom the land on the right bank belonged, sold it for the nominal consideration of \$1, on condition that the dam should be a movable one. A petition to the same effect was likewise received from the authorities of the city of Pittsburg, from the Pittsburg Chamber of Commerce, and from the Engineers' Society of Western Pennsylvania. In all these petitions the reason alleged for desiring that the dam be a movable one was the fear that the height of floods in the Allegheny River would be increased by a fixed dam.

The question of the proposed change was referred to the Secretary of War by the Chief of Engineers, and on September 29, 1890, it was ordered that the dam at Herr Island be made a movable one. This change in design necessitated a corresponding change in the estimated cost of the work; the estimate for the present design is \$600,000.

It was supposed that after this action there would be no further cause for delay, but a number of riparian owners on the left or Pittsburg side of the river, to whom no allowance for damages had been made, for the reason that the proposed lock was to be outside of the high-water line, brought suit in the circuit court of the United States for compensation. The case was placed in the hands of the United States attorney for the western district of Pennsylvania, where it still remains.

The Davis Island Dam pool gives permanent navigation on about 2 miles of the lower portion of the Allegheny, and during the calendar year 1891 the local commercial movements on that part of the river amounted to more than 830,000 tons.

The legal questions which stand in the way of the commencement of this important work should be settled, and it is hoped that some action will be taken at an early day by those having the matter in charge.

This work has been under the immediate supervision of Mr. J. W. Arras, Assistant Engineer.

Money statement.

July 1, 1891, balance unexpended.....	\$70, 648. 23
June 30, 1892, amount expended during fiscal year.....	1, 951. 38
July 1, 1892, balance unexpended.....	68, 696. 85
Amount appropriated by act approved July 13, 1892.....	40, 000. 00
Amount available for fiscal year ending June 30, 1893.....	108, 696. 85

{ Amount (estimated) required for completion of existing project.....	484, 500. 00
{ Amount that can be profitably expended in fiscal year ending June 30, 1894.....	200, 000. 00
{ Submitted in compliance with requirements of sections 2 of river and harbor acts of 1866 and 1867.	

D D 12.

ICE HARBOR AT MOUTH OF MUSKINGUM RIVER, OHIO.

The ice harbor is the lower pool of the Muskingum River, which has been created by the slack-water dam at Marietta, and the object of the work under consideration is to build a passway through this dam, of such a size as will permit Ohio River packets and coal fleets to take up from ice in this pool. The passway in question consists of a lock, which, owing to the local conditions controlling the only available site, was built with independent axes of entrance and exit, the angle between these axes being $11^{\circ} 15'$. This involved the necessity of widening the lock chamber at both ends so as to permit boats to change direction while in the lock. The plan of the lock chamber was therefore somewhat like an hourglass, the lock being narrowest in the middle. The maximum rectangle which can be passed through the lock has a length of 365 feet and a width of 56 feet. The work has been in progress since 1880, but has been kept back by inadequate appropriations and the occasional absence of any appropriation. The work is now nearly finished and is in use.

The following is a statement of the amount and date of all appropriations for the work:

March 3, 1879	\$30,000	August 5, 1888	\$37,500
March 14, 1880	50,000	August 11, 1888	60,000
March 3, 1881	30,000	September 19, 1890	30,000
March 2, 1882	40,000		
June 5, 1884	50,000	Total	327,500

The work was under the immediate supervision of Lieut. Cassius E. Rice, Corps of Engineers, U. S. Army, until April 20, 1892. Since that time Mr. Edmund Moeser, Assistant Engineer, has had immediate supervision of the work.

OPERATIONS DURING THE FISCAL YEAR.

(EXTRACT FROM REPORT OF MR. EDMUND MOESER.)

At the beginning of the year the masonry of the river wall was complete; the lock wall was complete with the exception of 155 feet, which was 12 feet above the miter sill. Work was carried on without interruption; the masonry of the lock wall was finished, being about 1,100 yards, the hollow quoins set, and the lock gates built and placed in position.

A contract was made with the Griffith and Wedge Company, Zanesville, Ohio, for iron work, consisting of mud valves, turbine culvert valves, head gates for mill race, machinery for operating lower gates, and all machinery for operating valves.

The under-water iron work was all placed as soon as finished, so that the lock was ready for use December 1, 1891.

Two thousand two hundred and eighty-seven cubic yards of mud was taken from the lock chamber and filled into the old mill race on the lock lot.

The lock will not be available for Ohio River boats until a draw is placed in the timbers and Ohio Southwestern Railroad Bridge just below it. No move has been made by the railroad company towards changing the bridge.

Money statement.

July 1, 1891, balance unexpended	\$20,114.29
July 30, 1892, amount expended during fiscal year	20,063.51
July 1, 1892, balance unexpended	50.78

REPORT OF THE ENGINEER IN CHARGE OF THE ARMY.

of proposals for construction of the Harbor Lock at Mouth of Muskingum River, Ohio.

Name and address of bidder.	Proposals for construction of Harbor Lock at Mouth of Muskingum River, Ohio.				Proposals for construction of Harbor Lock at Mouth of Muskingum River, Ohio.			
	Wrought iron per pound	Cast iron per pound	Steel per pound	Brass per pound	Wrought iron per pound	Cast iron per pound	Steel per pound	Brass per pound
The Griffith & Wedge Co., Janesville, Ohio.	4.50	3.50	5.00	1.00	4.50	3.50	5.00	1.00
Lambert Iron & Co., Canton, Ohio.	4.75	3.75	5.25	1.10	4.75	3.75	5.25	1.10
W. Gandy Machine Co., Janesville, Ohio.	4.50	3.50	5.00	1.00	4.50	3.50	5.00	1.00
H. J. Hal, Steam Pump Works, Pittsburg, Pa.	9.50	7.50	10.00	2.00	9.50	7.50	10.00	2.00
A. J. Greenwald, Canton, Ohio.	13.00	7.00	13.00	1.50	13.00	7.00	13.00	1.50
W. S. Sear, Canton, Ohio.	10.75	9.80	14.00	2.00	10.75	9.80	14.00	2.00
	10.50	11.50	1.50	8.221.43	15	15.50	.30	.50

plus extra charge for patterns, \$25.

Accepted by the Engineer in Charge of the Army, and executed August 29, 1891.

Contract for constructing Ice Harbor Lock at mouth of Muskingum River, Ohio, for fiscal year ending June 30, 1892.

Improvement.	Date.	To ex.
Work for Ice Harbor Lock.....	Aug. 29, 1891	Dec. 1

Account is attached to the report for operating and care of the lock from

D D 13.

HARBOR LOCK AT MOUTH OF MUSKINGUM RIVER, OHIO.

On December 10, 1891, for the opening of the lock.

Under the immediate supervision of Lieut. Cassin, U. S. Army, until April 20, 1892. Since that date, the assistant engineer, has had immediate supervision.

Respectfully,
November 1, 1891.

OPERATIONS FOR THE FISCAL YEAR.

(EXTRACT FROM THE REPORT OF MR. EDMUND MOESER.)

Expenditures were for lock-keepers and assistant, materials for repair of gates, labor in placing machinery for operating gates and for dredging entrance to lock and moving pieces of coffer.

Final statement of expenses incurred during the fiscal year ending June 30, 1892, out of the appropriation for operating and care of canals and other works of navigation, in operating and keeping in repair the Muskingum Ice Harbor Lock, Ohio.

Month.	Salaries lock-keeper and assistant.	Labor.	Supplies.	Miscellaneous.	Dredging.		Total.
					Salaries.	Towboat.	
under		\$62.25					\$62.25
ary		14.70					14.70
ary			\$25.00				25.00
		24.45	2.00				26.45
	\$55.00						55.00
	105.00		14.38	\$4.80	\$157.65	\$180.00	461.83
	195.00	102.75					297.75
Total	265.00	204.15	41.38	4.80	157.65	180.00	852.98

Commercial statistics are attached to the report for operating and care of locks dams on the Muskingum River.

D D 14.

IMPROVEMENT OF MUSKINGUM RIVER, OHIO.

This report is limited to work carried on under the appropriation of August 11, 1888, for the construction of a lock at Taylorsville and the construction of the lock at Zanesville, Ohio, of \$102,000.

This work was under the immediate supervision of Lieut. Cassius E. Kette, Corps of Engineers, U. S. Army, until April 20, 1892. Since that time Mr. Edmund Moeser, assistant engineer, has had immediate supervision of the work.

OPERATIONS FOR THE FISCAL YEAR.

(EXTRACT FROM THE REPORT OF MR. EDMUND MOESER.)

The work of opening out the channel below new Lock No. 9 was completed. There was a well-defined channel running from the lower end of the new lock on the right side of the river, downstream, and finally across the river to near the mouth of Creek, 2,000 feet below new Lock No. 9. The excavation extended over a distance of 800 feet. The bottom of this excavation is on a level 2 feet below the top of the lower miter sill of the lock. The remainder of the distance to Salt Creek will require some dredging.

During the year 13,443 cubic yards of rock and 10,062 cubic yards of earth were excavated, at a cost of 80 cents per yard for rock excavation and 31½ cents per yard for earth excavation.

The coping of the new lock was repointed, balanced counter weights were added to the machinery for operating the cylindrical valves, and the lower gates were installed but are not yet placed.

The balance of the money expended at this place during the year was for plant material which are on hand and which are to be used in the construction of a cofferdam crib at the head of the new lock.

THE CHIEF OF ENGINEERS, U. S. ARMY.

of the appropriation relating to the reconstruction of the locks and dams on the Muskingum River.

of the vicinity to ascertain the feasibility of reconstructing the lock in another location. The lock is in a very dilapidated condition, but it is possible to reconstruct the gates.

of the report for operating and care of locks and dams on the Muskingum River.

Money Statement.

.....	\$35,301.
.....	19,788.
.....	15,517.
.....	370.
.....	15,147.

.....

RECONSTRUCTION OF DAMS AND LOCKS ON MUSKINGUM RIVER

of the amount and date of all appropriations for the reconstruction of locks and dams on the Muskingum River.

.....	\$155,200.00
.....	39,980.00
.....	1,200.00
.....	12,241.54
.....	1,043,980.54

for work. The reconstruction of the locks and dams on the Muskingum River began at the beginning of the fiscal year, 1891. The reconstruction of locks Nos. 2 and 5 was completed.

and minor repairs were completed, and the reconstruction of lock No. 10.

at the locks, where the bottom would be raised to a uniform depth below the sill, so that all locks would be of uniform depth, and errors in width and in the placing of the sills at a uniform depth would be avoided. The navigable depth of the river would be maintained. By raising the high-water line has been

interrupted by the closing of the locks, and resumed with the opening of the locks.

of Lieut. Cassius E. Gilchrist, dated August 20, 1891. Since that time the reconstruction has had immediate super-

low race, leading to the mill below the Ice Harbor Lock, was excavated back of the wall of the lock and the material thus excavated used in filling the old bay.

A small amount of stone fill and about 100 feet of gravel backing is all that is required to complete this dam.

Dam No. 2, Decols.—The repairs at this place were limited to such work as could be done without making it last longer than the work at other points and thereby the opening of through navigation. The project for reconstruction included removal of the old foundation timbers under floor of lock chamber, as the lock had to be founded on rock, making the lower miter sill of stone, and the insertion of mud valves in walls at lower gate recess. After the cofferdams were finished and watered, investigation developed the fact that the lock was not on rock, but on a tough red clay overlying a red rock. Owing to the lateness of the season a change of plan was necessary; the lower miter sill was built of timber, and the mud valves were omitted. The lock was closed for repairs July 20, 1891, and reopened to navigation January 22, 1892.

The following was the work accomplished: Built new upper and lower gates and rails; six slide valves were placed in the two lower gates. Put in upper miter with filling culverts. Inserted cylindrical filling valves in head of old lock chamber. Raised 117 feet of lock wall 3.2 feet, to make guard of lock standard height. Made repairs to face of old lock walls and stopped all leaks with concrete. Put out mud and filled spaces between old foundation timbers with concrete. Rebuilt timber floor of lock chamber. A crib breakwater, 9 feet by 9 feet by 180 feet, put on the bank below the lock in order to break the eddy caused by dam and wall.

Made repairs to dam. The lower apron of the dam had raised, letting out the filling; the apron was forced back into place and fastened down with anchor bolts for a distance of 6 and 7 feet. The dam was refilled with stone where necessary and the lower apron was sheeted with new timber for a distance of 480 feet. To complete the repairs at this place the following work remains to be done: To raise the lock walls, to build a small guiding crib below the river wall, to put iron cover plates over cylindrical valve machinery, and to put in place lock gates and new gate-operating machines. About 170 yards of masonry remains to be built.

Dam No. 3, Lowell.—This lock has been open to navigation throughout the year. The section of needle dam across the head of the lock was operated as necessary, and the lower end of the canal kept in good navigable condition. There has not as yet been an opportunity of testing the practicability of keeping the upper end of the canal clear by this method, but a recent sudden flood in Catskill, which empties into the canal near the upper end, has put out into the canal a heavy bar of sand and gravel, and a test is to be made on this bar as soon as the

Dam No. 8, Eagleport.—At the close of last year the repairs on this lock were completed, but the cofferdams had not yet been removed. These were not removed until the work on the dam was completed, and the lock was not opened until November. A dry masonry wall was built connecting the old land wall with the high ground, and new gate-operating machines were put in position on the lock walls.

The new dam was pushed as rapidly as the materials could be delivered. It was rebuilt, with the exception of a short piece put in last season. The new dam was raised 4 feet higher than that of the old dam, in order to meet the normal conditions for which new Locks Nos. 8 and 9 were designed. The design of this dam is expected to cause a deposit at the foot of the dam, and by the use of a barge immediately below the dam add to its stability.

A new abutment was built at the right end of the dam, and the embankment on the left with the high ground, was raised to the height of the new abutment. The upstream slope paved.

Dam No. 9, Taylorsville.—As the new lock at Taylorsville can not be made until a draw is placed in the county bridge just below it, all navigation at this place has been through the old lateral canal and lock. In anticipation of the opening of the new lock nothing was done to the canal, and the time required for the passage of boats at this place has not been shortened.

The dam at this place must be raised 2 feet to produce the normal conditions for which Lock No. 9 was designed. If this were done it would be of immediate benefit to the boats running through the canal and would avoid the necessity of dredging the canal. The old dam was extensively repaired and is in good condition.

Dam No. 10, Zanesville.—Minor repairs were made to the dam.

The dam has been in operation throughout the year. Considerable trouble and expense has been incurred by the large amount of mud which accumulates in the chamber behind the gates, rendering them difficult of operation. The middle and upper gates were in a very poor condition. They should be rebuilt at an early day.

BOATS.

The *Mattie* was in commission seven months. She did good service in removing the mud from Locks Nos. 2, 5, 6, 7, and 8, in digging out the canal at McConnelsville in preparation for the completion of Lock No. 7, and in dredging channel and removing snags and obstructions at various points between Marietta and Eagleport.

For new dump scows have been prepared and the timbers for building new scows bought and stored at McConnelsville.

The tug *Wagoner* and the tug *Wagoner* were engaged in general towing between Locks Nos. 7 and 8.

The work for the dredge was done by the hired steamboat *Lizzie Cassel*.

...ham, 10 Wallace, 132. It appears from the evidence that at
... of said notice the defendants had no funds with which to
... change, and that under the statutes of Ohio, they, as commission-
... levies for bridge purposes at their March or June sessions in
... of which would be collectible not before the 20th of December
... . It also appears from the evidence that the defendants ap-
... authorizing them to raise the funds with which to make the
... and that their application failed, and the defendants introduced
... to prove that the cost of the required change will exceed the sum
... which, however, is denied by witnesses for the Government. The de-
... the statutes of Ohio, can not expend in constructing, altering, or
... a sum in ex-cess of \$10,000 without special authority from the
... or without submitting the same to a vote of the people of the county at
... election, and there was no general election after the service of notice,
... the spring election, on the first Monday of April and the State election
... Tuesday after the first Monday of November. The first of these dates
... and the last was after the limit prescribed by the
... authority in the matter excepting as county com-
... They had no bridge fund to draw upon and no authority of law to incur
... their individual responsibility. It would be mani-
... them to proceed without the authority of local law
... own responsibility, to incur the expenses involved in
... the required changes, whether the cost would have been less or more than
... The notice was not reasonable, and, therefore, if upon no other ground, the
... must be set aside.

... main question, and that which goes to the root of the matter, is whether
... has the power to confer upon the Secretary of War the authority attempted
... ferred by the act. In accordance with its terms, whenever he has good
... believe that a bridge is an unreasonable obstruction to navigation he is to
... e to the parties owning or controlling the same, after first giving them
... opportunity to be heard, to make such alterations as he may specify, and
... failure or refusal to make the same within a reasonable time they are to
... guilty of a misdemeanor, and the Secretary may direct the institution of
... proceedings. The power of the Secretary depends upon his having adjudged
... ridge is an obstruction and his adjudication is made final and conclusive.
... judicial power. The question is one of fact, or a mixed question of law
... and it can not be determined by a court without a jury unless the defend-
... sta. It was held in *Grant vs. Raymond*, 6 Peters, 242, Chief-Justice Mar-
... nencing the decision, that the Secretary of State of the United States is
... in whom, under the Constitution, judicial power can be vested. In
... the Secretary had gone through with the form of reissuing a patent for an
... It is true that there was not then any statute authorizing a reissue.

ent, original or renewed, is only prima facie evidence of an exclusive right in patents, and it is open to all defenses, including, in the case of a reissued patent, those involving an investigation into the question whether there was in fact such inadvertence, accident, or mistake as was requisite to authorize the reissue while here the Secretary of War finds and decides conclusively and finally whether the bridge is an obstruction, what changes shall be made, and within what time and the only questions left open to be tried in the criminal prosecution for misdemeanor, which he is authorized to set on foot, are whether he has made the finding and decisions, ordered the changes, given the proper notices, and whether the defendants have complied with his orders.

In this case the bridge was built about 1874 by the board of commissioners of Muskingum County, by virtue of a grant from the State of Ohio under the act of the legislature of March 25, 1870. The Muskingum River is entirely within the State of Ohio. Since 1890 and until the date hereinafter mentioned, it has been under the control of the State through its board of public works, which maintained a system of slack-water navigation until the cession of the river and its improvement by the State of Ohio to the Federal Government March 21, 1887. Since that time the General Government has caused to be constructed in a dam at the head of the rapid above said bridge, on its west side, the bridge, an artificial channel. It has also raised the locks and dam water above, some 4 feet. These changes and changes furnished the occasion for requiring the proposed bridge. The right of the State to erect or authorize bridges over which should not interfere with free lawful structures. But it is urged for the Government that they were subject to the power of Congress at an time to act upon the subject of such structures should be regarded as in Philadelphia, 3 Wal. 711. There is a case cited in *United States v. Leitch* containing the general proposition asserted is whether Congress could do in the premises to the Secretary of reasons for this conclusion are set out in *United States v. Kozak and H. Bridge* case, and to express as I do my own opinion therein.

The verdict against the defendants will be set aside, and the judgment of the court will be that sections 4 and 5 of the river and harbor act of September 19, 1890, upon which the information is based, are unconstitutional, and that the defendants go hence without day.

GEO. K. SAGE,
U. S. District Judge.

MAY 14, 1892.

Muskingum River leases for year ending April 30, 1892.

Location.	Lessee.	Dated.	Expires.
Dam 1, Marietta.....	Phoenix Mill Company.....	May 1, 1873	May 1, 190
Dam 2, Devel.....	Gates & Payne.....	May 1, 1889	May 1, 190
Dam 3, Lowell.....	Milton King.....	Nov. 1, 1888	Nov. 1, 190
Do.....	F. Wilking & Co.....	May 1, 1890	May 1, 191
Do.....	Reichsteiner Brothers.....	May 1, 1890	May 1, 191
Do.....	E. W. Sprague.....	Dec. 2, 1879	Dec. 15, 190
Do.....	George Rice.....	Sept. 1, 1883	Sept. 1, 190
Dam 4, Beverly.....	Robbins Brothers.....	May 1, 1889	May 1, 190
Do.....	Stall & Jumper.....	May 3, 1888	May 3, 190
Do.....	George S. Worstell.....	May 1, 1890	May 1, 191
Do.....	Mary L. Fahlwin.....	May 1, 1890	May 1, 191
Do.....	D. T. Brown.....	May 1, 1890	May 1, 191
Do.....	Isaac D. Spooner.....	May 1, 1890	May 1, 191
Dam 6, Stockport.....	Joseph Newberry.....	May 1, 1889	May 1, 190
Dam 7, McCounselsville.....	E. M. Staubertry.....	Sept. 1839	
Dam 8, Duncans Falls.....	John Miller.....	Dec. 31, 1836	
Dam 9, Taylorsville.....	Frazier & Son.....	Dec. 31, 1836	
Dam 10, Zaesville.....	John T. Drone.....	May 1, 1890	May 1, 191
Do.....	Edward Johnson.....	May 1, 1890	May 1, 191
Do.....	do.....	May 1, 1889	May 1, 190
Do.....	Joseph Shaw.....	May 1, 1890	May 1, 191
Do.....	Muskingum Coffin Company.....	May 1, 1890	May 1, 191
Do.....	Gary & McLaughlin.....	May 1, 1890	May 1, 191
Do.....	Beaumont & Blankenbuhler.....	July 1, 1889	July 1, 1909
Symmes Creek.....	Sarah V. Plummer.....	Apr. 2, 1889	Apr. 2, 1894

Muskingum River leases for year ending April 30, 1892—Continued.

Lessee.	Subject.	Cubic feet of water per minute.	Annual rental.	Rebate allowed.	Rents collected.
Flour Mill Company	Water power	3,000	\$350.00	\$257.73	\$92.27
Gates & Payne	do.	9,000	108.00	32.73	75.27
Miles King*	do.	2,600	100.00		
F. Wiking & Co.	do.	7,280	174.72		174.72
Echsteiner Brothers	do.	4,446	106.70		106.70
E. W. Sprague	Land		5.00		5.00
George Rice	do.		6.00		6.00
Baldins Brothers	Water power	1,904	100.00	4.85	95.15
Stall & Jumper	do.	5,293	127.03	6.16	120.88
George S. Worstell	do.	5,600	114.24	5.54	108.70
Mary L. Baldwin	do.	5,000	100.00	4.85	95.15
D. T. Brown	do.	6,500	118.30	5.74	112.56
Isaac D. Spooner	do.	4,000	100.00	4.85	95.15
Joseph Newberry	do.	6,390	220.04	28.58	201.46
E. M. Stanberry†	do.	(1)			
John Miller‡	do.	(3)			
Frazier & Son†	do.	(5)			
John T. Drone	do.	7,500	453.60		453.60
Edward Johnson	do.	6,029	361.74		361.74
do.	do.	7,399	443.82		443.82
Joseph Shaw	do.				
Muskingum Coffin Company	do.	4,619	207.85		207.85
Gary & McLaughlin	do.	4,794	186.97		186.96
Keasont & Blankenhauer	do.	7,741	278.68		278.68
Sarah V. Plummer¶	Land				
Total					3,221.67

* Lessee insolvent. Water shut off.

† Perpetual free lease of water power.

‡ Enough to propel 10 run of 4' 5" millstones.

§ Enough to propel 15 run of 4½" millstones.

|| Lease transferred to Muskingum Coffin Company, November 24, 1891.

¶ Terms five years in advance.

Estimate for fiscal year ending June 30, 1893.

Lock and Dam No. 1	\$1,100
Lock and Dam No. 2	2,270
Lock and Dam No. 3	450
Lock and Dam No. 4	2,145
Lock and Dam No. 5	2,470
Lock and Dam No. 6	1,340
Lock and Dam No. 7	1,615
Lock and Dam No. 8	1,845
Lock and Dam No. 9	7,000
Lock and Dam No. 10	1,000
Rent, fuel, gas, and water	300
Repairs to steamer <i>Vega</i>	500
Repairs to quarter boat	500
Operating steamer <i>Vega</i> eight months	3,000
Building two new dump scows	3,000
Dredging	9,600
Salaries and wages	12,288
Contingencies	5,000
Total	55,423

Statement of expenses incurred on Muskingum River, etc.—Continued.

LOCK AND DAM No. 1.

Month.	Labor.	Material.	Supplies.	Boat and engine hire.	Total.
.....	\$468.34	\$2,248.91	\$17.35	\$265.56	\$3,000.16
.....	2,452.02	1,814.12	96.42	104.76	4,467.32
.....	4,192.14	2,213.86	112.08	149.87	6,667.95
.....	6,480.43	3,179.77	475.45	165.49	10,301.14
.....	869.99	1,264.08	199.52	90.76	2,334.35
.....	46	46
.....	13.55	13.55
Total	14,462.92	10,734.75	810.82	776.44	26,784.93

LOCK AND DAM No. 2.

.....	\$699.74	\$970.69	\$6.38	\$3.00	\$1,679.81
.....	3,090.01	619.31	42.61	45.00	3,796.93
.....	3,047.57	1,473.69	143.83	41.50	4,706.59
.....	2,969.24	1,974.32	42.90	92.83	5,079.29
.....	1,988.82	1,764.63	45.19	11.50	3,810.14
.....	1,918.33	480.82	18.40	10.98	1,528.53
.....	569.95	456.00	1,025.95
.....	44.30	44.30
.....	4.80	2.00	6.80
.....	3.90	3.90
.....	2.40	2.40
Total	13,439.06	7,739.46	299.31	206.81	21,684.64

LOCK AND DAM No. 3.

.....	\$209.70	\$657.17	\$22.83	\$57.00	\$1,006.70
.....	1,305.00	360.26	1.00	38.00	1,704.26
.....	2,357.46	1,740.82	7.60	50.00	4,155.88
.....	2,368.14	1,130.08	13.01	47.16	3,558.39
.....	555.10	344.36	37.00	936.46
.....	30.00	16.67	46.67
.....	3.6060	4.20
.....	3.15	3.15
Total	6,862.15	4,262.69	45.04	245.83	11,415.71

LOCK AND DAM No. 4.

.....	\$2,550.08	\$1,334.64	\$15.65	\$174.16	\$4,074.53
.....	2,390.11	26.45	1.85	30.00	2,448.41
.....	1,655.92	128.64	.50	20.00	1,805.06
.....	479.57	19.70	.12	13.33	512.72
.....35	4.50	4.85
.....	56.30	56.30
.....	20.25	20.25
Total	7,152.23	1,509.78	22.62	237.49	8,922.12

LOCK AND DAM No. 5.

.....	\$44.50	\$44.50
.....	\$1,030.75	101.56	\$3.76	1,136.07
.....	3,566.28	771.06	22.77	\$85.00	4,445.11
.....	3,570.36	1,911.77	6.54	117.00	5,605.67
.....	4,182.65	1,019.21	217.36	85.00	5,504.22
.....	1,672.11	1,104.40	33.20	5.00	2,814.71
.....	886.83	56.09	942.92
.....	628.05	269.75	13.70	911.50
.....	1,029.21	384.68	21.39	8.00	1,443.28
.....	237.00	6.00	243.00
Total	16,803.24	5,663.02	318.72	306.00	23,090.98

2008 REPORT OF THE CHIEF OF ENGINEERS, U. S. ARMY.

Detailed statement of expenses incurred on Muskingum River under the appropriation for operating and care of canals and other works of navigation, for the fiscal year ending June 30, 1892.

GENERAL ADMINISTRATION.

Month.	Salaries.		Plant.	
	Office force.	Lock-keepers, bridge-tender and watchman.	First cost.	Repairs.
July.....	\$530.00	\$504.00	\$517.16	\$148.44
August.....	530.00	449.00	332.79	128.78
September.....	530.00	449.00	1,412.31	135.13
October.....	530.00	414.17	118.45	128.51
November.....	504.00	504.00	307.97	10.86
December.....	408.50	.95
January.....	150.00	534.00	2.80
February.....	410.00	579.00	107.28
March.....	380.00	579.00	.50	2.29
April.....	524.00	2.10	.59
May.....	524.00
June.....	380.00	449.00
Total.....	3,440.00	5,915.67	2,802.31	553.96

Month.	Buildings.	Supplies.	Transportation.	Miscellaneous.	Total.
July.....	\$74.08	\$152.85	\$127.01	\$402.57	\$2,357.11
August.....	30.73	11.25	72.53	1,551.03
September.....	42.34	10.25	127.71	2,708.74
October.....	49.40	10.89	128.66	1,379.89
November.....	5.54	13.15	109.18	969.79
December.....	8.58	.60	105.41	522.04
January.....	7.99	.50	41.28	739.57
February.....	14.60	110.68	1,221.56
March.....	34.18	.25	72.15	1,066.26
April.....	20.95	.25	50.51	596.51
May.....	1.00	33.18	559.18
June.....	1.20	870.20
Total.....	74.08	354.76	188.66	1,153.86	14,482.70

UNITED STATES DREDGE MALTA.

Month.	Salaries.	Equipment.	Supplies.	Repairs.	Towboat.	Total.
July.....	\$332.10	\$126.74	\$185.51	\$125.96	\$1,220.00	\$2,190.31
August.....	545.45	34.10	64.80	8.96	620.00	1,269.31
September.....	599.45	6.30	47.90	600.00	1,253.65
October.....	694.75	0.45	80.21	620.00	1,314.41
November.....	550.25	38.36	57.90	600.00	1,346.51
April.....	481.77	84.00	480.00	1,023.77
May.....	299.47	25.00	240.00	564.47
June.....	112.00	2.50	114.50
Total.....	3,655.24	167.14	457.52	273.03	4,380.00	8,912.98

UNITED STATES STEAMER VEGA.

Month.	Salaries.	Equipment.	Supplies.	Repairs.	Total.
July.....	\$354.55	\$52.44	\$19.05	\$426.04
August.....	355.00	\$0.50	30.75	21.00	407.25
September.....	352.50	8.30	3.90	364.70
October.....	355.5054	356.04
November.....	352.50	68.45	2.73	423.68
December.....	1.50	2.00	3.50
January.....	1.10	1.10
Total.....	1,770.05	8.80	156.66	42.78	1,978.29

2010 REPORT OF THE CHIEF OF ENGINEERS, U. S. ARMY.

Detailed statement of expenses incurred on Muskingum River, etc.—Continued

LOCK AND DAM No. 6.

Month.	Labor.	Material.	Supplies.	Boat and engine hire.	Total.
July.....	\$4,568.55	\$3,405.89	\$130.38	\$244.00	88
August.....	4,339.92	3,164.15	5.81	72.50	7
September.....	2,372.62	1,783.86	7.44	55.00	4
October.....	2,329.20	953.55	28.20	3
November.....	2,347.40	370.71	28.70	30.00	2
December.....	27.75	32.00
January.....	14.70	348.26
February.....	5.40
March.....	4.95
April.....	436.50	222.50
May.....	18.70
Total.....	16,465.69	10,280.92	200.53	401.50	27

LOCK AND DAM No. 7.

July.....	\$2,626.83	\$3,492.54	\$97.76	\$69.25	86
August.....	2,540.77	2,125.67	47.44	82.50	4
September.....	3,967.27	2,200.69	45.00	6
October.....	6,117.47	2,588.85	87.54	15.00	8
November.....	3,031.89	931.71	.67	17.00	3
December.....	939.95	665.95	20.00	1
January.....	143.94	347.76
February.....	169.09
March.....	102.50
April.....	84.60	222.75
May.....	49.05	8.36
June.....	8.40
Total.....	10,787.58	12,584.29	230.41	248.75	32

LOCK AND DAM No. 8.

July.....	\$3,419.45	\$3,209.29	\$39.33	\$201.67	86
August.....	3,469.30	3,177.85	54.13	87.25	6
September.....	4,040.93	3,454.80	367.50	7
October.....	3,749.10	2,278.10	498.05	6
November.....	2,547.33	600.12	266.50	3
December.....	318.52	220.75	31.00
January.....	24.00	235.52
February.....	32.57
March.....	5.10	49.57
June.....	2.55	1.56
Total.....	17,599.85	13,227.55	93.46	1,451.97	32

LOCK AND DAM No. 9.

July.....	\$2.00
August.....	13.32
September.....	\$440.55	532.37
October.....	914.77	433.16	\$1.35	1
November.....	40.50	2.20
May.....	212.73
Total.....	1,395.82	1,195.78	1.35	2

LOCK AND DAM No. 10.

July.....	\$50.92	\$3.00	\$1.75	\$147.00	4
August.....	401.02	13.46
September.....	496.90	630.33	1
October.....	159.15	221.33
November.....	50.25	1.89	2.20
December.....	1.40	10.72
February.....	15.90
March.....	22.95	1.00
April.....	4.80
June.....	28.95
Total.....	1,232.24	881.73	3.95	147.00	2

SUMMARY.

General administration	\$14,482.70	Lock and Dam No. 6	\$27,348.64
United States dredge Malta	8,912.93	Lock and Dam No. 7	32,851.03
United States steamer Vega	1,982.91	Lock and Dam No. 8	32,372.83
Lock and Dam No. 1	26,784.93	Lock and Dam No. 9	2,592.95
Lock and Dam No. 2	21,684.64	Lock and Dam No. 10	2,264.92
Lock and Dam No. 3	11,415.71		
Lock and Dam No. 4	8,922.12	Total	214,707.29
Lock and Dam No. 5	23,090.98		

Detailed statement and cost of dredging on the Muskingum River under the appropriation for operating and care of canals and other works of navigation, for the fiscal year ending June 30, 1892, to wit:

UNITED STATES DREDGE MALTA.

Date.	Locality.	Materials excavated.	Total cost.
July	Bear Creek	Gravel, at 18 cents	\$1,005.30
	Luke Chute	Gravel, at 15 cents	1,185.00
August	Stones Ripple	Gravel, at 18 cents	63.32
	Baldwins Ripple	Gravel, at 15 cents	1,200.00
September	Silver Heels	Gravel, at 17 cents	955.65
do	do	Stone, 1	228.00
do	do	Snags, 1	70.00
October	Stockport	Gravel, at 17 cents	1,249.41
do	do	Stone, 1	70.01
do	do	Snags, 1	15.00
do	do	Piling	20.00
November	do	Cofferdam	100.00
	McConnellsville	Stone, 1	20.00
do	do	Gravel, at 24 cents	1,126.51
April	Eagleport	Piling	35.00
	McConnellsville	Piling	30.00
do	do	Gravel, at 9 cents	904.77
do	do	Cofferdam	80.01
May	Stockport	Cofferdam	40.00
	Luke Chute	Cofferdam, 1, at \$1	80.00
do	do	Gravel, sand, and mud, 480 cubic yards, at 15 cents	72.00
do	do	Wrecked barge, 1, at \$20	20.00
do	Devol	Gravel, sand, and mud, 1,600 cubic yards, at 11½ cents	190.00
do	do	Cofferdam, 40 linear feet, at \$1	40.00
do	Marietta	Gravel, sand, and mud, 316½ cubic yards, at 15 cents	47.47
do	do	Piling, 3 at \$5	15.00
June	do	do	114.50
		Total	8,912.93

Commerce of Muskingum River during the fiscal year ending June 30, 1892.

LOCK NO. 1.

Month.	Steam-boats.	Barges.	Miscellaneous.	Total.	Number of lock-ages.
December	2	2	6	10	10
February	1	11	13	25	25
March	5	8	7	20	20
April	58	17	66	141	141
May	54	10	31	95	95
June	81	13	41	135	115
Total	201	61	164	426	406

LOCK NO. 2.

January	8	12	4	24	24
February	37	11	4	52	48
March	54	8	6	68	66
April	54	9	2	65	64
May	69	17	5	91	91
June	82	23	7	112	112
Total	304	80	28	412	406

Washington River during the fiscal year ending June 30, 1892—Continued.

LOCK No. 8.

Month.	Steam-boats.	Barges.	Miscellaneous.	Total.	Number of lock-ages.
.....	60	2	62	62
.....	26	1	27	27
.....	62	4	66	64
.....	11	11	11
.....	30	30	30
.....	85	6	4	95	90
.....	85	7	1	93	93
.....	77	1	4	82	82
.....	98	2	6	106	106
.....	534	20	18	572	565

LOCK No. 9.

.....	1	7	8	7
.....	1	6	7	7
.....	1	7	2	10	10
.....	50	14	2	66	66
.....	52	1	3	56	56
.....	62	5	4	71	68
.....	13	13	13
.....	30	30	30
.....	86	2	2	90	88
.....	81	9	5	95	92
.....	78	11	89	85
.....	108	14	7	129	122
.....	562	64	38	664	644

LOCK No. 10.

.....	54	54	54
.....	54	54	54
.....	54	5	59	54
.....	54	3	1	58	56
.....	48	4	2	54	49
.....	64	3	67	64
.....	12	12	12
.....	48	48	48
.....	80	1	81	81
.....	76	76	76
.....	76	2	78	78
.....	101	3	104	104
.....	721	21	3	745	730

SUMMARY.

	Steam-boats.	Barges.	Miscellaneous.	Total.	Number of lock-ages.
.....	201	61	164	426	406
.....	304	80	28	412	405
.....	402	136	22	560	495
.....	239	188	21	448	334
.....	110	32	10	152	123
.....	102	28	14	144	136
.....	165	41	19	225	201
.....	534	20	18	572	565
.....	562	64	38	664	644
.....	721	21	3	745	730
.....	3,340	671	337	4,348	4,041

112 REPORT OF THE CHIEF OF ENGINEERS,

Commerce of Muskingum River during the fiscal year ending June 30,

LOCK NO. 3.

Month.	Steam-boats.	Barges.	Miscellaneous.
July.....	28	24	
August.....	33	30	
September.....	22	17	
October.....	11	7	
November.....	15	8	
December.....			
February.....	36	5	
March.....	54	9	
April.....	54	7	
May.....	65	13	
June.....	85	16	
Total.....	402	136	

LOCK NO. 4.

July.....	46	44	
August.....	28	20	
September.....	13	26	
October.....	14	29	
November.....	20	18	
December.....		3	
February.....	12	4	
March.....	14	10	
April.....	20	9	
May.....	31	11	
June.....	41	14	
Total.....	239	188	

LOCK No. 5.

July.....	18	10
August.....	19	8
May.....	31	6
June.....	42	8
Total.....	110	32

LOCK No. 6.

October.....	10
November.....	
December.....	8
April.....	9
May.....	32
June.....	43
Total.....	102

LOCK No. 7.

.....	20
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IVER, IN-
PERATING
KY.

OF ENGI-
DING JUNE
RKS.

and Illinois.

OFFICE,
July 9, 1892.

reports on oper-
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AT LOUISVILLE, KEN-

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2014 REPORT OF THE CHIEF OF ENGINEERS, U. S. ARMY.

List showing amount of principal items of freight and the number of passengers carried on the Muskingum River between July 1, 1891, and June 30, 1892.

Articles.	Amount.	Articles.	Amount.
Passengers..... number..	31,868	Wool..... tons..	74
General merchandise..... tons..	6,000	Lumber..... do..	7,200
Coal..... do..	3,345	Brick..... do..	155
Live stock..... do..	245	Hay..... do..	4,822
Oil..... do..	7,121	Salt..... do..	288
Wheat..... do..	621½	Potatoes..... do..	124
Corn..... do..	1,174½		

The above statement of the commerce of the Muskingum River is complete, but is only for part of the fiscal year, as the locks were shut off for repairs and there was no through navigation until May 6, 1892.

lower end, in such manner as to form capacious basins, or harbors, at those points.

The project for the enlargement at the head of the canal was inaugurated in 1883, and modified in 1890, as recommended in the report of the Board of Engineers, dated January 28, 1890. The general width of the canal before enlargement was 90 feet; when the work is completed, as shown on the sketch submitted herewith, the width for a length of 2,400 feet will be from 210 to 325 feet; immediately to the eastward of this enlargement of the canal proper a basin about 800 feet wide and 2,200 feet long will be constructed, which will be closed by a bear-trap structure 800 feet long on the west and by a concrete dam 2,800 feet long on the north. The execution of this project requires, as its principal features, the excavation of about 325,670 cubic yards of solid rock and 270,000 cubic yards of earth; the construction of about 5,200 linear feet of new canal wall and masonry dams, containing in all about 26,000 cubic yards of masonry, and the removal of about 6,200 linear feet of old canal wall, dikes, and timber dams.

The work completed prior to June 30, 1891, comprised 124,027 cubic yards of rock excavation, 197,769 cubic yards earth excavation, and 10,307 cubic yards of masonry in new canal wall.

Operations for the past fiscal year were commenced July 21, 1891, when the river had fallen to a stage sufficiently low to permit the area of excavation to be cleared of water, by pumping and the erection of temporary dam, and the active work of excavation was begun July 26. It was continued until November 23, with a temporary suspension from August 28 to September 7 on account of high water. Up to August 28 the work was carried on by day only, but after resuming in September a night force was also organized, and was continued during the remainder of the working season. No masonry was laid on this portion of the work during the year, operations being limited to earth and rock excavation; but preparations for completing the new canal wall during the ensuing season were commenced June 15, 1892. The amounts excavated during the year were 15,299 cubic yards earth and 42,856 cubic yards rock, an average of 650 cubic yards daily during the working period. A considerable portion of the area remaining to be excavated has been drilled ready for blasting.

For blasting the rock in place 9,624 holes, aggregating 49,642 feet in length, and 18,362 block holes, aggregating 13,772 feet, for breaking the blasted rock, were drilled during the season; 29,567 pounds of dynamite and 875 pounds of black powder were used for blasting.

The total work done to June 30, 1892, is as follows: Earth excavation, 213,068 cubic yards; rock excavation, 166,882 cubic yards; masonry in canal wall, 10,307.5 cubic yards.

The principal items of work remaining to be done to complete the improvement is, approximately, as follows: Rock excavation, 158,787 cubic yards; earth excavation, 53,000 cubic yards; masonry, 16,000 cubic yards, and the removal of about 6,200 linear feet of old wall, dikes, and dams.

The other portion of the improvement, viz, the canal enlargement above the locks, has been carried on in accordance with the project authorized in the Annual Report of the Chief of Engineers for 1887, the purpose being to construct at that point a basin wherein boats may lie and tows may be properly formed, before or after passing the locks without interfering with navigation through the canal. The work approximately estimated as necessary to this end was the excavation of about 124,000 cubic yards of earth, 13,000 cubic yards of rock, the con

struction of about 5,200 cubic yards of masonry wall, and the removal of 1,050 feet of old canal wall on the north side. The result will be a basin at the lower end of the canal 1,500 feet long, with a width varying from 210 to 250 feet.

At the close of the fiscal year ending June 30, 1891, there had been excavated 133,617 cubic yards of earth, 14,581 cubic yards of rock, and 1,908 cubic yards of masonry had been laid in the new wall.

Operations for the fiscal year just closed were commenced early in July, 1891, and carried on with but little interruption until November 20, when high water and cold weather caused a suspension for the winter; they were resumed May 14, 1892, and continued until June 7, when temporarily suspended because of lack of available funds. Progress was in the highest degree satisfactory, and for this fact great credit is due to the energy and skill of the assistant engineer, Mr. R. E. Jones, in immediate charge; for it was through his exceptional capacity that the work received the full benefit of an unusually favorable season. He is entitled to this official recognition of his valuable services.

The results accomplished during the year were 11,760 cubic yards of earth excavated, 3,364.6 cubic yards of rock excavated, and 3,704.7 cubic yards of masonry placed in new wall, besides 195 cubic yards of temporary dry stone wall laid. In addition about 1,570 cubic yards of rock were removed from the old canal wall.

But little work is required to complete this portion of the improvement, being the removal of the remains of the old canal wall, the excavation of the underlying rock, and the construction of a length of about 50 feet of the new canal wall. It is anticipated that all this will readily be accomplished during the ensuing working season.

The amount expended under appropriations for improving the Falls of the Ohio River at Louisville, Ky., to June 30, 1892, was \$619,079.43, of which sum \$152,225.60 was expended during the past fiscal year, including liabilities outstanding July 1, 1892. The estimated cost of completing the improvements contemplated by the present project is \$610,008.89.

The following statistics of traffic and commerce indicate the extensive river tonnage directly interested in the substantial and complete improvement of the Falls of the Ohio. They show a traffic of 7,939 boats passing this point during the past fiscal year, carrying 2,172,661 tons of freight. The annual average for the past eleven years is 7,025 boats, with about 2,500,000 tons of freight. Of coal alone the amount passing the falls to points below during the past six years has averaged 1,731,308 tons annually.

Statement of vessels passing Falls of the Ohio River via Louisville and Portland Canal and Indiana Chute, June 30, 1882, to June 30, 1892.

Kinds of vessels.	1882-'92.		1892.		Total for 11 years.	
	No.	Tons.	No.	Tons.	No.	Tons.
Passenger boats	11,156	4,506,402	949	425,746	12,105	4,932,148
Tow boats	11,476	1,510,731	1,477	198,218	12,953	1,708,949
Government boats	1,107	76,536	143	12,522	1,250	89,058
Flat boats and barges	43,287	14,202,133	5,162	1,811,723	48,449	16,013,856
Small craft	2,251	201	2,452
Others	54	7	61
Total	69,331	20,295,802	7,939	2,448,209	77,270	22,744,011

Annual average for past 11 years: Boats, 7,025; tons, 2,067,638.

2018 REPORT OF THE CHIEF OF ENGINEERS, U. S. ARMY.

Statement of commerce passing the Falls of the Ohio River via Louisville and Port Canal and Indiana Chute, from June 30, 1887, to June 30, 1892.

Articles.	From 1887 to 1891.	From 1891 to 1892.	Total six years
	Tons.	Tons.	Tons.
Coal.....	8,672,543	1,715,308	10,387,851
Salt.....	63,691	8,532	72,223
Oil.....	44,823	2,839	47,662
Whisky.....	12,869	1,934	14,803
Tobacco.....	56,021	4,329	60,350
Cotton.....	66,193	10,068	76,261
Lumber.....	2,838,793	69,769	2,908,562
Corn and wheat.....	42,834	18,529	61,363
Iron ore.....	254,516	16,383	270,899
Iron.....	20,229	111,776	132,005
Steel rails.....	205,065	46,075	251,140
Produce.....	33,412	9,736	43,148
Hay and straw.....	118,457	6,375	124,832
Flour.....	8,639	1,613	10,252
Stock.....	42,347	5,890	48,237
Sugar and molasses.....	42,114	7,280	49,394
Staves and shingles.....	215,537	3,722	219,259
Peanuts.....	982		982
Malt.....	695		695
Cement.....	9,549	8,025	17,574
Miscellaneous.....	297,407	125,073	422,480
Total.....	13,046,056	2,172,661	15,218,717

Annual average during past six years, 2,536,453 tons.

Reports of R. R. Jones and G. W. Shaw, assistant engineers, containing details of operations in their respective charges, are herew submitted.

Money statement.

July 1, 1891, balance unexpended	\$193,144
June 30, 1892, amount expended during fiscal year.....	148,182
July 1, 1892, balance unexpended	44,962
July 1, 1892, outstanding liabilities	4,043
July 1, 1892, balance available	40,919
Amount appropriated by act approved July 13, 1892	60,000
Amount available for fiscal year ending June 30, 1893	100,919
{ Amount (estimated) required for completion of existing project.....	550,000
{ Amount that can be profitably expended in fiscal year ending June 30, 1894	300,000
{ Submitted in compliance with requirements of section 2 of river and harbor acts of 1866 and 1867.	

REPORT OF MR. R. R. JONES, ASSISTANT ENGINEER.

LOUISVILLE, KY., June 30, 1892.

COLONEL: I have the honor to submit the following report of operations for proving Falls of Ohio River at Louisville, Ky., enlarging canal basin at head locks, for the fiscal year ending June 30, 1892:

IMPROVING FALLS OF OHIO RIVER, ENLARGEMENT OF BASIN.

Cofferdam at dry dock, pier, etc.—A cofferdam was constructed parallel to about 30 feet distant from the old pier, separating dry dock from canal. This d was 16 feet high and about 400 feet in length, and behind this protection the c construction of wall between dry dock and basin was carried on.

The excavation of material was carried on from the pit until November 20, the

rial being removed on cars by the hoisting engine and incline and by carts. The
er were used only toward the close of the season's work for removal of part of
outside bank, under the protection of which the work had been heretofore carried
. The pit was allowed to fill November 20, and after that the regular canal dredges
re employed, whenever the stage of water permitted, in the removal of old wall
f the earth backing behind same.

Excavation.—The following material has been excavated from the commencement
the work to the close of the present fiscal year: Earth, 141,593 cubic yards; rock,
44.6 cubic yards; total, 159,337.6 cubic yards.

f this total of 159,337.6 cubic yards there were excavated during the fiscal year:
th, 11,760 cubic yards; rock, 3,364.6 cubic yards; total, 15,124.6 cubic yards.

he removal of what remains of the old canal wall will be done with dredges. The
k under the wall will be blasted under water and also removed with dredges.

ew canal wall.—The work of building new canal wall was carried on during the
son of 1891 up to November 2, and was resumed May 14, 1892, and carried on at
ervals whenever the stage of water permitted up to June 7, 1892.

uring the season of 1891 the work was conducted in the pit by means of derricks
l traveling crane.

1892 the work was done by means of a derrick boat, as the old canal wall was
that time partly removed.

he wall is now completed excepting the short curve joining the new wall to pier
d of new locks. This will be done under the protection of a small cofferdam to
constructed as soon as the water falls sufficiently.

he total quantity of masonry laid from the commencement of work to the close
fiscal year was:

	Cubic yards.
concrete foundation	169.20
masonry	5,443.98
dry wall	194.20
Total	5,807.38

Of the total quantity of masonry reported—viz, about 5,807.38—there was laid
during the fiscal year ending June 30, 1892:

	Cubic yards.
concrete foundation	61.00
masonry	3,643.68
dry wall	194.20
Total	3,898.88

The facilities for handling and laying stone, more particularly the traveler used
in construction of double wall between dry dock and basin, proved admirable. The
same used in construction of this wall weighed from 8,500 to 10,000 pounds, the
rollers being 10 feet long, and they were handled quickly and securely.

EXCAVATION FOR NEW CANAL WALL AT HEAD OF CANAL.

Much preparatory work was done in the way of laying railroad tracks, bridging
an outlet to new portion of canal, and building earth embankments to protect the
excavation.

Work was commenced June 15, 1892, the water then for the first time being low
enough to permit the prosecution of the work described.

The material excavated was as follows:

	Cubic yards.
earth	1,424
loose rock	107
Total	1,531

This work was mostly done in the neighborhood of the Fourteenth Street Railroad
bridge, and was for the purpose of connecting the end of the new canal wall as now
built with the old canal wall, about 705 feet west and south of the north drawpiers of
the bridge in question.

Very respectfully, your obedient servant,

R. R. JONES,
Assistant Engineer.

Lieut. Col. G. J. LYDECKER,
Corps of Engineers, U. S. A.

REPORT OF MR. G. W. SHAW, ASSISTANT ENGINEER.

LOUISVILLE, KY., June 30, 1892

COLONEL: I have the honor to submit the following report of operations on proving falls of the Ohio River at Louisville, Ky., enlarging the head of the Louisville and Portland Canal for the fiscal year ending June 30, 1892.

The work of enlarging the head of the Louisville and Portland Canal was begun in 1885, and was let by contract in three sections. Section 1 was completed November, 1886, and the contracts for sections 2 and 3 expired December 31, 1887. The work has since been carried on with hired labor, and that of last season directed mainly to finishing sections 2 and 3. Work on section 4, an additional area of section 3, was commenced during the last season. The material removed consisted principally of solid rock, ranging from 3 to 6 feet deep, the grade being of the canal bottom.

There was also removed a considerable quantity of earth, mud, and deposit which accumulates during high water.

The season of 1891-'92 was particularly favorable for work, excavation having been carried on for ninety days. Preparations for beginning work were completed early in July, but high water interfered with pumping until July 21. Steam drills were put in operation on the 26th and excavation commenced on the 27th of July. A dam in the river suspended general work from August 28 to September 7. Excavation was resumed on the latter date and continued until November 23 at noon, at which time the river flooded the inclosure and work was suspended for the season.

It is estimated that the following quantities were excavated and removed during the season: Earth, mud, loose rock, etc., 15,299 cubic yards; solid rock (measured in place), 42,855.49 cubic yards.

Beginning with September 14 an additional force of men was employed at night. The inclosure was illuminated with gasoline torches and the excavated material loaded into dump cars. It is estimated that the night force excavated and loaded into cars 8,448 cubic yards of solid rock, which was removed by 632 trains, requiring 11,882 cars, averaging about seven-tenths of a cubic yard per car, measured in place.

Two 12-inch centrifugal pumps were used in pumping out the excavated area. Leakage through the guiding dike and old canal wall was controlled by temporary earth dams, which carried the water around the edge of the inclosure into the river north of the improvement. The upper part of the guiding dike was repaired and paved with stone taken from the excavated material.

The following statement shows approximately the work done to June 30, 1892: Earth excavation, 213,068.06 cubic yards, of which 15,299 cubic yards were removed in the season 1891-'92; rock excavation, 166,882.40 cubic yards, of which 42,855.49 cubic yards were removed in season 1891-'92; masonry in new canal wall laid to date of fiscal year, 10,307.56 cubic yards.

There remains to be done the following: Earth excavation, 53,000 cubic yards; rock excavation, 158,787 cubic yards; masonry in canal wall, 6,013 cubic yards; moving old canal wall, 7,500 cubic yards; removing part of dam and guiding dike, 2,900 linear feet; building concrete dam, 4,822 cubic yards; building bear-trap structure, 800 linear feet; slope revetment and wall on south side of the canal between Tenth street, 2,300 cubic yards; and the construction of 4 guide piers or cribs at the north line of the improvement above Tenth street.

Herewith is submitted a sketch showing progress of work to June 30, 1892.

Very respectfully, your obedient servant,

GRANVILLE W. SHAW,
Assistant Engineer

Lieut. Col. G. J. LYDECKER,

Corps of Engineers, U. S. A.

E E 2.

IMPROVEMENT OF INDIANA CHUTE, FALLS OF THE OHIO RIVER

The Indiana Chute is the main river channel over the Falls of Ohio; originally very crooked, filled with dangerous rocky points projecting from the sides and bottom and with swift changing currents it was available for navigation by skilled pilots only when the river was above a stage of about 11 feet on the gauge of the Louisville and Portland Canal. Prior to 1890 some work had been done towards its improvement by blasting out the most dangerous points whereby

of navigation was increased, but the channel was not thereby available for heavy traffic at stages of water materially lower than stated above. On the 28th of January, 1890, a project, having its object the radical improvement of the channel so as to make it navigable at all stages above 8 feet on the canal gauge, was presented by a Board of Engineers after close study of the question. This project was approved by the Chief of Engineers, January 18, 1890. (See Report of Chief of Engineers for 1890, pages 2217-2218.)

It contemplates the better concentration in this channel of the water passing the falls by the erection of guiding dikes and the excavation of rock within the limits of proposed channel to a designated depth. The improved channel is designed to have a least width of 400 feet and a clear depth of not less than 8 feet at its lowest navigable point. The principal items of work in the execution of this project are the excavation of about 60,000 cubic yards of solid rock, the construction of about 3,700 linear feet of stone dikes, and of a concrete wall about 1,000 feet long. The project contemplates, ultimately, some modification of the present system of dams at the head of the falls when these are replaced by permanent structures, but the details of such a plan have not yet been decided.

The operations during the past year were limited to raising the old stone dike—575 feet long on the north side of the chute at its entrance to 12 feet, the first 50 feet constituting the pier head being raised to 15 feet, or 11 feet in all; the cross dam, connecting this dike with the Indiana shore, was rebuilt above the water line, and some minor repairs were made to the balance of the cross dam. About 72,900 feet of oak timber and 344 cubic yards stone filling were applied to the repairs. The stage of water did not permit any rock excavation, or the appliances available, until October 5, when the work was commenced by hand-drilling; on the 9th the steam-drilling scow below the Louisville and Portland Canal was put on the work, and operations were continued until November 17, when a rise in the river necessitated a suspension for the season. The results accomplished during the report period were 3,163 holes, aggregating 6,474 linear feet, drilled by hand; 673 holes, aggregating 2,592 linear feet drilled by steam drills; and dug out in all about 1,474 cubic yards of rock, using for this purpose 825 pounds dynamite. This is the only work so far done in connection with the present project, no suitable plant for the work in view has been procured, while all operations have been restricted to short periods of dead low water, and to the primitive and expensive method of hand-drilling. It is proposed to procure proper machinery for carrying out the work in the future, and make provision for operations during periods of low water, at least.

The relief which this improvement is designed to afford the heavy traffic—especially of coal—passing the falls, is a matter of high importance. A coal tow in reaching this point frequently contains twelve or more boats and barges carrying about 320,000 bushels (about 1,216 tons) of coal, bound for points on the Ohio and Mississippi between here and New Orleans; under existing conditions these tows, when unable to pass the Indiana Chute, must be broken on arrival at this point, and the boats, through the Louisville and Portland Canal and locks in sections, are again consolidated before continuing their journey to destination; all this imposes great delay and expense on this traffic, and not infrequently it happens that, when a rise in the river has brought the tows down to this point, the later arrivals after finally passing the locks find the river too low to continue their journey, and are compelled to lie over indefinitely, sometimes for an entire season. An im-

to permit the gate such a condition of affairs is of course not to be desired, and having been undertaken, it should be completed as soon as possible.

In comparing this report and the report in relation to the Wabash and Canal show that during the past year the Falls of the Ohio, of which 1,232,998 tons of freight is incident to transit through the canal. With the improvement now under consideration it is highly probable that most a very insignificant percentage, of the freight will be subject to any such delay, while general traffic will be benefited from the blockade frequently of the river. The total traffic through the canal during the past year was 6,027 boats, carrying 1,620,586 tons of freight. The total traffic via the Indiana Chute included 1,916 boats, carrying 2,007,638 tons of freight; total passing by canal and river 7,943 boats, with 2,007,638 tons of freight.

The amount appropriated by the Board of Engineers for the improvement available for the purpose was \$25,000, a total to June 30, 1894, of \$60,000. The amount estimated below for the present project is \$35,000. It is estimated on the idea that the river and harbor congress will appropriate for the work \$25,000. If such appropriation be not made the amount estimated below is increased in order that the project be completed.

The estimate given below, and the report in relation to the immediate charge of this improve-

.....	\$48,885.20
.....	21,361.44
.....	27,523.78
.....	14.63
.....	27,509.13
.....	33,000.00
.....	62,509.13
.....	57,249.53
.....	60,000.00

STATE OF KENTUCKY.

REPORT OF THE BOARD OF ENGINEERS, U. S. ARMY.

STATE OF KENTUCKY, June 30, 1892.

The report of operations on improvement of the river and harbor at Louisville, Ky., for the year ending June 30, 1892, is herewith submitted. The guiding dike at the locks was completed. Right Hand Reef. Fifty feet of the dike, 524 1/2 feet, was completed. The storehouse at the locks, the

work of raising the dike began on September 21, 1891. In removing the old decking for the guide pier it was found that the timbers above low water were rotten, and new material was substituted. In the remainder of the dike the new timbers were placed on top of the old structure, care being taken to remove such as were found to be defective. Corrections in alignment were made and the new work filled with stone taken from the vicinity and from the excavation on Right Hand Reef. The upper end of the guide-pier was covered with three-eighths inch boiler iron as a protection against ice and drift. The work on the dike was completed on November 19.

Repairs to the cross dam and abutments consisted in replacing defective timbers and decking; in rebuilding the south abutment of the middle chute, and placing therein new clutches for the movable dam; in the construction of a crib abutment at the end of the dam, north of the Indiana Chute, and in rebuilding that portion of the cross dam immediately north of the Indiana Chute.

Excavation in the chute was directed to the right-hand reef and to some projections near the lower end of the guiding dike. On October 2, the river having fallen to a working stage, a breakwater was placed at the lower end of the guiding dike and hand drilling was begun on the 5th. On the 9th the drill boat belonging to the canal was placed in the chute near the lower end of the reef and the boiler utilized for drilling by steam. Steam drilling was continued until the 29th, at which time all the exposed portions of the reef had been drilled. Blasting on this part of the reef was commenced on October 11 and continued until November 17, on which date a rise in the river compelled a suspension of work for the season. Material removed from the reef was piled north of the line of improvement, and it is proposed to utilize part of it in the new wall which is to extend from the bridge eastwardly along the north line of improvement. During the season there were drilled 3,836 holes, ranging from 1 to 5 feet deep. It is estimated that 2,513 cubic yards of solid rock were blasted, of which 763 were removed. Tools and appliances were stored at the locks at the end of the season.

Very respectfully, your obedient servant,

GRANVILLE W. SHAW,
Assistant Engineer.

Lieut. Col. G. J. LYDECKER,
Corps of Engineers, U. S. A.

COMMERCIAL STATISTICS.

Statement of vessels passing Falls of the Ohio River, via Indiana Chute, for fiscal year ending June 30, 1892.

Kind of vessel.	No.	Tonnage.
Passenger boats	344	177, 286
Towboats	469	75, 574
Square barges	1, 024	582, 572
Model barges	73	58, 400
Total	1, 910	893, 832

Days navigable: Ascending, 51; descending, 285.

Statement of commerce passing Falls of the Ohio River, via Indiana Chute, for fiscal year ending June 30, 1892.

Articles.	Tons.	Articles.	Tons.
Coal	482, 310	Iron	7, 366
Salt	1, 084	Miscellaneous	25, 240
Oil	750	Produce	517
Whisky	315	Flour	169
Wine	579	Sugar and molasses	519
Beer	1, 509	Hay and straw	710
Meat	2, 521	Live stock	648
Staves	831	Staves and shingles	3, 722
Shingles	236		
Iron ore	3, 517	Total	582, 125
Iron rails	19, 582		

Number of passengers, 7,246.

whenever opportunity offered. Its object is to secure
damage by striking against projecting points that
left in the lower courses of the canal wall and on the
underlying rock. This work should be continued until all
have been removed.

NEW MACHINE SHOP.

Evening of February 12, 1892, the old frame building which
the engine and boiler house and machine shop was totally
by fire, the cause of which remains a complete mystery.
struction, as a fireproof structure, was immediately under-
at the close of the fiscal year had been nearly completed.
of the new structure are of stone masonry and the roof of
ated iron. The destroyed building was surrounded by others
inflammable character, viz: The blacksmith shop, carpenter shop,
two storehouses, all of which were saved only by the greatest ex-
had there been any wind they must have been completely de-
oyed. These buildings should be reconstructed in masonry as op-
portunity offers.

NEW WINDING ENGINE.

This engine was purchased and put in position in January, 1892, and
operly housed in a small masonry building. It is used for handling
s in their passage through the locks, and their lockages are very
easily facilitated and expedited thereby.

GENERAL REPAIRS.

Considerable repairs were made to the towboat; they included raising
e pilot house four feet; extending the cabin forward, thereby adding
much-needed room to the cabin accommodation; building a new
heel, and placing new canvas deck and roof covering. New canvas

OPERATING AND CARE

The work of operation is done by a regular manager, a lock master, 1 lock hands, 1 bridge hand, and 1 general regulation through the canal for the purpose of two reliefs, one in lock master, and the periods of day and night, it is only proper that he should adopt this course.

During the past year the locks were closed 23 days being 1,620,588 feet of water.

The care of the locks and the immediate neighborhood of all improvements during the year were two new middle boats, and built a machine shop with 12, 1892. The

This service boat, and is a necessary for clearing deposits of mud to considerable depth. The cost was \$6,825 in addition to this, from the old dam, under construction, material has

The construction of the canal was maintained on the canal. The canal is a straight line, and the locks are built on the banks of the canal. The canal is 100 feet wide and 10 feet deep. The locks are 100 feet long and 10 feet wide. The canal is 100 feet long and 10 feet wide. The locks are 100 feet long and 10 feet wide.

Item	Amount
Salaries	100,000
Locks	50,000
Boats	20,000
Machine shop	12,000
Material	6,825
Other	10,000
Total	200,825

The usual statements of expenditures and commerce on the canal are given below, and the report of R. R. Jones, assistant engineer, charged with the care of the canal, is transmitted herewith.

Statement for operating and care of the Louisville and Portland Canal for the fiscal year ending June 30, 1892.

Expenses:	
Dockage.....	\$165.42
Rent of land.....	285.00
Total.....	450.42
Expenditures:	
Office and general administration.....	5,976.52
Canal and locks.....	40,593.85
Dredging.....	18,499.58
Total.....	65,069.95
Rebuilding two mud scows.....	1,155.75
Trimming canal wall and rock below.....	1,573.76
Building new middle gates.....	5,153.66
New winding engine.....	1,026.14
Aggregate.....	73,979.26

Statement of expenditures for operating and care of Louisville and Portland Canal for the fiscal year ending June 30, 1892.

Office and general administration:	
Salaries.....	\$4,910.03
Supplies.....	114.07
Miscellaneous.....	952.42
Total.....	5,976.52
Canal and locks:	
Labor.....	21,687.16
Supplies.....	3,400.64
Repairs and extra labor.....	15,506.05
Total.....	40,593.85
Dredging:	
Labor.....	10,445.67
Supplies.....	4,344.21
Repairs.....	3,709.70
Total.....	18,499.58
Grand total.....	65,069.95
Rebuilding two mud scows.....	1,155.75
Trimming canal wall and rock below.....	1,573.76
Building new middle gates.....	5,153.66
New winding engine.....	1,026.14
Aggregate.....	73,979.26

Abstracts of proposals for furnishing and delivering forage at the Louisville and Portland Canal received in response to the advertisement dated April 25, 1892, and opened May 26, 1892, by Lieut. Col. G. J. Lydecker, Corps of Engineers.

Articles.	Quantity (more or less).	(1) Bid of B. B. Conner.	(2) Bid of Geo. Becker & Co.*
timothy hay.....	pounds.. 10,000	† \$0.80	† \$0.80
black oats.....	bushels.. 300	.44	.40
ear corn.....	do 100	.50	.55
wheat straw.....	pounds.. 100	† .40	† .40
middlings bran.....	do 600	† .70	1.75
		266.60	259.90

*Accepted.

† Per hundredweight.

NEW MIDDLE GATES.

completed during the season of 1891 and were allowed to remain in place until the high-water season occurred in the spring, when they were removed on April 23, 1892. Since this time they have been floating in place at such a time as will least interfere with navigation.

ANCHORS AND GUY RODS, MIDDLE GATE COLUMNS.

The anchors which secured the guy rods used to stay the suspension columns had been removed from the masonry, in which they were embedded to a depth of 20 feet. A new system of anchorage has been put in, consisting of bars $3\frac{1}{2}$ inches in diameter, extending 20 feet down into the masonry of lock walls. These anchors, six in each column (one more than the old system), were placed in holes drilled through the masonry to the depth named, and cemented therein with English Portland Cement.

The ends of anchor bars have stout heads 2 inches larger in diameter than the bars. The upper ends have carefully forged eyes through which pass the guy rods, $2\frac{1}{2}$ inches diameter, with cross bars, connecting with the guys, which are attached to the top of the column.

The new system of anchorage was successfully completed without interfering with the operation of the gates and holds perfectly firm and secure.

REPAIRS TO LOCK GATES.

Repairs were made to the upper, middle, and lower gates of the new locks as occasion demanded. Nine cast-iron bearings, or valves, were replaced and three bearings of the same were put in place. The replacement of the latter was a very troublesome operation, involving the use of a partly submerged dock, in which the men were stationed. A number of the truss rods were also replaced when broken.

PAINTING.

The engine houses and portions of the gates and iron work were painted.

TRIMMING CANAL WALL.

This work was continued during the year whenever the water was sufficiently low. The season for such work is necessarily very short, as the most troublesome ledges are only exposed during a very low stage of water in the river, and then for but a brief period.

The manner of conducting this work was the same as in preceding years, viz, by stonecutters dressing the stone while working from flats.

BRIDGES.

Repairs were made to the flooring and stringers of bridge at new locks and temporary repairs to lower chord of bridge at old locks.

The bids received for construction of a new iron drawbridge at new locks having been unsatisfactory, all bids were rejected, and a new opening of bids will become necessary.

The old wooden drawbridge at old locks is in a very insecure condition, and the temporary repairs alluded to above will only suffice to carry the bridge along for a few months, within which time a new structure should be erected.

SHOPS AND OTHER BUILDINGS.

A fire, which occurred on the evening of February 12, destroyed the frame building used for boiler and engine house and machine shop. The origin of the fire was unknown. The city fire department and the regular canal force, with our own fire apparatus, succeeded in confining the fire to the building in which it originated. The building was entirely destroyed, and the shafting, pulleys, and machinery more or less injured. The work of rebuilding in stone was commenced within a short time after the fire, and the engine and boiler houses are now completed. The entire structure will be in shape like two Ls joined together—the boiler and fuel rooms, 18 by 14, constituting one L; the machine shop, 26 by 50, another L, and the two united boiler and fuel rooms, 14 by 18. The fuel rooms are rendered fireproof by brick arches supported on 7-inch iron I-beams. The exhaust fan, for conveying shavings from

INDIANA AND ILLINOIS.

of late years been carried on applicable to improvements in Ind. Excepting the construction of the Mount Carmel and Rapids, near Mount Carmel, of the same nature. With the completion of this lock and dam, operations would be necessary to secure a navigable low-water channel by excavating channels through the rapids, to concentrate the flow by properly located locks, and to clear the channel at some points, and to clear the

STETS BE

ENNES.

section of the river at Mount Carmel. The work has been done and completed, and earth filling and structures auxiliary to the main work remains to put in the covering apparatus and all preparations for the work, and, the material having been delivered, the work commenced September 25 and completed November

the past year was on the beginning of the year the work had been about completed. During the past fiscal year secured in positions, gates were laid and graded behind the work of construction re-commenced in working order besides the gates.

building the west abutment of the dam, and, the material having been delivered, the work commenced September 25 and completed November

on the lock during the year comprised the laying of masonry; 4,676 cubic yards earth were hauled and laid on the lock wall, and 1,554 cubic yards of mud and silt were removed from the lock chamber. The total masonry in the lock is 1,200 cubic yards, and the whole work is of a most substantial and permanent construction. Work on the abutment of the dam comprised the laying of 1,200 cubic yards of earth, laying 655 cubic yards of masonry, and 1,100 cubic yards of earth against the abutment.

In connection with this permanent work at Mount Carmel the snag boat "Ford" was employed up to December 1 in removing snags from the river at that point and the mouth of the river, principally in the vicinity of New Harmony, Little Chain, and Black's Cut-off. At these points the accumulation of snags was such as to establish a complete blockade. The crib dike at Little Chain was repaired with stone; 429 snags, weighing in all 1,786 tons, were removed. The result of this work has been to materially improve the condition of this portion of the river; but there remains a shoal, where, at low water, a navigable depth of only 15 feet exists.

The worst place is at New Harmony, where, the reconstruction of a dam which formerly closed the cut-off at that point, the channel depth has been reduced to 12 inches at low water, but 2½ feet, as was the case when the river was held to a

The reconstruction of the dam is of great importance,

2030 REPORT OF THE CHIEF OF ENGINEERS

planing mill to fuel room, is located directly above this boiler and engine rooms is entirely of iron, both trusses

The roof of machine shop will have corrugated-iron covering. The floors throughout are of concrete. The stone used was taken up by the dredges from old canal walls and

A new stationary engine was purchased, and also a furnishing power to the planing mill. This shafting is 30 feet long.

REPAIRING MUD SCOWS

The timber and iron work has been purchased for old mud scows. These boats have been pulled and work of rebuilding commenced.

The fire, which disabled the engine used for the dressing of lumber, but this cause of delay

Very respectfully, your obedient servant,

Lieut. Col. G. J. LYDECKER,
Corps of Engineers, U. S. A.

COMMERCIAL

Statement of vessels passed through the Locks
year ending

Vessels.
Passenger boats.....
Towboats.....
Government boats.....
Coal boats and barges.....
Small craft.....
Rafts.....
Total.....

Lockages, 3,600.

Statement of commerce passed through
year

Articles.
Coal.....
Salt.....
Oil.....
Whisky.....
Tobacco.....
Cotton.....
Lumber.....
Corn.....
Wheat.....
Miscellaneous.....

Total tons, 1,620,586.

ely through the lock walls
osing are of cast iron and of

s, each gate is opened and shut
that this will be amply sufficient.

	Cubic yards.
.....	1,442
.....	6,890
.....	7,532
.....	4,676
.....	1,554
.....	\$7,870

er miter sill were bolted down to the bed
15 feet long.
of abutment was furnished by the Romona
upt contracted for being delivered by Octo-

uilding of storehouse for cement and tools had
was all received the work of excavation for
gor.

eed in going to the bed rock on account of quick-
the rock. A small cofferdam was found necessary.
reby increased. Work was begun September 25
1891.

masonry wall 10 feet thick at the bottom, 7 feet at the
of wall at right angle to the dam proposed, line of
the height of proposed dam, the face of remainder
with quarry-face masonry.
as pertaining to the abutment:

	Cubic yards.
.....	655
.....	1,520
behind abutment	1,100
.....	\$7,229.11
.....	2,965.18
.....	10,194.29
.....	1.53

REMOVAL OF SNAGS.

h the project the snag boat *Richard Ford* was employed in remov-
of snags between Mount Carmel and the mouth of the river.

snags were removed between Grayville and New Harmony, but
number were removed at Little Chain and Black's Cut-Off, where
snags had accumulated to such an extent as to shut off the channel
these places were almost entirely cleaned out. During some two
river was at the very lowest stage the crew of the snag-boat was
repairing and building new cribs in the dike at Little Chain and remov-
yards of loose rock which had rolled into the chute.

at returned into winter quarters on December 1st, remaining at Mount
June 25th, when she was sent to Vincennes to remove snags on the
r above Vincennes.

done by the snag-boat was as follows:

snags removed and destroyed.....	429
snags removed and destroyed..... tons..	1,786
days repairing cribs at Little Chain.....	10

CONDITION OF THE RIVER.

removal of snags during last season has undoubtedly been of great benefit to
gation, but nevertheless, the river, at very low water, is not practicable for
wing over 15 to 18 inches of water, except in a few places where deep pools
between shoals.

Wabash River, below Vincennes, for fiscal year ending June 30, 1892.

Articles.	Tons.	Approximate value.
.....	25, 635	\$659, 347. 50
..... and rafted	25, 250	153, 200. 00
.....	60, 885	\$812, 447. 50

Statement of commerce, Wabash River, above and below Vincennes.

Years.	Tons.	Approximate value.
.....	122, 729	\$1, 817, 947. 00
.....	58, 614	535, 096. 00
.....	106, 513	1, 573, 698. 00
.....	100, 443	641, 107. 00
.....	103, 415	1, 162, 298. 00
.....	122, 535	2, 037, 242. 40

IMPROVEMENTS ABOVE VINCENNES.

It was the intention at the beginning of the year to clear a number of miles from this section of the river, but it was found impracticable to make any suitable arrangements for doing the work. The snagging boat owned by the Government was totally unserviceable, and no contract for doing the work could be obtained from private parties on satisfactory terms; as a result no work was done.

Towards the close of the year the snag boat *Richard Ford*, which was engaged on the lower river during the last season of operations, was sent up to Vincennes, and will be kept at work above that point during the ensuing season. In the meantime a new hull for the steamer *Wabash* is in process of construction and will be fitted for snagging work; when completed it will be an easy matter, with the two boats, to keep the entire river clear of snags with annual appropriations of \$5,000 for the operation of each boat.

Besides the removal of snags an additional expenditure of about \$100,000 would be of great benefit to navigation on this section; it is proposed for opening channels through the worst shoals, the construction of levees for a proper concentration of flow, and for shore protection in several localities.

The commerce on this section for the past fiscal year is reported at 60,885 tons of freight, valued at about \$1,250,000.

I transmit herewith report of O. L. Petitdidier, assistant engineer, containing commercial statistics gathered by him.

In referring to the opinion expressed in the first part of this report, I would invite consideration to the propriety of making a single appropriation of \$60,000 for "improving the Wabash River, Indiana and Illinois" for the year ending June 30, 1894, in lieu of separate appropriations of \$40,000 and \$20,000 for improvements below Vincennes and above Vincennes, respectively.

Money statement.

1891, balance unexpended.....	\$5, 742. 26
1891, amount expended during fiscal year	974. 28
1892, balance unexpended.....	4, 767. 98
1892, outstanding liabilities.....	440. 00
1892, balance available.....	4, 327. 98
Amount appropriated by act approved July 13, 1892.....	5, 000. 00
Amount available for fiscal year ending June 30, 1893.....	9, 327. 98

The worst impediment to navigation outside of the Grand Mound is at New Harmony, where a cut-off, at one time shut off half of the water of the main river, which could formerly be navigated by boats drawing 24 to 30 inches of water, is now impassable stage by boats drawing 12 inches.

The portion of river thus deprived of navigation is 12 miles in length. As the island which is now between the cut-off and the main river at high water and is composed of a sandy soil of very loose nature, it is possible to devise any sort of closing for the cut-off which would be necessary in low season, and this in the present enlarged size of the cut-off would be a great improvement.

At Grayville Bend Cut-Off.—Where an examination was made in 1881, the levee built by the United States is still in good order, while the work accomplished what was expected of them. At this place the bank is still in good order and it is probable that some additional work of protection will be required in the future.

Grand Rapids.—Near Mount Carmel, Ill., where the greatest impediment to navigation is at present, the improvement, consisting of the removal of snags, can be completed by the end of the present fiscal year.

In all in all, at very low water, navigation is suspended, not only at New Harmony, but also at the Grand Rapids, and the improvement which the United States are necessarily inefficient, but the work accomplished in the river seems either to diminish or the reefs seem to be in good order, which grow longer as the drainage of the river is improved.

PROJECT.

In the present fiscal year it is contemplated to complete the improvement at New Harmony, consisting of the removal of snags. Should any new appropriation be made available during the present fiscal year, it would be possible to complete the improvement at New Harmony. The timber for dam to be delivered during next summer could be quarried during the present summer season, and the stone which will be needed for the dam. This stone could be transported during the high water of 1893, and no delay would be experienced in the construction of the dam. The outlet already built are useless until after completion of the dam. The statistics of the river below New Harmony, with the commercial statistics of the river below New Harmony, for the past six years.

The increase in the tonnage has taken place while the commerce has nearly doubled; this difference being due to greater production of goods.

The increase of commerce is principally due to the high water of the last three months.

Very respectfully submitted.

Your obedient servant,

O. J.

Lieut. Col. G. J. LYDECKER,
Corps of Engineers, U. S. A.

COMMERCIAL STATISTICS.

List of steamboats and barges plying on Wabash River, below Vicksburg, for the year ending June 30, 1892.

Name.	Kind.	Tonnage of boat.	Tonnage of barge.	Tonnage to
Emma Cooper and 5 barges.....	Towboat.....			
Eugene and 5 barges.....	do.....			
E. S. Ragon.....	Stern wheel.....			
J. P. Drouillard.....	do.....			
John Fowler.....	do.....			
Emma Evans and 4 barges.....	do.....			
Rosedale and 3 barges.....	do.....	147	385	
Alex Perry.....	do.....			
D. A. Nesbit and 6 barges.....	Towboat.....		1,450	
Cumberland and 1 barge.....	Stern wheel.....	400	80	
Irene and 2 barges.....	Side wheel.....	60	60	
Peankishaw and 1 barge.....	Stern wheel.....	53	7	
Diana and 2 barges.....	do.....	60	1	

2036- REPORT OF THE CHIEF OF ENGINEERS, U. S. ARMY.

{ Amount (estimated) required for completion of existing project..... \$
 Amount that can be profitably expended in fiscal year ending June 30, 1894
 Submitted in compliance with requirements of sections 2 of river and
 harbor acts of 1866 and 1867.

REPORT OF MR. O. L. PETITDIDIER, ASSISTANT ENGINEER.

MOUNT CARMEL, ILL., June 5.

COLONEL: I have the honor to submit the following report upon the improvement of the Wabash River above Vincennes during the fiscal year ending June 30, 1894.

The plan of operations contemplated at beginning of fiscal year consisted in the removal of numerous snags which were obstructing the channel between Vincennes and Terre Haute, a distance of 90 miles.

As the steamer *Osseo*, which had been used formerly in towing the snag boat, became entirely unserviceable, an effort was made to secure the services of another boat to take its place, but owing to unacceptable restrictions put on by the Government of towboat the offer could not be accepted.

It was determined to build a new hull and fit it up with machinery of a different type and the snag scow, thus giving us a more manageable single snag boat and the necessity of employing a double crew as formerly.

In continuation of this plan the necessary timber was ordered early in the fall of 1892, and work was begun as soon as stage of water was such as to make a low bank which was to be used as a shipyard.

The long-continued high water of the spring of 1892, lasting nearly three months, having delayed the building of new hull until it was found that the new boat could not be prevented from passing over the Grand Rapids near Mount Carmel before the water set in, the snag boat *Richard Ford* was sent up to Vincennes, and put in readiness for the work of removing snags between Vincennes and Terre Haute.

Condition of river.—The condition of the river remains unchanged since the last report, the snags still forming at low water a very great obstruction to navigation.

The commerce is still considerable, and, as will be seen by the commercial statistics here appended, shows a very decided increase over the previous year. This is due to the long-continued high water of this spring, which has been very favorable to navigation.

Project.—During the present fiscal year it is intended to resume snagging operations and push work vigorously until funds on hand are exhausted.

I forward herewith tabular statements showing amount of freight carried during the part of the river during the past fiscal year.

Very respectfully submitted.

Your obedient servant,

O. L. PETITDIDIER,
 Assistant Engineer.

Lieut. Col. G. J. LYDECKER,
 Corps of Engineers, U. S. A.

COMMERCIAL STATISTICS.

Statement of commerce, Wabash River above Vincennes, for fiscal year ending June 30, 1894.

Articles.	Tons.
Grain.....	55,204
Lumber.....	873
Stave belts.....	882
Salt.....	373
Wood.....	328
Shingles.....	2,021
Merchandise.....	2,820
Stock.....	
Total.....	

Passengers, 17,404.

1901
June 1st

Balance
\$181,000
71,145
137,855
258,330
93,150
174,000

ending June

EE

1901
Zleton.

STATEMENT OF

Receipts and Disbursements
of the
Department of
the Interior
for the
Year ending
June 30, 1901

REPORT OF MR. O. L. PETITDIDIER, ASSISTANT ENGINEER

MOUNT CARMEL, ILL., J

COLONEL: I have the honor to present the following report of work on the White River, Indiana, during the fiscal year ending June 30, 1892:

The work intended to be done at the beginning of the fiscal year consisted of the completion of improvement at Kellys Ripple and removal of snags from the river.

Owing to the very low stage of water prevailing on the Wabash River during the whole summer, it was found impracticable to take the snag boat *Richard*, which it had been intended to remove snags, up to White River, so the work was confined to completing the improvement at Kellys Ripple.

The work done at this place has consisted of the construction of a dike 350 feet long, 10 feet wide, and 5 feet high, from the head of the south bank to the left bank of river; the widening of entrance of chute for a distance of 240 feet; and rebuilding for a distance of 240 feet, of the north dike, which had been destroyed since its construction.

I append the following data in regard to the work done:

Linear feet of logs used
 Linear feet of driftbolts used
 Cubic yards of stone filling used
 Approximate cost of work

Work was begun on July 16 and continued until its completion, October 1, 1892.

The improvement, as completed, is now of great assistance to navigation.

Condition of river.—With the exception of Kellys Ripple, where the improvement has been completed, the condition of the river is no better than in the previous year; numerous snags render navigation difficult and dangerous, while the caving and cutting of banks brings in fresh obstructions; such as the Eddy, three-fourths of a mile above Deckers, where a row of piling was driven to protect the bank is now by reason of successive erosion behind it, a pile of the channel, and the obstruction most complained of by steamboats.

The bridge at Rodgers is still unprovided with a draw, but no complaint has been heard in regard to it, there being no navigation in this portion of the river.

Project.—It is intended during the present year to resume the removal of snags and obstructions as soon as practicable, and until the present funds are exhausted.

Should any additional funds become available during the present fiscal year, work of snagging could be continued through the whole season.

I append commercial statistics for the present fiscal year, also comparative statistics for the five years previous. It will be seen that there has been quite a decline in the commerce of the river; this has been mainly due to the favorable stage of the river during the last three months.

Very respectfully submitted.

Your obedient servant,

O. L. PETITDIDIER,
 Assistant Engineer.

Lt. Col. G. J. LYDECKER,
 Corps of Engineers, U. S. A.

COMMERCIAL STATISTICS.

Statement of commerce, White River, Indiana, fiscal year ending June 30, 1892.

Articles.	Tons.
Grain.....	4,630
Logs and lumber, boated and rafted.....	18,500
Total.....	

Locks 7 and 8 were completed in 1892, as well as parts of the foundations of dams 7 and 8. When these two locks and dams are added to the finished works there will be a very great gain in the availability of navigation at all seasons on the river.

Mr. A. M. Scott has continued to exercise the local charge in his usual efficient manner. His report, which is appended, treats in detail of the operations of the year.

As the improvement of the river has progressed the commerce on it, notably the shipment of coal, has greatly increased.

The telephone line has been maintained between the central office at Charleston and the locks, and it is used by night as well as by day being found indispensable for the proper oversight and direction of the operations, as well of construction and of maintenance.

A gauge-reader has been kept at Kanawha Falls, near which place the Gauley joins the New to form the Kanawha River; and another at Hinton, where the Greenbrier empties into the New River. The compensation of these men is less than \$10 a month each. They send to the central office daily reports, by postal cards, of the stage of the river at their respective stations and by telegraph when there is a rapid rise. These reports are necessary as warnings to the central office in Charleston, in order that such maneuvers of dams, etc., may be had in time as the height and duration of the freshets may require.

For perfect security a similar station should be occupied at some point on the Upper Gauley, and perhaps also on the Elk.

The following are the amounts and dates of appropriations for improving Great Kanawha River, West Virginia:

March 3, 1873.....	\$25,000.00	August 2, 1882.....	\$200,000.
June 23, 1874.....	25,000.00	July 5, 1884.....	200,000.
March 3, 1875.....	300,000.00	August 5, 1886.....	187,500.
August 14, 1876.....	270,000.00	August 11, 1888.....	350,000.
June 18, 1878.....	222,000.00	September 19, 1890.....	300,000.
March 3, 1879.....	150,000.00		
June 14, 1880.....	200,000.00	Total	2,629,500.
March 3, 1881.....	200,000.00		

UNITED STATES ENGINEER OFFICE,
Baltimore, Md., January 8, 1892.

GENERAL: The estimate for the project for the improvement of the Great Kanawha by the method of locks and movable dams was prepared in 1875, seventeen years ago.

Several circumstances seem to make the present a suitable time for its final revision. A revised estimate is therefore submitted below. One reason why this has not been done sooner was the hope that in the project for the Ohio it might be known whether it was likely a dam would be built below the mouth of the Kanawha near enough to raise the water to such a distance as to influence the location of the dam in the Kanawha near its mouth and thus perhaps save the expense of one site in the Kanawha.

In a letter of September 26, 1891, from Col. Merrill he says: "I would therefore advise you to make your location on the Great Kanawha River without regard to what may be done on the Ohio."

The original estimate amounted to \$4,000,000. The appropriations since have amounted to \$2,579,500.

This would leave a sum of \$1,500,000 to be still provided if the original estimate were adhered to, but reasons have arisen for changing it. In four or five years became apparent that the upper dam of the series estimated for would not be necessary, as the commerce above the pool of No. 2 would not justify so large an expenditure as the occupation of Site 1 would have required.

When the original estimate was made it was necessarily founded on French drawings and experience, as a movable dam of the Chanoine type had not been built in America. The knowledge of the river since gained by our own experience and more detailed surveys than the hurried ones upon which the first estimates were based has also been great. We have thus been enabled to rearrange the locations of the sites

to be occupied and to omit another of the twelve locks and dams originally provided for. For these considerations the estimate was reduced by \$600,000 in 1878.

The amount asked for in the last Annual Report for completion of the work in the occupation of ten sites was \$670,000. The following revised estimate was lately made with great care by Mr. A. M. Scott, the resident engineer at Charleston:

"COLONEL: In compliance with your instructions I have the honor to submit below an estimate for completing the Great Kanawha slack-water improvement to the mouth of the river:

For Lock and Dam No. 9, complete, not including inspection, engineering, office expenses, etc.....	\$320,000
For Lock and Dam No. 10, complete, not including inspection, engineering, office expenses, etc.....	325,000
For Lock and Dam No. 11, complete, including purchase of site, not including inspection, engineering, office expenses, etc.....	485,000
Amount required to complete locks and dams Nos. 7 and 8, in addition to money now available.....	15,000
For additional lock houses at locks Nos. 2, 3, 4, 5, 6, 7, and 8, sixteen houses in all.....	12,000
Dredging in pools at heads of shoals and approaches.....	30,000
	<hr/>
	1,187,000
For inspection, engineering, general expenses, and incidentals, 10 per cent.....	118,700
	<hr/>
Total.....	1,305,700

"The cost of No. 11 over 9 or 10, is due partly to the greater width of the river at site 11 (being from 100 to 210 feet wider than at the other sites), but also to the greater depth of foundations. The bed rock at 11 and in that vicinity is of a harder nature. The foundations will average from 6 to 8 feet below low-water mark, and the masonry, concrete, excavation, and expenses of cofferdams, will be more extensive than at any of the other sites, increasing materially the cost of the work.

"The estimate submitted is regarded as a 'full estimate,' and it is believed the actual cost of the completed work will be made to fall below it. It was thought advisable, however, to make the estimate large enough to cover considerable uncertainty, unavoidably connected with such works.

"Col. WM. P. CRAIGHILL,
"Corps of Engineers."

The cost of what has been done is considerably greater than it would have been if the work had not been protracted through so many years instead of being entirely finished in three, as it might have been had money been available. The whole work could have been done in the same time as at each site, and three years is a full allowance of time for any site.

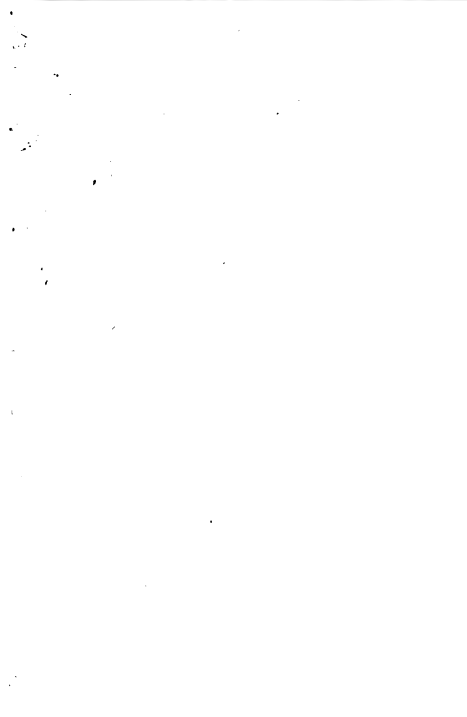
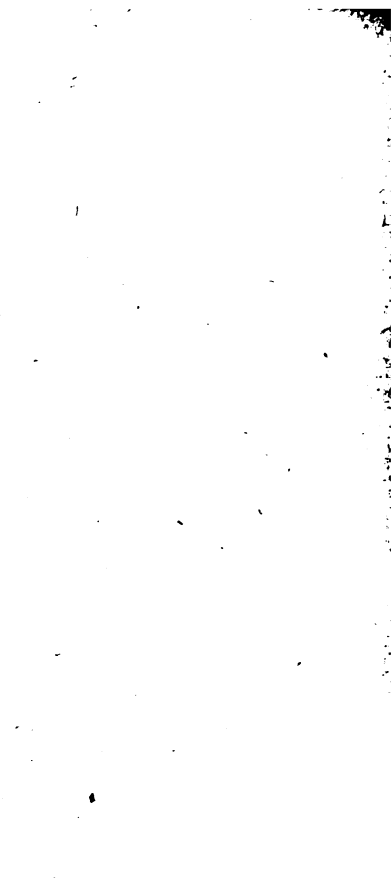
Moreover, the size of the locks has been increased by proper authority beyond what was originally proposed.

Work is nearly completed at sites 7 and 8. The completion of No. 8 marks an unusually important step in the improvement of this river, which has been a success from the commercial as well as the engineering point of view.

When the dam of No. 8 is finished, boats can at all times come as low as its pool for a harbor, and when there are often within reach of the back water of the Ohio in its freshets which may be in progress when the Kanawha is not.

Under such circumstances the coal, etc., from the Kanawha may at once pass over the unimproved lower part of that river and down the Ohio to a market at Cincinnati and the other great cities on the Ohio and Mississippi rivers and their dependencies by water and rail.

Moreover, should the backwater from the Ohio not come fully to the pool of Dam 8, it is thought it may at times be supplemented by using the water of pool 8 and others above it to flush the portion of the Kanawha below 8 sufficiently to enable boats to reach the Ohio at some stages when such movement would otherwise be impossible, when there would be a boating stage in the Ohio and not in the Kanawha.



APPENDIX F F.

IMPROVEMENT OF GREAT KANAWHA, ELK, AND GAULEY RIVERS, WEST VIRGINIA, AND OF NEW RIVER, VIRGINIA AND WEST VIRGINIA.

REPORT OF COL. WM. P. CRAIGHILL, CORPS OF ENGINEERS, OFFICER IN CHARGE, FOR THE FISCAL YEAR ENDING JUNE 30, 1892, WITH OTHER DOCUMENTS RELATING TO THE WORKS.

IMPROVEMENTS.

- | | |
|--|--|
| <ol style="list-style-type: none"> 1. Great Kanawha River, West Virginia. 2. Operating and care of locks and dams on Great Kanawha River, West Virginia. | <ol style="list-style-type: none"> 3. Elk River, West Virginia. 4. Gauley River, West Virginia. 5. New River, Virginia and West Virginia. |
|--|--|

(For letter of transmittal see Appendix I.)

F F I.

IMPROVEMENT OF GREAT KANAWHA RIVER, WEST VIRGINIA.

The object of the improvement has been to give a depth of not less than 6 feet all the year round throughout the whole river, 96 miles. The means are locks and dams. The locks are about 300 by 50 feet, above Charleston, and about 340 by 55 feet below. The following table shows the present condition:

No.	Distance in miles from Charleston.	Style of dam.	Completed in—	Remarks.
2	26 miles above	Fixed	1887	In operation.
3	21 miles above	do	1882	Do.
4	15 miles above	Movable	1880	Do.
2	9 miles above	do	1880	Do.
6	4 miles below	do	1886	Do.
7	14 miles below	do		Lock finished and dam under contract.
8	22½ miles below	do		Do.
9	32 miles below	do		Site purchased September, 1890.

Two more sites will require to be occupied below No. 9. Some dredging is also necessary in some of the pools, as well as the occasional removal of snags and rocks.

dam and foundations, is given on page 198 to 1889, in 1891, owing to misprint, the 4 instead of 41, the correct length, and is set 1890.

Reference may also be made here to the description of the lock and dam, finished, page 304, photo-ethographed by the C. E.

The cost of labor referred to coffer-dam gates. It included coffer-damming, put work, and all materials except the iron needed by the United States and placed the prices and payments on this contract.

Grubbing and clearing site, 1,111,111,111
Crib logs in cofferdam, 53,903 linear ft.
Sheathing in cofferdam, 31,870 feet, B. S.
Piling in cofferdam, 7,361 cubic yard
Excavation, common, including dredge
Excavation, hard pan, 3,061 cubic y.
Excavation, rock, 11 cubic yards, at
Embankment, 11,031 cubic yards, at
Puddle, 2,23 cubic yards, at \$1.50
Concrete, 600 cubic yards, at \$5,111
Concrete below Ref. 530, 2,713 cubic
Backing masonry, 5,221 cubic yard
Rock face masonry, 2,017 cubic ya.
Pointed face masonry, 1,836 cubic
Cut stone, bush hammered corners
Sill, 219 cubic yards, at \$18,111
GROUT, 77 cubic yards, at \$25
Course, 377 cubic yards, at \$25
Stone filling in cribs under paving
Riprap, hand-placed, 1,987 cubic
Paving, 1,171 cubic yards, at \$5
Timber in permanent construction
Bolt holes drilled in masonry, 1

Amount, 1,111,111,111

The cost of labor on attached fastenings, pivot plates, ladder \$1,000,000. The lock gates, the coffer-dam and the gates built are \$1,800, making the cost of the \$10,525.

Foundations and masonry.

described generally on pages 189, 1891. They are also fully set

on the report of Dec. No. 7 above. They are among the very

described in building the four

of the foundation of concrete

of the foundation of concrete

of the foundation of concrete

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The service bridge and wicket irons, trees, and wicket boxes for the navigation of the other irons mentioned above, embracing 38,500 pounds of cast iron. The works are meeting with delay about getting the account of the strike now prevailing in some delay will be but temporary. The Chief of Engineers for 1891, pages 2416 and

COMPLETED WORKS.

in 1887. (Located 85 miles from the mouth of the river.) Regular operation during the year. Locking was suspended by high water. The commerce through the lock

.....	bushels..	1,277,800
.....	tons..	16,020
merchandise, produce	by steamboats..do	5,506
.....	feet, B. M..	96,800
.....	number..	296
.....	do.....	1,674
.....	do.....	164
.....	do.....	25,000
.....	do.....	10,500
.....	do.....	5,938
.....	do.....	1,766

did some damage and made it necessary to riprapping; 1,500 cubic yards of stone were used in extending the riprap below the dam; 850 yards of this were taken from the old quarry near the site, the rest being either from the old quarry or below the dam. The work was done by regular lock hands, assisted by some hired labor and teams. The riprap on the downstream side now extends 1,210 feet and on the upstream side now extends 1,210 feet and on the

in 1882. (Located 80 miles from the mouth of the river above Charleston.) Regular working order during the year. Locking was suspended by high water. The commerce through the lock was as follows:

.....	bushels..	1,796,000
.....	tons..	16,020
merchandise, produce, etc., by steamboats	..do	7,148
.....	feet, B. M..	477,200
.....	number..	429
.....	do.....	1,682
.....	do.....	70
.....	do.....	55,000
.....	do.....	72,000
.....	do.....	8,870
.....	do.....	1,778

In addition to the ordinary repairs about the works, 530 cubic yards of stone were used in repairing and extending the riprapping below the dam; 1,210 yards of this, which were taken from the bar below the dam, and 425 yards of earth moved in grading the banks. The work was done by regular lock hands assisted by some hired labor. The riprap on the downstream side now extends 910 feet and on the lock side 1,212 feet and on the upstream side of the dam.

The working of the new valves put in last year (Report of the Chief of Engineers for 1891, pages 2420, 2421) has been entirely satisfactory.

THE MOVABLE DAMS.

All movable dams with their locks were in good working order during the year. As usual a good deal was done at each lock by regular hands, in keeping the works in order. The most important ones mentioned below under that head. A brief account of the movable dams here follows:

2050 REPORT OF THE CHIEF OF ENGINEERS, U. S. ARMY.

Lock No. 4.—Movable dam, finished in 1880. Located 74 miles from the of the river and 16 miles above Charleston.

The dam was up 150 days during the year and down the remainder of the till was down 12 days to make repairs and the rest of the time on account of sufficient stages of water. The maneuvers of the dam and some of the principal items of commerce at No. 4 are given in the following tables:

Maneuvers of dam.	Date.	Time taken.	Men employed.	Remarks.
		<i>h. m.</i>		
Lowered.....	August 23.....	3 30	5	No difficulty.
Raised.....	September 14.....	8 00	5	Do.
Lowered.....	November 25.....	4 00	5	Some trouble with pier tripping bar
Raised.....	December 1, 2, and 3.....	15 00	5	Water rather too high at beginning, trouble with chains breaking at drift.
Lowered.....	December 5.....	3 00	5	No difficulty.
Raised.....	December 23 and 24.....	13 30	5	
Lowered.....	December 27.....	2 30	6	Lock bar failed after tripping 10 ft rest put down by hand without aid
Raised.....	May 16 and 17.....	16 30	6	Found tripping bar off guides; men spent repairing panels of wickets.
Lowered.....	May 20.....	2 30	6	Some of the bar wickets lowered with no particular difficulty.
Raised.....	June 15, 16, and 17.....	28 00	6	Found lock tripping bar bent and set also some trouble with drift snags etc.

	Through the lock.	Through the navigation pass.
Coal.....bushels.....	2, 249, 000	6, 154, 000
Coke.....tons.....	1, 080	14, 940
Lumber and logs.....feet B. M.....	307, 560	164, 000
Coal barges.....number.....	502	1, 138
Steamboats.....do.....	784	1, 811
Other craft.....do.....	29	54
Number of lockages.....do.....		

Prominent repairs at No. 4.—Owing to trouble with the shortened tripping bar, it was still further reduced in length last fall and 12 ft of shunting hurters put in, leaving 14 of the wickets on that section to be used by the bar. This short bar got off the guides and badly bent again this year, on the whole, made so much trouble that it was decided to take it out, which was done in June. It will not be put back. The remaining shunting hurters on the lock section will be replaced during low water this year with a new pattern. There are still 13 wickets on the pier section operated by the bar.

New top timbers were put on three of the lock gates last winter, they being badly decayed. These gates are now twelve years old. They require occasional small repairs, to last several years yet.

Lock No. 5.—Movable dam, finished in 1880. Located 68 miles from the of the river and 10 miles above Charleston.

The dam was kept up 162 days during the year. The maneuvers and the most important items of commerce at No. 5 are given in the following tables:

Maneuvers of dam.	Date.	Time taken.	Men employed.	Remarks.
		<i>h. m.</i>		
Lowered.....	August 23.....	3 00	5	No difficulty.
Raised.....	September 4 and 5.....	11 30	5	Two hours on track.
Lowered.....	November 25.....	3 10	6	No difficulty.
Raised.....	December 2 and 3.....	14 30	6	About 100 ft of tripping bar broken.
Lowered.....	December 5.....	4 00	7	Lowered 10 ft.
Raised.....	December 23.....	10 00	7	
Lowered.....	December 27.....	3 00	6	
Raised.....	May 16.....	12 00	7	
Lowered.....	May 20.....	1 30		
Raised.....	June 14 and 15.....	18 30		

and estimate was for locks with "clear in: width and from 285 to 300 feet in length." side in the clear and from 300 to 311 feet long was built, the first below Charleston, it was late the coal trade, particularly large-sized of the locks below Charleston 55 feet wide quoins. The coal barges are from 24 to 26 the locks are designed to pass four barges at

was begun, as before stated, in 1875. Progress etc., of each lock and dam are shown on the out features and dimensions of each work, is

No.	Lock dimensions.			Location—miles from mouth.	Remarks.
	Total.	Clear width.	Length between quoins.		
	<i>Feet.</i>	<i>Feet.</i>	<i>Feet.</i>		
1	524	50	308	85	Finished in 1887.
2	564	50	311	80	Finished in 1882.
3	458	50	300	73½	Finished in 1880.
4	515	50	300	67½	Do.
5	558	55	342	54½	Finished in 1886.
6	564	55	342	44½	To be finished in 1892.
7	540	55	342	36	Do.
8	*548	55	342	25½	Not begun yet.
9	*538	55	342	18½	Do.
10	*668	55	342	1½	Do.

* Approximate.

and dams.—The commercial effect of the locks and Kanawha, and the additional benefits to result from the mouth of the river are shown in some detail in reports for 1887, page 1921, etc., and by commercial reports, and will be but briefly referred to here.

Completed in 1880; No. 3 in 1882; No. 6 in 1886, and No. 7 by river for the year ending June, 1881, was 385,148 tons and in 1891 the river shipment was 1,030,454 tons.

Mines above the head of the Charleston pool (the site of the dam) and they were sending out altogether not to exceed now 17 mines above No. 5 shipping by river. The river last year was 546,020 tons. It is evident too that this part of the valley is but a small part of what it is capable of. The coal business is being increased and new mines opened

and about 900 barges engaged in the coal trade on the

passenger traffic has increased and is still increasing and carried about 41,000 tons of miscellaneous freight—

of them being local boats, the others running from Cincinnati, Pittsburg, Gallipolis, or other points on the

THE MOVABLE DAMS.

Chanoine wicket type, operated from trestle-service they are all like Dam No. 7, illustrated and described in the report. They were completed and put in operation in 1880, and were built with slack-water improvement built in America—1886. Nos. 7 and 8 are now building; both are well completed during the present season, 1892.

on the Great Kanawha, the number of, and time taken to get up, number of days the dams are kept up, the cost, etc., each year, are fully described in the Annual Reports.

The movable dams on this river has on the whole been very satisfactorily maneuvered (in these respects Dam No. 6 and

ran up to Cannelton, in good stages of water, generally "winding" through the upper shoals. After the railroad was built steamboats seldom went above the foot of Paint Creek Shoal (now the site of Lock 3), and in low or even medium stages there was but little done by river above the Charleston pool.

In reference to coal shipments from the upper river: Locks Nos. 4 and 5, the first built on the river, were completed and put in operation in 1880. At this time there were but two mines above the site of No. 5 shipping by river, and they were doing but little, shipping altogether not to exceed 8,000 tons per year. Several attempts had been made to ship coal from above the foot of Paint Creek Shoal, at Cannelton, Armstrong Creek, etc., but owing to the risks and uncertainties of navigation this had been entirely abandoned before the slack-water improvement began. The immediate practical effect on commerce of the building of locks and dams in this part of the river will be referred to farther on.

IMPROVEMENT BY LOCKS AND DAMS.

The first appropriation for the improvement of the river by locks and dams was made by Congress in March, 1875. Col. (then Major) William P. Craighill, the engineer officer in charge of the river, in a project for the expenditure of this appropriation, dated April 30, 1875 (Report of Chief of Engineers, 1875, page 90), says: "The system of locks and dams may be considered as affording the most reliable navigation at all seasons of the year; and, as the ordinary construction is open to the objection of delaying boats and breaking up tows, the adoption of movable dams seems the best expedient available." * * * "These will furnish an unobstructed navigation during such times as the river will give sufficient depth of water, which will be not less than six months in each year. The system has not been fairly tested in this country, but its long successful use in France would seem to supply the deficiency and justify its adoption under such favorable conditions as are found on the Kanawha."

"The profile represents approximately the position and height of the movable dams, the lifts of which vary from 6 to 8 feet up to Paint Creek Shoal. Above that point the fall is greater, and it may be better that the movable dam system be not applied to it, but that the rise be overcome by three locks of 15 feet lift each."

On this project a Board of Engineers, consisting of Lieut. Col. H. G. Wright, and Maj. Craighill and Poe, made a report to the Chief of Engineers, under date of May 25, 1875 (Annual Report for 1875, page 94). The following is an extract from the report of the Board:

"To meet the necessities of such economical transportation a depth of at least 7 feet water should be secured at all times. This, as stated by Major Craighill in his report of April 30 last, may be obtained in three ways: By a system of locks and permanent dams; by a system of locks and movable dams, and by a combination of the two; that is, by locks and movable dams from the mouth of the river to a certain point, with locks and permanent dams above."

"They therefore recommend that movable dams be adopted from the mouth of the river to Paint Creek, the first permanent dam being at this point," etc.

This report was approved by the Chief of Engineers and the lock and dam at No. 5 and the lock at No. 4 were put under contract, and the work on them begun in the course of a few months.

The fixed dams, lifts, and number of.—The first project as stated above contemplated three fixed dams of 15 feet lift each above the foot of Paint Creek Shoal carrying the improvement to the foot of Kanawha Falls. It was afterwards deemed advisable to change the plan by reducing the lifts of the fixed dams to 12 feet, and Locks and Dams Nos. 2 and 3 have been so built, the No. 2 pool reaching to the foot of Loup Creek Shoal, as shown on the profile. This is nearly or quite to the upper line of the best coal deposit on the Great Kanawha (being about where the Lower Coal measures run out and the thick top sandstone of the Conglomerate series appears), and it is proposed not to continue the improvement further upstream until the locks and dams are all completed below. The reduction in the lifts will make two more fixed dams necessary if the slack water is carried to the foot of the falls, making four in all instead of three as first proposed. It may be added that the experience at Nos. 2 and 3, particularly in regard to the scour of the banks below the works, has fully justified the change of plan and shown that the height above (for maximum 12-foot lifts) is as great as either of these dams should have built.

Detailed drawings of Lock and Dam No. 2, uniform with those of No. 7 are now under way for publication by the department.

APPENDIX F F—REPORT OF COLONEL

size of the locks.—The first project and estimate was for locks of dimensions of about 48 to 50 feet in width and from 100 to 120 feet above Charleston are 50 feet wide in the clear and 60 feet between quoin. Before Lock No. 6 was built, the river was so silted and ruined, in order to better accommodate the coal trade, that boats in the lower river, to build all of the locks between the clear and 342 feet long between quoin. The locks are designed to pass one or three barges and a towboat.

The building of the locks and dams was begun, as before stated, to date, relative locations, lifts, etc., of each lock and dam profile. This, with some other important features and details, are given in the following table:

No. of lock and dam	Style of dam.	Maximum lift.	Length of dam.			Lock dimensions	
			Naviga-tion pass.	Weir.	Total.	Clear width.	Lock width.
		Feet.	Feet.	Feet.	Feet.	Feet.	Feet.
1	Fixed	12			524	50	
2	do	12			564	50	
3	Movable	7	248	210	458	50	50
4	do	7	250	265	515	50	50
5	do	8½	248	310	558	55	55
6	do	8	248	316	564	55	55
7	do	8½	248	292	540	55	55
8	do	6½	248	*300	548	55	55
9	do	7½	248	*290	538	55	55
10	do	10	248	*420	*668	55	55

* Approximate.

Commercial effects of the locks and dams.—The locks and dams first built on the Great Kanawha, and the subsequent ones completing the improvement to the mouth of the river are detailed in the Report of the Chief of Engineers for 1887, page 127, and the statistics in later Annual Reports, and will be referred to hereafter.

Dams Nos. 4 and 5 were completed in 1880; No. 6 was completed in 1887. The shipment of coal by river for the year 1880 was 200,000 tons. It has increased steadily, and in 1891 the shipment was 1,000,000 tons.

In 1880 there were but two mines above the mouth of the river (Lock 5) shipping by river, and they were shipping only 5,000 tons per year. There are now 17 mines above the mouth of the river, and the output of these 17 mines by river last year was 550,000 tons. The present output of coal from this part of the river is 1,000,000 tons per year, and will become, as the coal and coke business is being developed, 1,500,000 tons per year.

There are now 21 towboats and about 400 barges on the river.

The ordinary freight and passenger traffic has increased rapidly. In 1880 these boats carried about 100,000 tons of merchandise, farm produce, etc. Last year they carried 1,000,000 tons, nine packets in the trade, five of them being for Cincinnati, Pittsburg, Charleston and above, to Cincinnati, Pittsburg, and above, to Ohio.

THE MOVABLE DAMS

The movable dams are of the Chanoine wicket type, and are 100 feet high. In general features they are all like those shown herein. Dams Nos. 4 and 5 were completed and put in operation in 1880, the first movable dams in connection with slack water navigation. Dam 6 was completed in 1886. Nos. 7 and 8 were completed in 1887, and will probably be completed during the year 1892.

The operation of these dams on the Great Kanawha has been very successful in the maneuvers, difficulties met with, number of accidents, etc., are shown on Pl. XI, Report of the Chief of Engineers, and will be referred to hereafter.

The experience with movable dams on the Great Kanawha is detailed on Pl. XI; details of the construction are given on Pl. XII.

those now under construction have considerable advantage over those first built the expense of operation and maintenance is but little if any more than with fixed dams, and they prove highly satisfactory to the river interests.

Advantages over fixed dams.—The movable dams are kept up whenever there is no water enough in the river for coal-boat navigation and down at other times. The advantages over the ordinary fixed dams for a commerce and river like the Great Kanawha are decided, furnishing the benefits of the usual slack water without its most serious drawbacks. With fixed dams everything must pass through the lock with them navigation is entirely suspended, too, when the river is near or above the top of the lock walls. With movable dams the locks are only used when the discharge of the river is so small as to make them necessary. At all other times they are down, practically on the river bottom, out of the way, affording unobstructed open navigation. This is of great advantage to all classes of commerce, and is particularly so with coal, transported as it is, and empty barges returned, in "fleets of large barges. More barges can, of course, be taken by a towboat, and much better time made by all kinds of craft in "open river," when there is water enough for such navigation, than when the stage or discharge compels the use of the locks.

The movable dams being down in high water, there is comparatively little difficulty in protecting the banks about the works from scour. In this respect they have considerable advantage, too, over the fixed dams.

Modifications, cost of operating, etc.—Experience with the dams has naturally suggested improvements, and No. 6, the last one completed, has considerable advantage over those first built in strength and durability of construction, facilities for rapid maneuvering, and cost of operation and maintenance. Dams 7 and 8 have been still further improved in some of their details.

No. 6 has been in operation over five years. The average cost of operating and maintaining the lock and dam has been \$2,515 per year. This covers wages, supplies, repairs, including considerable addition to the riprapping, and all expense connected with the work. The entire cost during the five years of repairs on the dam proper and on all of its apparatus, including paints, one of the principal items has been something less than \$250, or an average of \$50 per year.

This dam is put up by four or five men in from seven to twelve hours; the usual time is about eight hours. It is lowered with the same force in about two hours. No material difficulty has ever been met with in any of the maneuvers at No. 6.

Four men are employed regularly at each work, the same as at the fixed dams. In raising and lowering the dams one or two extra men are often hired.

Maneuvering the dams.—The operation of raising and lowering the dams is generally understood, or will be inferred from the drawings, but may be briefly described as follows: In raising the pass the bridge is first put up trestle by trestle (they are connected by chains as shown), beginning at the lock. As the trestles come up, and with them the aprons that make the walk, the rails forming the connections at the winch track are placed. In raising the trestles the winch (Plate 10) is used by means of the small top crane and sheave. After the bridge is up the wickets are pulled up one by one with the winch and wicket chains until the props drop in the hurter seats. The wickets are not erected or "righted" as fast as pulled up but left "on the swing" (*en bascule*), that is, with the horse erect, the end of the prop, the hurter seat and the wicket in a horizontal position at the top of the horse. In this position the water passes freely under the wicket. If righted as fast as pulled up, the head of water becomes so great that the last wickets can not be safely handled with the winch. After being put on the swing clear across, they are all rapidly righted; this is done with the drum and brake on the winch and wicket chain the butt of the wicket being held against the pressure of the water and let again the sill without shock. In lowering the pass the wickets are pulled upstream a few inches with the winch by a simple line and grab connection at the top of the wicket. This carries the foot of the prop out of the seat into the descending channel of the hurter, when the grab is disengaged and the wicket falls. After the wickets are lowered the bridge is put down. The maneuvers briefly described above refer particularly to the navigation pass. The weir is maneuvered on the same general plan, but the weir wickets being smaller than those of the pass, they can be raised or lowered, put on the swing, or righted with full head whenever desired. The maneuver of the weir when the dam is up is governed by the stage or discharge of the river, it being kept wholly or partly raised as required to regulate the surface of the pool. A pass wicket, for reasons given above, is never lowered or swung unless the whole dam is to go down.

Telephone line, equipments, etc.—Concert of action is necessary between the dams and regulating the pools, and the different works are connected and with the central office at Charleston by telephone. The Kanawha Falls to give notice of floods, and daily communication by telegraph when necessary, is had with Hinton at the Falls, 15 miles above the Falls.

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..... 34,012
..... 343,600

section, which requires the
 such expenses, I have the honor

June 30, 1892, out of the general ap-
 plications and other works of navigation," in
 bills on the Great Kanawha River, West

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.....	\$2,304.83	
.....	1,172.61	
.....	110.88	
.....	4.64	
	<hr/>	\$3,492.96
.....	2,325.06	
.....	449.40	
.....	118.60	
.....	7.62	
	<hr/>	2,800.68
.....	2,320.39	
..... and extra labor.	623.09	
.....	141.28	
.....	5.96	
	<hr/>	2,990.72
.....	2,242.66	
.....) and extra labor.	811.53	
.....	165.76	
.....	6.29	
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.....	2,278.06	
..... (engine-rooms) and extra labor.	728.09	
..... fuel, tools, etc.	128.42	
.....	3.63	
	<hr/>	3,138.20
..... expenses:	3,485.31	
.....	273.99	
.....	9.10	
	<hr/>	3,768.40
..... running expenses of steamboat:	1,263.40	
.....	829.77	
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..... general work.....	53.59	
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.....	<hr/>	22,003.96

FF 5.

IMPROVEMENT OF NEW RIVER, VIRGINIA AND WEST VIRGINIA.

There has been no work on this river in the year ending June 30, 1892. The following are the amounts and dates of appropriations for improving New River, Virginia and West Virginia:

August 14, 1876.....	\$15,000	August 2, 1882.....	\$12,000
June 18, 1878.....	15,000	August 5, 1886.....	10,000
March 3, 1879.....	12,000		
June 14, 1880.....	24,000	Total.....	112,000
March 3, 1881.....	24,000		

Money statement.

July 1, 1891, balance unexpended.....	\$2,341.79
July 1, 1892, balance unexpended.....	2,341.79
(Amount (estimated) required for completion of existing project.....	159,000.00
{ Submitted in compliance with requirements of sections 2 of river and	
{ harbor acts of 1866 and 1867.	

1. A. B. C. D. E. F. G. H. I. J. K. L. M. N. O. P. Q. R. S. T. U. V. W. X. Y. Z.

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APPENDIX G G.

IMPROVEMENT OF CERTAIN RIVERS IN KENTUCKY AND WEST VIRGINIA.

REPORT OF MAJOR D. W. LOCKWOOD, CORPS OF ENGINEERS, OFFICER IN CHARGE, FOR THE FISCAL YEAR ENDING JUNE 30, 1892, WITH OTHER DOCUMENTS RELATING TO THE WORKS.

IMPROVEMENTS.

1. Tradewater River, Kentucky.
2. Operating and keeping in repair locks and dams on Green and Barren rivers, Kentucky.
3. Rough River, Kentucky.
4. Kentucky River, Kentucky.
5. Operating and keeping in repair locks and dams on Kentucky River, Kentucky.
6. Licking River, Kentucky, between Farmers and West Liberty.
7. Big Sandy River, West Virginia and Kentucky.
8. Levisa Fork of Big Sandy River, Kentucky.
9. Tug Fork of Big Sandy River, West Virginia and Kentucky.
10. Guyandotte River, West Virginia.
11. Little Kanawha River, West Virginia.
12. Operating and keeping in repair the lock and dam on Little Kanawha River, West Virginia.
13. Buckhannon River, West Virginia.

UNITED STATES ENGINEER OFFICE,
Cincinnati, Ohio, July 8, 1892.

GENERAL: I have the honor to transmit herewith, in duplicate, the annual reports on the works under my charge at the close of the fiscal year ending June 30, 1892.

First Lieut. William L. Sibert, Corps of Engineers, has been on duty under the direction of this office throughout the year.

Very respectfully, your obedient servant,

D. W. LOCKWOOD,
Major of Engineers.

Brig. Gen. THOMAS L. CASEY,
Chief of Engineers, U. S. A.

G G 1.

IMPROVEMENT OF TRADEWATER RIVER, KENTUCKY.

The Tradewater is a tributary of the Ohio and empties into it 79 miles below Evansville, Ind.

The project for the improvement, adopted in 1881, contemplated originally the removal of obstructions, such as bars, snags, etc., so as to

F F 3.

IMPROVEMENT OF ELK RIVER, WEST VIRGINIA.

Between July 1 and 11 a small party continued the work in progress June 30, 1891, at and near Jarretts and Porter shoals, which are, respectively, 12 and 7 miles from the mouth of the river. Twenty-three snags and sunken trees were taken out and cut up, and a number of leaning trees cut down or the projecting limbs removed. A towpath was also made high up along the bluff at Porter Shoal, and two ring-boats were put in for use in winding boats through the shoal.

Some other work was done in July, August, and September below Clay Court-House, at Yankee Dam, Big Laurel, Big King, Little Spread, Queen, and Porter shoals, in building, repairing, and modifying chute walls and dikes and removing rocks.

In the stretch between the 34 and 38 miles above Sutton, which is itself 100 miles from the mouth of the river, a number of shoals were worked over which had been left in November, 1888. The principal operation was the removal by blasting of about 4,500 cubic yards of large rock to facilitate the movement of much lumber to market in the shape of oose logs. Above Sutton there is no boating or rafting of any consequence, but below there is a considerable movement of rafts and separate logs of valuable timber, railroad ties, and sawed lumber in rafts, all downstream, and of push-boats with merchandise upstream.

The operations of the United States on the river have greatly improved it for the kind of navigation on it. There are still several mills which are unreasonable obstructions to navigation. They have been reported as such to the proper authorities in compliance with the law.

The following are the amounts and dates of appropriations for improving Elk River, West Virginia:

June 18, 1878.....	\$5,000	August 11, 1888.....	\$3,000
June 14, 1880.....	5,000	September 19, 1890.....	2,500
March 3, 1881.....	5,000		
August 2, 1882.....	2,000	Total.....	24,000
August 5, 1886.....	1,500		

Money statement.

July 1, 1891, balance unexpended.....	\$2,754.31
June 30, 1892, amount expended during fiscal year.....	2,595.42
July 1, 1892, balance unexpended.....	158.89
Amount appropriated by act approved July 13, 1892.....	2,500.00
Amount available for fiscal year ending June 30, 1893.....	2,658.89
{ Amount that can be profitably expended in fiscal year ending June 30, 1894.....	5,000.00
{ Submitted in compliance with requirements of sections 2 of river and harbor acts of 1866 and 1867.....	

REPORT OF MR. ADDISON M. SCOTT, RESIDENT ENGINEER.

UNITED STATES ENGINEER OFFICE,
Charleston-Kanawha, W. Va., December 16, 1891.

COLONEL: I have the honor to submit the following report of operations since July 1 on the improvement of Elk River, West Virginia.

The work was stopped October 21 for want of funds, the appropriation being practically exhausted.

With the exception of a few days' work early in July, near the mouth of the river, at removing snags, etc. (in continuation of that reported for the year ending July 1, 1891), work was directed to blasting a channel for logs through the large loose rock on the shoals in the upper river near Webster, Court-House, and in building, repairing, and altering chute walls and dikes and removing rock between Clay Court-House and the mouth of the river.

REMOVING ROCK IN THE UPPER RIVER.

This work was begun September 1 with a hired force under Mr. W. A. Porter, overseer, 5 miles below Addison (Webster Court-House), where it was left November 24, 1888. (Report of Chief of Engineers for 1889, page 1956, etc.) The weather and stage of water were favorable and the work went on without interruption until stopped, October 21, as stated above. This, with the exception of some work at Mill Run and Canoe Tree Shoals, left unfinished, owing to high stages of water in 1888, brought the work up to Addison, being all that was contemplated this season in this part of the river with the funds at hand.

The names and location of the shoals at each are given in the following table.

data connected with the

The distances are given from Sutton is 100 miles from the mouth

(rt-House) as in former table.

Name of shoal.	Distance from Sutton, miles.	Rocks blasted.	Holes drilled and blasts fired.	Rocks blasted and disposed of (approximate).
				<i>Cubic yds.</i>
Falster Lick Shoal	34	50	74	234
Miller Shoal	34½	59	144	1,129
Skidmore Shoal	34½	17	31	272
Christine Shoal	35	23	35	241
Big Island Shoal	35½	38	97	974
Cut Hole Shoal	35½	18	20	151
Fadry Shoal	36	53	66	789
Upper Cut Hole Shoal	36½	43	57	444
Flares Shoal	37	2	9	67
Fall Hole Shoal	37½	15	34	148
Shoal just below Addison	38	2	13	19
Total		320	580	4,418

There is a large quantity of fine timber on the Upper Elk, and all of the work done by the Government on this part of the stream has been to assist in driving and floating loose logs. Owing to the character of the stream above Sutton no rafting or push-boating is done above that point.

As in 1888, and described in the report referred to, the work in this part of the river consisted in blasting away the worst rocks on the shoals to make a narrow channel, generally about 25 feet wide, for logs to run in.

On most of the shoals worked on the passage could be made wider to advantage by more blasting, and in several places short dikes and dams would be useful in keeping logs out of bad lodging-places in pockets and behind islands.

WORK BELOW CLAY COURT-HOUSE.

This work was done by hired labor in July, August, and September. Mr. Leslie Frame, a pilot and lumberman of Braxton County, was the overseer on the most of this work. It consisted principally, as stated above, in building, repairing, and changing chute walls and dikes and removing rock on the most troublesome shoals. The names and location of the shoals and a brief description of what was done at each here follows:

Yankee Dam Shoal (46 miles from mouth of river and 7 miles below Clay Court-House).—Three riprap dikes were built, two at the head and one near the foot of shoal, to concentrate and direct the water in log and rafting stages. In addition, wall bar or tow-head and a piece of an old dike were taken out on the right, near head of the shoal, the bar being reduced by horse scrapers. About 554 cubic yards of stone were used in building the dikes, all of which were on the shoal, and mostly out of the rafting channel. Material moved out of old dike, about 210 yards.

2066 REPORT OF THE CHIEF OF ENGINEERS, U. S. ARMY.

Big Laurel Shoal (37 miles from mouth of river).—Built a timber and stone dam at head of shoal out from left shore, past head of island, 594 feet long, 7 feet wide on bottom, and about 3 feet high. Also repaired and strengthened an old dike on the right shore; 1,170 linear feet of round timber and about 415 cubic yards of new stone were used in the dam and dike.

Big King Shoal (31 miles from mouth of river).—Built a timber and stone dam at head of shoal 255 feet long, 9 feet wide at base and 6 feet at top, and 5 feet high from right bank to island. Material used in dam, 2,950 linear feet of round timber and about 255 cubic yards of stone.

Also moved an old riprap wall on left of channel and rebuilt it, lengthening both ends, near left shore. This wall, as first located, proved to be a good deal in the way of rafts and boats at certain stages of water, and was changed at the request of several of the best pilots. About 365 cubic yards of stone were handled in moving and lengthening the wall.

Little Spread Shoal (27 miles from mouth of river).—Built a timber and stone dike on from the left shore to hold up the water on the shoal and keep rafts from running into the bank. The dike is 440 feet long, 8 feet wide at base, 6 feet at top, and 4 feet high. In building it 3,555 linear feet of round timber and 327 cubic yards of stone were used.

Queen Shoal (25 miles from mouth of river).—Moved farther to left and rebuilt part of the old riprap wall on left to improve the channel for rafts and boats; moved and placed about 550 cubic yards of stone.

Porter Shoal (7 miles from mouth of river).—The work done here consisted in blasting or breaking up with sledge and removing a number of large rocks from along the left bank that were quite an obstruction to rafts and steamboats, particularly, the latter, in high stages of water. The rock disposed of aggregated about 140 cubic yards.

REMOVING SNAGS, ETC.

The small party engaged in May and June in removing obstructions below Queen Shoal, in the interest mainly of steamboat navigation, as described in the last Annual Report, continued operations until July 11, when it was disbanded. The work done by this party in July was at and near Jarretts and Porter shoals. Twenty-three snags and sunken trees were taken out and cut up and a number of leaning trees cut down or the projecting limbs removed. In addition, a "towpath" was made high up along the bluff bank at Porter and two ringbolts put in for use in winding boats through the shoal.

Very respectfully, your obedient servant,

ADDISON M. SCOTT,
Resident Engineer.

Col. WILLIAM P. CRAIGHILL,
Corps of Engineers.

COMMERCIAL STATISTICS.

UNITED STATES ENGINEER OFFICE.
Charleston-Kanawha, W. Va., July 18, 1892.

Commercial statistics for Elk River, West Virginia.

Year ending June—	Saw logs and lumber.	Railroad ties, oak.	Oak staves.	Hoop poles.	Hickory spokes.	Oak tan-bark.	Tonnage timber products
	<i>Feet, B. M.</i>					<i>Cords.</i>	<i>Tons.</i>
1883	5,200,000	100,000	898,334	24,7
1884	5,975,000	125,000	1,122,500	30,7
1885	5,100,000	250,000	1,433,750	33,5
1886	15,361,000	250,000	1,845,000	68,5
1888	15,900,000	330,000	1,210,000	71,1
1889	15,750,000	295,000	1,600,000	500,000	350,000	70,2
1890	26,650,000	330,000	1,380,000	150,000	568,000	300	89,8
1891	27,995,000	412,500	800,000	125,000	20,000	500	103,7
1892	44,400,000	380,000	1,405,000	240	139,8

The marked increase in the output of lumber will be noticed. The 44,400,000 feet of lumber is divided in kind about as follows: Poplar, 33,000,000; white oak, 8,000,000; hemlock, 1,500,000; the rest being mainly walnut and ash.

The amount of general merchandise and produce carried on the river is estimated at 10,000 tons, making the total for the year, with the timber products as above, 19,800 tons.

FF 4.

IMPROVEMENT OF GAULEY RIVER, WEST VIRGINIA.

The small amount of money available for this river was expended in July, August, and September, 1891. The unfinished channels made in 1890 through the shoals within 7 miles of Gauley Bridge were cleared of rocks and some new training walls were built for them. The channel for boats, 30 feet wide and 4 feet deep at low water, was completed to the foot of the Little Roughs, 1.5 miles.

A channel 30 feet wide was made through the Little Roughs by blasting out large bowlders, the object being to facilitate the downward movement of logs.

The formidable obstruction at the Big Roughs was also attacked by blasting the large bowlders which form it.

The total number of cubic yards removed was 3,184, and 6,980 linear feet of walls were built.

The work of the year has had the effect of causing the movement of much local freight by boats which otherwise have been hauled by teams at greater cost to the country.

The following letter explains itself:

UNITED STATES ENGINEER OFFICE,
Baltimore, Md., May 31, 1892.

GENERAL: In the money statement in the last annual report, under the head of Gauley River, West Virginia, the following is found: "Amount (estimated) required for completion of existing project, \$4,000."

The project under which Congress has given two appropriations contained two heads, viz, the improvement from the mouth of the river to the Roughs for bateaux navigation, \$10,000, and for the improvement of the Roughs, and above, \$65,000; total, \$75,000. See Annual Report for 1888, Part 3, page 1762. The appropriations up to this time have been, August 11, 1888, and September 19, 1890, each \$3,000, amounting to \$6,000.

Work was first begun on the portion below the Roughs, of which the cost was estimated to be \$10,000, and when \$4,000 was stated a year ago to be the balance estimated for completion of existing project it was understood to refer to that portion of the river. The statement should have read "Amount (estimated) required for completion of existing project below the Roughs, \$4,000," and that it did not so read was my fault.

In preparing the Annual Report for the current fiscal year doubt now arises as to what amount should be stated as that "required for completion of existing project."

The original estimate was \$75,000 for the two items as explained above, and I request authority to use this total in the money statement after deducting appropriations made prior to June 30, 1892.

Very respectfully, your obedient servant,

WM. P. CRAIGHILL,
Colonel, Corps of Engineers.

Brig. Gen. THOMAS L. CASEY,
Chief of Engineers, U. S. A.

The following are the amounts and dates of appropriations for improving Gauley River, West Virginia:

August 11, 1888	\$3,000.00
September 19, 1890.....	3,000.00
Total	6,000.00

Money statement.

July 1, 1891, balance unexpended	\$2, 938. 99
July 30, 1892, amount expended during fiscal year	2, 856. 20
July 1, 1892, balance unexpended	82. 60
Amount appropriated by act approved July 13, 1892.....	3, 000. 00
Amount available for fiscal year ending June 30, 1893.....	3, 082. 60
{ Amount (estimated) required for completion of existing project	66, 000. 00
{ Amount that can be profitably expended in fiscal year ending June 30, 1894	10, 000. 00
{ Submitted in compliance with requirements of sections 2 of river and harbor acts of 1866 and 1867.	

REPORT OF MR. WILLIAM PROCTOR SMITH, ASSISTANT ENGINEER.

BALTIMORE, Md., November 16, 1891.

COLONEL: I have the honor to report the following operations on the improvement of Gauley River, West Virginia, for the season of 1891.

The river being very low in May it was thought advisable to start the work, and accordingly everything was made ready by the 1st of June, when rains set in which kept the river up until the 29th, at which time work was begun and continued until the 22d of September.

It was found that the channels made in 1890 needed clearing out and that additional wing walls were desirable; so work was begun on them, beginning at the Big Creek, $1\frac{1}{4}$ miles above the mouth of the river, and extended to and through Mill Shoal $7\frac{1}{4}$ miles above the same point, from which were removed 712 cubic yards of loose sand and 12 cubic yards of solid rock, and 1,004 linear feet of main and wing wall were built.

Owing to the large amount of work absolutely required in this part of the river the project for the season could not be carried out in full.

The new work began at Humphreys Shoal, $1\frac{1}{4}$ miles above the mouth of the Gauley and channels were made through the shoals left last season, as far as the head of Mill Shoal, $7\frac{1}{4}$ miles above the initial point; from which place to Sugar Creek, $6\frac{1}{4}$ miles further up the river, the work was entirely new during the season.

The combined width of the shoals in the whole distance of the new work amounts to 8,623 feet, covering a river space of $7\frac{1}{4}$ miles, from which were removed 857 cubic yards of loose and 1,603 cubic yards of solid rock, with which were built 5,976 linear feet of main and wing walls.

The wing walls were built of loose stone, borrowed from points near by, mixed with brush and gravel and sand when the latter could be procured.

The total amount of loose rock for the season was 1,569 cubic yards; of solid rock 1,615 cubic yards; in all, 3,184 cubic yards.

The walls vary in width from 3 feet to 10 feet, and in height from 1 foot to 5 feet and amount to 6,980 feet in length.

The combined widths of shoals amount to 14,137 feet, or nearly $2\frac{1}{4}$ miles.

The total fall over the shoals and ledges from the mouth of the river to the head of the Big Roughs, 13.4 miles, is 79.8 feet, and the entire fall of the river between these points can be but little more, although the levels were not taken.

At Tritts Shoal, $9\frac{1}{4}$ miles above the mouth of the river, two channels were made one long and curved for extreme low water, the other short and straight for water at and above zero.

The bateau channel, 30 feet wide and 2 feet deep at zero or ordinary low water was completed to the foot of the Little Roughs, $12\frac{1}{4}$ miles, on the 8th of August, and there the force was reorganized, put in camp, and set at work at that point, Little Roughs, by blasting out the large bowlders for floating logs, through which was made a channel 600 feet long and 30 feet wide, requiring the removal of 30 rocks, containing 310 cubic yards. The Little Roughs, as shown on the tracing of map accompanying this report, is the beginning of the "Roughs" of Gauley, which extend from this point up the river for a distance of 26 miles, as mentioned in your report December 26, 1887, page 1761, Appendix D D 6 of the Annual Report of the Chief of Engineers, U. S. Army, for 1888, and the shoal called Big Roughs, on the same map, is the fourth in this series.

This completed, the force was moved to Big Roughs, 1 mile up the river, a partial channel having to be made through Beaver Pond Shoal and Camp Shoal to admit the passage of the boats.

Guide cribs.—Constructed upper and lower guide cribs. Upper river crib is 200 feet long, 34 feet high, and 15 feet wide. Upper land crib is 100 feet long, 12 feet wide, and 10 feet high. Lower river guide crib is 110 feet long, 34 feet high, and its mean width is 13 feet. Lower land crib is 550 feet long; 250 feet of it is 34 feet high, and 300 feet of it is 15 feet high. Total cubic yards of crib work built, 12,497.

Repairs to lock.—Pumped out lock and repaired gates, placing four new arms in each lower gate. Cut down recess for lower gate in old land wall so as to make it vertical. Cut recesses in walls to receive gate castings when gates were opened. Lengthened ladders and cleaned out lock pit. Made more clearance under lower gates by picking off stone forming bottom of lock. Rebolted lower miter sills.

General work.—Drove 29 piles and capped same, forming fender along island above lock. Split up the necessary stone and paved 675 square yards of bank. Moved 1,600 cubic yards of earth in grading bank. Built up wing walls of lock so as to conform to grade of bank. Dredged off point above entrance to lock, and drove forty-five piles around this point for a fender.

Mr. B. O. Lermond, assistant engineer, was in local charge at this lock.

LOCK AND DAM NO. 4, GREEN RIVER.

Repairs to dam.—Removed old sheeting from dam; rebuilt an 80-foot section of lower slope and placed new sheeting on dam throughout. Replaced all missing stone in dam. Constructed and sunk a crib 90 feet long, 16 feet wide, and 22 feet deep, so as to form an apron below the dam next to the abutment and at the same time filling a hole washed out below dam, endangering same. The sheeting has been torn off this crib during the winter, evidently due to a head caused by leak under dam from above.

Guide cribs.—Constructed and repaired guide cribs above and below lock. Lower land guide crib is 275 feet long, average width 12 feet, and 30 feet high. Upper land crib is 140 feet long, 18 feet high, and 15 feet wide. Upper river crib rebuilt from water up, 220 feet long, 10 feet wide, and 10 feet high. Total cubic yards in guide cribs, 5,837.

General work.—Constructed retaining crib along mill race, 450 cubic yards. Split up and laid 408 square yards of paving. Repointed coping of land wall of lock. Placed new gate-operating apparatus in position. Dredged entrances to lock.

Mr. George N. Bratt was in local charge at this lock.

LOCK NO. 7, BARREN RIVER.

Graded bank. Quarried and laid 804 square yards of paving; cut, hauled, and set 1,465 square yards of sod. Set out shade trees and cleaned up premises. Placed 500 cubic yards of sawdust behind dam.

UNITED STATES DREDGE BOAT.

Work done.—Lock No. 1, Green River: Removed 5,621 cubic yards of material and 36 logs from lock pit and entrances to lock.

Lock No. 2, Green River: Removed 5,200 cubic yards of material from entrances to lock.

Lock No. 3, Green River: Removed 12,975 cubic yards of material in preparing foundations for guide cribs and in cleaning out lock entrances and pit. Tore up and removed 6,000 linear feet of old crib timber. Dredged and blasted off remains of old lock wall and cofferdam to low-water level. Dredged a channel across river so as to barge stone from ledge below dam.

Lock No. 4, Green River: Removed 2,920 cubic yards of material from entrances to lock.

General work.—Dredged off point at Boat Island, removing 1,000 cubic yards gravel. Dredged off bar at mouth of Green River, removing 1,000 cubic yards of material. Painted dredge.

UNITED STATES SNAG BOAT WM. PRESTON DIXON.

Removed 1,043 snags from channel of river; cut 252 trees and deadened 220. Pulled out and removed cofferdam at No. 1, Green River. Pulled one dump scow out on its. Did all towing required on Green and Barren Rivers. Towed a dredge from Carmel, Ill., to Catlettsburg, Ky. Total miles run 8,773.

Very respectfully, your obedient servant,

WM. L. SIBERT,
First Lieutenant of Engineers.

D. W. LOCKWOOD,
Corps of Engineers, U. S. A

2070 REPORT OF THE CHIEF OF ENGINEERS, U. S. ARMY.

Table showing the distances from Gauley Bridge, W. Va., to foot of shoal or ledge, their length, and difference of level between head and foot of same; also the quantity and quality of material excavated in making and improving the channel in Gauley River, West Virginia, from its mouth to Sugar Creek, in Fayette County, W. Va., 1891.

Name of shoal.	Number.	Distance.	Length.	Fall.	Loose rock.	Solid rock.	Total.	Blasts.	Holes.	Walls.	Bowlders.	Drilling.
		<i>Miles.</i>	<i>Feet.</i>	<i>Feet.</i>	<i>Cu. yds.</i>	<i>C. yds.</i>	<i>C. yds.</i>	<i>No.</i>	<i>No.</i>	<i>Feet.</i>	<i>No.</i>	<i>Feet.</i>
Junction of New and Gauley Rivers.....	1		200									
Scrabble Creek ledges.....	2		330	8								
Buck Ford Ledges.....	3		339	1.2								
Humphreys Shoal.....	4	1 1/4	100	.4	24		24			50		
Big Creek Shoal.....	5	1 1/4	846	3.2	12		12					
Pool Shoal.....	6	2	283	.1	10		10					
Winding Shoal.....	7	2 1/2	400	1	15		15			30		
Kincaid Shoal.....	8	2 1/2	525	1.6	75		75					
Durden Shoal.....	9	2 1/2	460	1.9	108	4	112	3	3	168		4
Durden Ripple.....	10	3	168	.2	2		2					
Long Shoal.....	11	3 1/4	976	2.8	105	1	106			246		
Twenty-mile Creek Shoal.....	12	5	1,464	4.1	90	2	92	1	1	96		2
No. 1, above.....	13	5 1/2	205	1.2	118		118	6	6	196		5
No. 2, above.....	14	6 1/4	143	.2	20		20			84		
No. 3, above.....	15	6 1/2	100	.3	18		18			130		
Foster Shoal.....	16	6 1/2	482	2.3	83	6	89	4	4	894		8
No. 1, above.....	17	7	42	.6	8		8			84		
No. 2, above.....	18	7 1/2	37	.2	6		6			37		
No. 3, above.....	19	7 3/4	265	.9	69		69			265		
Mill Shoal.....	20	7 1/2	1,075	6.5	135	5	140	3	3	54		5
Church Hollow Shoal.....	21	9 1/2	580	2.5	43		43			579		
Simms Shoal.....	22	9 1/2	178	1.1	18	2	20	1	1	178		2
Tritts Shoal.....	23	9 1/2	745	3.6	218	12	230	10	10	1,400		9
Ford Shoal.....	24	10	200	.3	9		9			200		
Laurel Shoal.....	25	10 1/2	200	2.5	112		112			253		
Upper Shoal.....	26	10 1/2	690	4.1	124		124			1,388		
Little Ledges.....	27	12 1/2	204	1.1	123	24	147	21	21	348	10	24
Little Roughs.....	28	12 1/2	690	9.1		310	310	70	66		30	138
Beaver Pond Shoal.....	29	12 1/2	900	8.7	17		17			200		
Camp Shoal.....	30	12 1/2	500	.8	7		7			100		
Big Roughs.....	31	13 1/4	900	14.8		1,249	1,249	152	150		31	406
Total.....			14,137	79.8	1,569	1,615	3,184	271	265	6,980	71	603

COMMERCIAL STATISTICS FOR GAULEY RIVER, WEST VIRGINIA.

Year ending June 30—	Saw logs.	Lumber.	Staves.	Tonnage of timber.	Produce, etc.	Total from Gauley.	Merchandise to Gauley.
	<i>Feet.</i>	<i>Feet.</i>			<i>Tons.</i>	<i>Tons.</i>	<i>Tons.</i>
1888.....	691,800	2,535,300	177,400	7,121	36	7,151	750
1889.....	890,400	3,615,000	274,200	9,491	78	9,569	1,322
1890.....	384,800	4,452,500		9,676	90	9,766	1,985
1891.....	384,700	4,701,300	100,000	10,347	89	10,436	1,964

This commerce is carried on with four bateaux and four flatboats, drawing about 18 inches of water and of 18 tons capacity each.

Commercial statistics for Gauley River, West Virginia, from July 1, 1891, to June 1, 1892—eleventh months.

Lumber.....	feet..	5,249,200
Saw logs.....	do.....	261,170
Staves.....	number..	1,200,000
Shingles.....	do.....	111,000
Total timber in.....	tons.....	9,527
Produce, etc.....	do.....	103
Total from Gauley.....	do.....	9,630
Total to Gauley.....	do.....	3,063

F F 5.

IMPROVEMENT OF NEW RIVER, VIRGINIA AND WEST VIRGINIA.

There has been no work on this river in the year ending June 30, 1892. The following are the amounts and dates of appropriations for improving New River, Virginia and West Virginia:

August 14, 1876.....	\$15,000	August 2, 1882.....	\$12,000
June 18, 1878.....	15,000	August 5, 1886.....	10,000
March 3, 1879.....	12,000		
June 14, 1880.....	24,000	Total.....	112,000
March 3, 1881.....	24,000		

Money statement.

July 1, 1891, balance unexpended.....	\$2,341.79
July 1, 1892, balance unexpended.....	2,341.79
{ Amount (estimated) required for completion of existing project.....	159,000.00
{ Submitted in compliance with requirements of sections 2 of river and harbor acts of 1866 and 1867.	

G G 4.

IMPROVEMENT OF KENTUCKY RIVER, KENTUCKY.

ntucky River is a tributary of the Ohio, and empties into it on, Ky., about midway between Cincinnati, Ohio, and Louis-

sent project for the improvement of the river was adopted in object being to repair the five locks and dams built by the Kentucky, and extend slackwater navigation for a draft of 6 e construction of additional locks and dams to Beattyville, a f 261 miles from the mouth of the river.

WORK DONE DURING THE YEAR.

6.—Work on this lock was commenced April 27, 1891, by g for lock walls, and on December 2 the first boat was locked The cofferdam inclosing the lock site consisted in the main f piles joined by stringers and wale pieces, against which a w of sheet piles was driven, the whole being backed inside ith gravel put in position by dredges. The lower return y nk was left open at first, so that excavation of the incl l be effected by the dredges. Afterwards the low r e bank was constructed, the inclosed area pumped a to the rock made by shoveling, wheeling out the g it out with derricks. Although the coffer was floo ng the season, it held well, leaked very little, and answered pose.

onstruction of the lock and abutment, the following quanti- ne, etc., were put in place:

	Cubic yards.	Cubic feet.
.....	439	25
.....	693	25
stone.....	1,830	5
obs.....	1,557	26
.....	1,287	1
.....	3,999	20
one.....	1,420	18
prop.....	2,028	2
.....	1,483	26
.....	1,836	19

ensions of the lock are as follows: Length of walls, 249 feet. chamber between hollow quoins, 185 feet. Width of chamber, Lift, 14 feet 2½ inches.

all the stone was on hand when the work commenced. onstruction of the dam the lower step is a crib filled with anding down to the rock and reaching from the abutment to wall of the lock. The portion of the dam extending upstream lower step is crib work, resting on 139 piles, and is also filled The upper breast of the dam is protected by a double row g extending down to the rock, and will be backed with

truction of the dam the following material was used:
 mber, 291,217 feet, B. M., sheathing, 16,361 cubic
 ; yards fascines, and 10,949 cubic yards sand,

Submitted to
harbor notes

REPORT

Sta: I live
River, Kent
That part
the banks
mouth to H
No work
Work done
the river;
ing trees;
river.

Very

Maj. D. W.
Capt

Com-

Logs.....
Lumber
Timber, feet

Total

out of the river, and at the same time, when necessary, snagging and doing general work connected with operation. The present boat can not do the work.

APPROPRIATIONS.

.....	\$100,000
.....	100,000
.....	125,000
.....	225,000
.....	250,000
.....	187,500
.....	180,000
.....	180,000
.....	1,347,500
.....	500
.....	1,348,000

Money statement.

1891, balance unexpended	\$151,816.59
1892, amount expended during fiscal year	147,834.43
.....	3.1
1892, outstanding liabilities	2.
.....	1
.....	4
.....	0
.....	151,854.13
.....	1,524,000.00
.....	500,000.00

REPORT OF MR. R. S. BURNETT, ASSISTANT ENGINEER.

UNITED STATES ENGINEER OFFICE,
Frankfort, Ky., July 1, 1892.

MR: I respectfully submit the following report on the improvement of the Kentucky River, Kentucky, for the fiscal year ending June 30, 1892:

LOCK NO. 6.

ABUTMENT.

ry.—Began excavating for abutment May 16. Completed excavating June 5. Completed setting stone June 5. Completed setting stone July 2. Set 7 cubic yards (6 feet) of coping on return wing into bank. The return wing into bank was 7 feet 6 inches by stepping back 6 feet 5 inches from face of abutment six feet 5 inches in length, rise 15 inches each.

LOCK.

excavating for foundation April 27. Completed excavation June 5. Set stone for foundation July 6. Completed foundation July 26. for lock walls July 27. Completed setting stone October 24,

Length of foundation on river-lock wall, 254 feet. width, 24 feet.

Year	1950	1951	1952	1953	1954	1955	1956	1957	1958	1959	1960
Population	100	100	100	100	100	100	100	100	100	100	100
...

The following table shows the results of the survey conducted in the year 1960. The data is presented in a tabular format, with columns representing different categories and rows representing specific data points. The table is organized into several sections, each with its own heading. The first section, titled 'General Information', provides an overview of the survey's scope and objectives. The second section, 'Detailed Findings', presents the core data, which is broken down into various sub-categories. The final section, 'Conclusions and Recommendations', summarizes the key findings and offers suggestions for future research or action. The data indicates a significant trend in the population over the period studied, with a steady increase in certain areas and a decrease in others. These findings are crucial for understanding the underlying factors and for developing effective strategies to address the issues at hand.

Timbers.—Constructed crib 18 feet wide, 18 feet high, and 350 feet long from the abutment to the cofferdam; sunk same with riprap stone to solid rock.

Laid the following amount of timber:

	Feet, B. M.
57 pieces 12 by 12 inches by 10 feet	6, 840
728 pieces 12 by 12 inches by 16 feet	139, 776
618 pieces 12 by 12 inches by 20 feet	148, 320
639 pieces 12 by 12 inches by 24 feet	186, 912
171 pieces 12 by 12 inches by 30 feet	61, 560
358 pieces 12 by 12 inches by 32 feet	137, 472
2, 581	680, 880

Sheathing.—Spiked the following oak sheathing on dam:

Oak lumber—	Feet, B. M.
2-inch	24, 216
3-inch	66, 291
5-inch	560
6-inch	34, 524
9-inch	165, 626
	291, 217

Spikes.—Used the following amount of spikes:

Spikes—	
$\frac{1}{2}$ by 6 inches	
$\frac{1}{2}$ by 8 inches	
$\frac{1}{2}$ by 12 inches	
$\frac{1}{2}$ by 16 inches	
$\frac{1}{2}$ by 24 inches	

Stone.—Placed 16,361 cubic yards 8 cubic feet of riprap stone in cribbing.

Backing.—Placed 10,949 cubic yards of sand, clay, and gravel, and 2,235 cubic yards of fascines behind dam.

UPPER SHORE GUIDE WALL.

Excavation.—Removed by picking, shoveling, wheeling, and hoisting, 2,586 cubic yards of sand, clay, earth, and gravel to solid rock for foundation.

Filling.—Drove 32 piles, aggregating 293 linear feet.

Timber laid.—Laid the following amount of timber:

	Feet, B. M.
17 pieces 6 by 12 inches by 16 feet	1, 632
181 pieces 12 by 12 inches by 16 feet	35, 328
172 pieces 12 by 12 inches by 20 feet	41, 280
135 pieces 12 by 12 inches by 24 feet	38, 880
20 pieces 12 by 12 inches by 30 feet	7, 200
46 pieces 12 by 12 inches by 32 feet	17, 664
571 pieces.	141, 984

Spikes.—Used the following number of spikes:

Spikes:	
$\frac{1}{2}$ by 12 inches	50
$\frac{1}{2}$ by 16 inches	1, 026
$\frac{1}{2}$ by 24 inches	114

Stone.—Placed 846 cubic yards of riprap stone in cribbing.

Filling.—Placed behind wall 212 cubic yards of riprap stone.

Dimension of wall.—One hundred and forty-three by 16 feet. Foundation of wall, 24 feet.

LOWER SHORE GUIDE WALL.

Excavation.—Removed by picking, shoveling, wheeling, and hoisting, 3,477 cubic yards of sand, clay, earth, and gravel. Blasted 165 cubic yards of solid rock for foundation 4 feet wide.

...
...umber from river
...t, B. M., of hewed
...e, removed same to
platform from coffer-
General work about

...ember 14, 1891, to take the
... latter had her boilers re-

Removed with scrapers from
... for dredge *Ward*, derrick
...ck.

... BOAT.

...t and other material for the works,
... foundations in river for derricks,
...ing cofferdam from upper and lower
... walls; handled cement and riprap
...und lock and dam.

...GE BOAT WARD.

...dged the following amount of material:
...t. clay, gravel, and stone; for dam, 20,010
...lock entrances, 12,723 cubic yards of sand,
...pper handle, and replaced same with new;
...uches.
...me: 1 dipper, 1 dipper handle, 1 spud, 2 cam
...ng wheel, 1 junction band, 1 lever, 1 pump
...ts; broke backing chain eleven times, hoisting

...S DREDGE BOAT WILLIE.

...dredged the following amount of material: For
...and, clay, and gravel; for dam, 8,677 cubic yards
...k entrances, 500 cubic yards of sand, clay, and

ending June 30, 1892.

	1890.	1891.	1892.
	<i>Tons.</i>	<i>Tons.</i>	<i>Tons.</i>
.....	156, 982	224, 844	72, 749
.....	10, 670	15, 256	13, 336
Miscellaneous	19, 554	21, 739	60, 618
.....	900	1, 602	8, 025
.....	4, 682	3, 761	21, 631
.....	2, 793	1, 636	11, 199
.....	4, 142	4, 306	4, 098
.....	1, 995	2, 023	3, 172
.....	316, 354	389, 219	431, 846

Kentucky River, Kentucky.

Length.	Breadth.	Depth.	Tonnage.
<i>Feet.</i>	<i>Feet.</i>	<i>Feet.</i>	
100	15	3	43.64
92	13.6	2.2	21.32

THE SIX LOCKS AND DAMS ON THE KENTUCKY RIVER.

In the condition of these locks and dams in 1892, more or less crippled condition, the miter sills decayed and the lock gates and operating machinery need absolute removal or extensive

entirely rebuilt, the others nearly so. The locks have been constructed, and the miter sills protected by timber cribbing to

have been built, the areas in rear of the locks and the grounds graded, sown with grass

Lock No. 6, slack-water navigation on the river which is 121 miles above the mouth of

general repairs have been made to the lock gates, being torn off by drift, and placing backing to stop leaks, etc.

The repairs have consisted in replacing defective miter sills by putting in new blocks and arms. When the miter sills have been dredged, so as to afford navigation to the lower miter sills, and the guide and proppings have been kept in repair.

to be freed from snags by the United States snag boat, which is belonging to the general improvement of the river. A large part of the time operated under the supervision of the operating and care of canals, etc., applied to the improvement of the river. In her present condition this boat is

the boatman's dwelling is required at Lock No 2.

Statement of expenses incurred in preserving and maintaining navigation on that of the Kentucky River, Kentucky, improved by locks and dams, etc.—Continued.

	Lock No. 5.				Lock No. 6.			
	Salaries.	Labor and material.	Current and contingent expenses.	Total.	Salaries.	Labor and material.	Current and contingent expenses.	Total.
1891.								
.....	\$213.00	\$110.00	\$29.28	352.28				
.....	253.00	48.10	1.40	283.50				
.....	165.00	97.70	6.30	269.00				
.....	229.00	374.56	17.23	620.79				
.....	227.00	115.30	6.95	349.25				
.....	253.00	50.36		283.36	\$95.00	\$27.92		\$122.92
1892.								
.....	186.68	1.80		188.48	186.68	182.55	14.40	
.....	153.34	9.20		162.54	153.34			
.....	153.34		2.40	155.74	153.34	56.78	2.40	
.....	153.34	72.00		225.34	153.34	131.72		
.....	207.51	89.85	1.08	298.44	207.51	11.40	1.08	
.....	203.34	165.35	14.47	483.16	203.34	106.60	25.32	
total	2,357.55	1,134.22	79.11	3,570.88	1,152.55	516.97	48.20	1,

Months.	United States snag boat Kentucky and steamer Fulton.				United States dredges Will		
	Salaries and labor.	Repairs.	Supplies.	Total.	Salaries and labor.	Repairs.	Supplies.
1891.							
.....	\$973.50	\$79.44	\$452.71	\$1,505.65	\$407.78	\$52.88	\$6.15
.....	1,021.00		450.45	1,471.45	290.00	11.20	58.91
.....	959.49	16.99	548.61	1,525.09			51.35
.....	936.82		199.28	1,136.10	433.38	451.06	
.....	948.50		984.44	1,932.94	427.50		
.....	932.50	11.10	704.05	1,647.65	435.00	47.81	
1892.							
.....	832.49		1.50	833.99	307.17		
.....	589.00			589.00	140.00		10.60
.....	446.00	120.68	54.56	621.24	204.00		
.....	491.00			491.00	214.84	33.70	
.....	780.32			780.32	323.83		
.....	972.99	3.13	654.60	1,630.72	390.00		496.15
total	9,883.61	231.25	4,050.20	14,165.06	3,573.50	596.65	623.16

SUMMARY.

a. 1. Kentucky River, Kentucky.....	\$9,826.17
a. 2. Kentucky River, Kentucky.....	2,884.08
a. 3. Kentucky River, Kentucky.....	3,053.44
a. 4. Kentucky River, Kentucky.....	4,148.11
a. 5. Kentucky River, Kentucky.....	3,570.88
a. 6. Kentucky River, Kentucky.....	1,712.72
States snagboat <i>Kentucky</i> and steamer <i>Fulton</i>	14,165.06
States dredges <i>Willie</i> and <i>Ward</i>	4,793.31
Super's dwelling, Lock No. 2.....	1,446.10
Super's dwelling, Lock No. 3.....	1,316.77
Super's dwelling, Lock No. 5.....	978.17
Boat.....	1,104.50
Grand total.....	48,999.40

With the exception of N
 ans ate in an effective
 erty extended downst
 out at once, high wat
 low. For details con
 ention is invited to th
 herewith:

1885
1886
1887
1888
1889
1890
1891
1892

Total

*Detailed statement of
 position of the Koh
 year ending June*

Months.

1891.

July
August
September
October
November
December

1892

January
February
March
April
May
June

Total

Month:

1891

.....
.....
.....
.....
.....
.....

1892

.....
.....
.....
.....
.....
.....

Total.

LOCK NO. 3.

knocked off by drift; respiked old sheathing; re-
 Completed construction of lock-keeper's dwelling; laid 66
 around dwelling; graded and sodded around dwell-
 was required at either lock entrance.
 old lock-keeper's dwelling; repaired and repainted lock
 irons; whitewashed fence around premises; removed drift
 and walls.
 was no suspension of navigation at this lock except for
 walls were submerged eleven days during the year.
 occurred April 22, upper gauge reading 24 feet, lower gauge
 occurred November 5, lower gauge reading 5 feet, upper gauge

LOCK NO. 4.

from steps of dam 324 pieces of defective step sheathing and re-
 new oak sheathing; repaired upper slope of dam.
 wall.—Spiked 1,188 feet B. M. oak sheathing on face of wall.
 —Repaired and repainted lock gates and operating irons; removed
 entrances and walls; repaired tool house; stopped leak in dam; re-
 tore crib wall; built fence at lower end of premises; whitewashed
 —Dredge *Ward* dredged 1,010 cubic yards of sand and mud from upper
 and 25 bags; from the lock pit, 200 cubic yards, and from the lower entrance,
 cubic yards of sand and mud.
 remarks.—There was no suspension of navigation at this lock except for
 during the year. The lock walls were submerged thirteen days.
 highest water occurred April 22, upper gauge reading 22.3 feet, lower gauge
 25.8 feet.
 lowest water occurred October 17, upper gauge reading 4.8 feet, lower gauge
 24.4 feet.
Lee Town bar.—This bar had shoaled so badly during the past year that it
 dredging. The dredge *Ward* has dredged, in deepening and widening the
 3,500 cubic yards of earth, sand, and gravel, and removed same in dump

LOCK NO. 5.

—To stop leak through the dam, 377 cubic yards of stone and clay and 20
 of brush were placed behind as backing. Respiked old step sheathing; removed
 five pieces and replaced same with new oak sheathing.
 -keeper's dwelling.—Completed construction of new dwelling for lock-keeper;
 land built brick walks around dwelling; constructed 1 tool house.
 work.—Tore down old dwelling and utilized material in building walks
 (Government land. Painted lock gates and assistant lock-keeper's dwell-
 removed drift from lock entrance and lock walls.
 remarks.—Navigation was suspended fifteen days on account of high water
 seven days on account of low water, caused by the construction of Lock No.
 locks in the dam at Lock No. 5. The entire river was shut off from below
 No. 6 for a period of twenty-eight days at the driest season of the year, when
 the water was being voided from the upper river, the lower river being depend-
 on small branches and springs to maintain the pool, while pool No. 6 was fill-
 This will account for all the low gauge readings at all the locks.
 lowest gauge reading occurred October 15, upper gauge reading 0.10 foot, lower
 reading 4.90 feet. The highest water occurred April 22, the upper gauge
 22.60 feet, the lower gauge reading 33.50 feet.

LOCK NO. 6.

lock was in operating condition for navigation November 21, 1891. For lack
 on the lower shore and river guide walls, as well as the upper river guide wall,
 not constructed.
 ing behind the lock wall, grading and paving the bank, and constructing lock-
 keeper's dwelling were also omitted. A sufficient quantity of backing was not
 behind the dam.

skages on Kentucky River, Kentucky, for fiscal year ending June 30, 1892.

Boats.	Days operated.	Days suspended.*	Going up.			Going down.			Total crafts and flats passing.	Total number lock ages.	
			Steamboats.	Barges and flats.	Miscellaneous.	Steamboats.	Barges and flats.	Flats.			Miscellaneous.
.....	336	30	439	153	64	453	192	223	79	1,564	1,230
.....	348	18	426	150	94	438	134	195	125	1,502	1,328
.....	355	11	290	149	89	280	147	114	114	1,103	1,063
.....	353	13	301	157	160	302	149	200	155	1,433	1,230
.....	338	28	179	69	105	175	59	1,431	134	2,146	1,447
.....	223	47	61	42	47	61	1,271	31	1,380	695
.....	1,682	733	554	1,765	732	1,433	649	9,498	4,999

h water and filling Pool No. 6.

G G 6.

MENT OF LICKING RIVER, KENTUCKY, BETWEEN FARMERS AND WEST LIBERTY.

ject for the improvement of this river was approved by the of War under date of September 20, 1888, and provides for f snags, detached rocks, and boulders, with a view to im- w-water navigation and rafting.

close of the last fiscal year a working party, which had com- ork at West Liberty, May 15, had reached Blackwater Creek, elow. During the balance of the season, which closed Sep- , the work of improvement was carried to Farmers, and the ninent obstructions removed or lessened in degree. The gen- each season has been to afford as much relief as possible in of general improvement of the river between Farmers and erty, a distance of 68 miles, and the small appropriations e it impossible to thoroughly complete the work in any lo-

the fiscal year ending June 30, 1892, the following work was

Class of work.	Number.	Cubic yards.	Average	
			length.	circumference.
			Feet.	Feet.
moved.....		2,061		
moved.....		78		
sd.....	429		5.4	3.2
sd.....	1-3		57.4	5.9
ved.....	5-5			

ver, under the present project, is not susceptible of permanent, for the reason that such obstructions as snags, leaning mps, etc., are liable to occur after each high water.

ial estimate of the cost of this improvement was \$17,680 iles of river between Farmers and West Liberty, and it is 1 that of the balance yet unappropriated \$5,000 be appro- a fiscal year ending June 30, 1894.

WEST VIRGINIA

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ses during low water, and thus afford advantages for mining coal at no other location for a single lock could give.

At the close of the last fiscal year the lock was completed and equipped, a part—80 feet—of the permanent dam had been constructed, and the timber for the remainder had been contracted for and partly delivered. The abutment had already been built. The construction of a fixed dam was opposed by the timber and shipping interests, and under date of May 20, 1891, a Board of Engineer Officers was constituted "to consider and report upon the subject of the dam to be built on the Big Sandy River."

In its final report, the Board recommended the substitution of a needle dam in place of a fixed dam, the clear height of the pool formed by it to be 13 feet above the sill of the Navigable Pass.

As the length of the Navigable Pass should be sufficient to accommodate the commerce that may use it, and as the transportation of timber by rafting is the principal industry as yet developed on the river, the pass should evidently be long enough to pass rafts of the ordinary size, particularly as rafting is carried on at stages as low as 6 feet.

Considerations of economy with regard to construction and operation require that the pass should be as short as possible, consistent with the requirements of commerce. The circumstances of the case appear to require that the pass should be about 130 feet long.

In determining the elevation of the sill of the pass, the general principle that it, the sill, must not be an obstruction to navigation, should govern; in other words there must be as much depth on it, as boats can carry to it. This consideration would fix its elevation at least as low as the highest shoals.

When the lock was planned the elevation of its lower miter sill was fixed at 9 inches below the then accepted low water. Since then there are indications that the low-water surface at Louisa is higher than it was formerly. This is evidenced by the gauge readings in connection with the general condition of the river at the present lowest stages, and the fact that a part of the old steamboat landing at Louisa, which was formerly out of the water at lowest stages, is now and has been for a number of years, submerged at the lowest water. Push-boat navigation, which is effective with a draft of 9 inches, forms a fairly good practical standard of comparison for determining the relation between lowest stages of different years.

Granting that the reading of the lowest water is greater now than it was in former years, an explanation for it may be found, possibly, in the fact that since the commencement of the construction of the lock there has been a great change in the condition of affairs at and near the site of the lock, due to the partial obstruction of the natural waterway; but whether the change in low-water level will continue to exist as it now is, increase or diminish, can hardly be stated definitely, although the last would appear to be the most unlikely to occur.

Should there be no further change, it would be safe, taking into account the gauge readings during the low-water season, to place the sill of the pass one foot above the lower miter sill of the lock.

The needles forming the dam, pass, and weir, are to be supported by trestles, with an escapement which will allow the escape of all needles between adjacent trestles at one time, thus doing away the slow and laborious method of taking out each needle sepa-

is and estimates of cost of the dam and its accessories will be stated at an early date.

site of Louisa is a movable dam of needles supported by trestles, and recommends the substitution of that type for the fixed dam now in course of construction. The estimated cost of constructing the movable dam proposed is \$93,000, and, as there are only \$17,000 on hand available for the fixed dam, a further appropriation of \$76,000 will be necessary if the views of the Board are to be carried out.

I concur in the recommendations of the Board and recommend that the report be transmitted to the Speaker of the House of Representatives by the information of the Committee on Rivers and Harbors.

Very respectfully, your obedient servant,

THOS. LINCOLN CASEY,
Brig. Gen., Chief of Engineers.

W. M. RAINFIELD PROCTOR,
Secretary of War.

BOARD OF OFFICERS OF THE CORPS OF ENGINEERS OF
THE ARMY AT SANDY RIVER, NEAR LOUISA, KENTUCKY.

NOVEMBER 10, 1891.

The Board of Officers of the Corps of Engineers, constituted by Circular No. 47, from your office, May 20, 1891, "to consider and report on the subject of the dam to be built in the Big Sandy River, Louisa, Ky., have the honor to submit final report,

which was authorized, and was under construction, of the dam on the Big Sandy River, Kentucky, was going on the subject of the dam to be built in the Big Sandy River, Louisa, Ky., because of a communication received from the Senator from Ohio, Mr. E. Nigh, of Ironton, Ohio, on the 10th of May, 1891, in which he requested the Secretary of War to cause a change to be made to the type of dam to be built in the Big Sandy River, Louisa, Ky. A request was expressed for a "wicket dam" instead of a "fixed dam" and a request was made "to have the dam built in accordance with the work." This request was referred to the Board and report by this Board, which was made on the 10th of November, 1891, in regard to the kind of dam to be built in the Big Sandy River, Kentucky.

The Board assembled at Louisa, Ky., on the 10th of November, 1891, and at the next day at 10 o'clock A. M. The Board considered the statement of parties in interest in the construction of the fixed dam, and the statement of parties in interest in the construction of a movable dam, and others, and decided to recommend the fixed dam to completion.

The Board also considered the report of representatives from the Big Sandy River, Louisa, Ky., and its forks, as to the relations with the dam to be built in the Big Sandy River, Louisa, Ky. The Board was unable to reach a decision on this subject in its discussion. Those representatives who were present in expressing their views on the subject of the dam to be movable, recommended that the dam be movable.

The Board also considered the report of the representatives of the Big Sandy River, Louisa, Ky., as to the relations with the dam to be built in the Big Sandy River, Louisa, Ky. The Board was unable to reach a decision on this subject in its discussion. Those representatives who were present in expressing their views on the subject of the dam to be movable, recommended that the dam be movable.

be useless, and for the same reason the exit from the lock will so fill in with sand as to prevent the passing of rafts even a great part of the rafting stages.

opposed to the change of type desire the work to be carried on as at present authorized, because—

the dam now under construction can be completed and made ready for use at an earlier period than a dam of the changed type can be

the system of fixed dams will cost less than a system of movable dams and consequently the river will be sooner improved with the appropriations made for the work.

Some parties dissent from the parties opposed in regard to the apprehensions of serious evils inherent to the fixed type when applied to the Sandy River.

The reports, and returned with the original case, are a number of communications in which parties in interest have expressed views on different sides of the question at issue and to which attention is invited.

The first dam, while one in a series of similar structures, is specially designed, in advance of the construction of others in the series, to provide a pool for the harboring of loaded barges during low water. These barges do not get out of the pool during low water because of the lack of a lock below the dam and out to the Ohio River. A rise from the lower river must then be awaited. The rises from the upper river bring large numbers of rafts, and it would then come about that the rafts accumulated in this harboring pool and the arriving rafts would be seeking passage through the lock during these periods of high water.

To what extent this demand would tax the capacity of the lock during periods of time can not be determined from any data now at hand.

To assume how many barges would be harbored in the pool at any given time would be mere guesswork. Information in regard to the number of rafts arriving per hour for any defined period is very incomplete. Attention is invited to the following tabular statement:

Timber output of Big Sandy River, 1882-'90.

Volume in cubic feet.	Number of lockages of 8,000 cubic feet each.	Number of days river was at rafting stage at Louisa, Ky., 6 to 15 feet.	Remarks.
24,000	690	-----	Record last half year only; no record for first half year.
45,000	587	10	
26,920	778	80	
44,565	543	68	
71,360	1,021	143	
89,040	1,211	65	Record for first half year only; no record for last half year.
25,210	665	29	
65,000	569	182	
18,735	602	142	

* Given in reports as 107,083 tons, including cross ties.

The lock is large enough to take two rafts at one lockage, and it is estimated that three lockages at least could be made in one hour. The most unfavorable year for the lock would have been 1887, when 1,211 lockages of rafts would have been required in sixty-five days, or less than twenty lockages per day. No other year for which the record is available would have required as great an average as ten lockages per day during the rafting stages. There is no doubt that the lock could

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determined whether there has been any deposit of sand to interfere with the passage through the lock. If such be found, the deposit removed and the lock put in perfect condition for use before the flood be closed. Thus navigation will not be prevented for an in-

practical use of a movable dam, as indeed of any dam, involves a large quantity of water to cover the leakage through it when closed; otherwise the maintenance of the pool to its normal or even to a useful level is impracticable. Measurements of the discharge of the river at various points varying from 16.2 feet on the gauge down to 1.98 feet on the dam have been made, and a tabulation of these measurements is included with this report. In a wicket dam the spaces between the wickets are 4 inches wide. Under the conditions at Louisa the discharge through one of these spaces at low water would be about 52½ cubic feet per second if the wickets were as high as at Dam No. 5, Kanawha River.

If we assume a weir with wickets 8 feet high, the discharge through the pass would be about 24½ cubic feet per second. If wickets be 4 feet from center to center throughout, this leakage through the spaces between the wickets of the pass would be about 13 cubic feet, and per foot of weir would be about 6 cubic feet per second. Assuming for the Louisa dam 124 feet for the pass and 176 for the weir, their leakage would be required about 1,300 cubic feet per second to supply this leakage; that is, an amount equal to the discharge of the stream at the gauge reading of 5 feet, or about that over.

If the spaces between the wickets, however, are covered at the ends by joint covers called joint covers, laid cornerwise upon them. An actual discharge measurement on the Great Kanawha River gave the following data: There are 62 wickets in the pass and 53 on the weir. Those in the pass are 13 feet vertical height from the sill to the top of the wicket, and inclined at an angle of 15½°; similarly on the weir are 5 feet high, and inclined at an angle of 12°. The dam below the dam stood 1.64 feet above the sill of the pass. The pool was maintained at fifteen-hundredths of a foot below the tops of the wickets. Under these conditions the discharge through each space between the wickets of the pass would be nearly 52 cubic feet per second through each space between the wickets on the weir about 12 cubic feet; that is, it would require 3,860 cubic feet per second to supply the leakage through these spaces of Dam 5. Joint covers had been placed over 60 of the spaces in the pass, and over 21 spaces on the weir. The discharge of the stream was measured below the dam and found to be about 1,130 cubic feet per second. This measurement was taken on the Elk River, and assuming the discharge of that tributary as 1,130 cubic feet, the balance of 1,100 cubic feet maintained the pool on 5 to the level and under the conditions stated; that is, this saving over the amount necessary to maintain the pool to 30 feet of that necessary to supply the discharge through all the spaces covered. There were 2 spaces in the pass and 32 on the weir uncovered. The estimated discharge through these spaces was about 1,100 cubic feet per second, leaving 612 cubic feet as the leakage through the joint-covered portion of the dam, and between the wickets and the dam of that portion not joint covered. Omitting consideration for the leakage under the wickets having no joint covers, the leakage through the joint-covered portion of the dam would be less than 1½ cubic feet per foot of the aggregate length of the joint-covered portions of the weir. But the leakage under the wickets is not to be neg-

b. G. W. C. S. B. F. H. O. B. I. C. S. C.

1. The first part of the document is a list of names and titles, including "The Hon. Mr. Justice" and "The Hon. Mr. Justice".

2. The second part of the document is a list of names and titles, including "The Hon. Mr. Justice" and "The Hon. Mr. Justice".

3. The third part of the document is a list of names and titles, including "The Hon. Mr. Justice" and "The Hon. Mr. Justice".

4. The fourth part of the document is a list of names and titles, including "The Hon. Mr. Justice" and "The Hon. Mr. Justice".

5. The fifth part of the document is a list of names and titles, including "The Hon. Mr. Justice" and "The Hon. Mr. Justice".

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10. The tenth part of the document is a list of names and titles, including "The Hon. Mr. Justice" and "The Hon. Mr. Justice".

will suffice to maintain the pool to a useful height. This addition, in the opinion of the Board, is easily feasible.

It is observed that the lowest gauge reading so far reached this year which measurement of discharge has been made is not so low as has been recorded in past years; the low water of 1883 was 1.23 feet. How much less the discharges were in the lower recorded years of past years can not be ascertained or even estimated. There are indications that the bottom of the stream has filled somewhat with silt in these latter years, and a result of this filling would be that the discharge would correspond to higher gauge reading. So that, if the gauge reading has been as much lower as stated, it does not seem probable that the discharge has been correspondingly low. The tabular record of the gauge indicates an obliteration of the lesser gauge readings as time progresses; such would actually result from filling of the pool.

After full and careful consideration of all the elements determining the best type of dam, the Board are of opinion that the best type of dam for the site near Louisa is a needle dam, and they recommend the substitution of a fixed dam for the fixed dam. It is further recommended that the height of the dam from the sill of the pass to the normal level of the pool be 13 feet, and that the dam be built across the river opposite the lock as is done in our practice, leaving all other details to be worked out by the officer in charge.

A needle dam for the site near Louisa will cost somewhat less than a fixed dam.

In 1899 the recommendation was made by the officer in charge to build a fixed dam, with a view of ultimately turning the fixed dam into a movable one. Since that time experience has been had in this with the operation and action of movable dams, and the Board are of opinion that the trial now made should be with that kind of

In this report are inclosed a number of letters and papers addressed to the Board, and a list of them is also herewith.

Estimated cost of constructing a movable dam of needles, supported on piles, Poirée system, on Big Sandy River, near Louisa, Ky.: sill of pass, 125 feet; weir, 175 feet; vertical height of needles of 125 feet; of weir, 8 feet.

PASS AND WEIR.

Materials:	
Sill, 223.75 cubic yards, at \$20	\$4, 475. 00
Restle stone, 102.5 cubic yards, at \$16	1, 640. 00
Red stone, 839 cubic yards, at \$7	5, 873. 00
Blue stone, 315 cubic yards, at \$10	3, 150. 00
White stone, 2,688 cubic yards, at \$7	18, 816. 00
Wrought-iron tie-rods, etc., 13,200 pounds, at 4 cents	528. 00
Iron anchors, 11,000 pounds, at 4 cents	440. 00
Wrought-iron journal-boxes, 17,625 pounds, at 7 cents	1, 233. 75
Nuts	300. 00
	<hr/>
	36, 455. 75
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Abutment:	
Blue stone, 79 cubic yards, at \$16	1, 264. 00
White face stone, 241 cubic yards, at \$8	1, 928. 00
Red stone, 341 cubic yards, at \$7	2, 387. 00
Blue stone, 1,169 cubic yards, at \$5.50	6, 429. 50
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IN SENATE

COMMITTEE ON THE JUDICIARY

HEARINGS

ON THE NOMINATION OF

JOHN EDGAR HOOVER

TO BE ATTORNEY GENERAL

AND

ON THE NOMINATION OF

WALTER G. BARKER

TO BE ATTORNEY GENERAL

mines have been opened at and near Peach Orchard, and quite a number of shipments are made by rail to Ashland, on the Ohio, where they are put in barges for transportation down that river. Could coal be shipped out by water from the mines the output of coal would be largely increased.

Work on this stream can only be done to advantage during low water. It was on September 10, when the working parties were sent out. The work of previous years has removed the most important obstructions that during the past season it was only necessary to clear out what had become obstructed by material washed in during the high waters, and take out such obstructions as logs, stumps and snags that are always found in the channel after a rise. The following is a summary of the season's work:

Work.	Cubic yards.	Number.	Average length.
Removed.....	320		
Removed.....	1,310		
.....		390	25.7
.....		150	69.3
.....		278	
.....		9	

The stream under the present project is incapable of permanent improvement, as the obstructions to navigation, such as snags, stumps, logs, and snags, liable to re-form at any time and an annual appropriation of \$500 is needed to keep it clear.

APPROPRIATIONS.

1880	\$2,500
1881	2,000
1882	5,000
85	5,000
1886	3,750
1891, 1890.....	2,500
Total	20,750

Money statement.

1891, balance unexpended	\$1,151.76
1892, amount expended during fiscal year.....	1,131.90
1892, balance unexpended	19.86
Appropriated by act approved July 13, 1892	2,500.00
Available for fiscal year ending June 30, 1893.....	2,519.86
That can be profitably expended in fiscal year ending June 30, 1894	2,500.00

expended in compliance with requirements of sections 2 of river and navigation acts of 1866 and 1867.

G G 9.

STATEMENT OF TUG FORK OF BIG SANDY RIVER, WEST VIRGINIA AND KENTUCKY.

The Tug Fork is the eastern of the two forks which unite at the town of Ashland, 26 miles from the Ohio, to form the Big Sandy River. It flows from the southwest corner of West Virginia and flows towards the

THE UNIVERSITY OF CHICAGO
DEPARTMENT OF CHEMISTRY
5800 S. UNIVERSITY AVENUE
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has been no change in the situation so far as I can determine, that the obstructions have been increased. The stream is incapable of permanent improvement, as the obstructions, by logs, snags, etc., are liable to re-form at any time, and an annual appropriation is needed to keep it free. Following is a summary of work done during the past fiscal year:

Work removed	cubic yards.	80
Work removed	do.	2, 147
Work moved	number.	392
Work moved	do.	392
Work removed	do.	350
Work moved	do.	192

APPROPRIATIONS.

1880	\$2, 500
1881	2, 000
1882	5, 000
1884	5, 000
1886	3, 750
1887 to 1890	2, 500
Total	<u>20, 750</u>

Money statement.

1891, balance unexpended	\$1, 421. 67
1892, amount expended during fiscal year	<u>1, 270. 33</u>
1892, balance unexpended	151. 34
1892, outstanding liabilities	<u>40. 55</u>
1892, balance available	110. 79
appropriated by act approved July 13, 1892	<u>2, 500. 00</u>
available for fiscal year ending June 30, 1893	<u>2, 610. 79</u>
Amount that can be profitably expended in fiscal year ending June 30, 1894, appropriated in compliance with requirements of sections 2 of river and port acts of 1866 and 1867.	<u>2, 500. 00</u>

G G 10.

IMPROVEMENT OF GUYANDOTTE RIVER, WEST VIRGINIA.

Guyandotte River rises in the southwestern part of West Virginia, flowing in a northwesterly direction, empties into the Ohio River above the mouth of the Big Sandy and 39 miles below the mouth of the Great Kanawha.

The project for the improvement was adopted in 1878, and contemplated the obtaining of a clear channel with a width of 30 feet and a depth of 18 inches during five months of the year by the removal of logs, snags, and other obstructions. This river was practically improved before work commenced, except during rises of considerable

The stream was once improved by the State of Virginia by the construction of six locks and dams. The locks were constructed of timber, and, as a matter of course, after a time, the charges for repairs, ren-

dered necessary by the natural decay of the wood, became very great, and a little further along the system was abandoned, and what had formerly constituted a useful improvement now became obstructions. The old locks and dams were carried away in part from time to time by floods, so that when the first report on this river was made by Maj. William E. Merrill in 1875 most of them had been partially washed away, but the ruins of one still constituted the worst obstruction on the river.

There are two milldams—Pecks, 7 miles below Logan, and Lamberts, 19 miles below Pecks—that still constitute serious obstructions to navigation, and have been reported to the Department as such. The authority for their construction in the first place was based upon a permit from the circuit court, and their continuance has been a nuisance and constant damage to the river interests.

The work in previous years has been to cut passage ways through the old dams where required, make channels through the shoals for push boats, and remove such obstructions as rocks, snags, stumps, logs, etc., existing in the channel, the river as would interfere with rafting at ordinary and low ranging stages. During the past year the work done has been with a view to assisting the running of rafts. The stream is incapable of permanent improvement, as the obstructions in the channel, such as trees, snags, logs, etc., are liable to re-form. The stream up to Logan, a distance of 81½ miles, has an average fall of about 22 inches to the mile.

For details concerning the work attention is invited to the report of Assistant Engineer B. F. Thomas, attached hereto.

APPROPRIATIONS.

June 18, 1878.....	\$2,000	July 5, 1884*.....	\$2,000
March 3, 1879.....	1,000	August 11, 1888.....	2,000
June 14, 1880.....	2,000	September 19, 1890.....	2,000
March 3, 1881.....	3,500		
August 2, 1882.....	2,000	Total.....	16,500

Money statement.

July 1, 1891, balance unexpended.....	\$1,397.00
June 30, 1892, amount expended during fiscal year.....	1,303.30
July 1, 1892, balance unexpended.....	93.70
July 1, 1892, outstanding liabilities.....	.25
July 1, 1892, balance available.....	93.45
Amount appropriated by act approved July 13, 1892.....	2,000.00
Amount available for fiscal year ending June 30, 1893.....	2,093.45
{ Amount that can be profitably expended in fiscal year ending June 30, 1894	2,000.00
{ Submitted in compliance with requirements of sections of river and harbor acts of 1866 and 1867.	

REPORT OF MR. B. F. THOMAS, ASSISTANT ENGINEER.

LOUISA, KY., June 30, 1892.

MAJOR: At the close of the last fiscal year a force was at work on Guyandotte River at Big Huff Creek widening the chute at that point. About 1,600 cubic yards of rock were removed during the first half of July, when the party began the descent of

* Made available by act of August 5, 1886.

working as they went. At Fish Trap Shoal a large rock was taken out; one which was in the chute. The point of the bar was cut off and a slip on bank was removed. At Neiberts Bend, 7 miles above Logan, a portion of Cloyds Island and a heavy rock slip on the east side of the river removed, forming a most useful and excellent piece of work. The point of a bar was cut off at Lawsons Mill, where the river makes a short bend. A large rock was taken out of the main channel at Logan and three more near King Shoal. Rocks were removed at Dry Island and also three at King. Some rocks were removed about 5 miles below Big Hart Creek. A great many trees, stumps, snags, were removed all along the river from Kings Shoal to Rogers Dam, a distance of 15 miles. At Barboursville a very bad piece of river was found and considerable work was done there, but the appropriation was exhausted without completing the improvement. There is a rocky island and also a rock bar that ought to be removed, both lie in a bend of the river and at certain stages of water are a great and dangerous obstruction. A detailed statement of work done is below:

Rock removedcubic yards..	424
Rock removeddo.....	4,280
Slip removed	15
Slip removed	32
Slip removed	7
Slips removed	300
Work done with boatsmiles..	1724
Boats employed (average)	10

Improvements made by the United States on this stream have been of great benefit to the people of that section, and it is estimated that the output of timber has greatly increased this year. A small steamboat now plies the river, and if the Government would remove the dams in the lower part of the streams there is nothing to prevent steam navigation to within 7 miles of Logan, or a distance of 73 miles. Of these there are other obstructions in the bed of the river and overhanging timber must be removed before navigation can be an entire success, but the principal object to be done is to secure the removal of the dams heretofore reported as obstructing navigation. I think that further appropriation for the Guyandotte River should be made for that purpose solely until the removal of all dams is effected. Results of the work conducted are respectfully submitted.

B. F. THOMAS,
Assistant Engineer.

D. W. LOCKWOOD,
Cincinnati, Ohio.

COMMERCIAL STATISTICS.

Commerce of Guyandotte River, West Virginia, for the fiscal year ending June 30, 1892.

Articles.	1891.	1892.
	Tons.	Tons.
Coal	80,000	12,500
Iron ore	1,128	90,000
Total	61,128	103,800

List of boats plying on Guyandotte River, West Virginia.

Name.	Character.	Length.	Breadth.	Depth.	Tonnage.
		Feet.	Feet.	Feet.	
Walker (tow)	Side wheel	73	8	2	7

G G II.

IMPROVEMENT OF LITTLE KANAWHA RIVER, WEST VIRGINIA.

The Little Kanawha drains the central portion of West Virginia rising in Upshur County; its course is a little north of west and empties into the Ohio at Parkersburg; its total length is about 14 miles.

The Little Kanawha Navigation Company owns and operates four locks and dams on this river, which furnish slack-water navigation from the Ohio up to a point two miles above the village of Burning Spring where the United States lock is located, but the service of this system is very poor, owing to the dilapidated condition of both locks and dams; breaks and washouts are of frequent occurrence during high water, and the interruption to navigation in consequence expensive and harassing.

The present project for the improvement of this river, adopted in 1876 and modified in 1880, contemplates the construction of a lock and dam to extend slack-water navigation for a draft of 4 feet a distance of 12 miles above the point reached by the navigation company, and the improvement of the natural channel of the upper river by the removal of obstructions, etc., for a distance of 80 miles, the object of the latter being to obtain a channel of a minimum width of 40 feet with a depth of 2 feet for at least four months in each year.

During the past fiscal year the lock and dam were completed to such an extent that the lock was opened to navigation December 2, 1891. It was found impossible to complete the backing of the dam before the winter set in, and this work was deferred until after the spring floods. The backing of the dam is now in progress and will be completed in a short time. As always happens where proper bank protection is not put in in advance, the banks just below the lock and abutment have washed considerably, but not to a dangerous extent. The completion of the lower shore guide wall and placing of rip-rap below the abutment will effectually secure the banks against further washing.

For details of work done during the past year attention is invited to the report of Assistant Engineer B. F. Thomas, appended hereto.

APPROPRIATIONS.

August 14, 1876.....	\$7,300	August 5, 1886.....	\$16,875
June 18, 1878.....	18,000	August 11, 1888.....	25,000
March 3, 1879.....	18,000	September 19, 1890.....	40,000
June 14, 1880.....	15,000		
March 3, 1881.....	40,000	Total.....	211,175
August 2, 1882.....	31,000		

Money statement.

July 1, 1891, balance unexpended.....	\$27,315.2
June 30, 1892, amount expended during fiscal year.....	23,805.8
July 1, 1892, balance unexpended.....	3,509.4
July 1, 1892, outstanding liabilities.....	986.6
July 1, 1892, balance available.....	2,523.4

of the space behind the lock
next month, as the cold weather
building the protection cribs
was done in September, and
used in filling the cribs. The
transported in cars down
the works. At the start the
quarry, but owing to the
ground without
handling up the empties.
at the top of the incline.
very well. Now that a p
to obtain stone from a qu

lock gates without pump
was across the lock jus
longer of use. When put
about half a mile below
the lock chamber an
the concrete was tal

an adjacent s
this had to
during the season
was pumped on
likely. The
by men
The material th
large loads
During
and
the same
working

list of
the li
necessary
the
the

able inconvenience and expense as well as delay was caused by the drawing pool to repair the Palestine Lock, belonging to the Little Kanawha Navigation Company. This so reduced the water that but small loads of stone could be and even these had to be tied up some distance below when needed and the loaded in barrows to its destination. At times the length of wheeling was

It was very tiring to men as well as expensive and slow. This year the contract for timber for dam was filled, and timber for the cribs purchased. T. W. Moore, the contractor, furnished 417,686 board measure lumber. Smith & Co. also furnished some timber for protection cribs. The same were furnished by Pattin, Hall & Pattin, of Marietta, Ohio. The same were fully submitted.

B. F. THOMAS,
Assistant Engineer.

T. W. LOCKWOOD,
Corps of Engineers, U. S. A.

COMMERCIAL STATISTICS.

of Little Kanawha River, West Virginia, for fiscal year ending June 30, 1892,

Articles.	1890.	1891.	1892.
	Tons.	Tons.	Tons.
.....	1,100	10,597	3,100
.....	8,540	28,677	20,486
.....	678	1,450	1,738
.....	24,500	26,994	10,050
.....	70,000	59,703	109,337
.....	27,650	48,312	92,200
.....	1,563	1,500	1,343
.....	6,084	13,575	6,090
.....	140,115	190,688	244,254

List of boats plying on Little Kanawha River, West Virginia.

[Character, stern-wheel.]

Name.	Length.	Breadth.	Depth.	Tonnage.
	Feet.	Feet.	Feet.	
packet)	104	19.5	4.2	75.78
Iron (packet)	105	22	3.6	67.54
Iron (tow)	99	17	2.6	80.45
(tow)	66.5	3	34.93
(tow)	80	13.6
(tow)	98.4	15.1	2.6	37.16

G G 12.

MAINTAINING AND KEEPING IN REPAIR THE LOCK AND DAM ON LITTLE KANAWHA RIVER, WEST VIRGINIA.

The Lock and Dam No. 5 of the Little Kanawha River were so far advanced towards ultimate completion that the lock was opened to navigation on December 2, 1891, and has since that time been continuously in operation, with the exception of a short time during June, when it was necessary to readjust the lower gates to the miter sill and put new screens in the chamber openings of the lower culverts.

Considerable damage was done to the dam during the winter by ice and ties going over it and pounding off the sheeting of the dam, but as the companies or firms engaged in running logs in

this manner have put in booms above the lock, it is hoped that it will be no trouble from this source in the future.

Allotment for year ending June 30, 1892, \$1,500.

Detailed statement of expenses incurred in preserving and maintaining navigation a portion of the Little Kanawha River, West Virginia, improved by lock and dam during fiscal year ending June 30, 1892.

Months.	Salaries.	Labor and material.	Current and contingent expenses.	T
1891.				
December.....	\$95.00			
1892.				
January.....	95.00		\$14.25	
February.....	95.00	\$21.25	3.65	
March.....	95.00		1.25	
April.....	95.00		1.25	
May.....	95.00			
June.....	95.00			
Total.....	665.00	21.25	30.40	

REPORT OF MR. B. F. THOMAS, ASSISTANT ENGINEER.

LOUISA, KY., June 30, 1892

MAJOR: Locking was begun on the Little Kanawha River, at Lock No. 5, on December 2 last, and nothing has occurred to interfere with navigation since except a short stoppage to repair a leak between the lower gates and miter sill in the present month. The lock has been in fine working condition ever since it was opened to navigation. While the approaches to the lock were being made after the completion of the dam, some of the timber firms operating on the became fearful that navigation through the lock would not be opened speedily, and made no preparations for holding their timber in the upper pool, but allowed it to escape over the dam. This was continued for several weeks after navigation was established and assured, and the dam was greatly injured by the loose logs and ties pounding the decking or floor. The lower step of the dam has been partly ried away and nearly all loosened, and the entire lower face of the work has been damaged more or less. For the past two or three months but few logs have come over the dam, and the lockage of rafts has greatly increased in consequence. The construction of this dam has lengthened the navigable river about 12 miles, and boats run regularly to Creston at the mouth of the West Fork. This is of great benefit, not only to people living in the vicinity of the pool, but also to those living in the upper part of the valley, as it brings the head of navigation much nearer to them.

The intention originally was to employ but one lock tender, as is done on the lower part of the river, and with this idea a lock house was built. The greatly increased business of the river, particularly in timber and cross-ties, which is always greatest at the upper lock, made it necessary to employ an additional lock tender, and it will be to the interest of the United States to erect another dwelling this season for his use.

No damage of consequence has resulted to the lock. Two wicket screens were torn off during the high water of the winter and spring, but caused neither inconvenience nor delay to navigation.

The river bank below the abutment has washed away considerably, and it will be best to protect it with riprap before another winter. Below the lock there has been some erosion of the land, but it has not yet progressed far enough to cause any easiness, and the probabilities are that it will stop without damage to the adjacent property.

Respectfully submitted.

B. F. THOMAS,
Assistant Engineer

Maj. D. W. LOCKWOOD,
Corps of Engineers, U. S. A.

ges on Little Kanawha River, West Virginia, for fiscal year ending June 30, 1893.*

sted	197
ended.....	13
boats	259
and flats	168
laneous.....	61
n:	
boats	361
and flats	146
.....	1,915
laneous.....	52
s and flats passing	2,913
ber of lockages.....	2,871

G G 13.

MOVEMENT OF BUCKHANNON RIVER, WEST VIRGINIA.

st for the improvement of this stream, approved in 1884, is that portion of the river between the Three Forks and the khannon, a distance of 24½ miles. The obstructions consist and bowlders, some of which are estimated to contain 500

et called for a channel 30 feet wide.

ion River is a tributary of the Tygart Valley, and rising in stern part of Randolph County, W. Va., flows a little east il it empties in the Tygart Valley River in Barbour County. nce from the Three Forks to the mouth is 47½ miles, and the of the stream is about 57 miles.

he past fiscal year work was commenced during the last gust, and continued so long as the balance of funds available ant.

large bowlders were removed, containing in all 3,510 cubic one, and 136 trees and 25 logs were taken from drifts and

onal appropriation is required for this work.

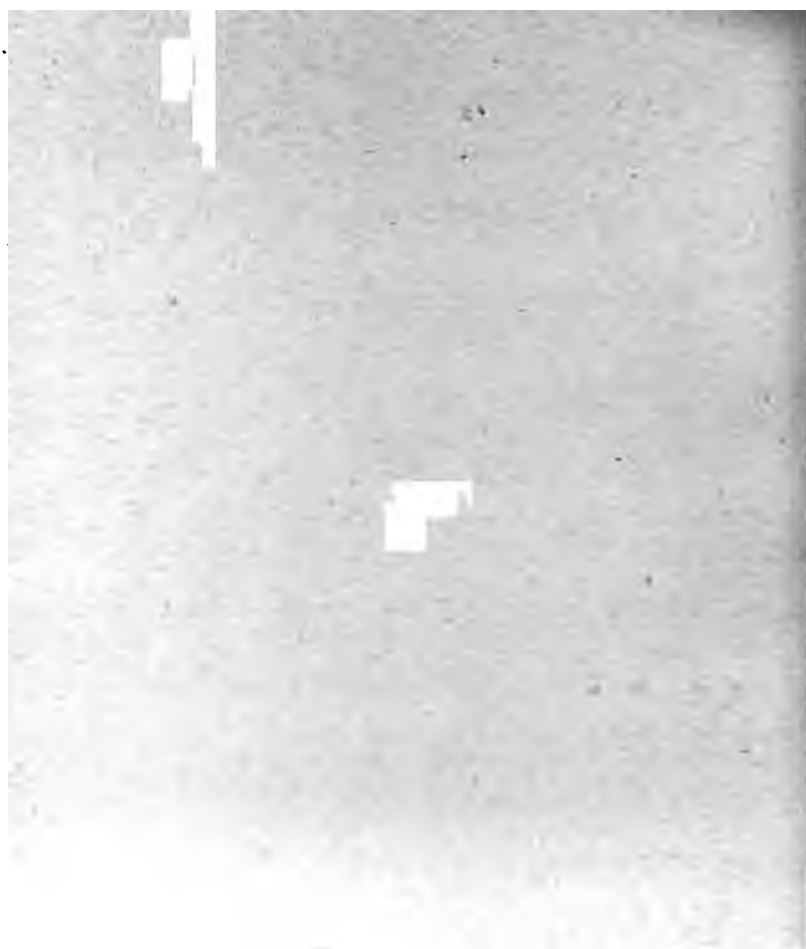
APPROPRIATIONS.

.....	\$1,500.00
.....	1,500.00
8	1,500.00
1890	1,000.00
.....	<u>5,500.00</u>

Money statement.

alance unexpended	\$1,027.40
amount expended during fiscal year.....	1,010.18
.....	<u>17.22</u>

1 through lock suspended six days on account of high water, and seven lock gate.



APPENDIX H H.

IMPROVEMENT OF HARBORS ON LAKE SUPERIOR.

REPORT OF CAPTAIN W. L. FISK, CORPS OF ENGINEERS, OFFICER IN CHARGE, FOR THE FISCAL YEAR ENDING JUNE 30, 1892, WITH OTHER DOCUMENTS RELATING TO THE WORKS.

IMPROVEMENTS.

- | | |
|---|---|
| 1. Harbor at Grand Marais, Minnesota. | 6. Harbor at Ashland, Wisconsin. |
| 2. Harbor at Agate Bay, Minnesota. | 7. Harbor at Ontonagon, Michigan. |
| 3. Harbor at Duluth, Minnesota. | 8. Eagle Harbor, Michigan. |
| 4. Harbor at Superior Bay and St. Louis Bay, Wisconsin. | 9. Harbor at Marquette, Michigan. |
| 5. Minnesota Point at Superior, Wisconsin. | 10. Harbor of refuge at Grand Marais, Michigan. |

EXAMINATION AND SURVEY.

11. St. Louis River, Minnesota and Wisconsin, from Grassy Point, in St. Louis Bay, to Fond du Lac.

UNITED STATES ENGINEER OFFICE,
Duluth, Minn., July 1, 1892.

GENERAL: I have the honor to transmit herewith annual reports upon works of river and harbor improvement at present in my charge for the fiscal year ending June 30, 1892.

Very respectfully, your obedient servant,

W. L. FISK,
Captain, Corps of Engineers.

Brig. Gen. THOMAS L. CASEY,
Chief of Engineers, U. S. A.

H H 1.

IMPROVEMENT OF HARBOR AT GRAND MARAIS, MINNESOTA.

On the north shore of Lake Superior there are very few localities where safe anchorage for vessels can be found, and Grand Marais offers the only harbor of refuge during storms between Agate Bay and Pigeon River, the international boundary, a distance of 120 miles. It is not yet a shipping port of much importance, but it is so near the rich iron ore deposits of the Vermillion Range that it is probable a railroad will soon be built to connect it with them.

The approved project of 1879 is to build two breakwater piers, each 50 feet long, from the east and west points of the bay, or one pier 700 feet long from the east point, and dredge an anchorage area of about acres to a depth of at least 16 feet, all at an estimated cost of

The object of this work is to provide a harbor for purposes of refuge and commerce, which it has fully accomplished as far as completed.

The improvement of the harbor commenced in 1880; at the present time the length of completed breakwater is 350 feet, or one-half of the whole, and the 16-foot anchorage is 21.5 acres in extent, or a little more than four-fifths of that contemplated by the original project.

The anchorage is compact in shape and fairly well protected, but this space is not of sufficient size to permit more than one or two vessels to maneuver with safety. The harbor is so small its entire area, 61 acres, should be utilized. To do this in the best manner would require 500 feet extension of the breakwater, and additional dredging to the amount of about 450,000 cubic yards. The extension of the breakwater would cost about \$100 per foot, or \$50,000; the additional dredging, at the present contract price of 20 cents per yard, would cost about \$90,000 more, which, with 10 per cent for contingencies, would make a total of \$154,000 to complete this excellent harbor. The original estimate of the cost of improving this harbor was \$139,669.40, but contemplated protecting and dredging only a part of it.

During the year small repairs were made to the breakwater, which is in good condition.

The contract under way at the date of the last annual report was completed the following August, 70,850.9 yards having been dredged during the fiscal year just closed, or a total under the contract of 100,780 yards.

This work is in the collection district of Duluth, Minn. Duluth, Minn. is the nearest port of entry. The nearest light-house is situated on the breakwater at Grand Marais.

Abstract of appropriations for improving harbor at Grand Marais, Minn.

By act of Congress—		By act of Congress—	
Approved March 1, 1879.....	\$10,000	Approved August 5, 1886.....	\$10,000
Approved June 14, 1880.....	10,000	Passed August 11, 1888.....	15,000
Approved March 3, 1881.....	20,000	Approved September 19, 1890	22,350
Passed August 2, 1882.....	20,000		
Approved July 5, 1884.....	10,000	Total	117,350

EXPENDITURES.

Amount expended under approved project to June 30, 1892	\$115,883.71
Balance available July 1, 1892	1,466.29

ESTIMATES.

Original (estimated) amount required to complete the improvement	\$139,669.40
Amount that can be profitably expended.....	100,000.00

Money statement.

July 1, 1891, balance unexpended*	\$22,922.18
June 30, 1892, amount expended during fiscal year	21,455.89
July 1, 1892, balance unexpended	1,466.29
Amount appropriated by act approved July 13, 1892.....	10,000.00
Amount available for fiscal year ending June 30, 1893	11,466.29
{ Amount (estimated) required for completion of existing project	12,319.40
{ Amount that can be profitably expended in fiscal year ending June 30, 1894	100,000.00
{ Submitted in compliance with requirements of sections 2 of river and harbor acts of 1866 and 1867.	

* The balance unexpended July 1, 1891, as shown in Annual Report for 1891 is \$22,888.85; the difference, \$33.33, is amount disallowed on Voucher No. 1, item 1, fourth quarter, 1891.

handling of merchandise. The commerce soon grew out of all proportion to the size of the harbor, and for the security of this commerce was necessary to supplement the natural protection by artificial means.

The approved project of 1887 is to construct two breakwater piers a line towards each other from the eastern and western points of the bay, to be 1,000 feet and 900 feet long, respectively, leaving an opening of 1,340 feet between their extremities and inclosing an area of 100 acres.

The original estimate of cost was \$213,000, which was increased to \$244,208 in Annual Report of 1887 on account of higher prices.

The object is to protect shipping at the wharves which otherwise would be greatly exposed. Work was commenced in 1887 on the eastern pier, which, including 200 feet built under the contract in progress at the date of last annual report, is now 750 feet in length, leaving 250 feet to be built to complete it, while nothing has been done on the western pier; the work is therefore a little more than one-third done, but the remaining two-thirds will cost considerably more than twice what has already been expended, for the reason that it is in much deeper water.

There is not yet perfect security from southwest storms for vessels lying at the merchandise dock, but it is no longer necessary to leave the harbor and seek security elsewhere. The favorable results already obtained with the portion of the breakwater now built are much greater than anticipated, and emphasize the advisability of speedily completing the remainder of the projected piers.

During the past winter the Duluth and Iron Range Railroad put in a third large ore dock, which it is now extending to about double capacity originally intended, and will soon put in two more, making five in all.

To protect vessels at these new docks as much as possible, the work should be done on the west pier, and in order to give good shelter the entire 900 feet of it should be completed; this would cost at least \$135,000, and this amount could be profitably expended during the next year.

The Iron Range is the only railroad using this harbor as a shipping point, and the business of the place has heretofore been entirely subsidiary to the iron-ore interests of the Vermillion Range, but the immense quantities of excellent iron ore recently developed in the Mesabi Range bid fair to be quite as important as the former and likely to seek an outlet through Agate Bay. Of iron ore alone there were shipped from this port during the season of 1891, 1,000,052 tons.

Agate Bay (known locally as Two Harbors) is in the collection district of Duluth, Minn., which is also the nearest port of entry. The nearest light-house situated on the east point, Agate Bay, Minnesota.

Abstract of appropriations for improving harbor at Agate Bay, Minnesota.

By act of Congress:

Approved August 5, 1886	\$22,500
Passed August 11, 1888	15,000
Approved September 19, 1890	25,000
Total	62,500

EXPENDITURES.

Amount expended under approved project to June 30, 1892	\$61,300
Balance available July 1, 1892	1,100

ESTIMATES.

Estimated amount required to complete the improvement	\$181,708.00
Estimated amount required annually for preserving and maintaining when improvement is completed	2,000.00

Money statement.

July 1, 1891, balance unexpended	\$26,244.21
June 30, 1892, amount expended during fiscal year	25,128.80

July 1, 1892, balance unexpended	1,115.41
Amount appropriated by act approved July 13, 1892	30,000.00

Amount available for fiscal year ending June 30, 1893	31,115.41
---	-----------

(Amount (estimated) required for completion of existing project	151,708.00
(Amount that can be profitably expended to fiscal year ending June 30, 1894	135,000.00
(Submitted in compliance with requirements of river and harbor acts of 1866 and 1867.	

COMMERCE AND TRADE.

Vessels arriving and departing.

Minnesota.

Year.	Vessels.	Estimated tonnage.	Increase.
1865	174	295,800
1866	263	460,000	164,200
1867	465	697,500	237,500
1868	749	1,436,000	738,500
1869	1,255	2,400,000	964,000
1870:			
Steam, 968	} 1,050	2,625,000	225,000
Sail, 82			
1871:			
Steam, 1,188	} 1,250	2,915,000	290,000
Sail and whaleback, 62			

Receipts and shipments.

Year.	Ore shipped.	Other freight received and shipped.	Total.
	<i>Tons.</i>	<i>Tons.</i>	<i>Tons.</i>
1865	225,484	10,895	236,379
1866	304,396	21,954	326,350
1867	394,252	6,620	400,872
1868	509,964	30,352	540,316
1869	924,054	31,731	955,785
1870	984,215	37,268	1,021,483
1871	1,000,052	41,181	1,041,233

Estimated value of freight received and shipped. (Exclusive of ore.)

1867	\$96,000
1868	212,000
1869	222,000
1870	246,000
1871	225,000

There were no new lines of transportation established last year,

IMPROVEMENT OF HARBOR AT DULUTH, MINNESOTA.

THE CANAL, OR PIER.

The question of ownership of the land occupied by the canal is unsettled, and is somewhat complicated owing to the deed of the city of Duluth to the piers lots having been accepted in part only by act of Congress of August 23, 1888, as the claims of the late Mr. Will Hoising to property not included in the deed but covered by the canal and to the fact that the city of Duluth has never vacated to the United States the underlying portions of streets.

No work of any kind was done on the canal piers during the year, but a contract has just been let for placing a new pierhead crib at the end of the south pier and repairing the old one, which has been damaged by ice and threatens the safety of the outer light-boat. There is no question of land ownership at this point.

The north pierhead has also been considerably damaged, although not so badly as the other. It will, however, soon require repairing.

There is now on hand for repairs to piers something over \$14,000, which has been reserved from various appropriations for the harbor, but the contract just let will require, including inspection expenses something over \$10,000, leaving available for emergencies only \$4,000. As the piers are old and the superstructure considerably aged and decayed, \$25,000 should be available for repairs. The new over 20 feet of water nearly the entire length of the canal, 12,000 yards of dredging will give an available depth of 23 feet three inches.

While the piers were built for a depth in the canal of only 12 feet they show no signs of giving way with an existing depth of from 15 to over 30 feet, which means that they will not have to be rebuilt at least for the new 23-foot channel.

THE HARBOR BASIN.

No work has been done upon the harbor basin during the year, and no complaints of shoaling have been heard. This basin should be enlarged where it joins the new channel east of Rice Point, as boats are constantly backing out from one wharf to another, and probably less than a half dozen exceptions all vessels bound to West Superior this season have come in through the canal and passed through this basin and the new channel over to the West Superior wharf. At least \$75,000 could be profitably expended in this work during next year.

RICE POINT CHANNEL.

The dredging for the new channel east of Rice Point was commenced in 1889, but as the funds allotted were not sufficient to give a navigable channel through, it was not carried beyond the old dike. When the river and harbor act of 1890 was passed the opening of this channel was considered of such importance that authority was given by the Secretary of War to continue the dredging upon the terms of the 1889 contract without losing the time necessary to advertise for new bids, and an agreement to that effect was entered into with Williams, Upham

o. the former contractors. The act of Congress was signed on the 9th of September, and dredging was begun on the 24th. When work stopped for the season sufficient had been done to permit the passage of large boats with careful handling by tugs.

An examination of the channel made just before the breaking up of the ice in April, 1891, showed that it had held unexpectedly well, but had a least width between 15-foot curves of only 45 feet. Work was resumed on the 28th of that month, as soon as the ice would permit, the channel carefully trimmed up and widened, until when the funds were exhausted, May 28, it had a least width of about 110 feet between 15-foot curves. An examination made during last winter after a full season's use of this channel showed surprisingly good results, the least width between 15-foot curves being 90 feet at two points while the average width throughout was 125 feet. The length of this channel is 1.6 miles. The present project calls for a width in this channel of 200 feet, but in my opinion it should be at least 400 feet to safely and conveniently accommodate the traffic passing through it. The custom-house records show 1,610 arrivals and 1,610 departures of vessels from West Superior during the season of 1890, the aggregate registered tonnage being 2,064,463, of which over 1,000,000 tons used this channel. In addition it is used by numerous tugs, with large rafts of logs in tow, and the large ferryboats make it in both directions. It is at present by far the most important harbor improvement upon which the Government has been profitably expended.

The estimated cost of the 200-foot channel is \$1,119,552, of which \$63,000 has already been allotted to complete.

NORTH-SHORE CHANNEL, ST. LOUIS BAY.

With the \$40,000 allotted for this channel from the appropriation made August 11, 1888, a channel of such width as could be made at a single cut of a dredge was dug between the deep water at Rice Point and that at Grassy Point. Though it had a depth of 14 feet throughout its length, its width was not sufficient to render it safely practicable for vessels of ordinary size. Important manufacturing industries are now located at Grassy Point, and the obliteration of this insignificant channel by neglect would work a great injury to these important interests and to the commerce dependent upon them. Aside from this, it is a matter of considerable economy to complete such channels as soon as possible, and thus avoid expensive redredging. The river and harbor act of September 19, 1890, gave \$60,000 for the continuation of this work. Dredging was begun under the new contract May 28, 1891, and completed on the 17th of October, when this channel had throughout a width of at least 100 feet, and the mills at West Duluth had begun shipping lumber through it. An examination made through the ice during last winter showed the channel to be holding well, as it was in practically the same condition as when work stopped.

The estimated cost of this channel is \$163,000. There therefore remains to be supplied \$63,000, which amount could be most advantageously expended during the next year.

PARK POINT CHANNEL.

Nothing has been done to this proposed channel.

HARBOR LINES.

Several instances of illegal dumping of dredgings in the harbor fall were reported to United States district attorney for Minnesota under the provisions of the last river and harbor bill. The offender plead guilty and a nominal fine was imposed. Since that time no other cases have been detected.

FURTHER IMPROVEMENTS.

In view of the increased depth adopted for the new lock at Sault Marie and connecting lake channels, it becomes necessary to submit revised estimate of the cost of the approved project for this harbor order that the necessary depths may be available here by the time they are in the lower lake channels.

The approved project calls for an anchorage area inside the canal 3,200 feet long by 1,450 feet wide; a channel east of Rice Point 200 feet wide; a channel 200 feet wide along the north shore of St. Louis Bay from Rice Point to the deep water of St. Louis River at Grassy Point and a channel along the 2,640 feet of Minnesota Point dock line nearest the canal. At present the anchorage area has a depth of 16 feet throughout; the Rice Point channel is 100 feet wide between 15-foot curves; the channel along the north shore of St. Louis Bay is also 100 feet wide between 15-foot curves; and nothing has been done on Minnesota Point (Park Point) channel.

With 21 feet in running channels the harbor and its channels should have a depth of not less than 22 feet in order that deep laden vessels starting and stopping shall not throw up troublesome lumps with their wheels, and on this basis the following amounts must be dredged,

(1) Canal, 12,258 cubic yards, at 15 cents	\$1,83
(2) Harbor or anchorage area, 847,617 cubic yards, at 15 cents	127,14
(3) Rice Point channel (200 feet wide between 22-foot curves), 444,661 cubic yards, at 15 cents	66,69
(4) Minnesota (Park) Point channel (200 feet wide between 22-foot curves), 343,211 cubic yards, at 15 cents	51,48
(5) St. Louis Bay channel (200 feet wide between 22-foot curves), 758,204 cubic yards, at 18 cents	136,47
2,405,951 cubic yards	383,63
Contingencies, 10 per cent.	38,37
Total	422,00
Required to complete project as it now exists (16-foot depths)	224,53
Increase	197,47

It will probably be necessary to dump most or all of the dredged material in the lake, and on account of the long tow the St. Louis work will cost more than the rest.

Of the above items (1), the greater part of (2), and (3) may be classed as urgent, while (5) can probably be done gradually, as its use for present is almost entirely by lumber vessels, which will not require maximum depth; (4) is not necessary at present, although it may be done in a short time.

The St. Louis River above Grassy Point has a depth of over 10 feet for several miles and 8 feet to Fond du Lac Station. Four or five feet can be carried up to the foot of the rapids, from which point up the river has a fall of about 600 feet in less than 7 miles. The utilization

this immense water power has already been commenced, the first one located at Thomson, being now about completed and giving a head of 36 feet, but it appears that a deep navigable channel to Fond du Lac is essential to the success of any enterprises in this locality. The estimated cost of a channel 100 feet wide and 16 feet deep from Grassy Point to the deep pool near Fond du Lac is \$112,822.88 (see separate report of survey of St. Louis River herewith),* and \$50,000 could be profitably expended on it during next year.

CONDITION OF WORK.

The canal piers are in fairly good condition, but need extensive repairs, particularly the superstructure. The entire work will eventually require to be replaced with more durable material.

The ruling depths in the portions of the harbor dredged by the United States are:

	Feet.
the canal.....	17
the inner basin or harbor.....	16
the channel on north shore of St. Louis River.....	16
the new channel east of Rice Point.....	16

Résumé of recommendations.

For next year:		
For canal piers.....	\$25,000	
For the Rice Point Channel.....	100,000	
For North Shore Channel, St. Louis.....	63,000	
For enlargement and development of basin.....	75,000	
For St. Louis River above Grassy Point.....	50,000	
Total.....	313,000	

This work is in the collection district of Duluth, Minn., which is also the port of entry. The nearest light-house is situated on the outer end of the south pier of the Duluth Canal and a range light is located on the inner end of the same pier.

Abstract of appropriations for improving harbor at Duluth, Minnesota.

By act of Congress approved—	
March 3, 1871.....	\$60,000.00
June 10, 1872.....	50,000.00
Alotted from act passed March 3, 1873.....	36,049.20
By act of Congress approved—	
June 23, 1874.....	10,000.00
March 3, 1875.....	35,000.00
August 14, 1876.....	15,000.00
June 18, 1878.....	30,000.00
March 3, 1879.....	25,000.00
June 14, 1880.....	25,000.00
March 3, 1881.....	40,000.00
By act of Congress passed August 2, 1882.....	45,000.00
By act of Congress approved—	
July 5, 1884.....	45,000.00
August 5, 1886.....	56,250.00
By act of Congress passed August 11, 1888.....	80,000.00
By act of Congress approved September 19, 1890.....	100,000.00
Total.....	652,299.20

The following statement shows the manner in which the appropriations were expended. The amount expended under the different classes

*Appendix H H 11.

REPORT OF THE CHIEF OF ENGINEERS, U. S. ARMY.

of - as the cost of soundings, superintendence, buoying,
 COI

Total amount expended to June 30, 1892:

Breakwater.....	\$110, 00
Canal piers, etc.....	77, 50
Dredging.....	447, 84
Total.....	635, 35

Expended prior to present project:

Breakwater.....	110, 00
Canal piers, etc.....	45, 60
Dredging.....	114, 95
Total.....	270, 65

Expended under present project (adopted in 1881):

Canal piers, etc.....	31, 81
Dredging.....	332, 88
Total.....	364, 69

Money statement.

July 1, 1891, balance unexpended.....	\$62, 83
June 30, 1892, amount expended during fiscal year.....	45, 82

July 1, 1892, balance unexpended.....	17, 01
July 1, 1892, outstanding liabilities.....	\$62, 23
July 1, 1892, amount covered by uncompleted contracts.....	10, 240. 00
	10, 30

July 1, 1892, balance available.....	6. 70
Amount appropriated by act approved July 13, 1892.....	125, 00

Amount available for fiscal year ending June 30, 1893..... 131, 70

{ Amount (estimated) required for completion of existing project.....	122, 02
{ Amount that can be profitably expended in fiscal year ending June 30, 1894.....	313, 00
{ Submitted in compliance with requirements of sections 2 of river and harbor acts of 1866 and 1867.	

Abstract of proposals received for constructing a protection crib and repairs to canal piers at Duluth, Minn., opened June 14, 1892, at Duluth, Minn.

No.	Name and address of bidder.	For protection crib, including riprap.	For repairs to old pier-head.	Total amount bid.
1	Andrew J. Wenzell, Grand Rapids, Mich.....	\$8, 600	\$3, 070. 00	\$11, 670
2	Louis Meining, Duluth, Minn.....	7, 859	2, 789. 20	10, 648
3	Francis L. McDonald, Duluth, Minn.....	8, 549	1, 675. 00	10, 224
4	Fred. H. Quinby and Francis Omeis, Duluth, Minn.....	8, 300	1, 700. 00	10, 000
5	William E. Davis and Peter P. Ferguson, Duluth, Minn.....	8, 600	3, 183. 00	11, 783
6	James Edward Smith, Marquette, Mich.....	9, 141	2, 143. 00	11, 284
7	E. J. Amory, Duluth, Minn.....	14, 500	3, 500. 00	18, 000

Contract awarded to Fred. H. Quinby and Francis Omeis with the approval of the Chief of Engineers.

APPENDIX H H—REPORT OF CAPTAIN I

COMMERCIAL STATISTICS.

Arrivals and clearances of vessels at Duluth, Minn., for 1890 and 1891.

Designation:	1890.				1891.			
	Arri-vals.	Clear-ances.	Total.	Tonnage.	Arri-vals.	Clear-ances.	Total.	Tonnage.
Coastwise.....	1,102	1,125	2,227	2,554,020	1,291	1,301	2,592	
Foreign.....	164	143	307	186,334	153	150	303	
Total.....	1,266	1,268	2,534	2,740,354	1,444	1,451	2,895	3,268

Comparative statement of arrivals and clearances, 1890 and 1891.

Year.	Vessels.	Tonnage.	Ave toni
1890.....	2,594	2,740,354	
1891.....	2,895	3,268,631	
Increase.....	301	527,677	

Principal domestic commodities received and shipped by lake, 1891.

Receipts:	Tons.	Shipments:	To
Coal.....	778,982	Wheat.....	
Lime and cement.....	45,994	Flour.....	
Lime and building stone.....	33,312	Oats and corn.....	
Salt.....	21,741	Lumber.....	
General merchandise.....	270,174	General merchandise.....	726,682
Total.....	1,150,203	Total.....	

Receipts of coal and shipments of flour eastward for nine years.

Year.	Coal.	Flour.	Year.	Coal.	Flour.
1883.....	420,000	91,896	1888.....	1,435,000	171,223
1884.....	372,000	79,801	1889.....	1,045,000	198,053
1885.....	695,000	113,190	1890.....	* 778,982	149,112
1886.....	736,000	133,036	1891.....	778,982	170,774
1887.....	1,041,000	129,627			

* Coal receipts prior to 1890 are given for the head of the lake (Duluth and Superior), since then for Duluth only.

The storage capacity of Duluth elevator system is 21,300,000 bushels, and that of Duluth elevators proper 12,650,000 bushels. The following table gives the receipts and shipments of wheat for twenty-one years.

Year.	Receipts.	Shipments.	Total.	Year.	Receipts.	Shipments.	Total.
Ending Dec. 31--				Ending Aug. 31--			
1870.....	1,144,082	1,058,449	2,202,531	1880.....	40,430	43,610	84,040
1871.....	490,244	427,725	887,969	1879.....	45,722	44,617	90,339
1872.....	513,827	409,410	923,237	1878.....	54,092	54,000	108,092
1873.....	239,802	404,485	644,287	1877.....	13,817	15,117	28,934
1874.....	514,089	585,558	1,099,647	1876.....	43,536	41,292	84,828
1875.....	675,977	530,048	1,206,025	1875.....	34,137	32,436	66,573
1876.....	446,590	421,973	868,563	1874.....	97,224	72,725	139,949
1877.....	411,688	346,537	758,225	1873.....	59,444	47,495	106,939
1878.....				1872.....	27,943	28,531	56,479
1879.....				1871.....	16,703	16,345	33,048
1880.....	141,234	137,607	278,841				
1881.....	97,447	99,264	197,251				
1882.....	38,415	85,906	124,321				

of wheat at the head of the lake (Duluth and Superior com-

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Disbursements, for seven years.

Year.	Total.	Total.	Total.
1865	100	100	970
1866	100	100	1,081
1867	100	100	1,129
1868	100	100	

Comparative statement of arrivals and clearances of vessels at Duluth for fifteen years.

Year.	Arrivals.	Clearances.	Total.
1865	686	520	1,206
1866	1,426	982	2,408
1867	2,157	1,228	3,385
1868	2,150	1,210	3,360
1869	2,250	1,282	3,532
1870	2,280	1,288	3,568
1871	2,444	1,451	3,895

New line of transportation with capital of \$2,000,000, tonnage, 100,000 of the standard type of sailing vessels, Liverpool, England, via St. Louis to New York, via Cape Horn, 1872.

The American Steel Barge Company, of five steamers and twelve barges, the steamers carried a cargo of grain thence to New York and via Cape Horn, Ogdensburg, N. Y.; tonnage, 100,000.

Comparative statement of arrivals and clearances.

Year.	Domestic.	Foreign.	Total.	Value of goods landed and warehoused.	Dredges collected.	Domestic ports.
1865	27,000,000	80,000,000	107,000,000		87,000,000	\$1,207,450
1866	31,000,000	4,200,000	35,200,000		2,000,000	1,455,447
1867	31,000,000	11,000,000	42,000,000	800,000	2,000,000	1,520,211
1868	32,000,000	11,000,000	43,000,000	400,000	4,700,000	2,000,170

Opening and closing of navigation.

Year.	Opening.	Closing.	Year.	Opening.	Closing.
1865	Apr. 27	Nov. 15	1866	Apr. 21	Dec. 4
1866	May 1	Dec. 15	1867	Apr. 16	Dec. 9
1867	May 14	Nov. 15	1868	Apr. 30	Dec. 9
1868	May 11	Nov. 15	1869	Apr. 10

H H 4.

IMPROVEMENT OF HARBOR AT SUPERIOR BAY AND ST. LOUIS BAY, WISCONSIN.

The natural channel connecting Superior Bay with Lake Superior is at the southern extremity of Superior Bay. It was originally obstructed by shifting bars with scarcely 9 feet of water over them, to remedy which the citizens of Superior, previous to 1866, had made some attempts at pier work, which was subsequently taken in hand by the United States and the improvements continued until the piers have reached an aggregate length of 5,650 feet.

When the improvement was commenced 12 feet depth in the channel

was more than sufficient to meet the requirements of the largest vessels. This depth has been increased and maintained for some time at 17 feet, and at no distant day will have to be still further increased to 22 feet. The crib piers which define this channel are in most part badly conditioned for the present depth of 17 feet, they having been intended originally for not over 12 feet. A further increase in the depth of the channel may possibly make it necessary to replace them with better-proportioned structures.

In the meantime the superstructures will have to be extensively repaired. In fact, extensive repairs are urgently needed now, but the \$15,000 held in reserve for this purpose is not safely sufficient to meet the damages liable to result from a single severe storm. It is in reality an emergency fund, and until something more is in sight can not be properly used in making general repairs. It is in the interest of economy as matters stand at present to postpone all general repairs to the latest moment pending the consideration of the subject of reconstruction, but it is hoped that the greater portion of the present substructure cribs can be retained. In any event it will be necessary to maintain a reserve fund for repairs of all kinds until the general repairs are completed. At present this should not be less than \$15,000.

The shore on the Wisconsin side is advancing rapidly, and soon the sand which drifts around the end of the Wisconsin pier will necessitate the extension of this pier. If this extension is to provide for 22 feet depth it will cost \$30,500.

CHANNEL IN SUPERIOR BAY.

The channel in Superior Bay has neither the directness nor width to permit vessels to reach Connor Point safely without the assistance of a tug or local pilot, though its condition has been vastly improved in the past few years. Its present condition is good, but vessels bound for West Superior will not use it so long as the shorter and much easier route through Duluth Canal is in good condition. It is used considerably, however, by rafts, and should not be allowed to deteriorate.

In order that vessels may navigate this channel at all times safely it is essential that it be widened and straightened considerably.

During the next year \$50,000 could be advantageously expended in continuation of this work.

QUEBEC DOCK CHANNEL.

No work was done on this channel during the year.

THE NEMADJI RIVER.

The commerce on the Nemadji River is increasing, owing to the location upon its banks of sawmills and brickkilns which depend upon the water transportation it affords to get their products to market.

The bar at its mouth is a serious obstruction and was dredged during last year, but will probably require redredging each year after the spring floods to maintain a serviceable channel; this will require about \$5,000 annually.

CHANNEL ALONG WISCONSIN DOCK LINE, ST. LOUIS BAY.

The projected channel between Connor Point and Grassy Point along the West Superior dock line will be about 12,500 feet in length; of this

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The following statement shows the manner in which the appropriations have been expended. The amount expended under the different classes of work include the cost of examinations, soundings, superintendence, booying, and contingencies:

Repairs and beach protection.....	\$13, 22
Construction and repairs to piers.....	318, 44
Dredging.....	222, 66
Total.....	554, 34

Money statement.

July 1, 1891, balance unexpended.....	\$71, 67
June 30, 1892, amount expended during fiscal year.....	50, 13
July 1, 1892, balance unexpended.....	21, 54
Amount appropriated by act approved July 13, 1892.....	70, 00
Amount available for fiscal year ending June 30, 1893.....	91, 54

Amount (estimated) required for completion of existing project.....	156, 78
Amount that can be profitably expended in fiscal year ending June 30, 1894.....	275, 50
Submitted in compliance with requirements of sections 2 of river and harbor acts of 1896 and 1897.	

COMMERCIAL STATISTICS.

Arrivals and clearances of vessels at Superior, Wis., for two years.

Year	Arrivals and clearances.	Total
1890.....	1, 256	1, 541
1891.....	1, 610	2, 064
Increase.....	354	523

Average vessel tonnage.

1890.....	1,
1891.....	1,
Increase.....

Of the number of vessels reported above for 1891, 1,453 were steam and 158 sailing vessels.

The storage capacity of the Superior elevators is 8,300,000 bushels.

Shipments and receipts by lake of leading articles during season of 1891.

Shipments:	Tons.	Shipments—Continued.	Tons.
Wheat.....	524, 736	Copper ore.....	15,
Other grain.....	2, 654	Lumber.....	10,
Flour.....	155, 860		
Wool.....	1, 000	Total.....	710,
Receipts:			
Coal.....			877,
Cement.....			2,
Sugar.....			13,
General merchandise.....			12,
Total.....			906,
Total receipts and shipments.....			1, 616,

Comparative statement of receipts and shipments for 8 years.

Year.	Tons.	Year.	Tons.
1884.....	17,462	1888.....	327,327
1885.....	33,626	1889.....	1,006,542
1886.....	117,027	1890.....	1,493,885
1887.....	170,020	1891.....	1,616,648

Comparative statement of arrivals and clearances of vessels for 8 years.

Year.	Vessels.	Tonnage.	Year.	Vessels.	Tonnage.
1884.....	194	119,258	1888.....	812	915,816
1885.....	200	113,519	1889.....	900	1,122,048
1886.....	316	283,787	1890.....	1,256	1,541,777
1887.....	462	404,780	1891.....	1,610	2,964,463

H H 5.

IMPROVEMENT OF MINNESOTA POINT, AT SUPERIOR, WISCONSIN.

The approved project for this improvement consisted in building about 1,000 feet of sand fence near the old light-house on the southern extremity of the point to catch the drifting sand and prevent its blowing over into the Superior Channel, which is very close to the shore of the point in this vicinity, and about 200 feet at a low and narrow part of the point, known as "The Opening," about 2 miles north of the old entrance, to form a bank which would prevent the lake from cutting through into the bay during storms.

The fences were built according to the plans published in House Ex. Doc. No. 51, Fifty-first Congress, first session.

Work was begun by the contractors shortly before the opening of navigation and completed during the season of 1891.

This work is in the collection district of Duluth, Minn. The nearest light-house is situated on the outer end of north pier of the Superior entry.

Money statement.

July 1, 1891, balance unexpended	\$368.41
June 30, 1892, amount expended during fiscal year.....	322.49
July 1, 1892, balance unexpended	45.92

H H 6.

IMPROVEMENT OF HARBOR AT ASHLAND, WISCONSIN.

The southern end of Chequamegon Bay forms the harbor of Ashland, and as the length of the bay is considerable it was necessary to protect the wharves from the waves to enable vessels to use them at all times. The approved project is for the construction of a pile, slab, and rock breakwater 8,000 feet long, dredging a channel of the necessary length along the dock line, and closing the breach in Chequamegon Point with a brush and stone dike.

The breakwater consists of three rows of piling filled in with slabs which are held in place by large rock ballast. The construction seems somewhat light, considering the ice in the bay, but has so far answered

the purpose—, recently, with the great recommendation that it is very cheap, cost—, under last season's contract only \$25.95 per running foot as against about \$140 per foot for the usual form of crib breakwater constructed in about the same depth of water.

In 1889 a length of 4,650 feet was built, of which 50 feet was subsequently carried away. Under last season's contract 1,080 feet were added to it; its total length now 5,680 feet, and the breach in Chequamegon Point was closed by a brush and stone dike 4,200 feet long. In addition to the new work repairs to the old portion were made to the amount of \$3,782. These were rendered necessary by the settlement of the slab filling and the shifting of considerable of the ballast, which was apparently of too small size.

The breakwater is still of insufficient length to give protection to all the wharves of the city, but its influence in diminishing the turbulence of the waters of the harbor is—, apparent. To build the remaining 2,320 feet of the breakwater—, estimated at present contract prices, will cost \$94,000, including—, necessary repairs and contingencies.

A harbor line has been established—, the city, but there is not a uniformity of depth along it. A—, channel 200 feet wide and 17 feet deep along this dock line—, 90 cubic yards of material will have to be excavated. This—, cost—, 3,500, provided no rock is encountered.

In order to give access to the wharves—, for vessels drawing the full depth that the new lock at Sault Ste. Marie—, is expected to give, a channel not less than 200 feet wide, and—, depth of 22 feet, should be dredged along the dock line by the—, the new lock is completed, and to that end work should be begun soon.

This channel will require the removal of 903,497 cubic yards of material, and at 18 cents per yard would cost \$162,629.46; adding 10 per cent for contingencies makes the total estimated cost of this channel \$178,892.40.

The principal shipments from Ashland are iron ore and lumber.

There is an iron furnace and general machine shop in operation and it is presumed that the other enterprises which are expected to locate here will materially increase the shipments of manufactured articles and general merchandise.

There has been expended during the year just closed \$59,400.87 and the total amount expended under approved project to date is \$137,138.67.

RECOMMENDATIONS.

For 2,320 feet extension to the breakwater	\$70,000
For repairs and contingencies	24,000
For dredging	93,500
Total	187,500

This work is in the collection district of Superior, Mich., Marquette, Mich., being the port of entry; nearest light-house, La Pointe, on Chequamegon Point, at entrance of Chequamegon Bay.

Abstract of appropriations for improving harbor at Ashland, Wis.

By act of Congress approved August 5, 1886	\$22,500
By act of Congress of August 11, 1888	60,000
By act of Congress approved September 19, 1890	60,000
Total	142,500

Money statement.

1891, balance unexpended.....	\$64,762.20
1892, amount expended during fiscal year	59,400.87
1893, balance unexpended.....	5,361.33
amount appropriated by act approved July 13, 1892.....	45,000.00
amount available for fiscal year ending June 30, 1893	50,361.33
amount (estimated) required for completion of existing project	142,500.00
amount that can be profitably expended in fiscal year ending June 30, 1894	142,500.00
amount admitted in compliance with requirements of sections 2 of river and harbor acts of 1866 and 1867.	

COMMERCIAL STATISTICS.

Arrivals and clearances of vessels at Ashland, Wis., for two years.

Year.	Number.	Tonnage.
.....	4,489	2,750,000
.....	4,099	No record.
Decrease.....	390

Principal articles of export and import for 1891.

Exports:	Tons.	Imports:	Tons.
Iron ore.....	1,253,493	Coal.....	560,480
Pig iron.....	37,192	Cement.....	2,222
Stone, building.....	938,420	Salt.....	8,254
Lumber.....	404,904	Fire brick, fire clay.....	326
Lath.....	9,750		
Shingles.....	2,500	Total.....	571,282
Total.....	2,646,259	Total receipts and shipments.....	3,217,541

Comparative statement of receipts and shipments for five years.

	Tons.
1887.....	1,401,454
1888.....	1,357,472
1889.....	1,971,213
1890.....	2,816,924
1891.....	3,217,541

Arrivals and clearances of vessels for five years.

1887.....	892
1888.....	2,375
1889.....	3,171
1890.....	4,489
1891.....	4,099

H H 7.

IMPROVEMENT OF HARBOR AT ONTONAGON, MICHIGAN.

In 1887 the Ontonagon River, which forms the harbor, had fairly deep water in it, but its mouth was obstructed by a bar which had but

7 feet depth over it. The project adopted then for the harbor it offe was to build parallel piers projecting into the lake on either side of river's mouth, the expectation being that the current of the river we have sufficient force to maintain a depth between the piers of 12 or more. This expectation has not been fully realized, although piers are nearly of the proposed length, the east pier being 2,315 in length and the west pier 2,675 feet.

It appears that the river during the freshets is heavily charged with sand, and the bar at the entrance forms as fast as the piers are tended. The channel which the river is able to maintain through the bar is shifting and uncertain.

Unless the piers can be speedily extended into deep water, which owing to the gradual slope of the lake bottom and consequent great expense, is hardly practicable, it does not appear advisable to extend the pier work any farther at present. The improvement of the harbor can probably be more economically pursued by dredging a channel through the bar each year.

The portion of the piers composed of piling is very much decayed and needs replacing. This will cost \$15,600. The dredging will amount about \$10,000 annually.

The remaining 150 feet of superstructure on the west pier was completed in August last under the contract in progress at the date of last annual report.

The amount expended during the year was \$9,585.39.

The total cost of the improvement to June 30, 1891, was \$302,700.

As a result of the improvement there was a channel with least depth of 16 feet, but it is likely to shoal again.

This work is in the collection district of Superior, Mich.; nearest port of Marquette, Mich. A light is shown on the outer end of the west pier at Ontonagon, Mich.

Abstract of appropriations for improving harbor at Ontonagon, Mich.

By act of Congress—

Approved March 2, 1867.....	\$
Approved July 7, 1870.....	
Approved June 23, 1874.....	
Approved March 3, 1875.....	
Approved August 14, 1876.....	
Approved June 18, 1878.....	
Approved March 3, 1879.....	
Approved June 14, 1880.....	
Approved March 3, 1881.....	
Passed August 2, 1882.....	
Approved July 5, 1884.....	
Approved August 5, 1886.....	
Passed August 11, 1888.....	
Approved September 19, 1890.....	

Total..... \$

EXPENDITURES.

Amount expended under approved project to date.....	\$306,
Balance available July 1, 1892.....	1,
Estimated amount required annually for preserving and maintaining (dredging).....	10,
Estimated amount required for repairing piers.....	15,
Total.....	25,

APPENDIX H H—REPORT OF CAPTAIN FISK

Money statement.

1891, balance unexpended *
 1892, amount expended during fiscal year
 1892, balance unexpended
 appropriated by act approved July 13, 1892
 available for fiscal year ending June 30, 1893..... 21.
 (estimated) required for completion of existing project...
 that can be profitably expended in fiscal year ending June 30,
 fitted in compliance with requirements of sections 2 of river
 or acts of 1866 and 1867.

COMMERCIAL STATISTICS, ONTONAGON, MICHIGAN.

ing able to obtain the statistics for the years 1889, 1890, or 1891, I
 the calendar year 1888.

Arrivals and clearances of vessels.

Year.	Arrivals.	Clearances.	Tonnage
.....	174	174	222,000
.....	120	120	188,100

Principal articles of export and import.

Articles.	1887.	1888.
.....	<i>Tons.</i> 28,929	<i>Tons.</i> 42,858
.....	119	143
.....	140	12
.....	200	98
.....	29,388	43,111
.....	1,600	1,665
.....	600
.....	504
.....	26
.....	47
.....	500	1,050
.....	26
.....	22
.....	3,277	2,763

H H 8.

IMPROVEMENT OF EAGLE HARBOR, MICHIGAN.

harbor was improved in order to form one of the harbors of ref-
 this shore of Lake Superior. The entrance to the small bay

ance unexpended July 1, 1891, as shown in Annual Report for 1891, is
 the difference, \$52.37, is amount credited to this appropriation on account
 a by Maj. Charles E. L. B. Davis, Corps of Engineers, U. S. Army, as per
 lansen T Auditor's Office, November 12, 1891.

made for the commencement of this extension in the river
bill of August 11, 1888.

Contract under this appropriation was for 180 feet extension of
breakwater, but after getting all the embankment and cribs in place
the latter were carried away by a severe storm, leaving as the
result of the season's work 180 feet of embankment and one crib 60
feet high. Considerable damage was also done to the remainder

of the breakwater as much speculation as to the integrity of the riprap founda-
tion. Examination made through the ice, and since, proves that it
is not injured. From this it appears that where there are no defects
or obstructions reaching to or above the surface of the water
a little force exerted by the waves at 14 feet depth.

Extension of the breakwater is in an exposed place, and it is un-
desirable to attempt to carry on work upon it during the stormy seasons of

the year. There does not, however, appear to be any necessity for altera-
tion of construction materially, for where the cribs and super-
structure are finished the pier is sufficiently strong. As a precaution

in the future building, however, the cribs should be decked over with
timber as soon as filled with rock, as otherwise the rock is liable
to be washed out by the waves and the stability of the crib destroyed.

It can be safely done in those cases like Marquette, where the cribs
rest upon a substantial riprap foundation, as there would be but
little chance for the ballast to escape, and even if some did escape the
structure would not be affected, and therefore the stability of the
structure would be preserved.

Contract in progress at date of last annual report was for 120
feet of work and superstructure on the 120 feet of embankment,

and the cribs were taken by the storm, and 120 feet of entirely
new embankment, cribs, and superstructure, making a total exten-
sion of breakwater of 240 feet. The remaining 700 feet of extension

is much needed, and the estimated cost at last contract prices is
for contingencies, etc., \$9,800; a total of \$107,800. In addition,

it is not less than \$15,000 will be required for repairs to old work
during the period of construction.

Superstructure of the portion of the breakwater commenced in
1875 and finished in 1875 has been extensively repaired, but is never-

theless not as secure as is to be desired. Much of the timber has already
rotted in place several years longer than the usual life of timber in this

climate, and the time is short, if it has not already arrived, when this
work must be replaced by more permanent material.

Plans for a concrete superstructure was prepared, and was ap-
proved on February 27, 1890. This superstructure is estimated to cost

\$107,800, of which sum \$100,000 could be expended judiciously in
the future.

Harbor lines were established by the honorable Secretary of War in
1889, and a reconsideration of the subject occurred upon the

application of the Duluth, South Shore and Atlantic Railway Company.
Upon the action of the honorable Secretary of War upon this new mo-
tion the railroad company extended the ore dock, as prayed for in the

bill. I am not advised as to the final action upon the railroad com-
petition, but no complaints from interested citizens of Mar-
quette from owners or masters of vessels visiting this port have so

far been received regarding these extensions beyond the harbor lines,

1114 REPORT OF THE CHIEF OF ENGINEERS, U. S. ARMY.

From which it is to be inferred that the attachment is rather view as advantageous to all concerned.

CONSTRUCTION FOR FISCAL YEAR.

For year extension.....	\$107,
For annual expenditures on all work.....	100,
General repairs.....	15,
Total.....	222,

Abstract of appropriations for improving harbor at Marquette, Mich.

By act of Congress approved—	
March 2, 1867 (allotment).....	\$85,
April 24, 1868 (allotment).....	25,
July 22, 1869.....	25,
March 2, 1871.....	60,
June 23, 1872.....	50,
March 2, 1873.....	15,
June 23, 1873.....	15,
March 2, 1875.....	15,
August 14, 1875.....	2,
June 24, 1878.....	2,
March 2, 1879.....	1,
June 14, 1880.....	1,
By act of Congress passed August 2, 1882.....	15,
By act of Congress approved—	
July 5, 1883.....	5,
August 3, 1886.....	10,
By act of August 11, 1888.....	25,
By act approved September 22, 1890.....	40,
Total.....	394,

Money statement.

July 1, 1891, balance unexpended.....	\$44,433.
June 30, 1892, amount expended during fiscal year.....	29,788.
July 1, 1892, balance unexpended.....	14,645.
July 1, 1892, outstanding liabilities.....	100.
July 1, 1892, balance available.....	14,545.
Amount appropriated by act approved July 13, 1892.....	80,000.
Amount available for fiscal year ending June 30, 1893.....	94,545.
{ Amount (estimated) required for completion of existing project.....	275,736.
{ Amount that can be profitably expended in fiscal year ending June 30, 1894.....	222,800.
{ Submitted in compliance with requirements of sections 2 of river and harbor acts of 1896 and 1897.....	-

COMMERCIAL STATISTICS, MARQUETTE, MICHIGAN.

Arrivals.....	1,
Cleanances.....	1,
Tonnage.....	2,867,

Principal articles of export and import.

Exports:	Tons.	Imports:	Tons.
Iron ore.....	1,057,830	Coal.....	148,560
Pig iron.....	4,862	Limestone.....	1,057
Lumber.....	40,000	Sandstone.....	2,857
General merchandises.....	3,000	Lumber.....	2,475
Total.....	1,105,692	Brick.....	3,087
		General merchandise.....	20,000
		Total.....	178,036

Vessel tonnage.

Year.	Tonnage.	Average tonnage.
1889.....	3,077,566	1,061
1891.....	2,867,020	931
Decrease.....	210,546	130

There have been no new lines of transportation established.

H H 10.

IMPROVEMENT OF HARBOR OF REFUGE AT GRAND MARAIS, MICHIGAN.

The harbor of Grand Marais, Michigan, was only accessible for vessels drawing less than 9 feet. Once within the bay, there is ample depth to float the largest vessels. As a harbor of refuge it is of pressing importance to the shipping navigating the Lakes in this vicinity, as the many wrecks in this neighborhood bear witness.

The project for the improvement of this harbor was adopted in 1881, and has for its object the creation of a safe entrance into the bay for vessels of the largest size. The entrance channel is to be about 300 feet in width, protected by crib piers on either side. The west pier is now 1,406 feet long, including 100 feet of pile dike, and the east pier is 853 feet long, including 100 feet of pile dike.

A channel 150 feet wide and 17 feet deep was dredged out between the piers in 1889. As the sand was piled up between the piers at the inner ends to the height of the superstructures, it was not supposed that the dredged channel would remain open, but it was believed that while the waves and currents might level the sand between the piers and obliterate the channel no accessions of sand were anticipated. An examination was made in June, 1890, and, compared with the survey made in 1888, showed considerable change. The sand had been leveled and distributed over a rather larger area than was expected, and, although no accession of sand appears to have occurred, the character of the bar and the condition of the shore at the inner ends of the piers would indicate that a large quantity of sand had been washed into the channel and afterwards washed out.

The work of December 22, 1890, consisted mainly the end of the pier with its construction, including between the piers near the end of the east pier. It is now badly damaged to be replaced or better work, avoiding the same expense.

A more substantial form of protection at the inner ends of the pier that originally constructed appeared essential, particularly if the pier is to be used for anchorage.

The contracts were made for work at this place last year. One for building new shore-end on east pier with protecting spurs on the side, reconstructing and strengthening a mass, and repairing the damaged ends. The other was for dredging between the piers. In addition was expended in repairing damage that occurred after the end of the pier.

The new shore-end of the east pier, with its protecting spurs, is to have fully accomplished its purpose of closing the cut around end, and when last visited had gathered a large quantity of sand, partially covering the shore connection.

Although properly the dredging should have been delayed until piers were finished, the pressing need of a channel of some kind for harbor warranted the commencement of this work, and during year a channel 157 feet wide with least depth of 17 feet was dredged through and used last fall by several vessels seeking relief from storms.

During the dredging the crib mentioned as having been left in channel by the storms in December, 1890, was carried out by an gale and deposited to the eastward of the east pier, where it is out of the way.

To complete the work according to the approved project, 40 piers are to be added to the west and 900 feet to the east pier. Completion of the pier continues with reasonable velocity, probably completed in full in 1897 will not be objected, but will probably be delayed until the commercial interests of this port are better than the present in the harbor of this essential.

The commercial interests in connection with this harbor, which is made a shipping port in the event of there being a sufficient of water in the harbor to accommodate the vessels engaged in business, and I think that it would not be long after obtaining a good channel into it before the harbor would claim other distinction than that of a harbor of refuge.

Original estimate see Report of Chief of Engineers, 1881, page 256) \$1
Appropriated 2

This work is in the collection district of Superior, Mich. Nearest light-house, Mich.

Abstract of appropriations for improving harbors and refuges at Grand Marais, I.

B. Act of Congress.	
Approved June 14, 1890	3
Approved March 3, 1891	
Passed August 2, 1892	
Approved July 5, 1894	
Approved August 5, 1895	
Of August 11, 1898	
Approved September 19, 1899	
Total	2

Money statement.

1891, balance unexpended	\$50,389.20
Do, 1892, amount expended during fiscal year	45,727.26
<hr/>	
1892, balance unexpended	4,641.94
1892, outstanding liabilities	19.20
<hr/>	
1892, balance available	4,622.74
Not appropriated by act approved July 13, 1892	30,000.00
<hr/>	
Not available for fiscal year ending June 30, 1893	34,622.74
<hr/>	
Amount (estimated) required for completion of existing project	188,750.00
Amount that can be profitably expended in fiscal year ending June 30, 1894	100,000.00
mitted in compliance with requirements of sections 2 of river and harbor acts of 1866 and 1867.	

COMMERCIAL STATISTICS.

Arrivals and clearances of vessels at Grand Marais, Mich.

Vessels.	Number.	Tonnage.
Steamers	116	12,000
Sailing vessels	30	8,000
Total	146	20,000

Principal articles of export and import.

orts:	Tons.	Imports—Continued.	Tons.
Fish	300	Coal	150
Shingles	32	General merchandise	100
orts:		Miscellaneous	150
Lumber	20		
Hay and grain	600	Total	1,356
Iron	4		

Receipts and shipments for five years.

.....	Tons.
.....	1,910
.....	6,270
.....	8,686
.....	9,405
.....	1,356

he balance unexpended July 1, 1891, as shown in Annual Report for 1891, is \$11.15. The difference, \$28.05, is amount credited to this appropriation on account of fuel drawn by Maj. Charles E. L. B. Davis, Corps of Engineers, U. S. Army, in transfer settlement from Third Auditor's Office, November 12, 1891.

ready these interests are being established along the river to Fond du Lac, and it is probable that long before any considerable progress can be made towards improving the channels of approach by water the necessity for them will be pressing, in my opinion it is not too early to make the surveys required for the preparation of a general project for improving St. Louis Bay, and examining the river upon a scale adequate to meet the prospective demands of commerce.

I believe that St. Louis Bay, from Grassy Point, in Minnesota, to Fond du Lac, is worthy of improvement by the General Government, and I recommend that the within project be examined and approved for its survey.

O. M. POE,
Colonel, Corps of Engineers,
Engineer, Northwest Division.

THE ST. LOUIS LAND IMPROVEMENT COMPANY OF DULUTH, MINNESOTA.

DULUTH, MINN., September 25, 1890.

SIR: In the matter of a preliminary survey of the St. Louis River from Grassy Point to Fond du Lac, and the demands of commerce for the immediate improvement of the same, it may be of assistance to you to know something of the interests and purposes of the land companies who are now engaged in developing properties on said river.

The St. Louis Land Company, which I represent, owns 2,600 acres on the Wisconsin side, comprising sections 14, 15, 22, and 23, and lots in front of said sections on the river in township 48 north, range 15 west, Douglas County, about 2½ miles from the St. Louis water power. Our capital stock is \$1,000,000, \$600,000 of which is paid in these lands. The balance of the stock is to be sold and the cash obtained therefor used for the developing of the same. We have already sold 250,000 shares of the balance ready for sale when we need the money. The cash received from the sale of this stock and also from the sale of lots, so far as may be necessary, will be used for securing manufacturing plants, railroads, improving our water front, and general developing of our town site. The Duluth and Southern Railroad, a new line, will have its terminals, car shops, etc., upon our town site, and will run in a westerly direction, intersecting the Northern Pacific, the Great Northern, the Soo, and a number of other roads in its course to Minneapolis and St. Paul, Red Wing, and some miles further south.

Our engineers have already commenced the survey of this road, and over 17 miles are already under contract to be built within this year. The Northern Pacific is also developing a route to reach the river at this point. From this point to St. Paul will be about 20 miles shorter than any other road now connecting St. Paul with the west of the lakes. The effect of congregating these different systems of roads at this point must of necessity establish a large system of coal docks, grain elevators, saw mills, etc.

We have already secured a flouring mill with a capacity of from 6,000 to 7,000 bushels a day, that will commence putting its foundations in within the next ten or twelve days. Also some six or seven manufacturing plants, each of which is a heavy iron producer. We purpose continuing to induce manufacturing plants to establish themselves upon our town-site, and expect to establish here a large manufacturing town in a very brief time. The chief reason for locating manufactories at this point is on account of its nearness to the falls of the St. Louis River, with its almost constant supply of power. A company has already been formed, with a capital of \$1,000,000, to purchase this water power of Jay Cook and his associates and immediately commence developing the same by a series of dams that will produce at least 100 horse power. This power will be used to generate electricity, which will be transmitted to the manufacturing plants, which are located sufficiently near to render it feasible. The effect of the development of this water power must necessarily be the building of large manufacturing industries along the line of the navigable channels of the river, all of which must of necessity be very much handicapped if they can obtain the benefit of lake navigation.

For more detailed statement of our plans will be of assistance to you I shall be glad to render it at your suggestion.

Very truly,

General Manager, St. Louis Land and Improvement Company.

A. C. OTIS,

J. JAMES B. QUINN.

MEMORANDUM TO THE CHIEF OF ENGINEERS, U. S. ARMY.

LETTER OF NEW DULUTH LAND COMPANY.

DULUTH, MINN., September 29, 1890.

We desire to call your attention to the necessity of dredging and improving the St. Louis Bay and St. Louis River from Grassy Point to Fond du Lac.

The New Duluth Land Company have secured and are now improving some 2,000 acres in sections 2, 3, 4, 9, 10, and 11, in township 48 north, range 15 west, Douglas County, Minn.

They already secured industries, both wood and iron working, to employ 1,200 men, and are now negotiating with other industries to employ as many more. The prospect of locating them during the coming winter.

Industries already located will be two large sawmills, which will need transportation for their output; large iron, brass, and wood working establishments will need large quantities of coal to be delivered by water. The St. Louis Power Company have contracted, as soon as the falls are improved, to supply power for still larger industries, and there is every reason to believe that they will be improved so as to furnish this power within the next twelve months.

Fond du Lac are large heavy-stone quarries, shipping large quantities of stone, and are obliged to barge down the river.

The Duluth, Northern, and St. Paul and Duluth, and Duluth Short Line railroads pass through our property and have partial terminals at New Duluth, out of the city.

The Duluth and Southern, a new road projected from Duluth to Red Wing, all roads between St. Paul and Duluth, is already under contract and will be built first 100 miles this winter, and expect to bridge the St. Louis Bay on St. Louis and New Duluth and have their terminals at New Duluth.

The Duluth and Winnipeg Road will also have their terminals in part at New Duluth. Steam and electric, the Soo Road, and Superior Terminal Company will run from New Duluth over the bridge of the Duluth and Southern, and will make New Duluth their point of transfer.

We are negotiating now with the Duluth and Northern and expect to secure the third road at New Duluth. Between Duluth and New Duluth, the St. Louis and Company and the Ironston Land Company in Minnesota, and St. Louis Bay in Wisconsin, have already secured large tracts of land and large numbers of men, all wanting facilities for water shipment.

The New Duluth Land Company we respectfully request that in making report to the War Department you recommend an appropriation for dredging St. Louis Bay and River for such an amount as you deem advisable.

Very respectfully, yours,

NEW DULUTH LAND COMPANY,
By FRANK E. WESSLER,
President of the New Duluth Land Company.

M. J. QUINN,
Chief of Engineers.

LETTER OF NEW DULUTH LAND COMPANY AND ST. LOUIS BAY LAND COMPANY.

DULUTH, MINN., September 29, 1890.

We desire to call your attention to the necessity of dredging and improving the St. Louis Bay and St. Louis River from Grassy Point to Fond du Lac.

Within the last six months four important land companies have organized and are making large tract improvements on the St. Louis Bay and River, and all of said companies have secured one or more important industries to be built in the near future. The St. Louis Bay Land Company owns land in sections 14 and 20, and the Ironston Land Company land in section 23, township 49, range 15, St. Louis County, Minn., and has secured the Ironston Steel Works, a manufacturing establishment which will supply sheet iron, structural iron, etc., and employ in the neighborhood of 1,000 men.

They will build a dock and expect to receive coal and other freight from the East, and part of their manufactured products by boat to Milwaukee, Detroit, and other cities on the chain of lakes. The other land companies referred to are the New Duluth Land Company, which has 2,000 acres of land in sections 2, 3, 4, 9, 10, and 11, township 48, range 15, St. Louis County, Minn., and the St. Louis Land Company, which owns land in sections 14 and 15, 22, 23, 48-49 in Douglas County, Wis., both of which have already secured industries which will want facilities for receiving coal and freight from the East and shipping part of the products East also by boat.

on behalf of the St. Louis Bay Land Company and the Ironton Land Company we respectfully request that in making your report to the War Department you recommend an appropriation for dredging and improving said St. Louis Bay and River of an amount as you may deem necessary and advisable.

Most respectfully,

IRONTON LAND COMPANY,
By E. L. EMERY,
Manager,
ST. LOUIS BAY LAND COMPANY,
By C. E. LOVETT & CO.,
Agents.

Maj. JAS. B. QUINN,
Corps of Engineers.

SURVEY OF ST. LOUIS RIVER, MINNESOTA AND WISCONSIN, FROM GRASSY POINT, IN ST. LOUIS BAY, TO FOND DU LAC.

UNITED STATES ENGINEER OFFICE,
Duluth, Minn., November 10, 1891.

GENERAL: I have the honor to submit the following report on the survey of "St. Louis River from Grassy Point, in St. Louis Bay, to Fond du Lac, or the State line between Minnesota and Wisconsin."

The preliminary examination was made by Maj. James B. Quinn, Corps of Engineers, who reported the river worthy of improvement and recommended an allotment of \$3,400 to make a complete survey of the portion under consideration. The amount allowed, however, was but \$600, and with this it has only been possible to check up a survey made in 1884-'85, as described in the report of Assistant Engineer J. H. Darling, herewith, to which reference is made for details.

The portion of the river susceptible of improvement terminates about 1 mile above Fond du Lac, the village being $14\frac{1}{2}$ miles by water above the Grassy Point Railroad bridge, and $20\frac{1}{2}$ miles from the Duluth Canal. About $4\frac{1}{2}$ miles above Fond du Lac is the foot of the Dalles, which extend about 4 miles up the river, and in which the fall is 480 feet.

From the natural pool at the boom just below Fond du Lac down to Grassy Point there is practically slackwater, and as the river carries little or no sediment any improvements made would be practically permanent. As will be seen from the chart,* this part of the river consists of a succession of long, deep pools (usually with 18 feet or more of water in them) separated by comparatively short bars, with minimum depths of $7\frac{1}{2}$ to 12 feet over them, so the amount of excavation necessary to make a channel the entire distance 100 feet wide at bottom and 16 feet deep, including 8 per cent increase in volume for scow measurement, is but 410,265 cubic yards. The character of the material to be excavated could not be accurately determined, as the small sum available for the survey would not permit of making borings, but from the best available information it is believed to be sand covered with mud in the lower part, running into clay covered with mud in the upper part of the portion of the river considered. The disposal of the dredgings would be somewhat difficult, as it would probably be necessary to tow to the lake for a dumping ground or rehandle it. However, as dredging has been done this year in St. Louis Bay for 14 cents, probably 25 cents per cubic yard would be a fair estimate, and at this figure the cost of the work would be as follows:

410,265 cubic yards, at 25 cents.....	\$102,566.25
Contingencies, etc., 10 per cent	10,256.63
Total	112,822.88

* Not printed.

It is believed that improvement should be undertaken, as there are already very considerable manufacturing interests along this part of the river, and the banks will be the natural location for the new manufacturing establishments, making the head of the lakes.

The manufacturing interests already located on the river above Fond du Lac represent actual investment in buildings and plant of over \$1,000,000, and exclusive of the sawmills have a capacity sufficient to employ, in nearly or quite 2,000 men.

Among these interests are a large and very complete blast furnace and iron works, a large establishment for the manufacturing of railroad iron and material, wood furniture factories, saw and flour mills, iron works, and other factories, hardware manufacturing, brick works, and other industries.

The development of the water power on the river by a company controlled by the State at the Fall in the Dulles has been begun, and when completed will furnish over 60,000 horse power on the minimum discharge of a 1,000 cubic feet per second. Of the ten dams contemplated, the first, near the head of the Dulles, was built last spring, gives an available head of about 40 feet, and furnishes power to a large establishment for making a fine quality of pressed brick by patented process, and the coal which is found near it in unlimited quantities; Dam No. 10, near the lower end of the Dulles is under contract. The ultimate use seems to be to distribute the power in the form of electricity, and it is very probable that the interests using the water would be located on the river below Fond du Lac; this is, of course, merely speculative as yet, but the considerable interests already located there seem to me to deserve an improvement which can be made at a comparatively small cost.

I am, Sir, very respectfully, your obedient servant,
W. L. FISK,
Captain, Corps of Engineers.

Very truly yours,
A. S. L. CASSY,
Major, Corps of Engineers, U. S. A.
Division Engineer, St. Louis River, Corps of Engineers, Division Engineer,
Northwest Division.

First interview.
U. S. ENGINEER OFFICE,
Detroit, Mich., November 19, 1891.

My dear Sir, I have the honor to acknowledge the receipt of the Chief of Engineers. I con- sider the same as a matter of course in this respect.
O. M. POE,
Captain, Corps of Engineers, etc.,
Division Engineer, Northwest Division.

Very truly yours,
ASSISTANT ENGINEER.

U. S. ENGINEER OFFICE,
Detroit, Mich., August 31, 1891.

I have the honor to acknowledge the receipt of the survey of the St. Louis River and to inform you that the same was under your direction and in accordance with the instructions of the Chief of Engineers, U. S. Army, and for which the sum of \$300 was appropriated.

Survey was made in April and May, 1891. The character of the survey and the results accomplished are as follows:

SOUNDINGS.

covered the main channel of the river beginning at a point (marked A on map) near Grassy Point and proceeding up to three-fourths mile above the village of Du Lac, this being a little above the foot of the rapids, and as far up as it was possible to use a sounding boat.

Length of this main river channel is 13 miles. Soundings were also made in the secondary channels, also navigable, and separated from the main channel by islands. Soundings are plotted as taken, correcting only for error of lead and no reduction made to any particular plane or stage.

Gauge was set at New Duluth on April 7, with its zero at surface of the water, the datum being 5.169 feet below the bench mark. This gauge was established by the St. Louis Railway Company and St. Louis and Superior Railway Company, and is 503.83 feet above their datum plane. This plane has not been referred to Lake Superior nor to any known elevation as far as can learn. The gauge readings may be of use in comparing subsequent surges with this one.

Following are the dates on which soundings were made, and the readings of the gauge at New Duluth and at Duluth:

[+ is above zero; — is below zero.]

Location of soundings.	Date.	New Duluth gauge.	Duluth gauge.	Remarks.			
near Grassy Point, to B, near New Duluth. Soundings taken through the ice.	1891.	<i>Feet.</i>	<i>Feet.</i>				
	April	1	-0.5			
		2	-0.5			
		3	-0.5			
		4	-0.4			
		6	-0.5			
		7	0.9	-0.4			
		8	-0.3			
		9	+0.7	-0.4	Northeast gale on 9th.		
		11	+0.4	-0.6			
		15	+0.2	-0.4			
		16	+0.4	-0.3			
		+0.2*	-0.4		
		near New Duluth, to the rapids above Du Lac. Soundings taken from a boat.	April	25	+0.8	-0.3	
				28	+0.8	-0.2	
				29	+0.8	-0.1	
30	+0.7			-0.3			
May	1		+0.5	-0.3			
	2		+0.6	-0.2			
.....		+0.7	-0.2			

* This mean is taken on the assumption that the stage for April 1, 2, 3, 4, 6, and 8, on which readings were made, was somewhere between 0.9 and +0.17, which I consider likely from what I observed over and weather. See also a memorandum on next page.

Additional gauge readings may be found in the field-notes sounding book.

Duluth gauge is in Duluth Harbor, near the canal, its zero at the plane of which is a low-water stage of the lake and harbor, but not extreme low water. The mean of 1873 is 601.2 feet above sea level, and the mean level of the lake is 600 feet above sea level.

At the time of setting the new Duluth gauge, April 7, 1891, I made this memorandum:

"Mr. Cooper, at New Duluth, who seems to be a builder or contractor and is now engaged in a sawmill, tells me that the river has not risen any yet this spring, excepting a recent heavy northeaster when the water backed up and rose 1 foot, and then went down again; that the water is considered to be now at about the lowest it ever gets, and that it has been known to reach a point 7 feet higher than the present stage."

"There has been no rain yet to speak of; the snow is thawing gradually for the last one or two days quite rapidly, and considerable snow now remains. Not a drop has fallen through the canal."

Mr. Parkinson's angles as I remeasured show a tolerable agreement. These were measured not only for testing them, but in order to confirm the identity of the found.

In computing my additions to Parkinson's triangulation I found it necessary to compute his triangles from the line $\Delta 4$ — $\Delta 5$ to line $\Delta 20$ — $\Delta 23$ (this being a system on the river), taking the angles out from his field notes on account of Parkinson's computation being lost. I also computed the azimuths and bearings of his stations and mine. These computations are in Triangulation, an appendix to this book.

The triangulation stations platted on the map of this survey (excepting $\Delta 4$ and $\Delta 1$ also $\Delta 3a$, which falls outside of sheet 1, are marked by iron gas pipe, one in number. Each pipe is $1\frac{1}{2}$ inches in external diameter, 3 feet long, with a point at lower end, and driven vertically into the ground, so as to leave $4\frac{1}{2}$ inches projecting above the surface. Further description of the stations is in the field notes of this survey.

The work of measuring the angles, marking the stations with iron monuments, marking the triangles, azimuths, and coördinates, and plotting the stations was done by

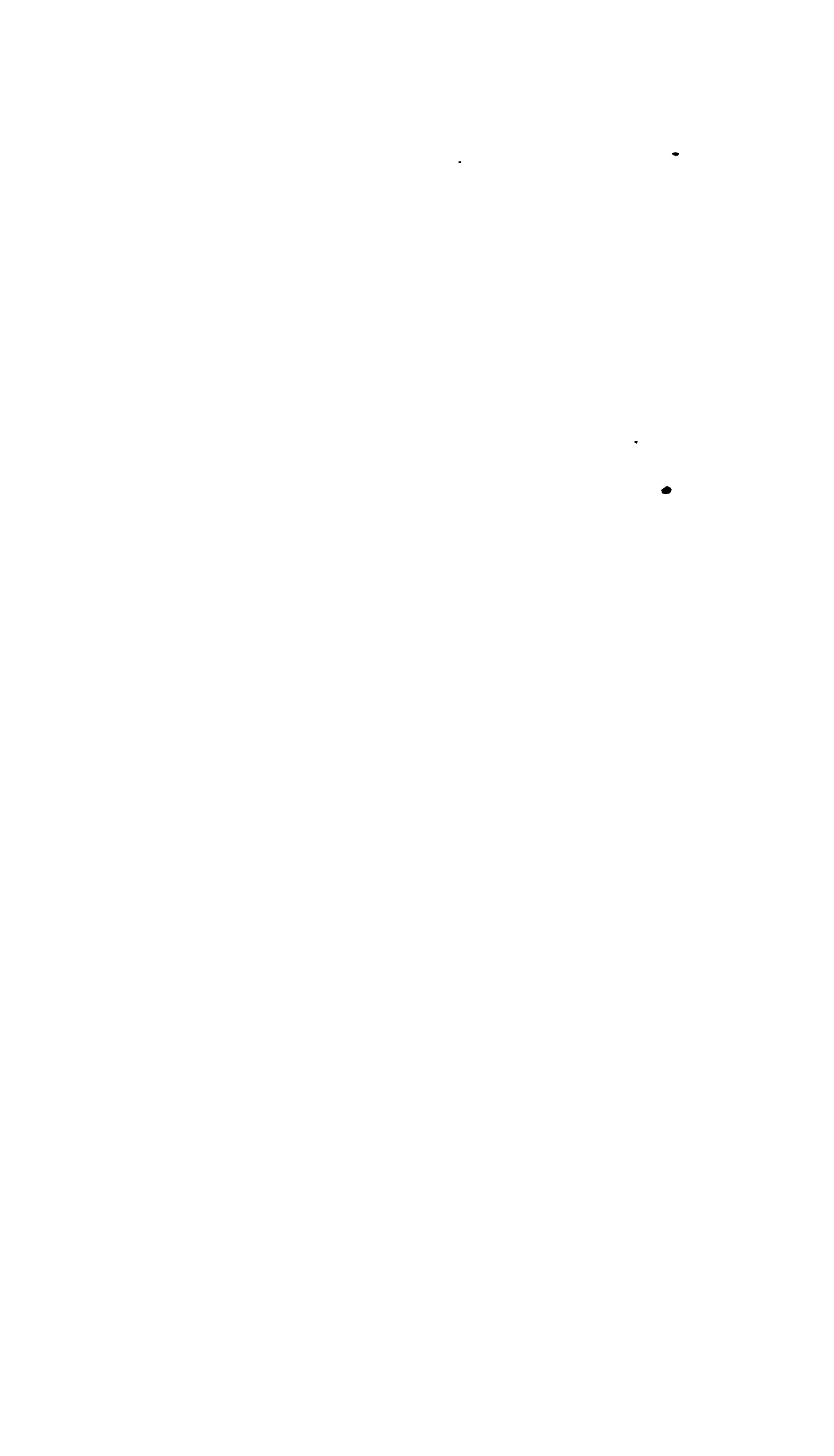
TOPOGRAPHY.

As shown in a note on sheet 1, the results of this survey are mapped with black ink on certain portions of the map which it was impossible to cover by this survey. The original maps, have been copied in sienna color, from the last previous survey, made by Parkinson in 1884-'85, already referred to.

The method of mapping has been as follows: The results of the present survey are mapped on mounted drawing paper, which map is to be retained at this time in its incomplete state, and additions made to it later if desired. A copy of the map was made with black ink on tracing linen, sheets 1 and 2 (these forming a book designed for transmittal to Washington). These sheets were then placed over Parkinson's map of 1884-'85, adjusted into position by means of the triangulation stations common to the two maps, and the copy made in sienna color of the original map desired to be added. In joining common portions of shore line and lines of the two surveys, discrepancies were found, as might be expected; some very small, usually not large, but occasionally large, especially in the case of swamps, which rise but little above the water. Here a difference in the position of the river for the two surveys would give different positions for the shore line. In the survey of 1884-'85 was made in the winter, and during at least a portion of that survey the snow was deep, as I know, and this would make it hard to tell the position of some portions of the swamp line, and thus cause error. Wherever a discrepancy was found in copying from Parkinson's map the line of his map was bent aside for several hundred feet, usually back to the point of meeting, so as to join on the line of the present survey, the latter being changed in any case. The present survey as a rule located the shore line by stadia, and of all channels in which soundings were taken, the portion below New Duluth was located mainly by cross-sectioning the channels at time of making the soundings. Other topography by stadia, angles, and taping. Some points were located by computed triangles.

The survey was located instrumentally section corners and subdivision corners, and in number. These have a small circle drawn around them on the map. Some of these were found marked by stone monuments, a few of them by wooden monuments, and one by an iron monument. The permanent monuments established and located at stations and at section corners will be valuable hereafter in laying out the channels or in establishing harbor lines; in making public and private improvements that may be made and adding same to the map, or in making detached surveys of localities on the river; and in enabling the surveyors at any time to be compared with this and with Parkinson's surveys of West Duluth, New Duluth, Ironton, and St. Louis have sprung up since Parkinson's survey. These, and that of Fond du Lac, have been mapped instrumentally one or more street or block corners and the direction of the streets, and the filling out the remaining streets (as far as deemed best) from plats or other maps and plats considered authentic. Much of the outer portions of West Duluth and of the plat of St. Louis, and to a less extent of the plats on and Fond du Lac have been omitted for want of room or other good reasons.

For filling out the streets of West Duluth Roe's Atlas was made use of, showing the car works, blast furnace, and woolen mills. With this except all the buildings and docks shown on the map were located instrumentally. The work of locating shore line and other topography was by Assistant Dever.



APPENDIX I.

IMPROVEMENT OF PORTAGE LAKE AND LAKE SUPERIOR CANALS, ACROSS KEWEENAW POINT, MICHIGAN, OF HARBORS ON WESTERN SHORE OF LAKE MICHIGAN NORTH OF CHICAGO, ILLINOIS, AND OF FOX AND MENOMINEE RIVERS, WISCONSIN.

REPORT OF MAJOR JAMES F. GREGORY, CORPS OF ENGINEERS, OFFICER
IN CHARGE, FOR THE FISCAL YEAR ENDING JUNE 30, 1892,
WITH OTHER DOCUMENTS RELATING TO THE WORKS.

IMPROVEMENTS.

Portage Lake and Lake Superior canals, across Keweenaw Point, Michigan.	11. Keweenaw Harbor, Wisconsin.
Manistique Harbor, Michigan.	12. Two Rivers Harbor, Wisconsin.
Sedar River Harbor, Michigan.	13. Manitowoc Harbor, Wisconsin.
Menominee Harbor, Michigan and Wisconsin.	14. Sheboygan Harbor, Wisconsin.
Menominee River, Michigan and Wisconsin.	15. Port Washington Harbor, Wisconsin.
Deonto Harbor, Wisconsin.	16. Harbor of refuge at Milwaukee Bay, Wisconsin.
Pensaukee Harbor, Wisconsin.	17. Milwaukee Harbor, Wisconsin.
Green Bay Harbor, Wisconsin.	18. Racine Harbor, Wisconsin.
Harbor of refuge at entrance of Sturgeon Bay Canal, Wisconsin.	19. Kenosha Harbor, Wisconsin.
Shnapee Harbor, Wisconsin.	20. Waukegan Harbor, Illinois.
	21. Fox River, Wisconsin.
	22. Operating and care of locks and dams on Fox River, Wisconsin.

UNITED STATES, ENGINEER OFFICE,
Milwaukee, Wis., July 8, 1892.

SIR: I have the honor to transmit herewith annual report for works of river and harbor improvement in my charge for the fiscal year ending June 30, 1892.

Very respectfully, your obedient servant,

JAMES F. GREGORY,
Major of Engineers.

Brig. Gen. THOMAS L. CASEY,
Chief of Engineers, U. S. A.

about 2 miles from Lake Superior, on the northwest side of Keweenaw Point, receiving from the east the overflow of Torch Lake. The depth of both lakes is ample for all purposes of commerce, present and future. They drain, by Portage River into Keweenaw Bay, the great reentrant of the promontory and the southern shore of Lake Superior. The original route could be carried through Portage River into the lake was from 3 to 5 feet. Its depth on many interior rivers large expenditures are annually made by the Government.

Duluth and Houghton are the chief shipping ports of the copper industry. They are situated on Torch Lake, about 10 miles from Lake Superior. When the demand of the mines had reached such a point as to render the demand for facilities of communication imperative, the country was upon the eve of civil war was inaugurated and has been completed without assistance from the Government.

The idea of cutting a ship canal through the ridge of sand and harican separating the end of Portage Lake from Lake Superior was also formed during the same time and was incorporated for the purpose. The present canal has received various changes in organization, which will be considered more in detail hereafter. The General Government has aided this part of the work by appropriating money from the public lands. No other assistance was received from the General Government or from the State of Michigan.

The improvements, although owned by separate corporations, are now controlled by the same individuals.

Importance of Portage Lake Route.—Before going into details respecting the present condition, and present value of this waterway, its national importance will be considered from two points of view: That of interstate commerce and that of copper production.

Commerce.—The records of the Sault Ste. Marie Canal afford precise data to judge of the extent and nature of the commerce of Lake Superior. The following figures exhibit the freight which has passed through the canal during the following years:

Years.	Registered tonnage.	Actual freight.
.....	2,062,521	1,567,741
.....	2,468,083	2,020,521
.....	2,042,259	2,267,105
.....	2,997,837	2,874,537
.....	3,035,937	3,256,628

* * * * *

M. Poe, Corps of Engineers, in his reports for intervening years, the following statistics of registered tonnage and actual freight through the St. Marys Falls Canal from 1886 to 1890, inclusive:

Years.	Registered tonnage.	Actual freight.
.....	3,529,184	3,701,014
.....	4,388,691	4,882,802
.....	4,741,976	5,581,169
.....	7,899,604	8,288,580
.....	8,454,435	9,041,213

at a few ports on the northern shore of Lake Superior at which our vessels are unable to trade. They naturally hug the southern shore, on which are situated the centers of the iron, copper, and lumber industries; where are imported supplies of coal, lime, building materials, machinery, provisions, and sundry other kind, and where are exported iron, copper, and lumber. At Duluth and Superior the exports of grain and imports of every kind needed to develop the region opened by the Northern Pacific and other railroads swell the traffic enormously and increasing figures given above. Keweenaw Point, as already mentioned, is so far out into the lake and obstructs this natural route of trade.

The prevailing storms of the Lake Superior region are from the northwest. The natural route of commerce near the southern side lies, throughout its whole extent along a lee shore. During the summer months

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The act of Congress

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...on the west, a direct
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been brought to bear upon the company to reduce the tolls were reduced by nearly 50 per cent below what they were since 1863. The demand now is that the tolls shall be reduced so that the waterway shall be further enlarged. None of the shareholders have any interest in the corporation, and we are informed that the canal is now owned entirely by persons nonresidents in the district.

Portage Canal, Railway, and Iron Company.—The opening of Portage Canal for the improvement of Portage River, to vessels navigating Lake Superior, was to go within a trifle over 2 miles of Lake Superior on the west side of Keweenaw Point. The advantage which would accrue from making the canal from the east side to the west side of Keweenaw Point was involved the construction of an artificial canal 2 miles long, with its entrance from Lake Superior. The expense of such a work has caused the persons immediately interested in the local trade of the benefits to result from it would be national rather than local, and it is to be expected from the tolls would probably not pay interest upon the investment in many years.

The Portage Lake and Lake Superior Ship Canal Company was organized under the State laws of 1864, its articles of agreement being filed with the Secretary of State of Michigan July 15, 1864. By a joint resolution of the Michigan Legislature dated January 21, 1865, Congress was asked to aid the enterprise with 200,000 acres of land. The Minnesota legislature passed a resolution to the same effect on February 24, 1865. Congress responded by granting to the State of Michigan, by act approved March 3, 1865, 200,000 acres of land. By act approved March 3, 1865, the lands were conferred by the State upon the above-mentioned corporation on certain conditions. A plan of construction was then adopted, which was more liberal than had at first been contemplated. Additional aid was sought from the legislatures of Michigan, New York and New York passed resolutions February 27, 1866, asking Congress to aid the enterprise. A number of cities also sent in petitions to the same effect. Congress responded by granting 200,000 additional acres of land by act approved July 3, 1868.

The canal was to be 13 feet deep and 100 feet wide and to be provided with a breakwater at Lake Superior end. For several years operations were limited to surveying, procuring machinery, and the exploration of lands to be selected for grant. The original surveys and plans were made by Mr. I. N. Greene, State Engineer for the State. It was not until 1868 that the work of excavation was actually begun, and at the end of that year only a few thousand yards had been excavated. In the spring of 1869 the work was resumed with great activity, and was pushed during that and the following year. Mr. John H. Forster succeeded Mr. Greene as engineer for the State June 1, 1869. In 1871 the work was embarrassed for want of funds, and made unsatisfactory progress. The title of the canal was transferred to Lake Superior Ship Canal Railroad and Iron Company. In this company failed, and a receiver was appointed by the United States circuit court for Michigan with authority to borrow money for the purpose of completing the work advanced slowly during the two following years.

The act of Congress making the first grant of land required that the works should be completed by March 3, 1867. The act making the second grant extended this time to March 3, 1870. The time was subsequently extended to March 3, 1873, and again to March 3, 1873, and finally to December 1, 1873. On the 18th of November, 1873, the State engineer, Mr. Forster, certified that the works were conforming to law, and on the 29th of November, 1873, the governor of Michigan certified that the works were built according to law, but that the title to the lands which they were situated being vested in the officers of the company as individuals and not in the company itself, he did not consider the works completed in accordance with the act of Congress. It was not until June 25, 1875, that the Governor issued his final certificate that the works were completed, the delay being due to the transfer of legal title required by him. Technically, therefore, the works were not completed in 1875, but as a matter of fact they were completed in 1875.

In 1877, after protracted litigation, the property was sold under decree of the court. It was purchased by Messrs. Mann and Wilson, for the bondholders of the old company, who organized the present company, under the name of Lake Superior Ship Canal, Railroad, and Iron Company. The consideration for the deed to Mann and Wilson was \$870,000. In the deed from them to the present company the consideration was \$100, subject to prior incumbrances amounting to \$6,365 and accrued interest at 10 per cent. The present company had no connection with the old company.

The title to the lands attaching thereto, has not proved a financial success. The income from the tolls has been almost wholly expended in keeping the canal open.

It appears from the foregoing facts that part
by Keweenaw Point and partly to secure a need
of a ship-canal to perfect the Portage Lake rou
public interests of the lakes area, if private ent
not already begun the improvement. * * *

ENGINE OF THE PORTAGE

(2) *Portage River improvement.*—By the year
Portage Lake had acquired an importance
imperative a necessity. At the month
from less than 5 feet over the bar. Vessels arriv
weight was transferred by lighters to dock
ground covers, upon which it was towed to
out of \$4 per ton. The expense, delay, and
of procedure led those who suffered there
river and bar and constructing a canal
subscriptions, the contributors being the C
St. Mary's, Albany and Boston, and Lake
and land owners, Moses Sheldon and E
is highest and a contract was made with
14 feet deep and 30 feet wide at bottom.
The expense selected a point farther to
a canal leading to it through a tamarack
The outlet of the breakwater was to
was filling up the mouth of the canal
was built during the winter of 1868-69.
then used and the stone backed from
running from 20° to 30° below zero.
winter was piled through the can al
the contract for the improvement was
This work was done without the profit

In the following year an act was pass
the location of a canal to construct a
canal was approved and James W. M
and River Improvement Company was
who had begun the improvement. The
and drawing were then 5 feet, the main

The improvement then he made pro
responsible were the great amounts
expended annually in maintaining and
and placing locks and booms. As th
was were called for in the use of the

to dangers encountered at

of the route have been made
No outlay has been made
the cuts have been purchased
title to them, as set forth in

soundings indicate a depth of
at water surface is 100 feet,
maintaining 100 feet nearly
(lake) traverses a swampy region,
enlargement. The northern half
old tamarack swamp and a range
underlapping near Lake Superior.

ment consists of a front row of
center, capped a few inches above
piling; a row of anchor piles is
front row by timber braces. The
piles are often decayed and the
position. The superintendent
where the route traverses sand
double sheet piling should be
checked the piles are pressed out
canal in storms tear off the

to the canal is formed by two
which have resisted the storm
completed in 1873. The hardpan
been of marked advantage in
feet long, are of timber and
at the shore line, widening
depth at the date of our survey
ice during the winter is said
shore line, which tends to cause
ing of sand around the ends of

on to the canal, safe for large vessels during
Superior in the spring and autumn, is a diffi-
rise 6 feet above the surface of the lake when
ing a moderate gale from the westward on
with their solid crests fully up to the top of
presented surfaces normal to the direction of
into the air and solid water poured over the
oscillation extended into the canal quite
are informed that in the great northeast and
more directly in the line of the canal, wave

at this place it is a matter of vital importance
arms. A report upon the subject by Maj. H. M.
found in Senate Ex. Doc. No. 32, Forty-sixth

act of September 19, 1890, provided:
"Preserving and continuing the use and navi-
\$10,000 for each of the present and the
1."

project; but in the pending river and
or improving the water communication
Superior, from Keweenaw Bay to Lake
gan, for a navigable depth of 16 feet
at the bottom and for repairs to exist-

cks. The present works consist of
ces on Lake Superior and Keweenaw
dredged cuts and channel-ways;
navigation, as hereafter described.

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each side of the channel in about 12 feet of water. As these dikes were of great assistance to vessels in making the channels at points, they were left in place until the close of navigation. In May, 1892, soundings were again taken at the outer end of the piers and outside for a distance of 300 feet, where there was only from 12 feet of water on line with the center of the canal to 18 feet at the close of navigation last season.

Under agreement with Mr. James Pryor, of Houghton, Mich., dredging was begun May 16 and finished June 1, 1892, 6,384 cubic yards of material being removed, at a cost of 25 cents per yard.

This opened a channel about 60 feet wide and 17 feet to 18 feet in depth of water outside and about 15 feet between the piers.

A small amount of dredging (included in above-stated amount) was done in the channel, opposite the light-house, which gave relief at a very troublesome point.

As the funds being exhausted further dredging was suspended.

Harbor lines in Portage Lake.—For about 7 miles of Portage Lake, in the vicinity of Houghton and Hancock, harbor lines on each side were established by the Board of Engineers constituted by Special Orders, No. 40, 1890, headquarters Corps of Engineers, in accordance with act of Congress approved June 20, 1890.

The report of the Board was dated December 11, 1890, and received the approval of the Assistant Secretary of War January 5, 1891.

These harbor lines were established to prevent encroachments on the channel of Portage Lake by the various copper-mining companies and saw-stamp mills were located on the border of the lake.

At present the Franklin and the Atlantic are the only companies that dump their stamp refuse into the lake. They are erecting bulkheads to prevent the refuse getting into the channel, and their dumps are closely watched to see that they do not encroach on the harbor lines. Surveys and soundings are made from time to time to illustrate the condition of the banks.

Copper smelting works that dump slag and cinders, electric-light plants that dump ashes, and all other establishments that are likely to dump refuse into the lake have been furnished with printed circulars explaining the laws of Congress applying to such cases and have been warned not to infringe them.

Rules and regulations.—A new set of rules and regulations for the navigation of the canals were prepared and received the approval of the honorable the Secretary of War, April 5, 1892. They have been printed and distributed, and no objections on the part of those who use the canals have been made to their enforcement, except in the case of some representative lumbermen in regard to one of the provisions of Rule 5, which directs that "rafts shall be made up with logs parallel to each other, in the direction of raft lengths, secured and held closely together by frequent cross sticks, chains, or cables." These lumbermen think it a hardship and an unnecessary expense to them to be obliged to make their rafts as described in the rules. It appears to me both reasonable and necessary that rafts should be so made up that they will not inconvenience general navigation more than is necessary and do as little damage to the canal banks as possible, and that to accommodate these conditions the logs should be parallel to each other and not closely boomed. In order, however, that the matter may receive proper consideration upon sufficient data, I directed Lieut. H. E. Waterman,

the front and the two sides secured by a telegraph wire. ... into this row if possible. A second row follows, ... being put on about every 25 feet. When filled the ... boom. All of the logs in these operations are pushed in

... were then united to form a raft or half raft, as they call it.

... replaced by rope cross lines. "A" lines, as they are called, ... zigzag across the raft. All these lines are shown on the

... lightened by a Spanish windlass. One pole is held vertical, while ... a second in the rope, as shown in Fig. 2. The slack is then ... 3, and when tight enough the windlass is simply laid flat. The ... together, but should not ride, or there is danger of their slip- ... boom. The steamer is attached to the rear by three lines and pushes ... of towing it.

... application of the above methods in the Portage Lake Canals is con- ... principal difficulty is with the stiff boom. It is all right inside ... on Lake Superior, in a storm, nothing seems to hold as well as a loose ... logs not overlapping but connected by about 2 feet of chain. Rafts ... have been frequently abandoned during storms on the lake and after- ... unbroken.

... making up with the stiff boom as above described would not exceed ... M.

... stated that the following amounts will be towed through the Portage ... during the present season.

Owner.	Feet, B. M.	Down or up.
.....	7,000,000	Down.
.....	6,000,000	Do.
.....	2,500,000	Do.
.....	2,500,000	Do.
.....	2,000,000	Do.
.....	1,000,000	Do.
.....	3,000,000	Up.
.....	4,000,000	Do.

not include the amounts which may be towed through the Upper Canal, probably be small.

noted, however, that the difficulties in using this method are much a case of rafts bound up Portage River than those bound down. The use a stiff boom through the river and run their logs into a loose one at or crossing Keweenaw Bay, but to reverse the operation requires the of the raft in Keweenaw Bay, which might be a serious matter.

the visit to Houghton I found that Mr. Hebard and Mr. Funke were mak- port to conform to the rule, though not employing the stiff boom. Their de up of a series of short strings, one in front of the other. Each string in the four sides by eight boom sticks loosely connected, thus forming a t 30 feet on each side. This square is closely filled with logs laid par- ry nearly preserves its shape, and would seem a fair compromise if not

towed up Portage River, however, by Mr. Gregory are nothing but sack few cross lines, and constitute a serious obstruction. I would recom- be notified that he must make more of an effort to conform to the rules. cross line at each joint of the boom sticks would accomplish the result. e these can only be used successfully by arranging the logs in lines, all ne another.

tion. I would recommend no changes in the rules relating to rafting for

They should be given a fair trial by all the owners, as they are unques- the benefit of the canals.

respectfully, your obedient servant,

H. E. WATERMAN,
First Lieut. of Engineers.

ES P. GREGORY,
Corps of Engineers, U. S. A.

REPORT OF THE CHIEF OF ENGINEERS, U. S. ARMY.

Expenses made from appropriation for preservation of Portage and Lake Superior, State of Michigan, act of September 19, 1890.

	To what paid.	For what paid.	Am
1	Major Chas. E. E. Tamm	Mileage	
2	do	do	
3	First Lieut. H. E. Waterman	do	
4	G. A. Marr	Traveling expenses	
5	Peter Primeau	Recording deeds	
6	F. Mayer & Co.	Blue prints	
7	G. A. Marr	Services	
8	Paul Mondor	do	
9	S. Mott	do	
10	Hired men	Services August, 1891	
11	Des Forges & Co.	Stationery	
12	Albert H. Travet	Stationery	
13	James Pryor	Eng. fare	
14	J. R. Sturgis	Office rent	
15	Williams, Daugherty & Updegraff	Insuring	3.1
16	F. A. Douglas, agent	Express charges	
17	First Lieut. H. E. Waterman	Mileage	
18	do	do	
19	V. Reyer	Labor	
20	W. Cross	Stone, etc.	1
21	W. Smith	Timber	
22	W. H. H. Co.	Bolts, etc.	
23	W. H. H. Co.	Rope, etc.	
24	W. H. H. Co.	Stationery	
25	W. H. H. Co.	Services	4
26	W. H. H. Co.	do	
27	W. H. H. Co.	Office rent	
28	W. H. H. Co.	Services	
29	W. H. H. Co.	Hire of tug	
30	W. H. H. Co.	Services, September, 1891	3
31	W. H. H. Co.	Services, etc.	
32	W. H. H. Co.	Mileage	
33	W. H. H. Co.	Rules and regulations	
34	W. H. H. Co.	Books	
35	W. H. H. Co.	Mileage	
36	W. H. H. Co.	Services	2
37	W. H. H. Co.	do	
38	W. H. H. Co.	do	
39	W. H. H. Co.	Office rent	
40	W. H. H. Co.	Book	
41	W. H. H. Co.	Book	1
42	W. H. H. Co.	Mileage	
43	W. H. H. Co.	Services, October, 1891	
44	W. H. H. Co.	Services	2
45	W. H. H. Co.	Services	
46	W. H. H. Co.	Services	
47	W. H. H. Co.	Services	
48	W. H. H. Co.	Services	
49	W. H. H. Co.	Services	
50	W. H. H. Co.	Services	

statement of expenses made from appropriation for preservation of Portage Lake and Lake Superior canals, Michigan, etc.—Continued.

No. of voucher.	To whom paid.	For what paid.	Amount.
12	S. Mott	Services	\$30.00
13	Hired men	Services, May, 1892	189.50
14	James Pryor	Dredging	1,596.07
15	L. Hennes & Co	Rope, etc	9.89
16	Chas. J. Hodge	Steel, etc	3.12
17	Paul Mondor	Services	45.00
18	S. Mott	do	30.00
19	J. B. Sturgis	Office rent	19.00
20	James R. Dec	Use of telephone lines	20.00
21	G. A. Marr	Services	200.00
	Total		9,833.65

COMMERCIAL STATISTICS FOR THE CALENDAR YEAR ENDING DECEMBER 31, 1891.

tion district, Superior, Mich.; nearest port of entry, Houghton, Mich. There are house and range lights at each entrance to the canal. Range lights are at the head of Portage River, and a single light on the outer end of the canal at the Portage Lake entrance to the upper canal.

The charge of the canals was assumed by the United States tolls were collected. Complete commercial statistics were recorded. Since tolls were abolished the collection of statistics has been more difficult.

Blanks for making reports were printed last winter and issued to vessels passing through the canals, the officers of the St. Marys Falls Canal assisting in the distribution of the blanks by direction of Col. O. M. Poe, Corps of Engineers. Vessel masters or owners are required to make on these blanks complete returns of their cargoes, directly to the collector at the first United States post-office after leaving the canals. It is expected that this method will furnish next year complete statistics, or nearly so, of the cargoes of the canals. The statistics reported below are only for the months of September, October, and November, 1891, and are those furnished by vessel agents, masters, and others, and are probably within the limit of business done during these months.

Statement of class and tonnage of vessels passing through Portage Lake and Lake Superior canals during the months of September, October, and November, 1891.

Bound up.		Number.	Net tonnage.	Bound down.		Number.	Net tonnage.
		164	90,331	Steam	102	41,041	
		43	13,220	Sail	22	3,883	

Number of vessels, 331; total net tonnage, 148,475.

Statement of freight and passengers carried through the Portage Lake and Lake Superior canals during the months of September, October, and November, 1891.

Bound up.		Net tons.	Bound down.		Net tons.
		48,748	Copper		13,000
		1,490	Flour		2,649
		130	Building stone		202
		110	Pig iron		382
		23,067	Lumber		6,052
	number..	179	Logs		36,000
			Miscellaneous merchandise		786
			Passengers	number..	85

Weight, 132,616 tons; total passengers, 264.

The following principal lines of steamers used the canals during the season 1891:

Lake Michigan and Lake Superior Transit Company, Chicago, Ill.....	100
Lake Superior Transit Company, Buffalo, N. Y.....	
Ward's Detroit Line, Detroit, Mich.....	
Ward's Lake Superior Line, Detroit, Mich.....	

The principal lines of steamers for the season of 1892 are as follows:

Lake Michigan and Lake Superior Transit Company, Chicago, Ill.....	100
Lake Superior Transit Company, Buffalo, N. Y.....	
Ward's Lake Superior Line, Detroit, Mich.....	
Crescent Line, Detroit, Mich.....	
Union Transit Company, Buffalo, N. Y.....	

Besides the above regular lines of transportation there are a large number of stevedores and sailing vessels using the canals continually, and a large local business is done by tugs towing rafts of logs and scows with wood and lumber. This business is now arranged so that all of the above mentioned lines can be reported regularly, and the necessary after complete reports of statistics must be collected.

I I 2.

IMPROVEMENT OF MANISTIQUE HARBOR, MICHIGAN.

Object.—To secure a navigable channel from Lake Michigan into the mouth of the Manistique River, where the harbor of Manistique is situated.

Project.—The original project, adopted in 1880, provided for the excavation of about 20,000 cubic yards of material to complete a channel 100 feet wide and 12 feet deep between the piers constructed by local enterprise at the mouth of the Manistique River.

Present works.—No construction work was done by the United States Government. The piers were built and are still owned by private parties.

Depth of water.—Originally there was a depth of 7 feet, which was increased to 10 feet before any appropriation had been made by the Government. A survey made May 12, 1892, showed at that date a channel between the piers of navigable width with a depth of 13 feet. The outer bar was not clearly defined, but the ruling depth over it appeared to be about 11 feet. Dredging by private enterprise was in progress, increasing the depth of water over the outer bar.

Operations during the fiscal year.—There were no operations during the fiscal year ending June 30, 1892.

Remarks and recommendations.—The only work by the United States at this harbor has been the removal of 11,780 cubic yards of material in 1880, under a contract with the Chicago Lumbering Company.

In October, 1880, a survey of the harbor showed that the direction of the piers lay across the natural channel. The company which had built the piers, and had also the contract for dredging, found it necessary at this time to renew about 330 feet of the west pier which had been washed away. The superintendent of the company was notified by the officer in charge, Maj. H. M. Robert, that the pier lines would have to be rectified to conform to the natural channel. The company declined to comply with this demand, and their contract, which had been extended from December, 1880, to June 1, 1881, was annulled.

There have been no operations at this harbor since, and no money is set aside for its improvement.

Estimated cost (see Report of Chief of Engineers, 1890, page 1931) \$6,000

APPROPRIATIONS.

June 14, 1890	\$5,000
March 3, 1881	1,000
Total	6,000

Money statement.

Jan. 1, 1891, balance unexpended	\$2,600.78
Jan. 30, 1892, amount expended during fiscal year	31.96
Jan. 1, 1892, balance unexpended	2,568.82

COMMERCIAL STATISTICS FOR THE CALENDAR YEAR ENDING DECEMBER 31, 1891.

[Furnished by Mr. J. D. Mercereau, secretary Chicago Lumbering Company.]

Name of harbor, Manistique, Mich.; collection district, Superior, Mich.; nearest post-office, Poverty Island, Michigan.

Arrivals and departures of vessels.

Description.	Arrivals.		Departures.	
	Number.	Tonnage.	Number.	Tonnage.
.....	367	139,753	367	139,753
.....	299	83,789	299	83,789
Total	666	223,542	666	223,542

Principal articles of export and import.

Exports:	Tons.	Imports—Continued.	Tons.
Beer	23	Cheese	6
Cattle	8	Coal and coke	2,000
Fish	838	Corn	31
Flour	13	Eggs	27
Hides	17	Flour	360
Iron and steel	14,265	Furniture	30
Lath	4,271	Hay	320
Lime and cement	62	Hogs	31
Lumber	158,646	Iron and steel	967
Merchandise, general	102	Cement	174
Shingles	1,130	Merchandise, general	3,528
Pickets	2,775	Mill stuffs	487
Total	182,150	Oats	467
Total approximate value, \$1,718,960.		Oil	260
		Plaster, land	6
Imports:		Pork and beef	163
Agricultural implements	17	Potatoes	23
Apples	86	Salt	585
Beans	29	Sheep	3
Beer	147	Staves	190
Brick	80	Wagons and carriages	11
Butter	49	Total	10,145
Cattle	68	Total approximate value, \$343,484.	

I I 3.

IMPROVEMENT OF CEDAR RIVER HARBOR, MICHIGAN.

Object.—To secure a navigable channel from Green Bay into Cedar River, where the harbor of Cedar River is located.

Project.—The original project, adopted in 1883, provided for the construction of two parallel piers 200 feet apart, extending from the mouth of Cedar River to the 16-foot contour in Green Bay, and dredging a channel between them 14 feet deep; also removing an outer shoal dredging to a depth of 15 feet.

A modification of this project, approved in 1884, provided for continuing the piers in a direct line with the part already built, instead at an angle, as originally proposed.

Present works.—Pile piers and sheet piling: (1) East pier, 301 feet in length, 16 feet wide. All were built in 1884 and are in good condition.

Depth of water.—Originally 8 feet, obstructed by a 3-foot shoal in front of the mouth.

A survey made May 14, 1892, showed a channel 20 feet wide, 12 feet deep, and 10 feet deep with a width of 40 feet. Dredging private enterprise to improve the channel was in progress.

Operations during the fiscal year.—There have been no operations during the fiscal year ending June 30, 1892.

Remarks and recommendations.—Work was suspended at this harbor in November, 1885, and has not been resumed by the United States since that date. It is reported that some dredging was done by private enterprise in 1891 and 1892.

In view of the small amount of commerce likely to be benefited by its completion, no appropriation for continuing the improvement has been recommended since 1885.

Original estimate (see Report of Chief of Engineers, 1882, page 2121).... \$150,000

APPROPRIATIONS.

Act of—	
August 2, 1882	\$150,000
July 5, 1884	0
Total	\$150,000

Money statement.

July 1, 1891, balance unexpended	\$2,000
June 30, 1892, amount expended during fiscal year	0
July 1, 1892, balance unexpended	2,000
{ Amount (estimated) required for completion of existing project	108,000
{ Submitted in compliance with requirements of sections 2 of river and harbor acts of 1866 and 1867	0

COMMERCIAL STATISTICS FOR THE CALENDAR YEAR ENDING DECEMBER 31, 1892.

[Furnished by Mr. Jesse Spalding, president Spalding Lumber Company.]

Name of harbor, Cedar River, Mich.; collection district, Superior, Mich.; n light-house, on north pier head, Cedar River Harbor, Michigan.

a channel 100 feet deep, extending from the 16-foot channel in the Meteor channel to the 10-foot channel of the channel. The original estimate for the improvement of the channel is \$212,000.

A comparison of the channel in 1871 and January, 1872, shows that in the latter year the channel had advanced 1,100 feet from the original center line of the channel, and that 500 cubic feet of material had been deposited within the channel limits.

Of the 1,900 linear feet of channel in 1871-1872, at least 1,000 linear feet were damaged by a freshet in 1872. A section of the channel 100 feet wide was damaged by a freshet in 1872, rendering the channel face undermined, rendering it necessary to make repairs.

An appropriation of \$212,000 is needed for the repairs needed to this pier.

Original estimate (see Report of the Chief of Engineers, 1872, page 139) \$212,000

Act of--

March 3, 1871	212,000		
June 10, 1872	12,000		12,000
March 3, 1873	16,000		16,000
June 23, 1874	14,000		14,000
March 3, 1875	10,000		10,000
August 11, 1876	2,000		2,000
June 18, 1878	2,000		2,000
March 3, 1879	2,000		2,000
			212,000

Money statement

Original estimate	\$212,000
Actual amount expended	2,876,000
Balance on hand	578,000
Total	15,000,000
Balance on hand	15,000,000

STATEMENT OF RECEIPTS AND EXPENDITURES FOR THE YEAR ENDING DECEMBER 31, 1891

Receipts		Disbursements		Balance	
Amount	Number	Amount	Number	Amount	Number
100,000	177	100,000	177		
50,000	94	50,000	94		
150,000	271	150,000	271		
				184,000	

... dredging closed for the season. Work
... still in progress. Under this
... were removed during the fiscal
... and the use of United States dredg
... material were removed from the
... June 30, 1892. Dredge No. 2 w
... Dredge No. 1 was working

... purchase of materials in open

... The formation of a c
... was originally contemplated
... Engineer's Report for 1
... to reduce the upper 2,

... made in 1889 and 18
... considerable shoaling

... the channel an appropriati

... No. 5, Fifty-first Congre
... Engineers, 1891, page

.....
.....

.....

.....

.....

(estimated) required for completion of existing project* \$10,000.00
 that can be profitably expended in fiscal year ending June 30, 1894 10,000.00
 ted in compliance with requirements of sections 3 of river and
 or acts of 1886 and 1887.

STATISTICAL DATA FOR THE CALENDAR YEAR ENDING DECEMBER 31, 1891.

Commercial statistics for the Menominee River are the same as for Menominee
 Michigan, and Marinette, Wis.

I I 6.

IMPROVEMENT OF OCONTO HARBOR, WISCONSIN.

Object.—To secure a navigable channel from Green Bay up the Oconto
 to the city of Oconto.

Project.—The original project, adopted in 1882, provided for the
 formation of a channel 100 feet wide and 8 feet deep by extending the
 pier built by the city to the 10 foot contour in Green Bay, and
 piling between the piers and up the river to Section Street Bridge,
 a distance of about 2 miles, the piers to be parallel to each other and
 100 feet apart.

Construction works.—(1) North pier, 1,603 feet long, 20 feet wide. For
 the first 500 feet the piles are 5 feet apart; for the remaining 503 feet the piles
 are 2 feet apart on the channel side, and 2 feet apart on the outer side,
 the latter being provided with wale timbers, cross-ties, and tie-rods.
 The filling is composed of slabs and edgings ballasted with sand. (2)
 South pier, 2,151 feet long, 20 feet wide. For 1,850 feet the piles are 5
 feet apart; the remaining 351 feet is close piling. The filling is com-
 posed of slabs and edgings ballasted with sand, except the outer 301
 feet which is covered with 2 feet thickness of stone. (3) The outer side
 of the south pier, beginning 300 feet from the outer end for a distance
 of 850 feet, is protected from ice pressure by a line of close piling,
 shoreward for a distance of 850 feet by riprap. (4) Additional
 piling is given to 1,850 feet of the south pier by a line of piles on the
 inner side 3 feet apart, provided with wale timbers, cross-ties, and
 tie-rods at intervals of 9 feet.

Depth of water.—Originally 2 feet, increased to 3½ feet by local enter-

prise. The soundings taken in May, 1892, shows the governing
 depth at the harbor entrance to be about 7½ feet, and in the river as far
 as Spies Mill to be about 6½ feet.

Expenses during the fiscal year.—By hired labor and purchase of
 materials in open market, the north pier, which was seriously damaged
 by a vessel in April, 1891, was repaired by driving a line of piles along
 the inner face for a length of 750 feet. The piles were driven 3 feet
 from center to center, and provided with wales and binders.

Remarks and recommendations.—In order to maintain a depth of 8
 feet of water, periodical dredging will be necessary. The piers, being
 of a pile character, will require frequent repairs.

* For maintenance of channel.

2178 REPORT OF THE CHIEF OF ENGINEERS, U. S. ARMY.

For maintenance and preservation it is estimated that \$100,000 be needed for the fiscal year ending June 30, 1894.

Estimated cost (see Report of Chief of Engineers, 1893, page 1265).....

APPROPRIATIONS.

Acc of—

March 3, 1893.....
August 3, 1893.....
July 3, 1894.....
August 3, 1894.....
August 11, 1894.....
July 13, 1893.....
Total.....

Money statement.

July 1, 1893, balance unexpended.....	\$2,000.00
Amount expended during fiscal year.....	2,000.00
July 1, 1894, balance unexpended.....
Amount appropriated by act approved July 13, 1893.....
Amount available for fiscal year ending June 30, 1893.....
Amount estimated required for completion of existing project.....
Amount that can be probably expended in fiscal year ending June 30, 1894.....
Amount in compliance with requirements of sections 2 of river and harbor acts of 1890 and 1892.....

It is recommended that use of same in place for repairing by hired labor, and purchase of iron machinery, 50 horse, set of the north pier at Oconto, Wisconsin.

Article and quantities.	Price.	Amount.
.....	35.00	\$1,750.00
.....	1.00	100.00
.....	25.00	250.00
.....	2.25	225.00
.....	.25	25.00
.....	1.75	175.00
.....		2,525.00

APPROVED FOR THE FISCAL YEAR ENDING DECEMBER 31, 1893.

Witness my hand and the seal of the Army at Washington, D. C., this 10th day of July, 1893.

CHIEF OF ENGINEERS, MILWAUKEE, WIS.: nearest light.

.....

Name.	Percentage.	Percentage.	
		Number.	Percentage.
.....	10	11.11
.....	14	15.56
.....	44	48.89

Principal articles of export and import.

	Tons.	Exports—Continued:	Tons.
.....	137½	Butter	4
.....	7,500	Coal and coke.....	300
.....	150	Fish	50
.....	1,400	Flour	161½
.....		Hay	50
.....	9,187½	Lime and cement.....	43½
.....		Oil	45
.....		Sawlogs.....	72,000
.....	15	Stone	2,800
.....	23½	Wood	1,000
.....	30		
.....	4,000	Total	80,519

I I 7.

IMPROVEMENT OF PENSBAUKKE HARBOR, WISCONSIN.

—To secure a navigable channel from Green Bay into the Pen-
sbaukke river.

1.—The original project, adopted in 1883, provides for continuing
the pier, which had been built by private enterprise, until it should
be a 10-foot contour in Green Bay, and dredging a channel south
to a depth of 10 feet and width of 100 feet, connecting the deep
water of the river with the deep water in the bay.

2.—*Works.*—(1) A slab pier, 1,300 feet long and 20 feet wide,
built of slabs and edgings and ballasted with sand and stone. It
was built in 1883 and repaired in 1885 and 1891. (2) One thousand six
hundred feet of slab pier, built by private enterprise, was nearly all
destroyed by a storm in 1885. The destruction of this work left the
pier built by the United States a detached work.

3.—*Depth of water.*—Originally 2 feet; increased by private enterprise to
about 9 feet for a width of 30 feet. The destruction of the portion
of the pier built by private enterprise by the storm of October, 1885,
and in restoring the channel to about its original condition.

4.—*Survey made in May, 1890,* showed the governing depth to be 2.8 feet.
5.—*Repairs during the fiscal year.*—By hired labor and purchase of
materials in open market, about 600 linear feet of the west end of the
pier damaged by fire in September, 1891, were repaired in October and
November, 1891.

6.—*Costs and recommendations.*—For the preservation of the existing
pier it is estimated that \$1,000 will be needed for the fiscal year end-
ing 30, 1894.

7.—If further improvement of this harbor be deemed necessary it is
claimed that the modified project submitted in report dated February
1891, the estimated cost of which is \$8,800, would be all that is needed.
8.—Arrivals or departures of vessels were reported at this harbor for
the calendar year ending December 31, 1891. Several boats are engaged
in trade, and it is claimed that this business would materially increase
if the harbor entrance was deepened.

9.—Estimated cost (see Report of Chief of Engineers, 1883, page 1652)..... \$50,000

APPROPRIATIONS.

.....	\$10,000
.....	5,000
.....	15,000

SECTION

Section B
Section C

Section D
Section E
Section F
Section G

Section H

Section I

Section J
Section K
Section L

Section M
Section N
Section O

Section P
Section Q
Section R
Section S
Section T
Section U
Section V
Section W
Section X
Section Y
Section Z

Section AA
Section AB

Section AC
Section AD
Section AE
Section AF
Section AG
Section AH
Section AI
Section AJ
Section AK
Section AL
Section AM
Section AN
Section AO
Section AP
Section AQ
Section AR
Section AS
Section AT
Section AU
Section AV
Section AW
Section AX
Section AY
Section AZ

Section BA
Section BB

... 24, 1892, and suspended June 20, 1892, resulting in ... cubic yards of material.

Recommendations.—For continuing work on the 16-foot ... estimated that \$25,000 can be profitably expended during ... ending June 30, 1894, and for the preservation and main- ... the existing works \$5,000 will be needed for the same period, ...

Cost of the project adopted in 1881 (see Report of Chief of ... 1881, page 2069)	\$135,000.00
Cost for a channel 16 feet deep, submitted February 12, 1892.	91,915.00

APPROPRIATIONS.

		Act of—	
March 23, 1866	\$30,500	March 3, 1879	\$4,000
March 2, 1867	45,000	June 14, 1880	6,000
July 25, 1868 (allotted)	17,500	March 3, 1881	5,000
April 10, 1869 (allotted)	44,550	August 2, 1882	20,000
July 11, 1870	17,500	July 5, 1884	10,000
March 3, 1871	17,500	August 5, 1886	7,000
March 3, 1873	20,000	August 11, 1888	10,000
March 23, 1874	10,000	September 19, 1890	10,000
March 3, 1875	10,000	July 13, 1892	25,000
August 14, 1876	8,000		
March 12, 1878	5,000	Total	322,550

Money statement.

1891, balance unexpended	\$10,224.80
1892, amount expended during fiscal year	6,395.75
1893, balance unexpended	3,829.05
Appropriated by act approved July 13, 1892	25,000.00
Available for fiscal year ending June 30, 1893	28,829.05
Amount (estimated) required for completion of existing project	71,915.00
Amount that can be profitably expended in fiscal year ending June 30, 1894	30,000.00

... fitted in compliance with requirements of sections 2 of river and harbor acts of 1866 and 1867.

GENERAL STATISTICS FOR THE CALENDAR YEAR ENDING DECEMBER 31, 1891.

[Furnished by Mr. Chas. Hartung, deputy collector of customs.]

... of harbor, Green Bay, Wisconsin; collection district, Milwaukee, Wis.; light house, Grassy Island, Wisconsin.

Arrivals and departures of vessels.

Description.	Arrivals.		Departures.	
	Number.	Tonnage.	Number.	Tonnage.
.....	435	157,271	431	157,535
.....	262	49,079	271	48,545
Total	697	206,350	702	206,080

The figures show that a larger class of vessels than formerly is being employed in the Green Bay trade. Thus, while there were 14 less arrivals in 1891 than in 1890, the tonnage was 31,563 tons greater. There were 42 less departures in 1891 than in 1890, but the tonnage was 33,755 tons greater.

Remarks and recommendations.—The steam tugs at this harbor employed in the fishing trade and towage of rafts from the north end of Lake Michigan and Green Bay, therefore they are not available for towing sailing vessels seeking entrance into the harbor; when the wind is unfavorable they are obliged to "beat" in. Being shoal near the shore ends of the harbor piers, they sometimes touch bottom when well inside the harbor entrance. It will therefore be desirable to dredge the channel 12 feet deep the full width between the piers. This would require the removal of about 39,000 cubic feet of material.

For the formation of a wider channel, maintenance of the same and rebuilding decayed superstructure, an appropriation of \$12,000 is recommended for the fiscal year ending July 30, 1894.

Estimated cost (see Report of Chief of Engineers, 1871, page 123)..... \$265,

APPROPRIATIONS.

Act of—		Act of—	
March 3, 1871.....	\$25,000	March 3, 1881.....	
June 10, 1872.....	25,000	August 2, 1882.....	
March 3, 1873.....	25,000	July 5, 1884.....	
June 23, 1874.....	15,000	August 11, 1888.....	
March 3, 1875.....	15,000	September 19, 1890.....	
August 14, 1876.....	5,000	July 13, 1892.....	
June 18, 1878.....	10,000		
March 3, 1879.....	20,000	Total.....	
June 14, 1880.....	20,000		

Money statement.

July 1, 1891, balance unexpended.....	
June 30, 1892, amount expended during fiscal year.....	
July 1, 1892, balance unexpended.....	
Amount appropriated by act approved July 13, 1892.....	3,
Amount available for fiscal year ending June 30, 1893.....	3,
{ Amount (estimated) required for completion of existing project.....	59,
{ Amount that can be profitably expended in fiscal year ending June 30, 1894.....	12,
{ Submitted in compliance with requirements of sections 2 of river and harbor acts of 1866 and 1867.....	

COMMERCIAL STATISTICS FOR THE CALENDAR YEAR ENDING DECEMBER 31,

[Furnished by Mr. R. E. Mueller, mayor.]

Name of harbor, Two Rivers, Wis.; collection district, Milwaukee, Wis.; light-house, on north pier head, Two Rivers, Wis.

Arrivals and departures of vessels.

Description.	Arrivals.		Departures.	
	Number.	Tonnage.	Number.	Tonnage.
Steam.....	253	27,405	253	
Sail.....	63	16,523	63	
Total.....	316	43,928	316	

CHAPTER I. THE DISCOVERY OF AMERICA

IN 1492, CHRISTOPHER COLUMBUS, an Italian navigator, sailed from Spain in search of a western route to the Indies. He discovered the continent of America on October 12, 1492.

COLUMBUS'S DISCOVERY led to the European colonization of the Americas. The first permanent European settlement was established in 1493 at San Salvador in the Bahamas. Other settlements followed in the Caribbean and along the Atlantic coast.

THE SPANISH EMPIRE expanded rapidly, conquering large areas of Central and South America. The Spanish conquistadors, led by men like Hernan Cortes and Francisco Pizarro, established a vast empire that reached from the Gulf of Mexico to the Strait of Magellan.

THE ENGLISH AND FRENCH also sought to establish colonies in North America. The first English colony was founded in 1607 at Jamestown, Virginia. The French established settlements in Canada and the Mississippi Valley.

THE AMERICAN REVOLUTION broke out in 1775, leading to the independence of the United States. The Continental Congress declared independence on July 4, 1776, and the new nation was born.

THE UNITED STATES continued to expand westward, acquiring new territories and states. The Louisiana Purchase of 1803 doubled the size of the nation, and the Texas Annexation of 1845 added another large territory.

THE AMERICAN CIVIL WAR (1861-1865) was fought over the issue of slavery. The Union emerged victorious, preserving the nation and ending slavery. Reconstruction followed, but the South remained a separate society.

THE AMERICAN WEST was a land of opportunity and adventure. Cowboys, miners, and pioneers sought fortune and freedom in the vast, open spaces of the West.

THE AMERICAN INDUSTRIAL REVOLUTION transformed the nation into a major industrial power. Factories, railroads, and cities grew rapidly, changing the way Americans lived and worked.

THE AMERICAN IMPERIALISM of the late 19th century led to the acquisition of overseas territories. The United States emerged as a world power, competing with Europe for global influence.

THE AMERICAN WESTERN MOVIE became a popular form of entertainment, celebrating the adventures and heroism of the frontier. Cowboys and cowboys became icons of the American West.

THE AMERICAN WESTERN MOVIE continued to evolve, reflecting the changing values and attitudes of the American people. It remains a beloved part of American culture.

THE AMERICAN WESTERN MOVIE is a testament to the spirit of adventure and exploration that has shaped the American West. It is a story of courage, sacrifice, and the pursuit of the American Dream.

materials used and cost of same in place under contract dated December 13, 1890, for the Truman and George Cooper, of Manitowoc, Wis., for rebuilding above the 742 linear feet of the north pier at Manitowoc Harbor, Wisconsin.

Articles.	Quantity.	Price.	Amount.
12 by 12 inches feet B. M..	145,572	\$27.00	\$3,930.44
12 by 12 inches do.....	7,890	19.00	149.91
..... pounds..	8,628	.05½	301.98
..... do.....	335	.04	13.40
..... cords..	34	6.00	204.00
			4,599.73
per foot			\$0.20

STATISTICAL STATISTICS FOR THE CALENDAR YEAR ENDING DECEMBER 31, 1891.

[Furnished by Mr. Frederick Schuette, mayor.]

of harbor, Manitowoc, Wis.; collection district, Milwaukee, Wis.; nearest se, Manitowoc, Wis.

Arrivals and departures of vessels.

Description.	Arrivals.		Departures.	
	No.	Tons.	No.	Tons.
.....	778	490,073	781	489,278
.....	290	57,395	259	56,858
.....	1,038	547,468	1,040	546,136

Principal articles of export and import (harbor only).

	Tons.	Exports—Continued.	Tons.
cultural implements...	250	Wagons and carriages	225
y	47	Wheat	1,071
.....	175	Wool	5
.....	1,200	Total	72,069½
er	82½		
le	30	Imports:	
se	2,130	Agricultural implements ...	17½
.....	93½	Coal and coke	129,380
r	39,664½	Iron and steel	502½
iture	1,000	Lath	185½
.....	1,200	Leather	46½
s	25	Lime and cement.....	145
her	375	Lumber	6,000
ber	5,250	Merchandise (general).....	20,000
handise (general).....	2,500	Plaster, land	600
stuffs	6,750	Salt.....	2,670
.....	1,128	Saw logs	13,500
e	7,510	Stone	3,500
and beef	42	Wood	10,000
atoes and vegetables ...	870	Total	186,547½
.....	126		
gles	75		
(railroad)	245		

approximate value of exports, \$3,250,000; total approximate value of im-
750,000.

I I 14.

IMPROVEMENT OF SHEBOYGAN HARBOR, WISCONSIN.

Object.—To secure a navigable channel from Lake Michigan into the harbor of Sheboygan River, where the harbor of Sheboygan is situated.

Project.—The project for the improvement of this harbor was adopted in 1852, and had for its object the formation of a 12-foot channel a distance to the mouth of the Sheboygan River. This was modified in 1873 so as to secure a deeper channel by farther pier extension and dredging. Both projects were completed within their estimated cost and a channel was formed 100 feet wide, with a depth of 15 to 16 feet between the piers. A survey made in 1880 showed a depth of less than 12 feet between the piers and on the outer bar. The existing project was adopted in 1881, its object being to deepen the channel still further by extending the piers to the 20-foot contour in the lake and dredging to a depth of 18 feet between their outer ends, the depth decreasing to 14 feet at the shore line.

Present works.—Pile and crib piers: (1) North pier, 2,270 feet long, composed of 900 feet of pile and crib pier, built by the city, from 12 to 20 feet wide; 1,320 feet of cribs 20 feet wide, and 50 feet of cribs 30 feet wide. (2) South pier, 2,387 feet long, composed of 780 feet of pile and crib pier, built by the city, from 12 to 20 feet wide; 132 feet of pile pier, 20 feet wide, 1,425 feet of cribs 20 feet wide, and 50 feet of cribs 30 feet wide; built 1852-91. About 850 feet of the north pier and 900 feet of the south pier built, since 1871, are in good condition, the cribs having been sunk on a stone or pile foundation. Previous to 1871 they were sunk on the natural lake bottom, composed of shifting sand, causing them to settle very unevenly.

Depth of water.—Originally 4 feet. A survey completed April 2, 1892, showed the governing depth of water in the channel on that date to be about 13.7 feet.

Operations during the fiscal year.—Under contract dated December 13, 1890, with Messrs. Truman and Cooper, of Manitowoc, the north pier was extended 150 feet by the construction of three cribs, each 150 feet long, 20 feet wide, and 20½ feet high, including superstructure. These cribs were sunk upon foundations consisting of 24 piles for each crib, cut off 13½ feet below the established datum plane. Work under this contract was completed October 20, 1891.

Dredging by hired labor and the use of United States Dredge No. 1 for the improvement of the channel, was begun June 26, 1891, and progress at the beginning of the fiscal year. It was continued until August 15, 1891, when the work was closed and the dredge transferred to Port Washington. The dredging resulted in the formation of a channel 16 feet deep with a width of 90 feet for about two-thirds of its length and 60 feet wide for the remainder; 17,030 cubic yards of material were removed during the fiscal year.

Remarks and recommendations.—A channel of less than 16 feet does not meet the present requirements of the commerce of this harbor. A reduction in depth to less than 14 feet this season, with no funds available for dredging, has been the source of much annoyance and serious loss to shippers and others interested in navigation.

Several vessels laden with coal for this port, after vainly attempting to enter the harbor, have been obliged to seek other ports to discharge their cargoes; and steamers plying the west shore of Lake Michigan and advertising to stop here daily, have been obliged to pass in

without stopping, thereby causing serious loss and great annoyance to the owners of vessels, shippers, consignees, passengers, and others more or less concerned.

It is proposed that an appropriation be made for the fiscal year ending June 30, 1892, to be expended in pier extension and such dredging as may be necessary for the maintenance of the channel and an appropriation of \$67,000 is recommended.

Estimated cost of the present project, adopted in 1881 (see Report of Chief of Engineers, 1881, page 2104).....	\$150,000
Estimated cost of the present project, adopted in 1881 (see Report of Chief of Engineers, 1884, page 1866)....	45,000
Total	195,000

APPROPRIATIONS.

		Act of—	
June 30, 1862.....	\$10,000.00	June 18, 1878.....	\$4,000.00
June 30, 1864 (allotted).....	10,000.00	March 3, 1879.....	3,000.00
June 30, 1866.....	47,598.81	June 14, 1880.....	7,000.00
June 30, 1867.....	8,000.00	March 3, 1881.....	25,000.00
June 30, 1869 (allotted).....	14,850.00	August 2, 1882.....	30,000.00
June 30, 1870.....	15,000.00	July 5, 1884.....	28,000.00
June 30, 1871.....	15,000.00	August 5, 1886.....	15,000.00
June 30, 1872.....	18,000.00	August 11, 1888.....	15,000.00
June 30, 1873.....	10,000.00	September 19, 1890.....	15,000.00
June 30, 1874.....	10,000.00	July 13, 1892.....	25,000.00
June 30, 1875.....	12,000.00		
June 30, 1876.....	6,000.00	Total	343,448.91

Money statement.

Balance unexpended.....	\$12,525.43
Amount expended during fiscal year.....	12,392.75
Balance unexpended	132.68
Appropriated by act approved July 13, 1892.....	25,000.00
Available for fiscal year ending June 30, 1893	25,132.68
Estimated amount required for completion of existing project.....	42,000.00
Amount that can be profitably expended in fiscal year ending June 30, 1894.....	42,000.00
Amount available in compliance with requirements of sections 2 of river and harbor acts of 1866 and 1867.....	

Materials used and cost of same in place for extending the north pier 150 feet by 3 feet, 6 inches by 20 by 20½ feet, at Sheboygan Harbor, Wis., under contract dated December 15, 1890, with Horatio Truman and George Cooper, of Manitowoc, Wis.

Articles.	Quantity.	Price.	Amount.
Timber, 12 by 12 inches and 12 by 18 inches..... feet B. M..	98,856	\$25.00	\$2,471.40
Timber, 12 by 12 inches and 12 by 18 inches..... do.....	92,952	22.00	2,044.94
Timber, 8 by 12 inches..... do.....	9,240	18.00	166.32
Spikes..... number.....	72	11.50	828.00
Wails..... pounds.....	12,685	.03	380.55
Belts..... do.....	3,894	.04	147.77
Iron spikes..... do.....	282	.04	11.28
..... cords.....	636	6.50	4,134.65
			10,184.91

Lesser cost, \$97.50.

REPORT OF THE CHIEF OF ENGINEERS, U. S. ARMY.

REVENUE FOR THE CALENDAR YEAR ENDING DECEMBER 31,

At [redacted] J. A. Warner, collector of customs, Milwaukee, Wis., and Mr. John M. [redacted] Mayor of Shelbygan, Wis.]

Name of [redacted] Shelbygan, Wis.; collection district, Milwaukee, Wis.; [redacted] Shelbygan, Wis.

Arrivals and Departures of vessels.

Description.	Arrivals.		Departures.
	No.	Tons.	No.
.....	554	445,548	228
.....	616		611
Total	1,170	445,548	1,159

Exports and imports.	
Exports:	
.....
Boots and shoes
Chairs
Chests
Coal and coke
Eggs
Fish
Flour
Furniture
Hay
Malice ware
Leather
Lime and stone
Machinery
Mineral water
Pears	500
Salt	750
Wooden ware	750
Veneering	2,000
Total	80,626
Total approximate value	\$8,130,250
Imports:	
Bark (tan)
Coal and coke
Empty bottles
Glue
Hay and grain
Hides
Lime and cement
Lumber and lath
Merchandise (general)
Oil and varnish
Pig iron
Plaster (land)
Posts (fence) and ties
Salt
Shingles
Wines and liquors
Wood and slabs
Veneering
Total
Total approximate value	\$4,.....

IMPROVEMENT OF FORT WASHINGTON HARBOR, WISCONSIN

Object.—To secure a navigable channel entrance from Lake Michigan to a small artificial harbor formed by excavating two interior basins.

Project.—The original project for the improvement of this harbor adopted in 1869, and provided for the building of two parallel extending from the shore line to 10 feet of water in the lake, a excavation of a basin 600 feet long by 200 feet wide inside of the line. In 1870 a deflection of about 10 degrees to the southward made in the direction of the piers, this being considered a more able direction for their alignment. In 1876 a further modification the plan was made by excavating a second basin to the northward nearly at right angles to the first basin, and extending the piers 14-foot contour in the lake. This doubled the available area harbor and reduced the height of the entering waves very much.

This is the project now being carried out. The two interior basins have a combined area of about 5½ acres and area to be dredged

12 feet, with a channel of the same depth connecting them lake, the channel inclosed between two piers 150 feet apart being out to 14 feet of water.

works.—(1) North pier, 920 feet long, composed of 370 feet 4 feet wide; 500 feet, 20 feet wide, and 50 feet, 24 feet wide. pier 1,226 feet long, composed of 370 feet of cribs 14 feet 2 feet 20 feet wide, and 406 feet of pile revetment. The piers at 1871–1887, and are in good condition, with the exception near feet of superstructure, built in 1871, which is much decayed and requires renewal.

of water.—The original depth of water at the mouth of Sauk is about one foot. Soundings taken in May, 1892, indicated a depth in the entrance channel of 10½ feet.

done during the fiscal year.—By hired labor and the use of States Dredge No. 2, 14,360 cubic yards of material were removed from the entrance channel and basins. Dredging was suspended on May 15, 1891, and the dredge transferred to Menomonee, Mich. 10 cords of stone were transferred from the shore end of the piers for heads and used for refilling and riprap where undue settlement occurred.

works and recommendations.—The north pier was built the full extent contemplated in 1887. It is proposed to complete the south pier by extending it 100 feet as soon as funds are appropriated.

September, 1890, to May, 1892, there was a deposit in the channels and basins of about 7,000 cubic yards of material, reducing the depth of water in the entrance channel about 1½ feet. The removal of 3,000 cubic yards of material is necessary to maintain the required depth of water in the channel and basins. Three hundred and linear feet of the shore end of each pier, built in 1871, have seen one year's service. The superstructure of this portion of the piers is badly decayed and should be rebuilt above the water line. Estimated cost of the dredging and rebuilding 640 linear feet of superstructure is \$10,000.

It is recommended that an appropriation be made for the fiscal year ending June 30, 1893, to be expended for dredging, and rebuilding above the water line the shore ends of the north and south piers. An appropriation of \$16,500 is recommended.

APPROPRIATIONS.

		Act of—	
11, 1870.....	\$15,000	March 3, 1881.....	\$17,000
h 3, 1871.....	15,000	August 2, 1882.....	17,000
10, 1872.....	15,000	July 5, 1884.....	10,000
h 3, 1873.....	15,000	August 5, 1886.....	5,000
23, 1874.....	10,000	August 11, 1888.....	5,000
h 3, 1875.....	10,000	September 19, 1890.....	3,000
act 14, 1876.....	8,000	July 13, 1892.....	6,500
18, 1878.....	5,000		
h 3, 1879.....	7,500	Total.....	184,000
14, 1880.....	20,000		

Money statement.

1891, balance unexpended.....	\$3,034.70
1892, amount expended during fiscal year.....	2,901.34
	<hr/>
1892, balance unexpended.....	133.36
appropriated by act approved July 13, 1892.....	6,500.00
	<hr/>
available for fiscal year ending June 30, 1893.....	6,633.36

I I 16.

HARBOR OF REFUGE AT MILWAUKEE BAY, WISCONSIN.

1.—To provide a secure anchorage for vessels engaged in the commerce of the lakes, by inclosing the northern section of Lake Bay within a breakwater, formed of timber crib work, placed upon a foundation of stone.

2.—The project was adopted in 1881 and provided for the construction of a breakwater, the north arm of which commences at a distance of about 600 feet from the north shore of the bay, in a depth of 8 feet of water.

The direction is S. 25° 19' E. and its length 2,450 feet. From this the direction of the main arm of the breakwater is changed to S. 71° for the proposed distance of 5,200 feet, with an opening of 400 feet the distance of 1,000 feet from the angle, to provide a fair-weather entrance and exit for vessels.

3. **4.** **5.** **6.** **7.** **8.** **9.** **10.** **11.** **12.** **13.** **14.** **15.** **16.** **17.** **18.** **19.** **20.** **21.** **22.** **23.** **24.** **25.** **26.** **27.** **28.** **29.** **30.** **31.** **32.** **33.** **34.** **35.** **36.** **37.** **38.** **39.** **40.** **41.** **42.** **43.** **44.** **45.** **46.** **47.** **48.** **49.** **50.** **51.** **52.** **53.** **54.** **55.** **56.** **57.** **58.** **59.** **60.** **61.** **62.** **63.** **64.** **65.** **66.** **67.** **68.** **69.** **70.** **71.** **72.** **73.** **74.** **75.** **76.** **77.** **78.** **79.** **80.** **81.** **82.** **83.** **84.** **85.** **86.** **87.** **88.** **89.** **90.** **91.** **92.** **93.** **94.** **95.** **96.** **97.** **98.** **99.** **100.** **101.** **102.** **103.** **104.** **105.** **106.** **107.** **108.** **109.** **110.** **111.** **112.** **113.** **114.** **115.** **116.** **117.** **118.** **119.** **120.** **121.** **122.** **123.** **124.** **125.** 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**1112.** **1113.** **1114.** **1115.** **1116.** **1117.** **1118.** **1119.** **1120.** **1121.** **1122.** **1123.** **1124.** **1125.** **1126.** **1127.** **1128.** **1129.** **1130.** **1131.** **1132.** **1133.** **1134.** **1135.** **1136.** **1137.** **1138.** **1139.** **1140.** **1141.** **1142.** **1143.** **1144.** **1145.** **1146.** **1147.** **1148.** **1149.** **1150.** **1151.** **1152.** **1153.** **1154.** **1155.** **1156.** **1157.** **1158.** **1159.** **1160.** **1161.** **1162.** **1163.** **1164.** **1165.** **1166.** **1167.** **1168.** **1169.** **1170.** **1171.** **1172.** **1173.** **1174.** **1175.** **1176.** **1177.** **1178.** **1179.** **1180.** **1181.** **1182.** **1183.** **1184.** **1185.** **1186.** **1187.** **1188.** **1189.** **1190.** **1191.** **1192.** **1193.** **1194.** **1195.** **1196.** **1197.** **1198.** **1199.** **1200.** **1201.** **1202.** **1203.** **1204.** **1205.** **1206.** **1207.** **1208.** **1209.** **1210.** **1211.** **1212.** **1213.** **1214.** **1215.** **1216.** **1217.** **1218.** **1219.** **1220.** **1221.** **1222.** **1223.** **1224.** **1225.** **1226.** **1227.** **1228.** **1229.** **1230.** **1231.** **1232.** **1233.** **1234.** **1235.** **1236.** **1237.** **1238.** **1239.** **1240.** **1241.** **1242.** **1243.** **1244.** **1245.** **1246.** **1247.** **1248.** **1249.** **1250.** **1251.** **1252.** **1253.** **1254.** **1255.** **1256.** **1257.** **1258.** **1259.** **1260.** **1261.** **1262.** **1263.** **1264.** **1265.** **1266.** **1267.** **1268.** **1269.** **1270.** **1271.** **1272.** **1273.** **1274.** **1275.** **1276.** **1277.** **1278.** **1279.** **1280.** **1281.** **1282.** **1283.** **1284.** **1285.** **1286.** **1287.** **1288.** **1289.** **1290.** **1291.** **1292.** **1293.** **1294.** **1295.** **1296.** **1297.** **1298.** **1299.** **1300.**

I I 17.

IMPROVEMENT OF MILWAUKEE HARBOR, WISCONSIN.

et.—To secure a navigable channel from Lake Michigan into the Milwaukee River, which is the interior harbor of Milwaukee.

ject.—The original project adopted in 1852 provided for the formation of a channel 260 feet wide and 12 feet deep, by dredging across a point which overlapped the mouth of the Milwaukee River at the distance of 3,000 feet to the northward of its original outlet, and by the erection of parallel piers of crib work, each 1,120 feet in length, for the protection of the sides of the channel thus formed. In 1868 an extension of the piers became necessary for the requirements of commerce, and under a continuation of the original project both the north and south piers were extended 600 feet into the lake to 18 feet depth at their outer ends.

ent works.—The entire length of the channel between the piers is 1,120 feet; its width at the outer end has been increased through the use of crib work and settling of cribs to 284 feet.

The width of each of the 1,120-foot sections of piers first built is 20 feet. The pier extensions of 600 feet in length are 26 feet wide, except at their outer ends, which are each 30 feet wide and 50 feet long, protected by a bulkhead of piles and oak timbers.

The superstructure of the inner section of the north pier for 1,052.6 feet of its length by 20 feet in width, is of quarry-faced dry stone masonry, the side walls being 4 feet thick. Between these walls for a distance of 663 feet the filling is of stone packed with gravel, and covered with large paving stones of 9 inches in thickness. The filling between walls for the remaining 389.6 feet is of packed stone, with a layer of concrete pavement of 9 inches in thickness. Piles for the foundation of the piers have been driven along the channel faces. The superstructure over the pier extensions of the 600 feet was built in 1871 and 1872. In 1887 the superstructure of the 600 feet extension of the north pier was cut down and rebuilt. In 1889 the superstructure of the outer section of the south pier was cut down and rebuilt.

Depth of water.—Soundings were taken between the piers and in Lake Michigan beyond the extremities of piers on the 4th day of May, 1891, a plat of which shows that since similar soundings were taken in 1871, a deposit of about 0.8 feet of material has been made in the channel between piers, leaving a depth of waterway of 17 feet. Outside of the pier extremities the water is fully 18 feet deep below the datum plane.

Operations during the fiscal year.—Proposals were opened on the 1st day of July, 1891, for dredging the channel between the piers to a depth of 18 feet and for a width of 162 feet. Mr. Christopher H. Keane's bid for doing the work at 18 cents per cubic yard was the lowest offer received, and a contract was made with him on July 29, 1891, under which 25,896 cubic yards of material were removed from the channel. Ten and three-fourths cords of stone were purchased and used in the north pier in places where the concrete covering had been washed away by the waves.

Remarks and recommendations.—In the report of June 30, 1889, the condition of this harbor was stated in detail. The deterioration of the harbor from wear and climate has been steadily progressing since that time. In 1879 it was estimated that an average yearly expenditure of \$100,000 would be required for the maintenance of this harbor. Since

STATISTICAL STATISTICS FOR THE CALENDAR YEAR ENDING DECEMBER 31, 1891.

of harbor, Milwaukee, Wis.; collection district, Milwaukee, Wis.; nearest city, Milwaukee, Wis.; amount of revenue collected during the year \$365,-

Arrivals and departures of vessels.

Description.	Arrivals.		Departures.	
	No.	Tons.	No.	Tons.
	3,873	3,158,757	3,771	
	2,009	372,420	2,011	
	5,942	3,531,177	5,782	3,50

Following extracts taken from the annual report of the secretary of the chamber of commerce for the fiscal year ending April 4, 1892, show the receipts and shipments of some of the leading articles of commerce at the harbor and city of Milwaukee during the year:

Receipts and shipments.

	Tons.	Shipments:	Tons.
by (rail and lake)	235,030½	Barley	12
by (local consumption)	105,893	Butter	
er	1,997½	Coal	600
er posts	12,332	Corn	
se (Wisconsin)	6,452½	Flax-seed	
(by lake)	1,006,656	Flour	
(by rail)	149,377	Hides	
.....	32,569	Hogs	4,.....
.....	2,716½	Lard	3,376½
wood	231,032	Lumber	261,582½
seed	17,382½	Lath	1,831½
r	272,756½	Malt	51,816½
s	11,497½	Meat (bulk)	6,861½
s (manufactured into		Mill stuffs	90,624
other)	6,184½	Oats	38,698
.....	69,576	Pork, beef, hams, should-	
.....	1,057½	ers, and middles	25,016½
ore	134,324	Pig iron	38,481
ber	541,689	Potatoes	1,675½
.....	4,851½	Salt	84,346
.....	116,936	Shingles	11,895
stuffs	24,163	Wheat	82,932½
.....	76,795		
iron	35,292	Total	1,874,992½
atoes	5,385½		
.....	93,974		
gles	16,184½		
ow	499½		
sat	325,394½		
d	1,962½		
total	3,539,961½		

deposits \$365,401,940.58
 of office of internal revenue 3,276,630.42

number of manufacturing establishments in the city of Milwaukee is 3,258. Number of persons employed in these establishments is 55,890. The capital in manufactures is \$80,506,500. The value of products during the year, 1895. Two new lines of steamers have been established, connecting with lines on the east shore of Lake Michigan, one of which connects with St. Mich., the other with the railroad lines at Benton Harbor. Milwaukee has independent lines of steamers plying across the lake, besides the lines of steamers along the west shore.

...the harbor, with a view to providing

...the harbor was extended 600
...modification was
...the north pier 240 ft
...the length of wa
...the pier extended 30
...the south pier.

Length of piers — The north pier is about 1,200 feet long. Its
width is 20 feet in length x 12 feet wide; its middle section
is 20 feet wide, and its outer or eastern section is
20 feet wide. The south pier is 1,270 feet long; its
width is 20 feet wide. The north pier is 1,270 feet long; its
width is 20 feet wide, and its outer
section is 20 feet long and 20 feet wide. The north pier
now extends 120 feet farther into Lake Michigan than
the south pier.

Effect of piers — The natural level of water in the harbor is
...the water before additional improvement was
...The effect of a northeast storm would be to deposit
...to completely obstruct the entrance and as
...the water in the river became sufficient to remove
...and upon a change of wind sometimes a heavy windward
...would be scoured out.

...under the project previous to 1886, the water
...10 feet in depth. The extension of the piers
...which has been done since 1886, has
...of from 13 to 15 feet in depth, but the
...channel is dependent upon a frequent

...taken at this harbor in April, 1882, is
...conditions of weather a vessel drawing 13 feet

removed in its accomplishment was 14,273 cubic yards. contract made with Messrs. Truman and Cooper, of date 13, 1890, for extending the south pier into lake Michigan for of 200 feet, by constructing 4 cribs, each of dimensions 50 $\frac{1}{2}$ feet, and sinking them on a foundation of piles, the speci- was completed and a continuous superstructure built over height of 6 feet above datum, before the 10th of October,

and recommendations.—Until both piers are extended farther ke, a frequent use of the dredge will be necessary. The ture of the western section of the north pier 200 feet in 12 feet in width, and 635 feet of its middle section of 20 feet in a dilapidated condition, as is also 310 feet in length of the ture of the west end of the south pier. The damage to the e outer end of the north pier, caused by the collision of the *City of Ludington*, of the Goodrich Transportation Company, rning of the 20th of December, 1891, is of a serious character, of the expense and difficulty incurred in repairing structures fractured below the water surface. The broken timbers of will have to be removed and replaced by new material to a 4 or more feet below datum.

f the piers should be extended 300 feet. In 1880 it was esti- at an average yearly expenditure of \$8,000 would be required aintenance of this harbor. Since that time the total sum of ations made has been \$63,500, of which \$21,000 has been ap- pier extension, so that the average yearly expenditure for nce of channel and repairs to piers has been but \$3,541.66 per the past twelve years. This amount is inadequate to the nce of the harbor as is apparent from its present condition.

APPROPRIATIONS.

		Act of—	
15, 1844	\$12, 500	June 18, 1878	\$10, 000
ct 30, 1852	10, 000	March 3, 1879	6, 000
28, 1864	3, 600	June 14, 1880	6, 000
23, 1866	23, 910	March 3, 1881	6, 000
12, 1867	45, 000	August 2, 1882	7, 000
10, 1869 (allotted)...	22, 275	July 5, 1884	7, 000
11, 1870	10, 000	August 5, 1886	10, 000
13, 1871	10, 000	August 11, 1888	10, 000
13, 1873	20, 000	September 19, 1890	17, 500
23, 1874	10, 000	July 13, 1892	25, 000
13, 1875	10, 000		
ct 14, 1876	8, 000	Total	289, 785

Money statement.

\$1, balance unexpended	\$17, 643. 88
\$2, amount expended during fiscal year	16, 642. 71
	<hr/>
\$2, balance unexpended	1, 001. 17
ppropriated by act approved July 13, 1892	25, 000. 00
	<hr/>
available for fiscal year ending June 30, 1893	26, 001. 17
	<hr/>
(estimated) required for completion of existing project	39, 500. 00
that can be profitably expended in fiscal year ending June 30, 1894	39, 500. 00
ed in compliance with requirements of sections 2 of river and r acts of 1866 and 1867.	

I I 19.

IMPROVEMENT OF KENOSHA HARBOR, WISCONSIN.

—To secure a navigable channel from Lake Michigan into the basin at the mouth of Pike Creek, upon which the city and of Kenosha are situated.

1.—The original project for the improvement of this harbor, in 1852, was to secure a channel 12 feet deep from 12 feet of Lake Michigan to the interior basin or bayou by the constructors placed parallel to each other and 150 feet apart, and by g between them. In 1866 a modification of the original project le in order to provide a navigable channel 15 feet in depth.

ther modification or extension of the project was made in 1889, rovided for the extension of the north pier 300 feet and of the er 600 feet.

2. Works.—The north pier is 1,600 feet long. Its inner or section, which was built either by private parties or the city of a, is of pile work, 365 feet in length, and is 12 feet wide. The ad- section to the eastward, built under the original project of 1852, b work 610 feet long and 18 feet wide. The extension of this ce 1866 is 625 feet long, 250 feet of which is 30 feet wide. The er is 950 feet long and 20 feet wide, commencing at the shore d extending into the lake. The north pier extends 270 feet into the lake than the south pier.

3. Depth of water.—The natural depth of water at the mouth of Pike not more than 2 or 3 feet could be depended upon at the entrance. rk first done under the project furnished a channel of from 9 to in depth between the piers.

xtensions of the piers since 1866, together with periodical dredg- e generally provided a channel about 12 feet in depth, but in o maintain a greater depth of water frequent dredging has been ry.

it of soundings which were taken at this harbor in April, 1892, an available depth of but 12½ feet of water in the channel. A ank in mid-channel has but 12 feet of water over it. An exten- posit of sand has been made since dredging was done in June. To restore the channel to a depth of 15 feet and width of 80 feet aire the removal of about 15,000 cubic yards of material, scow ement.

contour line of 13 feet extends 200 feet beyond the outer end of th pier, and the bank to the southward of the harbor entrance t 600 feet outside the eastern extremity of the south pier. Under nditions it is improbable that a channel of greater depth than 2½ feet of water can be maintained, except by a constant use of dge.

4. Operations during the fiscal year.—At the close of the fiscal year end- ne 30, 1891, the three cribs which were built under the contract with Messrs. Truman & Cooper, of date December 13, 1890, for ing the south pier 150 feet, were sunk in position. The super- re was subsequently built and supplied with stone ballast and l with a decking of 3-inch plank. The contract was closed July 1. An agreement was entered into with Mr. S. O. Dixon on .1892, for restoring the channel to 15 feet in depth and 80 feet in r its whole length, at a cost of 18½ cents per cubic yard of ma-

Arrivals and departures of vessels.

Description.	Arrivals.		Departures.	
	No.	Tons.	No.	Tons.
.....	72	11,379	72	11,353
.....	195	27,812	194	27,510
.....	267	39,191	266	38,863

Principal articles of export and import (harbor only).

Tons.	Imports—Continued.	Tons.
..... 3	Hides	125
..... 6	Lath	302.5
..... 800	Lumber (hard and soft)	22,750
..... 1.5	Peaches	6
..... 112	Piles	120
..... 60	Posts (fence)	228
..... 6,750	Shingles	450
..... 7,729.8	Slabs	300
.....	Stone	5,600
.....	Wood	2,750
..... 16,500	Total	59,631½
..... 10,500		

II 20.

IMPROVEMENT OF WAUKEGAN HARBOR, ILLINOIS.

—To provide a shelter for the protection of vessels engaged in commerce of the city of Waukegan.

—In 1852 an appropriation of \$15,000 was made for the "improvement of the harbor and breakwater at Waukegan, Ill." The project was a breakwater parallel to the shore in 20 feet of water. It, 30 by 25 feet, was placed in position, but was carried away by a storm and the work then abandoned. In 1872 an examination and survey were made, as called for by the river and harbor act of that year. It contemplated a breakwater in 24 feet of water. No action was taken on this report.

The character of the improvement of this harbor is somewhat different from that of other points on the Great Lakes. Most of the improvements have consisted in deepening the mouths of streams emptying into the lake, but at Waukegan there is only a creek emptying into the lake, and it is of no importance for harbor purposes.

The present project was adopted in 1880, its intention being to construct an exterior basin of sufficient capacity to meet the requirements of local trade by inclosing a portion of Lake Michigan within piers constructed of pile work, and an entrance channel between piers from

12 feet of water in Lake Michigan to the basin; the channel to be excavated by dredging to the depth of 12 feet.

works.—The length of the north pier is 1,651.6 feet; the south pier is 1,226.9 feet, making a total length of pier of 2,878.5 feet. The north pier, which incloses the basin, is composed of the first of which runs easterly from inside the lake in 1879 for a distance of 345 feet into the lake.

This section is comprised of a single row of 12 by 12 inch square piles, reinforced at the angle or outer end for a length of 80 oak piles driven at a distance of 12 feet from the front row, the space being filled with stone ballast.

The next section of pile work, 16 feet wide, runs in a southern direction for a distance of 380.8 feet to the channel angle.

The next section of 16 feet in width, which runs in a southern direction for a distance of 533.8 feet from the channel angle, is comprised of two rows of closely driven white oak piles, divisions by cross rows driven at distances of about 32 feet apart, sheeted with 3-inch pine plank, and filled with stone ballast of 2 feet above datum.

The outer section, of 16 feet in width and 392 feet in length, is constructed in the same manner as the preceding section, and to the line of the south pier the distance between the piers is 100 feet.

The south pier is built as a prolongation of the line of the side of Madison street in the city of Waukegan. The inside of this pier, 140.3 feet, is composed of a single row of square piles, 12 by 12 inches in dimensions, with the exception of 24 piles, which are 22 feet in length and 12 inches square.

The next outer section of 16 feet in length is constructed of white oak piles, each 30 feet in length, on alignment at distance from center to center, and sheeted with two rows of dimensions 3 inches by 12 inches by 22 feet, driven close at joints, and spiked to the wales which inclose and secure the piles.

The next section to the eastward, of 178 feet in length, is formed of white oak piles, each 30 feet long, driven at distance from center to center and secured with binders. On the inside of this row pine piles of 8 inches by 12 inches by 25 feet are driven and bedded to the binder and reinforced with a pile 3 by 12 inches. The outer section of this pier is 178 feet in length, and is composed of two parallel rows of white oak piles, driven in at a distance of 14 feet from center to center of rows, and with sections of about 7 feet each. For a length of 100 feet from the pier the piles are 30 feet long, and for the remainder 2 feet rows of piles are secured and bound in place by means of the same binders, and the rows in a similar manner to those of the pier at the south pier.

On the south side of the pier, two rows of piles, 3 by 12 inches by 22 feet, are driven and spiked to the binder, secured to the second section. The distance of the piles from the center of the pier is a length of 12 feet, and the space between the two rows is 16 feet.

The second section of the pier, which is 178 feet in length, is composed of two parallel rows of white oak piles, driven in at a distance of 14 feet from center to center of rows, and with sections of about 7 feet each.

On the north side of the pier, two rows of piles, 3 by 12 inches by 22 feet, are driven and spiked to the binder, secured to the second section. The distance of the piles from the center of the pier is a length of 12 feet, and the space between the two rows is 16 feet.

more than 1,300 feet to the eastward of the line of

soundings taken at this harbor in April, 1892, shows that a channel of 7 feet in depth near the north pier is available for its draft.

during the fiscal year.—At the close of the fiscal year ending 1891, six sections of pile work, each 32 feet long, being completed in extension of the north pier, having been done by hired labor and the purchase of materials in

Subsequently six more sections, forming an additional 192 running feet, were built, making a total length of extension of north pier of 392 feet, under the provisions of the river and harbor act of September 19, 1890, which appropriated \$35,000 for the extension of Wanakegan Harbor. This extension was completed on the 1st of August, 1891, when the pile-driver was transferred from the north pier to the south pier for its further extension. Work was done on the south pier until November 11, 1891, at which date 202 feet of extension had been built, and the pile-driver laid up for the winter.

Wooden fences have been constructed for the purpose of arresting the drift of the beach to the northward of the harbor from being exposed to the channel and basin. One of these is 176 feet long and 18 feet wide. While they have not completely checked the drift, a quantity of sand has been retained upon the beach through the intervention of the fences, which otherwise would have been carried into the

harbor received and opened on March 16, 1892, in answer to advertisement of February 15, 1892, inviting proposals from contractors for the deepening of the entrance channel and harbor basin to a uniform depth below datum. The bid of the Green Bay Dredge and Pile Driving Company was the lowest offer made, and a contract was made with that company on the 23d day of March, 1892, to dredge and remove 100,000 cubic yards of material at 13½ cents per cubic yard, the work to be completed on or before October 31, 1892.

Under this contract 9,714.6 cubic yards of material have been removed under contract.

and recommendations.—The satisfactory manner in which the extension of the piers were made during the working season of 1891, forming a total length of 594 running feet, with one pile-driver and nine men, was partly due to the preparations which were made during the winter of 1890 and 1891 by the purchase and delivery of material, but chiefly to the systematic work done by the overseer and crew of men.

100 cords of stone are needed alongside the piers as riprap, to prevent the deficiency caused through a settlement of that material.

Prospect for a large increase in the commerce and manufactures of the harbor is very encouraging, and, in consequence, the value of the harbor has been greatly enhanced. The facilities rendered by the harbor are not considered by the citizens of the place as being commensurate with the requirements of the growing demands; in view of this opinion a petition was circulated among the business men for signature during the past winter, and addressed to the honorable the Secretary of the Treasury, asking that an ample appropriation be made for extending the harbor to the depth of 16 feet of water in Lake Michigan. A resolution on this subject was called for, and made by me, dated February



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Arrivals and departures of vessels.

Description.	Arrivals.		Departures.	
	No.	Tons.	No.	Tons.
	203	20,811	203	20,811
165	31,740	165	31,740	
	368	52,551	368	52,551

Principal articles of export and import.

	Tons.	Imports—Continued.	Tons.
	75	Furniture	562½
	1,575	Hides	300
	150	Hops	10
Green trees	500	Lime and cement	972
	500	Lath	825
Doors, and blinds	150	Lumber	21,000
	400	Malt	325
	1,200	Merchandise (general)	25,000
Works	75,000	Oil	675
	12½	Pork and beef	1,200
	79,562½	Posts (fence)	114
		Potatoes	180
		Salt	750
		Shingles	900
tan)	440	Ties (railroad)	700
	176	Wire-work products	75,000
	1,750	Wood	10,000
	18,000	Zinc ore	4,650
and coke	116,000		
	28,000	Total	309,464½
	1,935		

I I 21.

IMPROVEMENT OF FOX RIVER, WISCONSIN.

—Originally to secure a cheap route of water transportation Mississippi River to the Great Lakes and Atlantic seaboard. Improvement of the Wisconsin River having been abandoned, the object is to obtain a navigable channel, as far as existing works permit, from Portage, on the Wisconsin River, to the harbor of Bay, a distance of 160 miles.

—The original project called for slack-water navigation on River by means of locks and dams as part of the through navigation route. The present modified project is that recommended by the Board of Engineers of May 14, 1886, and is to deepen River by rock excavation and dredging from Montello to Green Bay 3-foot depth and from Portage to Montello to 4 feet depth; to deepen river channels to 100 feet throughout; to deepen the channel De Pere and Green Bay; to dredge the channel in the Neenah and to remove the bar at the mouth of the Fond du Lac River. *t works.*—The present works are as follows:

	Upper Fox.	Lower Fox.	Total.
	9	18	27
	7	10	17
	4	8	12
falls		2	2
	20	38	58



NDIX I I—REPORT OF MAJOR GREGORY

Money statement.

ce unexpended
 unt expended during fiscal year
 ce unexpended
 unding liabilities
 ce available
 ted by act approved July 13, 1892
 for fiscal year ending June 30, 1893
 ted) required for completion of existing project.
 be profitably expended in fiscal year ending June 30
 mpliance with requirements of sections 2 of river
 f 1866 and 1867.

ORT OF MR. SAMUEL WHITNEY, ASSISTANT ENGINEER.

UNITED STATES FOX RIVER IMPROVEMENT,
 ENGINEER OFFICE,
 Oshkosh, Wis., June 30,

the honor to submit the following report of operations on tl. ix
 , from Portage to Green Bay, for the fiscal year ending June 30, 1892.
 he year consisted principally in excavating a channel through the
 e river outlet of Deperre Lock, in completing the guard-gates at
 mpleting by dredging of a 6-foot channel in the Neenah River and
 from Scott Street Bridge at Fond du Lac to the 7-foot curve in Lake
 redging the channel at Grignon Rapids (lower Fox), in completing
 scows, and making repairs of Dredge No. 3.

Fox the water became so low during the month of September, 1891,
 lass of boats were obliged to lay up for the remainder of the season.
 closed by ice November 17, 1891, and resumed April 10, 1892.

Fox boats drawing 5 feet of water could run from Green Bay to
 out August 1, 1891, when the mills at Neenah, Menasha, Appleton,
 ad drawn the water so low below the crests of the dams that there
 set of water in the channel, and at times there was a depth of but 6
 ast wall of the Kaukauna, first lock. Navigation was closed by ice
 91, and resumed April 15, 1892.

outline of work done at the different points between Portage and
 r separate headings, viz: "Improving Fox River," and "Improv-
 consin rivers, Wisconsin," is respectfully submitted.

IMPROVING FOX RIVER, WISCONSIN.

redging channel through rock bar at river outlet of De Perre Lock.—The
 ing a channel 75 feet wide and 450 feet long, through the ledge of
 r outlet of the lock, was begun in November, 1891.

ant was put in good repair, placed on a scow and housed in. Drill-
 commenced on December 2 and completed February 10. One thou-
 ty-five holes, 2 inches in diameter, were drilled to a depth of about
 ed. The total depth of holes drilled was 4,227 feet.

was fitted out, and on February 19, 1892, commenced the removal of
 ousand one hundred and seventeen cubic yards in place were ex-
 icated at the left side of the channel, and as that side of the channel
 ace where the rock could be dumped, a large portion of it had to be
 e dredge. The rock is of a good quality of limestone suitable for
 l banks, etc. One thousand one hundred and ten cubic yards of
 , were dredged from a point just below the lower end of channel,
 scows, towed to deep water in the river by the tug *General G. K.*
 mped. The dredging of the channel was carried to a depth of 4 to
 the level of the top of the lower miter sill of the lock. The work
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2222 REPORT OF THE CHIEF OF ENGINEERS, U. S. ARMY.

List of boats navigating Fox River, between Portage and Green Bay, Wis., June, 1891.

Name of boat.	Draft.	Ton- nage.	Steam or sail.	Name of boat.	Draft.	Ton- nage.	Ste- am
	<i>Feet.</i>				<i>Feet.</i>		
Portage*.....	2	75	Steam.	D. W. Libbey (tug)†.....	3½	60	Ste
S. D. Arnold*.....	4½	93	Do.	Oshkosh (tug) †.....	3	46	I
City of Berlin.....	3	150	Do.	Thos. Spear (tug) †.....	10	41	I
R. F. Carter †.....	5	125	Do.	Time (tug).....	(5)	(5)	I
Oaslan Cook †.....	5	175	Do.	M. Brunette (barge).....	(9)	(5)	I
John Dennessen.....	4½	15. 40	Do.	Emma (scow).....	4½	45	Sail
Evalyn †.....	5½	150. 66	Do.	Eclipse.....	3½	60	I
Fashion †.....	3½	70	Do.	George (scow).....	5½	82	I
City of Fremont*.....	3½	100	Do.	Topsy (scow).....	4½	72	I
K. M. Hutchinson*.....	4½	189. 96	Do.	Juilia (scow).....	5	44	I
George Lacy.....	(5)	25	Do.	Sassy Jack (scow).....	4	26	I
Lilly.....	4	4	Do.	Rose Bnd (scow).....	5	75	I
Laura May †.....	3	5. 91	Do.	Venture (scow).....	5½	91	I
J. H. Marston †.....	4½	160	Do.	Red Fox (scow).....	3	40	I
Mark*.....	2½	30	Do.	Newbauer (scow).....	2½	20	I
Henrietta †.....	5	125	Do.	Barge (Morning Bell).....	3½	70	Tov
John Lynch*.....	3	50	Do.	Barge (Mark)*.....	2½	90	I
Schiller (tug).....	5½	13. 60	Do.	Barge (D. W. Cady)*.....	4	75	I
O. B. Reed †.....	3½	75	Do.	Barge (Indian Queen).....	4	45	I
Morning Bell (tug).....	5	9	Do.	Barge (Jumbo) †.....	4½	97	I
Viola (tug).....	3½	7	Do.	Barge No. 1 †.....	5½	145	I
Agnes C. (tug).....	6	10	Do.	Barge No. 2 †.....	5½	145	I
D. W. Cady (tug)*.....	2	20	Do.	Barge No. 3 †.....	5½	145	I
W. W. Neff (tug).....	3½	46. 44	Do.	Barge No. 4 †.....	5½	145	I
M. D. Moore (tug) †.....	3½	60	Do.				

- * Boats that run above Oshkosh.
- † Boats that run to Winneconne occasionally.
- ‡ Runs between De Pere and Green Bay.
- § Not given.
- || H. Collette.

Number of lockages on Fox River, Wisconsin, for the calendar year 1891.

No.	Lock.	Lock- ages.	No.	Lock.
1	De Pere.....	634	16	Appleton, second.....
2	Little Kaukauna.....	462	17	Appleton, first.....
3	Rapid Croche.....	429	18	Menasha.....
4	Kaukauna, fifth.....	482	19	Eureka.....
5	Kaukauna, fourth.....	476	20	Berlin.....
6	Kaukauna, third.....	538	21	White River.....
7	Kaukauna, second.....	555	22	Princeton.....
8	Kaukauna, first.....	551	23	Grand River.....
9	Little Chute, fourth } Combined	5342	24	Montello.....
10	Little Chute, third }	342	25	Governor Bend.....
11	Little Chute, second.....	328	26	Fort Winnebago.....
12	Little Chute, first.....	328	27	Portage City.....
13	Cedars.....	334		Total.....
14	Appleton, fourth.....	337		
15	Appleton, third.....	376		

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OPERATING AND CARE OF LOCKS AND DAMS ON FOX RIVER, WISCON

The expenditures for maintaining the existing depth of navigation throughout the Fox River and canals; for repairs to mechanical structures that have been completed and in use, but afterward injured by flood or otherwise; for current repairs to old locks and dams, lock houses, and for lock tenders' services, have been paid from the definite appropriation for "operating and care of canals and other works of navigation" provided by section 4 of river and harbor act of July 1884.

In accordance with this section an itemized statement of the expenditures is appended herewith.

The work during the last fiscal year has consisted principally in dredging the channel of the upper Fox River and in making repairs of locks, dredges, lock houses, locks, dams, and canal banks. For details of the work, attention is invited to the appended report of Assistant Engineer Samuel Whitney.

Money statement.

June 30, 1891, balance unexpended	\$63,948.29
June 30, 1892, amount expended during fiscal year.....	55,989.98
June 30, 1892, balance unexpended	8,008.31
June 30, 1892, outstanding liabilities	6,176.96
June 30, 1892, balance available	1,831.35
Amount allotted for expenditure in fiscal year ending June 30, 1893.....	40,000.00
Amount available for fiscal year ending June 30, 1893	41,831.35

REPORT OF MR. SAMUEL WHITNEY, ASSISTANT ENGINEER.

UNITED STATES FOX RIVER IMPROVEMENT, ENGINEER OFFICE,
Oshkosh, Wis., June 30, 1892.

MAJOR: I have the honor to submit the following report of operations under the title of "Operating and care of canals and other works of navigation on the Fox River" for the fiscal year ending June 30, 1892.

Work during the year consisted principally in dredging the channel of the Upper Fox River; in making repairs of boats, dredges, lock houses, and in making incidental repairs of locks, dams, and canal banks.

De Pere Lock.—Four cords of loose rock were removed from the lock chamber by hand and a hand dredge.

De Pere Canal Banks.—A revetment wall of cement masonry, 46 feet in length and 10 feet in height, was built to sustain a portion of the left bank of the canal immediately above the lock. The slope of the bank was raised to a level of the top of the wall with 340 cubic yards of clay, and the canal face of the bank for a distance of 10 feet above the head of lock riprapped with 10 cords of stone brought from the quarry cut below the lock.

Lock house at De Pere.—The old wooden foundation walls, which were badly decayed, were replaced by masonry walls, the roof reshingled, a new floor laid in the cellar, new stairs built to cellar and loft, front room replastered and papered, a porch built at the front of house, two new doors were fitted and hung in place; a foot walk built from the front to rear of house, and the woodwork of house, both inside and out, painted two coats. A new vault was made for the outhouse, and the outhouse put in good repair.

Little Kaukauna Lock.—Four solid timber gates were made, the old gates taken out, valves and gate hangings taken off and fitted to the gates, and the gates hung in place.

Little Kaukauna Dam.—No repairs were needed during the year.

Rapid Croche Lock.—The upper right gate having been knocked off its step by a passing boat, the gate was taken out, the step replaced and the gate rehung. Slight repairs were made to hangings of the other gates.

Rapid Croche Dam.—Four hundred and eighty-five cubic yards of clay were wheeled from the bank of the river and placed to repair the embankment at the rear of the right abutment that had been cut away by water passing through the sluice way; and to prevent further damage the foot of the embankment was well riprapped with cords of large stone boated from the quarry at Kaukauna.

Kaukauna Fifth Lock.—The upper left gate was removed, one of the valve rods repaired, and the gate replaced. New racks were put on the valve rods of the upper right gate.

Kaukauna Fourth Lock.—The valve chains of the platform valves were repaired.

Kaukauna Third Lock.—Slight repairs were made to the valve chains.

Kaukauna Second Lock.—Eight new iron rollers were put in place under the gate.

Kaukauna First Lock.—During the winter of 1891-'92, four new solid timber gates were made, and early in the spring the old gates were removed from the lock to the canal bank; the valves, gate hangings, etc., were taken off, fitted to the new gates

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Appleton Third Lock.—The house was moved from the head of the lock to a new place near the foot of the second lock; a cellar built under the dam newly clapboarded; a porch built at the front side, and the outside painted two coats.

g.—The suspension rods of the lower left gate were repaired and timber bars were purchased.

h. Canal Bank.—No repairs required.

i.—Slight repairs were made to the hoisting chains of the sluice gates.

—During July, 1891, a washout at the rear of the left abutment was 1,200 cubic yards of clay boated from a dredged bank 2 miles above the lock. 1,200 cubic yards of gravel boated from Berlin was placed along the river to the left of the dam. A break underneath the dam, at a point near the right abutment, caused by sinking 90 brush mats, 10 inches by 4 feet by 10 feet with 10 cords of stone, and placing over the mats 1,327 cubic yards of clay and gravel. A break was removed from the apron and the washout underneath apron was repaired by sinking 90 brush mats, 10 inches by 4 feet by 10 feet with 10 cords of stone, 17 cords of which were boated from Menasha and 12 cords from Berlin. Seventeen broken apron planks were replaced by new plank.

k.—The drawbridge over the navigable pass was entirely rebuilt and painted two coats. All of the posts and valves in the sluice way under the bridge were replaced, new posts and valves framed and put in place, and low places in the dam were filled with 160 cubic yards of gravel, completing the repairs. The apron was built of piles and timber on quicksand foundation, and needs frequent repair its being entirely washed away.

l.—Four gates, built at Oshkosh in the spring of 1891, were taken to the lock and hung in place of the old ones. Four new capstan platforms, new gate spars were made, and four new tripod capstans, for the gates, set in place, completing the repairs.

m. (brush).—During the months of August and September, 1891, low places in the dam were raised to the original height of dam by placing 373 brush mats of stone; and the shore protection at the ends of dam repaired with 48 cords and 15 cords of stone.

—No repairs needed.

n. House.—The kitchen floor was painted one coat and the eaves spout

o. Lock.—The fence around lock site was put in good repair.

p. Lock House.—No repairs needed.

q. Dam (brush).—Low places in the dam were repaired with 138 brush mats of stone.

r. River Banks.—A pier composed of piles, brush mats, and stone, was built to prevent the further washing away of the right bank of the river at the head of the River Lock. From a point about 200 feet below the lock, the pier extends a distance of 384 feet. Two rows of piles were driven at a distance of 12 feet from centers. The piles in the front row were driven at distances of 8 feet, and those in the back row 16 feet from centers. A wale, 10 inches in diameter, was placed along the back row of piles below the water surface; through the wale and piles tie-rods of 1-inch round iron were placed and the pier then filled with brush mats, sufficient stone being placed on each layer of mats to sink them in

the further caving into the river of the higher portion of the sand bank at the head of White River Lock and right end of dam, piles were driven at a distance of 12 feet from centers; between the row of piles and the face of the bank, brush mats were placed and covered with stone. The bank was then graded to a proper slope and rippaped with stone. The length of the pier is 531 feet.

s. Dam (brush).—Repairs of the dam were commenced on May 11, 1892, but on account of the high stage of water in the swamps, making it difficult to obtain the timber, etc., work was suspended on June 15. Up to the time work was suspended 1,016 mats, 10 inches by 4 feet by 10 feet made, and 15 cords of stone were sunk in place.

t. Lock.—No repairs required.

u. Lock House.—No repairs needed.

v. Lock.—No repairs needed.

w. Lock House.—No repairs needed.

x. Dam.—No repairs made.

y. Lock.—No repairs made.

z. Lock.—A gate spar was made and placed.

—Dredge No. 5 commenced the work of strengthening the levee on the right end of the dam. 56 cubic yards of material have been excavated from the bed of the river and placed at the sides and top of the embankment.

aa. Lock.—No repairs required.

No. 7.—A broken plank in the bottom of the hull was replaced by a new arboard spud guides rebolted to hull, the old dipper handle repaired, and the dipper ready for the iron work. A new door was put on the dipper and the dipper repaired and riveted in place.

The machinery was cleaned and put together, the outside of cabin and the tin lined one coat, completing the repairs. The dredge was put in commission, 25, 1892.

Scow No. 1.—The scow was hauled out in May, 1892; eleven graving in the bottom, side fenders and pocket door hinges repaired; two new mks fitted and spiked in place, and the seams at the lower end of the pocket and both rakes were recalcd, completing repairs.

Dump Scows.—The rakes and the top sides of the two center dump scows were during the month of May, 1892.

Scows.—The construction of five scows, 16 by 15 by 3 feet, to be used as tenders and dredges, was commenced in May, 1892, and at the date of this report three were about half built and the framework of the other two commenced.

DREDGING UPPER FOX RIVER.

Dredge No. 4.—The work of rebuilding the dredge having been completed on July, 1891, she was put in commission on the same date and employed close of the season in dredging bars between Omro and the Berlin Bridge, *viz*: From a bar opposite the mouth of Waukau Creek 8,137 cubic yards of material were removed; bar 5 miles below Eureka Lock, 11,015 yards; bar at Ricks Bend, 4,258 yards; two bars near the village of Eureka, 8,883 yards; bar lower end of cut-off below Eureka Lock, 4,683 yards; bar at Sacramento, 4,419 yards, and bar one-half mile below Berlin Bridge, 4,419 yards. The dredge was laid up for the work November 16, and was towed to Eureka Lock and laid up for the

work was resumed April 21, 1892, at the point where it was suspended in November. 98 cubic yards of sand and gravel were removed, completing the work of dredging a channel through the bar. On May 30 the dredge was towed to a point 3 miles above Berlin Lock by the steamer *For*, where the removal of a bar was commenced, and by the end of the fiscal year 5,282 cubic yards of sand had been removed. The number of cubic yards of material removed by the dredge during the fiscal year, 1891-2, was 369.

Dredge No. 5.—Dredge No. 5 continued the work of dredging bars between Omro and a point one mile below White River Lock, and removed material as follows, *viz*: from bars between Grand River Lock and the head of Lake Apuck, 448 cubic yards; from bars between the point where Mehan Creek enters the Fox River and the head of Princeton Lock, 10,191 yards; bar at the lower end of Princeton Lock, 1,301 yards; bar one and a half miles below Princeton Lock, 1,301 yards; bar one and a half miles below Princeton Bridge, 5,740 yards; bar 3 miles above White River Lock, 5,901 yards; bar below White River Lock, 12,145 yards. Work was suspended at White River Lock on Nov. 27 and the dredge laid up for the winter.

The dredge was put in good repair during the winter, and dredging resumed in May, 1892; 4,669 cubic yards of sand were removed, completing the dredging of a channel through the bar below White River Lock. On the 8th of May the dredge was towed by the *Boscobel* to a point about 4 miles above Princeton Lock, where 50 cubic yards of scattering bowlders were removed from the channel of the

The work was completed on the 17th of the month, and on the following day the dredge was towed by the *Boscobel* to Montello; 9,794 cubic yards of sand were removed from the bar just below the lock by the 2d of June, and the dredge then moved above the lock and commenced the work of strengthening the levee at the Buffalo Lake; 3,456 cubic yards of sand and mud were taken from the bed of the lake and placed on the levee.

The number of yards removed from bars during the year, 81,561.

The number of yards placed on the levee, 3,456.

Dredge No. 7.—August 25, 1891, Dredge No. 7 was transferred from Fond du Lac to the entrance of Fox River into Lake Buttes des Morts. A cut 1,942 feet long and 30 feet in width was made through the bar, from which 9,231 cubic yards of sand were removed. The work was completed on the 10th of September, and the following day the dredge was towed back to Fond du Lac.

During the month of April, 1892, the dredge was put in good repair, and on the 15th of the month was towed from Oshkosh to Preachers Bend to widen the channel through the bar at that place; 8,456 cubic yards of clay were removed from the bar, which work having been completed on May 14 the dredge moved up to a point below Omro and took out 4,029 cubic yards of sand. The dredge was then towed

Statement of expenditures for fiscal year ending June 30, 1892, etc.—Continued.

Character of work, etc.	Item of expense.	Amount.	Total.
Kaukauna Canal.....	Labor.....	\$39.50	\$39.50
Lock house, Kaukauna Lock.....	Materials.....	336.40	
.....	Labor.....	193.90	530.36
Spid Croche Lock.....do.....	27.00	27.00
Spid Croche Dam.....	Labor and transportation.	154.75	154.75
.....	Materials.....	7.10	
Kaukauna Fifth Lock.....	Labor.....	20.06	27.16
Kaukauna Fifth Level.....do.....	279.80	279.80
Kaukauna Fourth Lock.....do.....	15.50	15.50
Kaukauna Fourth Level.....do.....	87.17	87.17
Kaukauna Third Lock.....do.....	4.00	4.00
Kaukauna Second Lock.....	Materials.....	24.00	28.50
.....	Labor.....	4.50	
Lock house, Kaukauna Second Lock.....	Materials.....	511.05	1,042.26
.....	Labor.....	531.21	
Kaukauna First Lock.....	Materials.....	953.69	1,465.73
.....	Labor.....	512.04	
Kaukauna Canal Banks.....	Materials.....	63.46	205.21
.....	Labor.....	141.75	
Lock house, Kaukauna First Lock.....	Materials.....	469.67	779.93
.....	Labor.....	310.26	
Little Chute Combined Locks.....	Materials.....	55.03	166.46
.....	Labor.....	111.43	
Little Chute Combined Locks Level.....do.....	165.44	165.44
Little Chute Second Lock.....	Materials.....	14.00	18.00
.....	Labor.....	4.00	
Little Chute Dam.....	Materials.....	3,786.25	5,348.19
.....	Labor.....	1,561.94	
Cedars Lock.....	Materials.....	3.60	3.60
Cedars Dam.....	Labor and transportation.	1,035.47	1,035.47
Appleton Fourth Lock.....	Materials.....	173.10	323.97
.....	Labor.....	150.87	
Appleton Fourth Level.....do.....	111.37	111.37
Appleton Canal Banks.....	Labor and transportation.	272.34	272.34
Lock house, Appleton Third Lock.....	Materials.....	134.61	419.91
.....	Labor.....	285.30	
Appleton Second Level.....do.....	23.00	23.00
Appleton First Lock.....	Materials.....	2.20	335.20
.....	Labor.....	333.00	
Menasha Dam.....	Materials.....	4.38	4.38
Menasha Lock.....do.....	1.25	1.25
.....do.....	97.77	257.16
.....	Labor.....	159.39	
.....	Materials.....	678.19	1,792.29
.....	Labor.....	1,114.20	
.....	Materials.....	123.82	806.68
.....	Labor.....	682.76	

Statement of expenses made from appropriation for operating and care of canals and locks of navigation, indefinite, act of July 5, 1884, applied to Fox River, Wis.

To whom paid.	For what paid.	Amount.
Jones & Langhins, Limited	Chain	\$355.24
A. L. Smith	Hire of house	45.00
Green Bay and Mississippi Canal Co.	Rent of quarry	75.00
Hired men	Services	93.56
John Jansen	Lumber	11.11
Butler Bros.	Shovels, etc.	14.75
L. Lindauer	Coal	35.61
M. V. Morsehouse	Boiler covering, etc.	86.94
Battis Bros.	Labor	1.20
George F. Stroud estate	Cotton waste, etc.	14.21
Doman & Manuel	Iron castings	16.32
Gillingham & Son	Iron, etc.	19.99
D. P. Sanford	Oil, etc.	21.56
F. H. Josslyn	Sheets, etc.	20.48
McKenzie & Crawford	Wood, etc.	67.65
J. A. Barnes	Iron castings, etc.	95.38
K. M. Hutchinson	Rope, etc.	114.13
W. H. Crawford	Iron pipe, etc.	183.18
Conlee Lumber Co.	Lumber	864.30
C. A. Peck	Oil, etc.	15.73
H. Stedman	Lumber	25.81
Niels Johnson	Boiler rivets, etc.	41.69
William Bannerman	Stone	60.00
The Morgan Co.	Lumber, etc.	36.60
A. Ross Houston	Services	200.00
James Clear	do	30.00
John M. Paige	do	30.00
Jerry Parkinson	do	25.00
John Lewis	do	25.00
George T. Allanson	do	30.00
Alexander Sims	do	25.00
John A. Banker	do	25.00
Gottlieb Jahnke	do	25.00
George Gifford	do	25.00
Gabriel Wick	do	25.00
John Baeten	do	25.00
Richard E. Rice	do	25.00
C. L. Neumann	do	10.00
Joy Bros. & Co.	Rope	11.41
Des Forges & Co.	Stationery	34.28
Samuel Whitney	Services	200.00
Hired men	Services, July, 1891	2,311.97
Jones & Langhins, Limited	Chain	41.20
Julius Lando	Tapelines	15.50
J. M. Harford	Services	14.43
Louis Clairmont	Moving stone	225.00
Paul E. Thomas	Services	65.00
Samuel Whitney	do	200.00
John M. Paige	do	30.00
Alexander Sims	do	25.00
George T. Allanson	do	30.00
George Gifford	do	25.00
Jerry Parkinson	do	25.00
John Lewis	do	25.00
James Clear	do	30.00
John A. Banker	do	25.00
John Baeten	do	25.00
Gabriel Wick	do	25.00
Richard E. Rice	do	25.00
Hired men	do	256.11
Gottlieb Jahnke	do	25.00
Butler Brothers	Nails	6.90
Wamer & Knitter	Bolts, etc.	12.37
John Jansen	Lumber	129.70
Schlafer, Barrett & Tesch	Drift bolts, etc.	258.96
K. M. Hutchinson	Rope	4.20
Gillingham & Son	Lumber	6.00
McKenzie & Crawford	Coal	13.01
Ole Olson	Towing	24.00
Conlee Lumber Company	Lumber	876.75
Mitchel Garrow	Clam poles	20.00
C. A. Peck	Lye, etc.	16.71
Niels Johnson	Iron, etc.	111.57
Charles S. Morris	Coal	209.15
Trest & Garrow	Wood	263.90
Neumann	Services	10.00
Hired men	Services, August, 1891	2,316.34
Neumann & Tate	Paper	1.50

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Itemized statement of expenses, etc.—Continued.

No. of voucher.	To whom paid.	For what paid.	Amount.
16	74 John Schwark	Brush	85.00
20	75 Samuel Whitney	Services	200.00
30	76 John M. Paige	do	30.00
30	77 Alexander Sims	do	25.00
30	78 John Baeten	do	25.00
30	79 George T. Allanson	do	30.00
30	80 George Gifford	do	25.00
30	81 John A. Banker	do	25.00
30	82 Gabriel Wick	do	25.00
30	83 Gottlieb Jahnke	do	25.00
30	84 Jerry Parkinson	do	25.00
30	85 John Lewis	do	25.00
30	86 Richard E. Rice	do	25.00
30	87 James Clear	do	30.00
30	88 C. L. Neumann	do	10.00
3	89 Samuel Whitney	Traveling expenses	10.50
3	90 Elwin Bauter	do	3.62
3	91 Hired men	Services, Nov., 1891	1,638.88
4	92 Charles M. Cole	Traveling expenses	9.94
4	93 W. M. Bambert	Labor	2.00
4	94 Jno. Jansen	Lumber	11.64
4	95 Butler Bros.	Paint, etc.	16.72
4	96 J. H. Marston & Co.	Mortar	1.20
4	97 Schlafer, Barrett & Tesch	Paint, etc.	24.75
4	98 Gerry Lumber Co.	Lumber, etc.	55.12
4	99 Ryan Brothers	Handling out boat	60.00
4	100 H. Stedman	Lumber	1.56
4	101 C. A. Peck	Lath yarn, etc.	18.25
4	102 Chas. S. Morris	Coal	83.54
4	103 Priest & Garrow	Wood	98.60
4	104 George Zuehls	Depositing dredged material	8.05
5	105 John Flynn	Services	8.43
9	106 Henry Weber	do	5.25
31	107 Ramsay & Jones	Lumber	793.38
31	108 John M. Paige	Services	35.00
31	109 Alexander Sims	do	35.00
31	110 James Clear	do	30.00
31	111 Gottlieb Jahnke	do	25.00
31	112 John Lewis	do	25.00
31	113 Gabriel Wick	do	25.00
31	114 George Gifford	do	25.00
31	115 Jerry Parkinson	do	25.00
31	116 John Baeten	do	25.00
31	117 John A. Banker	do	35.00
31	118 George T. Allanson	do	30.00
31	119 A. L. Smith	Hire of house	45.00
31	120 Ramsay & Jones	Lumber	873.28
31	121 Ossian Cook	Rent of office	66.00
31	122 A. Sanford Logging Tool Co.	Cant hooks, etc.	5.38
31	123 C. A. Peck	Sole leather, etc.	3.95
31	124 Warner Hardware Co.	Nails, etc.	8.69
31	125 A. J. Wier	Lumber, etc.	129.55
31	126 Eugene Dietzgen & Co.	Tracing cloth, etc.	8.56
31	127 The Keith Lumber Co.	Timber	274.40
31	128 Samuel Whitney	Services	200.00
31	129 Richard E. Rice	do	25.00
31	130 Samuel Whitney	Traveling expenses	10.52
1892.			
4	1 Jones & Laughlins, limited	Bolts	244.56
5	2 Hired men	Services, December, 1891	884.11
5	3 Elwin Bauter	Traveling expenses	7.86
5	4 E. O. Hoffmann	do	6.94
8	5 Des Forges & Co.	Stationery	8.45
30	1 A. J. Weir	Lumber	40.23
31	2 John M. Paige	Services	35.00
31	3 Alexander Sims	do	35.00
31	4 George Gifford	do	25.00
31	5 James Clear	do	30.00
31	6 George T. Allanson	do	30.00
31	7 John Baeten	do	25.00
31	8 Gottlieb Jahnke	do	25.00
31	9 John Lewis	do	25.00
31	10 Gabriel Wick	do	25.00
31	11 Jerry Parkinson	do	25.00
31	12 Richard E. Rice	do	25.00
31	13 John A. Banker	do	35.00
31	14 Samuel Whitney	do	200.00
31	15 E. O. Hoffmann	Traveling expenses	1.51
31	16 " "	do	10.37
		Services, January, 1892.	1,655.73
		with	13.12

Itemized statement of expenses, etc.—Continued.

No. of number.	To whom paid.	For what paid.	Amount.
25	Alexander Sims	Services	\$29.66
26	John A. Banker	do	30.00
27	James Clear	do	30.00
28	Richard E. Rice	do	25.00
29	John Lewis	do	25.00
30	Jerry Parkinson	do	25.00
31	Gottlieb Jahnke	do	25.00
32	Gabriel Wick	do	25.00
33	George Gifford	do	25.00
34	George T. Allanson	do	30.00
35	John Baeten	do	25.00
36	Joys Bros. & Co	Rope, etc	82.81
37	Samuel Whitney	Traveling expenses	9.34
38	Elwin Baeter	do	2.06
39	Engene Dietzen & Co	Tracing cloth	8.40
40	Doman & Manuel	Marine boiler	104.09
41	Excelsior Iron Works	Dipper-handle, shaft, etc	260.26
42	Samuel Whitney	Services	200.00
43	Des Forges & Co	Stationery	28.04
44	Hired men	Services, March, 1892	2,713.60
45	John Gevers	do	18.00
46	Burdick, Armitage & Allen	Notices of law	7.00
47	Hired men	Services	245.00
48	Samuel Whitney	do	200.00
49	John M. Paige	do	35.00
50	Alexander Sims	do	25.00
51	John A. Banker	do	25.00
52	James Clear	do	30.00
53	Richard E. Rice	do	25.00
54	John Lewis	do	25.00
55	Jerry Parkinson	do	25.00
56	Gottlieb Jahnke	do	25.00
57	Gabriel Wick	do	25.00
58	George Gifford	do	25.00
59	George T. Allanson	do	30.00
60	John Baeten	do	25.00
61	William Jansen	Sand	30.75
62	Louis Clairmont	Moving stone	579.70
63	Hubert Zegers	Lime	11.50
64	Luther Lindauer	Tile, etc	4.60
65	Charles Wheaton	Plastering, etc	80.13
66	Butler Bros	Locks, knobs, etc	25.75
67	Kaukauna Lumber and Mfg. Co	Lumber, etc	95.29
68	Schlafer, Barrett & Tesch	Nails, etc	13.60
69	J. H. Marston & Co	Moving stone	70.47
70	Gerry Lumber Co	Lumber	159.50
71	Julius Fleweger & Sons	Iron, etc	1.25
72	B. H. Soper & Co	Window shades	5.00
73	August Schroeder	Hire of jackscrews	7.25
74	Gillingham & Son	Screw bolts, etc	17.66
75	Hay Hardware Co	Files, etc	13.07
76	C. W. Johnston	Towing	18.00
77	Orville Beach	Rent of land	25.00
78	W. H. Crawford	Washout closet, etc	33.30
79	F. H. Josselyn	Carpet, etc	33.47
80	Stond & Thomson	Oakum, etc	35.50
81	Doman & Manuel	Iron castings, etc	36.11
82	The Morgan Co	Lumber, etc	39.23
83	D. P. Sanford	Oil	58.71
84	McKenzie & Crawford	Wood, etc	63.75
85	Hay Hardware Co	Rope, etc	89.31
86	W. H. Nichols and S. E. McPartlin	Boiler covering	84.41
87	H. M. Harmon	Painting	94.99
88	Doman & Manuel	Iron castings, etc	158.77
89	McKenzie & Crawford	Moving stone	455.77
90	Doman & Manuel	Horizontal engines, etc	1,937.00
91	August Ziemer	Lumber	3.28
92	John W. Slater	Oil, etc	8.27
93	Niels Johnson	Packing, etc	26.81
94	C. A. Peck	Soap, etc	32.87
95	H. Stedman	Coal, etc	179.42
96	F. T. Yahr	Lath, yarn, etc	27.01
97	Priest & Garrow	Wood	423.87
98	C. L. Neumann	Services	10.00
99	Samuel Whitney	Traveling expenses	5.32
100	Des Forges & Co	Stationery	13.80
101	Excelsior Iron Works	Steel pinions	30.40
102	Hired men	Services, May, 1892	3,871.17
	James F. Gregory	Mileage	17.44
	do men	Services, May, 1892	577.83
	do Zuehle	Depositing dredged material	14.00

APPENDIX J J.

MENT OF CHICAGO AND CALUMET HARBORS, ILLINOIS; OF
T RIVER, ILLINOIS AND INDIANA; AND OF ILLINOIS RIVER,
S; ILLINOIS AND MISSISSIPPI CANAL.

OF CAPTAIN W. L. MARSHALL, CORPS OF ENGINEERS, OFFICER
ARGE, FOR THE FISCAL YEAR ENDING JUNE 30, 1892, WITH
DOCUMENTS RELATING TO THE WORKS.

IMPROVEMENTS.

Harbor, Illinois.
Harbor, Illinois.
River, Illinois and Indiana.
River, Illinois.

5. Operating and care of La Grange Lock
and Dam, Illinois River, Illinois.
6. Illinois and Mississippi Canal.

UNITED STATES ENGINEER OFFICE,
Chicago, Ill., August 18, 1892.

AL: I have the honor to transmit herewith annual reports
works in my charge for the fiscal year ending June 30, 1892.

* * * * *

ry respectfully, your obedient servant,

W. L. MARSHALL,
Captain, Corps of Engineers.

en. THOMAS L. CASEY,
Chief of Engineers, U. S. A.

J J I.

IMPROVEMENT OF CHICAGO HARBOR, ILLINOIS.

sent project was adopted in 1870 and modified in 1878, and
ates—

formation of an outer harbor or basin by inclosing a portion
Michigan just south of and adjoining the entrance to the river,
purpose of increasing the harbor facilities of Chicago and to
f to the overcrowded river.

construction of an exterior breakwater of crib work filled with
side the outer harbor and north of the entrance to Chicago
p water, to shelter the entrance to Chicago River (which
ie harbor of Chicago) and the outer harbor from northerly

2,542.5 linear feet of superstructure 6 feet in height were completed at the close of the fiscal year. The north pier of Chicago Harbor and the breakwater and its return, 5,240.3 linear feet, are in fine order.

Assistant Engineer Liljencrantz herewith gives the

breakwater, outer harbor, and the south pier at the present time are in bad condition and now require renewal. This work is situated 1 mile northeast of the harbor, the entrance to which it covers from northerly to southerly in length.

The breakwater was begun in 1881 and completed in 1890. The breakwater, except the outer 1,200 linear feet, is of crib work 30 feet high. The natural bottom in water varying from 18 to 20 feet deep. The outer 1,200 linear feet are built upon a stone foundation. The breakwater answers the purpose for which it has been built, and is of great value as forming a safe harbor of refuge during storms, thus affording a sheltered entrance to the harbor of Chicago, and is used by all parties interested in the commerce of this harbor.

During the fiscal year no work was done upon this breakwater, and it is necessary to renew the superstructure over three-fourths of its length. An estimate is herewith submitted for that purpose.

Chicago River.—The Chicago River constitutes the inner harbor of Chicago, and it has hitherto been customary to periodically dredge the river lying between the bridge nearest the lake and the pier heads. The last dredging was done in 1892, and at that date no further dredging has been necessary, and none is now being done.

DISPOSITION OF FUNDS NOW ON HAND, AND THOSE ASKED FOR THE FISCAL YEAR ENDING JUNE 30, 1893.

The amount now on hand will be applied to the survey of Chicago Harbor, the completion of the works, the harbors created thereby, and the improvement of the river and harbor bill now pending, to the completion of the project as far as advisable and to renewing superstructure of 225 feet of the South Pier, Chicago Harbor entrance, and parts of the southerly breakwater, outer basin, and \$80,000 for the renewal of superstructures over 3,140 linear feet exterior breakwater and for engineering and contingent expenses.

Money statement.

Balance unexpended.....	\$96,003.20
1892, amount expended during fiscal year.....	94,393.43
Balance unexpended.....	1,609.77
Contingencies.....	184.76
Total.....	1,425.01
Approved July 13, 1892.....	72,000.00
Ending June 30, 1893.....	73,425.01

were 10 by 12 inches by 28 feet.

placed between the cross-ties in juxtaposition to the side walls and in correspondence to the end walls of the 50-foot cribs of the substructure, secured to the cross-ties in same manner as the timbers are in the side forming solid walls.

Timbers were made of 10 by 12 inch timber. They were placed between the cross-ties, running continuously through the length of the work, with 16 inches between them, so placed as to come between two ties, and with drift-bolts passing through the two parts of the scarf and the tie below.

Shimming pieces.—Before placing the first or lowest cross-ties it was ascertained that the top of the tie of the substructure was in the same plane as the side timbers, leaving thus an insufficient space for the bottom long-ties provided by the plans. These were therefore omitted and short blocks were placed in their places under the bottom ties. Owing to irregular settling of the timbers in most places, wedge-shaped sticks had to be resorted to frequently to give a horizontal bearing for the new work. All these and other pieces required to be secured by the effects of irregularities of the old work were counted under this head. The bolts were of round iron, 1½ inches in diameter. Thirty-two-inch bolts were used in the side timbers and solid cross walls; 20-inch bolts in ties and long-ties, and 18-inch bolts partly in the top timbers of side and cross walls.

The decking consisted in 6 by 10 inch timbers laid lengthwise and spiked with wrought-iron spikes to the ties.

The ends, most subjected to the process of decay, were protected by 3 by 12 inch timbers spiked down crosswise, covering the joints.

Shoring.—The new work was filled to within 3 feet of the top, which was sufficient for safety now, since these works are protected against north-wind by the exterior breakwater.

In the progress of the work stone removed from the old work was transferred to the old portions and thus no new stone was required for the part—the northerly end of the easterly breakwater—which was first completed, and the north pier, finished last, required the greatest amount of new stone.

At the easterly end of the north pier and the westerly end of the "return," the places had for a number of years been in a deplorable condition, from being obstructed by vessels running into them, these places were surrounded by a wall consisting of single closely driven rows of piles, held together with walings on either side of the piles and screw bolts through both, and attached to the means of 1½-inch iron tie-rods.

The space was filled with stone, and riprap was placed outside the piles to counteract the pressure of the stone filling. Clumps of 7 piles in each, held together by walings and 2 rounds of iron dredge chain, were placed as additional protection around the pier heads just described and around the north end of the easterly breakwater. When constructed, was surrounded by riprap to within 18 feet of the surface.

In connection with the work done under the terms of the contract some materials were furnished and secured in the work under a special verbal agreement, viz. 10,348 feet B. timber, 10 turnbuckles, and 3 piles of special length, 2 being 44 feet each and the other 43 feet long.

The timber was used for waling pieces in the pier heads and for snubbing posts in the pier. The turnbuckles were used on the tie-rods in the pier heads, and the clumps.

Shoring was done during the year and none is required for the present.

Condition of the works.—The easterly breakwater and the north pier, as well as the south pier of the southerly breakwater, which were rebuilt in 1889, are now in a state of decay. So are the pier heads and most of the clumps of piles built during the year. One of the latter, at the end of the north pier, has been broken up, probably by some vessel having run into it.

The pier heads frequently found a very strong current at the different pier ends, which, in some cases, makes their boats at such times unmanageable. This is the principal excuse for the many collisions of this kind in times past, and the principal reason why during the year were much needed and are of considerable value as

The north pier is in extremely bad condition. The greater part of the timbers in the pier structure are rotten and numerous pieces are even detached from the main work and are blowing down the stone filling to issue forth into the river channel. The cribs of this pier were sunk in 1869 and 1870 and the superstructure was built in the latter year, and is now only 22 years old. Considering its age it has held out well, no account of not being, like the breakwaters, exposed to severe winds.

The south pier was built in 1879 to 1880. This work is 3,000 feet in length and was the effect of southerly or southeasterly gales and of general decay

COMMERCIAL STATISTICS.

t of revenue collected at nearest port of entry (Chicago) during fiscal year, 0.17.

Arrivals and clearances of vessels during the year, port of Chicago.

	Arrived.		Cleared.	
	No.	Tons.	No.	Tons.
.....	5,222	3,581,859	5,351	3,702,645
.....	3,768	1,123,788	3,815	1,144,157
.....	8,990	4,705,647	9,166	4,846,802

Receipts and shipments by lake during calendar year 1891.

	Tons.	Shipments:	Tons.
.....	1,246,106	Flour	172,150
and iron ore.....	90,871	Grain	2,061,251
.....	12,590	Lard	21,463
.....	2,510	Meats.....	3,036
ber.....	2,746,219	Lead.....	70,028
ur, salt and sugar.....	132,032	Seed.....	155,196
ent.....	26,441	Oil and oil cake.....	36,013
atoes.....	6,613	Hay.....	9,647
cellaneous merchandise	281,956	Iron.....	4,077
		Miscellaneous merchandise	136,566
total.....	4,545,338	Total.....	2,669,427

J J 2.

IMPROVEMENT OF CALUMET HARBOR, ILLINOIS.

object of this work is to provide a deep entrance to Calumet and the port of South Chicago, Ill.

is effected in the usual manner by dredging a channel and projecting the dredged area by parallel piers, 300 feet apart, projecting into the river's mouth.

CONDITION OF THE WORK JUNE 30, 1892.

It was begun on this harbor in 1870, and at the beginning of the year 2,020 linear feet of the south pier, and 3,640 linear feet of the north pier had been constructed, which completed the existing project. The pier work is concerned, but the south pier should be prolonged to protect the channel from drifting sands.

Account of the present urgent necessity for maintaining the harbor ready done, the superstructure of the piers being rotten over an extent of 3,300 linear feet and in bad condition over 600 feet in addition. Estimate herewith submitted is entirely for maintenance, and no further extension of piers at the present time.

Under the harbor act of September 19, 1890, appropriated \$20,000 and at the beginning of the fiscal year work was in progress with W. A. McGillis & Co., of South Chicago, Ill.,

contracts for improving Calumet Harbor, Illinois, in force during the fiscal year ending June 30, 1892.

Address of contractor.	Nature of contract.	Date.	To expire.	Contract extended to—
McGillis & Co., South	Dredging 80,000 cubic yards at 13½ cents per cubic yard.	Dec. 10, 1890	Aug. 31, 1891	Nov. 30, 1891

completed October 16, 1891.

REPORT OF MR. G. A. M. LILJENCRANTZ, ASSISTANT ENGINEER.

UNITED STATES ENGINEER OFFICE,
Chicago, Ill., June 30, 1892.

I have the honor to submit herewith a report of operations in Calumet Illinois, during the fiscal year ending June 30, 1892.

The work done during the year consisted in continuing dredging between the harbor to a depth of 17 feet below the United States harbor datum, under contract with W. A. McGillis & Co., of South Chicago, Ill., dated December 10, 1890, at 13½ cents per cubic yard.

Under the contract commenced on the 27th of April, 1891, and was completed on the 16th of October following.

At the beginning of the fiscal year, 57,930 cubic yards of material were removed from the channel, which, together with the 29,087 cubic yards removed during the year from April 27 to June 30, 1891, or in all 87,017 cubic yards, constituted the total material dredged under the contract.

The work was done with the contractors' dipper dredge *A. B. Stetson*, a strong and reliable machine, with a dipper of 2½ cubic yards capacity. It was built two years

ago. The time devoted to this contract was 129.8 working days of 10 hours each, or

1,298 hours, 1,024 hours 20 minutes were spent in actual work, the remainder 273¾ hours 40 minutes representing time lost on account of repairs to plant, machinery, and other causes. The proportion of time utilized and time lost to the total time is accordingly:

	Hours.	Per cent.
Actual work	1,024½ =	78.92
.....	273¾ =	21.08
Available time	1,298 =	100.00

The average daily work amounted to 87,017 ÷ 129.8 = 670.4 cubic yards. The capacity of the dredge having been indicated as 84.95 cubic yards per hour, or 849.5 cubic yards per day of 10 hours of uninterrupted work.

The contract should have expired on the 31st of August, 1891, but an extension of the contract to the 30th of November of the same year, was, at the request of the contractor, granted by the Chief of Engineers, U. S. Army, on August 31, 1891.

The channel has now, as indicated on a map respectfully submitted herewith, a depth of at least 17 feet below the United States harbor datum, and a width of

at least 100 feet. The Illinois Steel Company's slip eastward to Lake Michigan, periodical maintenance has been done by the Steel Company, at their own expense, to secure a deep channel for the heavily laden ore vessels entering the slip at their

wharf. Work was done during the year, for want of funds, to the westerly ends of the pier, but this is very much needed. Neither was any new work done in extending the south pier, as per project heretofore submitted.

Work on the piers.—The stone filling placed in the outer, more exposed portions of the piers in 1888 is yet in very good condition, having settled but slightly.

in the approved dock lines have been dredged out and the proved, and it is understood that the ejection suit has been

for the specifications to the contracts heretofore made it has been found that the material to be encountered is mud, sand, and clay. As dredging progressed, however, the clay became more and more hard until finally it became so hard as apparently to require special machinery for its removal. Solid rock, apparently part of a narrow reef or shoal channel, was also encountered. After repeated vain attempts to remove this material, a supplementary agreement was made in 1892, by authority of the Secretary of War, by which the contract was modified so far as to require the contractors to dredge only dredgeable material only, within the proposed channel way. Under this supplementary agreement a section of the channel 650 feet long was not excavated to the full width and depth, but there is a depth of 10 feet in depth at shoalest and narrowest point, 80 feet in remainder of the 200-foot wide channel being from 11 to 16 feet, as shown upon the map accompanying this report. This depth is sufficient for all present requirements of navigation on this river for probably many years in the future. The amount of hardpan and rock so far revealed is too small to justify special appliances for its removal, and in the further prosecution of the work it is advisable to continue the dredging of all dredgeable material, and to define the hardpan and rock within the proposed channel. Whether material requiring blasting be discovered contracts may be based upon definitely known material and definite quantities. It is believed that the rock and hardpan is of great extent, but it is advantageous to the work to uncover it by dredging than to locate the limits of probably small areas of hard material by a few borings indefinitely multiplied over such an extent of the channel. It will be recommended, therefore, that future contracts shall be for dredgeable material, or such that may be removed without special appliances, until a sufficient amount of hard material to justify special appliances for its removal is discovered, or it is definitely ascertained that a sufficient amount now in sight covers all such material to be removed. The improvement of this river is attended by a gratifying increase in the commerce of the port of South Chicago and by the rapid increase of trading and shipping interests along its borders, and the influence of the work demanding either a radical widening and deepening of the river (other entrances to) Chicago River, which constitutes the inner harbor of Chicago, or a diminution of its commerce by water, seem to indicate a still more rapid development of the uses of Calumet Harbor for commerce by water.

In the several previous years the returns of the collector of customs indicated an annual increase in the tonnage of the port of South Chicago of about 30 per cent per annum. During the past fiscal year the returns indicate an increase of 47 per cent in tonnage, but only 10 per cent in the number of arrivals and departures of vessels, and the report only indicates the value and effect of the increased depth of the harbor and river, especially by increasing the draft of vessels trading here. Much of this increase, however, is due to the great improvements in plant and otherwise of the Illinois Steel Company's works near the mouth of the Calumet and to the construction of their own harbor north of the entrance to the Calumet River, which increased facilities for handling ores shipped by water have been gained.

nel, exclusive of superintendence and all incidental expenses, was making an average cost per mile of \$44,197.42.

Under the last contract has proved a very unprofitable one for the contractor on account of the loose, slushy material from the marshes adjacent running into completed sections of the same, requiring repeated dredging at the same place, frequently two and three times after having been completed and partly by the interference of one A. A. Westengard, who brought the contractors *et al.* for trespass on his alleged property, a great port was located in the natural river bed and covered by 12 feet of water at the stage of the river.

The great drawback, however, was the encountering of a stretch of hardpan places, underlying rock, which was found in the neighborhood of One twelfth street and for a distance of 800 feet along the channel. For the first of this distance, the northerly half, the hardpan reached across the width of the channel, diminishing gradually in extent southward.

The hardpan was struck at an average depth of 12 feet below the United States surface, underlying rock being found in patches, rather irregular both in extent and depth.

The contractors labored faithfully with the removal of the hardpan, applying whatever that could be devised and applied to either the elevator or the dipper dredge in this work, but with the most discouraging results. The two kinds of these dredges was repeatedly broken, and much more time and expense spent in repair of machinery than in doing actual work, until the contractors asked, in a letter dated May 5, 1892, to be relieved from the work on rock and hardpan found in the section covered by the contract, which was granted by the honorable the Secretary of War on the 4th of June following.

In order to ascertain the condition of the bottom of the river, to ascertain as near as practicable the extent of the rock and hardpan to be found within the limits of the area contracted for, a sounding was made in April by Mr. Paul Heinze, overseer.

The sounding indicated the existence of 1,332 cubic yards of rock and 14,459 cubic yards of hardpan. The hardpan was extremely hard to penetrate with the pointed steel dredge for the purpose, and it was only with the utmost exertions that it could be removed.

At the very low price at which the dredging was done, 12½ cents per cubic yard, the difficulties encountered by the contractors felt all the more.

In view of the removal of rock and hardpan, the contractors have been relieved in excavating the overlying strata of mud, sand, and clay. Even this has proved expensive. The elevator dredge did the greater part of this work, the hardpan being impenetrable by the spud of that dredge, around which it was working, it was found necessary to make use of the dipper dredge for the purpose of holding it in position while working.

By using the strength of and otherwise modifying various parts of the dredge, as has been from time to time suggested by the experience of two years, this dredge has done very good work.

It is entirely impossible, however, to present any data indicating its actual cost for the following reasons:

The dredges have worked at times alternately, at other times simultaneously, in different sections of the river. The material removed has been measured "in place" in scows, in which latter case, if used, the work done by each dredge would be estimated separately.

The material running into a completed portion of the channel to a very great extent necessitated (1) the redredging of such portion on some occasions as often as three times, the dredge thus handling the same material several times; (2) the dredging to a depth of generally not less than 2 feet below that required by the contract, to make sure of leaving the required depth at the completion reported for examination. Besides these reasons it may also be mentioned that frequently, after the material along the sides of the projected channel had been removed by the elevator dredge, the dipper dredge was employed in transferring material from the center of the channel to the excavated trenches on the sides. It was afterwards rehandled by the former dredge, which, by means of a conveyor and a powerful stream of water, would remove it far enough beyond the channel lines to prevent its returning into the completed channel.

In view of the experience of the last year, it is reasonable to expect that no such success in dredging in this river will be obtained as has been heretofore, unless a provision is made in the next specifications, which will hold the contractor from excessive loss, should rock and hardpan be encountered again, which is not probable according to appearances, still must be (and certainly will be) considered as possible.

COMMERCIAL STATISTICS.

The river and harbor are so closely connected in their commercial statistics are given in this place, but will be found under the Harbor. There is practically no navigation on the river between the harbor. As soon, however, as the river channel is opened by the locks and business along the river will develop very fast. It awaits

J J 4.

IMPROVEMENT OF ILLINOIS RIVER, ILLINOIS.

The main object of this improvement is to furnish a through route of navigation by water from the southern end of Lake Michigan to the Mississippi River, of sufficient capacity for its navigation by the class of Mississippi River steamboats that can reach the mouth of the Illinois River.

Dimensions that have been adopted for the locks to meet this requirement are:

	Feet.
Lock chamber	350
Chamber	75
Low water over sills	7

The project now under execution is for the improvement of the lower portion of this route, extending from the mouth of Copperas Creek to the Mississippi River, a distance of 137 miles. Another section of the route, 5 miles in length, from the mouth of Copperas Creek to La Salle, Ill., is improved by the State of Illinois by the construction of two locks and dams, one at Copperas Creek and one at Henry, Ill.

In the latter section the State still collects tolls, the United States Government having accepted the conditions imposed by the act of cession of the Illinois legislature, which conditions are such as to deprive the works of their value while demanding a great expenditure by the United States Government for works to be substituted therefor.

In regard to this subject reference is made to the Annual Report of the Chief of Engineers for 1889, page 2121, *et sequitur*.

Under the act of Congress of August 11, 1888, surveys and separate estimates have been made, based upon low-water depths of 8 and 14 feet, for carrying the improvement from La Salle to Lake Michigan via the Illinois and Desplaines Rivers, and a cut across the Chicago divide, in which section there is a fall in water surfaces, Lake Michigan being the upper limit, of about 141 feet.

The report upon this survey and estimates has been printed as House Report No. 264, Fifty-first Congress, first session, and is also published in the Annual Report of the Chief of Engineers for 1890, page 2419 and following pages.

Additional surveys and estimates have also been made by the trustees of the sanitary district of Chicago, organized under laws of the State of Illinois, for cutting a capacious channel carrying a large discharge from the Chicago River to the Illinois River for drainage and sanitary purposes. The Chief of Engineers, U. S. Army, for the reports and estimates of engineers acting in connection with the city of Chicago, and later under the trustees of the sanitary district of Chicago, relating to this matter will be found.

etc. Twelve thousand five hundred cubic yards of material were used from the approaches to the lock. The completion of this lock ended the tonnage passing this point, even with a less number of

at *Kampsville*.—The remainder of the mud overlying the foundation constructed in 1853 was removed, i. e., 9,000 cubic yards; making in all 14 cubic yards of deposit removed since 1833. Trestles 1,170 feet in length for stone travelers for laying the lock walls were erected. Also trestles for tramway track for delivering stone at lock walls 1,260 linear feet in length were constructed.

The laying of masonry began September 5, 1891, and was prosecuted until November 30, 1891. A greater part of the time work was prosecuted day and night in laying masonry. Ten thousand one hundred and fifty-five cubic yards of masonry were laid in the lock walls, commencing with the upper course L and lower courses of the lock. The foundation (pile, grillage, and grillage) for the abutment of the dam was constructed. The abutment laid to a height of 10 feet above the foundation. One thousand seven hundred and seventy-nine cubic yards of material were dredged above and below the lock pit. One thousand and fifty-four cubic yards of cut stone were purchased under contract, of which 788 cubic yards have been delivered at the present time; 6,915 cubic yards of rubble stone for the dam were purchased and delivered.

The purchase of the timber, iron and steel for the lock gates and maneuvering gear of the lock has been contracted for. Also, one-third of the material for the dam, but none of this material has yet been delivered. The floating plant has been kept in repair as well as may be, but it is nearly unserviceable as far as floating plant is concerned. The appropriations for this work have been so small and made at such intervals that the material has decayed, but not worn out, before the work for which it was intended, i. e., the dredging and channel work, has been more than

required. The dredging required can be done a complete new dredging plant must be purchased as far as hulls of floating plant is concerned. The greater part of the machinery is good.

It has been necessary to begin the construction of new dump scows, of which one is practically completed and the other under way. Six more will also have to be built before the dam can be constructed.

The water level of the river during the past spring, and until now, has attained and is at an unusually high stage, above all stages recorded since 1853. The water still remains above the banks and nearly daily rains afford little hope of a speedy decline.

PROPOSED APPLICATION OF FUNDS NOW ON HAND.

The funds on hand will be expended in completing the lock masonry, the upper floor, and lock gates, and purchasing the material for the dam.

PROPOSED APPLICATION OF FUNDS ASKED FOR, FOR FISCAL YEAR ENDING JUNE 30, 1894.

For the funds asked for herein to the completion of the Lock, and to dredging the river between the Grange and Kampsville, and as far as practicable the pools

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*Constructing Kampsville Lock and Dam, Illinois River, in force
the fiscal year ending June 30, 1892.*

Nature of contract.	Date.	To expire.
Furnishing and delivering at lock site 1,254 cubic yards of stone.	Feb. 10, 1891	*Nov. 30, 1891
Furnishing 197,790 pounds iron and steel fittings for lock gates.	Apr. 11, 1892	Aug. 1, 1892
Furnishing 246,722 feet, B. M., lumber.	May 31, 1892	Sept. 1, 1892
Furnishing 66,986 feet, B. M., lumber.	June 15, 1892	42 days sub- sequent to notification.

* Contract extended to August 31, 1892.

OF MR. C. V. BRAINARD, ASSISTANT ENGINEER.

UNITED STATES ENGINEER OFFICE,
Kampsville, Ill., June 30, 1892.

In honor to submit the following report of operations for im-
provement for the year ending June 30, 1892.

At the beginning of the fiscal year about 9,000 cubic yards of
dirt were cleared off the foundation. After removing 1,350 cubic yards
from the side of the cofferdam was closed and the water pumped
out from the wall sites was then removed. A part of this was
hoisted over the cofferdam. The most of it, however, was wheeled
into scale boxes, and hoisted outside. About 1,650 cubic yards
of dirt were hoisted. After the walls were partially up about 3,000 cubic yards,
in the center of the foundation, was wheeled back of the land
together about 34,000 cubic yards of dirt removed from the lock

and fifty-four piles and 54 sheet piles were driven. Sixteen piles and
driving the opening in the cofferdam, 228 piles for the traveler and
and 6 piles for a derrick support. There was built 1,170 linear feet
for laying the lock walls and 1,260 linear feet of railroad trestle
at the walls.

At the stone pile 16 of the old elm piles were replaced by
timbers.

For the railroad track with the necessary switches were laid from
the walls for the delivery of stone at the work.

Four hundred and twenty-six square yards of the foundation were
of concrete varying from 0.1 foot to 0.25 foot in thickness.

Setting the walls was begun September 5, 1891, and continued until
on the "L" course, making the walls 20 feet and 3 inches high, was
setting both miter sills and the breast wall, except the coping of the

From September 29 until November 18 the work was carried on
day and night. Night work on the walls was stopped at this time on
freezing weather at night. During this time, 4,610 cubic yards of
cubic yards of backing stone were set.

Two yards of broken stone were used for concrete in filling the joints

On the walls three overhead travelers were used, one over each wall and
of cut stone. Two derricks with a steam hoist handled the backing
stone. A railroad track about 10 feet above the foundation runs along
each wall for its entire length, the stone being brought down on cars
opposite where it belongs in the wall. It was unloaded by the traveler
and the wall to be used when needed.

The work was carried on by the aid of nine Wells lights of 1,200 candle power
the time these lights burned they averaged 2 barrels of coal oil each

and ninety-one pieces of quarry stone for the river wall were recut
giving them a batter of 3 inches to the foot. Twenty-four pieces of

This recutting was made necessary by a change in the plans of
as a part of the stone had been delivered. The lower miter sill
was set and bolted into position.

Work on dredging above and below the lock was begun and continued
which time no dredging has been done because of high water.
Work was carried on day and night. From the 20th to the

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of Illinois River at Kampsville, Ill., October 20, 1891.

	Partial areas.	No. of registrations.			Mid-depth velocities.	Mean velocities.	Partial discharges.	Remarks.
		For 300 seconds.		Per second.				
		Sec-onds.	Regis-trations.					
Shore.								
9	94.40	158	12	.076	.329	.309	29.2	Gauge.—7 a. m., —0.25; noon,—0.25; 6 p. m.,—0.25. Time, a. m. From west to east. Brisk breeze upstream. W. M. Childs, observer.
1.1	88.50	99	19	.192	.866	.857	75.8	
1.1	91.50	125	24	.192	.866	.857	78.4	
1.0	92.25	110	20	.182	.820	.810	74.7	
1.1	94.50	109	18	.165	.741	.729	68.9	
1.1	93.00	122	18	.148	.662	.647	60.2	
1.6	79.50	138	20	.145	.649	.635	50.5	
1.3	69.75	126	15	.119	.528	.508	35.4	
1.1	63.00	117	16	.137	.612	.592	37.3	
1.2	73.50	136	13	.096	.422	.401	28.5	
4.2	128.25	116	10	.086	.376	.359	46.0	
6.9	199.50	122	15	.123	.547	.539	107.5	
9.7	276.75	140	22	.157	.704	.704	194.8	
10.1	285.75	168	19	.176	.792	.795	227.2	
6.0	189.00	131	26	.190	.898	.890	169.9	
4.6	138.75	125	21	.168	.755	.747	109.6	
4.2	118.50	121	15	.124	.552	.538	63.8	
1.8	47.10	120						
Shore.								

cross section.....square feet.. 2,223.5
 rgecubic feet per second.. 1,452.7
 y.....feet per second.. 0.653



Readings of the upper gauge at Copperas Creek Lock, 1891.

[Plane of reference: Top of lower miter sill.]

	Jan.	Feb.	Mar.	Apr.	May.	June.	July.	Aug.	Sept.	Oct.	Nov.	Dec.
<i>Feet.</i>	<i>Feet.</i>	<i>Feet.</i>	<i>Feet.</i>	<i>Feet.</i>	<i>Feet.</i>	<i>Feet.</i>	<i>Feet.</i>	<i>Feet.</i>	<i>Feet.</i>	<i>Feet.</i>	<i>Feet.</i>	<i>Feet.</i>
12.18	11.80	13.78	14.68	17.70	12.20	13.40	11.70	11.60	11.50	11.40	11.40	12.10
12.20	11.80	13.85	15.02	17.55	12.15	13.22	11.75	11.60	11.50	11.40	11.40	12.10
12.25	11.85	13.93	15.20	17.35	12.25	12.97	11.75	11.60	11.50	11.40	11.40	12.10
12.30	11.95	13.95	15.33	17.10	12.30	12.80	11.75	11.60	11.45	11.40	11.40	12.10
12.30	11.98	13.88	15.45	17.02	12.27	12.75	11.70	11.60	11.40	11.40	11.40	12.10
12.30	12.00	13.78	15.55	16.85	12.25	12.60	11.70	11.60	11.40	11.40	11.40	12.30
12.30	12.00	13.72	15.72	16.60	12.37	12.50	11.70	11.60	11.40	11.40	11.40	12.40
12.30	12.00	13.70	15.80	16.23	12.37	12.55	11.70	11.60	11.40	11.60	11.60	12.40
12.30	12.00	13.67	15.83	16.07	12.55	12.45	11.70	11.55	11.40	11.60	11.60	12.40
12.30	12.00	13.40	16.07	15.87	12.65	12.45	11.70	11.50	11.40	11.60	11.60	12.40
12.30	12.00	13.40	16.10	15.68	12.80	12.35	11.70	11.45	11.40	11.60	11.60	12.40
12.22	12.00	13.40	16.57	15.53	12.85	12.30	11.70	11.45	11.40	11.60	11.60	12.30
12.20	12.00	13.40	16.93	15.33	12.90	12.20	11.70	11.50	11.40	11.60	11.60	12.40
12.15	12.00	13.33	17.17	15.12	12.90	12.17	11.78	11.50	11.40	11.60	11.60	12.45
12.10	12.00	13.35	17.53	14.87	13.00	12.02	12.00	11.50	11.40	11.60	11.60	12.45
12.05	12.00	13.35	17.75	14.68	13.00	12.00	11.70	11.50	11.40	11.60	11.60	12.50
12.05	12.00	13.35	18.05	14.37	13.07	11.97	11.70	11.50	11.40	11.60	11.60	12.50
12.05	11.95	13.48	18.25	14.07	13.50	12.00	11.70	11.50	11.25	11.60	11.60	12.50
12.00	12.00	13.62	18.45	13.83	13.55	12.00	11.70	11.50	11.20	11.60	11.60	12.50
12.00	12.32	13.73	18.57	13.68	13.70	12.00	11.70	11.50	11.15	11.67	11.67	12.50
12.00	12.40	13.77	18.70	13.45	13.72	12.00	11.70	11.50	11.15	11.72	11.72	12.50
11.90	12.57	13.80	18.73	13.42	13.80	12.00	11.60	11.50	11.20	11.80	11.80	12.53
11.90	12.68	13.80	18.80	13.20	13.90	12.00	11.60	11.55	11.20	11.85	11.85	12.50
11.90	12.85	13.92	18.77	13.10	13.95	11.90	11.60	11.60	11.20	11.95	11.95	12.55
11.85	13.20	14.02	18.73	13.00	14.00	11.90	11.60	11.65	11.30	12.00	12.00	12.55
11.85	13.43	14.07	18.57	12.70	13.80	11.90	11.60	11.65	11.30	12.05	12.05	12.55
11.85	13.62	14.37	18.50	12.50	13.75	11.85	11.60	11.60	11.25	12.05	12.05	12.55
11.85	13.75	14.45	18.37	12.50	13.70	11.80	11.60	11.60	11.25	12.05	12.05	12.60
11.80	14.02	14.52	18.17	12.45	13.60	11.75	11.60	11.60	11.40	12.05	12.05	12.60
11.80	14.70	17.92	12.37	13.50	11.70	11.60	11.50	11.40	12.08	12.08	12.08	12.60
11.80	14.75	12.23	12.23	11.70	11.60	11.60	11.60	11.40	12.08	12.08	12.08	12.60
mas	12.07	12.29	13.81	17.18	14.72	13.07	12.23	11.68	11.55	11.36	11.68	12.42

Readings of the lower gauge at Copperas Creek Lock, 1891.

[Plane of reference: Top of lower miter sill.]

	Jan.	Feb.	Mar.	Apr.	May.	June.	July.	Aug.	Sept.	Oct.	Nov.	Dec.
<i>Feet.</i>	<i>Feet.</i>	<i>Feet.</i>	<i>Feet.</i>	<i>Feet.</i>	<i>Feet.</i>	<i>Feet.</i>	<i>Feet.</i>	<i>Feet.</i>	<i>Feet.</i>	<i>Feet.</i>	<i>Feet.</i>	<i>Feet.</i>
8.65	8.00	12.72	14.53	17.58	10.25	12.53	7.50	7.78	7.08	6.95	6.95	8.65
8.70	8.00	12.85	14.67	17.40	10.10	12.25	7.58	7.70	7.00	6.95	6.95	8.65
8.80	8.10	12.98	14.82	17.23	10.73	11.93	7.62	7.73	7.00	6.95	6.95	8.70
8.87	8.20	13.05	15.00	17.03	10.95	11.75	7.65	7.70	7.00	6.95	6.95	8.80
8.93	8.33	13.05	15.10	17.00	10.75	11.60	7.55	7.70	7.00	6.95	6.95	8.90
9.00	8.40	13.00	15.22	16.97	10.30	11.50	7.55	7.70	7.00	6.95	6.95	9.00
9.10	8.40	12.87	15.38	16.60	10.30	11.20	7.55	7.65	7.00	6.95	6.95	9.00
9.17	8.38	12.80	15.40	16.13	10.42	11.23	7.55	7.57	7.00	6.98	6.98	9.10
9.20	8.35	12.80	15.45	15.73	10.60	11.00	7.53	7.55	7.00	7.00	7.00	9.10
9.20	8.30	12.60	15.75	15.40	10.77	10.87	7.52	7.50	7.00	7.00	7.00	9.10
9.20	8.28	12.50	15.80	15.10	11.00	10.47	7.55	7.40	7.00	7.00	7.00	9.10
9.17	8.20	12.40	16.23	14.83	11.15	10.38	7.55	7.40	7.00	7.05	7.05	9.10
9.10	8.13	12.40	16.55	14.70	11.42	10.18	7.60	7.40	6.97	7.08	7.08	9.10
9.07	8.10	12.40	16.93	14.40	11.50	10.08	7.93	7.40	6.95	7.13	7.13	9.17
9.00	8.10	12.40	17.33	14.30	11.73	9.67	8.60	7.40	6.88	7.20	7.20	9.15
8.83	8.10	12.40	17.60	14.20	11.85	9.37	9.00	7.37	6.77	7.20	7.20	9.15
8.80	8.10	12.40	17.90	13.93	11.85	9.25	9.50	7.30	6.80	7.18	7.18	9.27
8.75	8.10	12.62	18.17	13.78	12.83	9.25	9.50	7.30	6.80	7.15	7.15	9.40
8.65	8.13	12.75	18.35	13.48	12.80	9.18	9.40	7.30	6.80	7.17	7.17	9.40
8.60	8.73	13.03	18.42	13.28	12.90	9.03	9.30	7.27	6.80	7.35	7.35	9.40
8.47	9.47	13.02	18.50	13.15	12.98	8.90	9.17	7.25	6.85	7.77	7.77	9.40
8.40	10.13	13.00	18.55	13.00	13.20	8.73	9.00	7.25	6.90	8.00	8.00	9.45
8.32	10.70	13.00	18.60	12.80	13.30	8.47	8.77	7.25	6.90	8.05	8.05	9.50
8.22	11.02	13.12	18.55	12.73	13.33	8.20	8.57	7.25	6.90	8.20	8.20	9.60
8.20	11.58	13.35	18.50	12.52	13.33	8.00	8.25	7.25	6.90	8.35	8.35	9.60
8.18	11.93	13.42	18.40	12.22	13.20	8.00	7.92	7.25	6.90	8.50	8.50	9.60
8.10	12.28	13.75	18.35	11.80	13.15	7.93	7.72	7.20	6.95	8.50	8.50	9.60
8.05	12.63	13.87	18.22	11.47	13.10	7.80	7.80	7.20	6.95	8.50	8.50	9.60
8.00	13.97	18.03	11.12	12.95	7.75	7.80	7.15	6.95	8.50	8.50	8.50	9.60
8.00	14.10	17.78	10.73	12.72	7.65	7.80	7.13	6.95	8.63	8.63	8.63	9.60
8.00	14.28	10.47	10.47	7.50	7.80	7.80	6.95	6.95	8.63	8.63	8.63	9.60
	1.67	9.08	13.00	16.94	14.23	11.85	9.73	8.13	7.41	6.93	7.47	9.24

Readings of the gauge at Kampsville Lock.

[Plane of reference: Low water of 1879.]

Jan.	Feb.	Mar.	April.	May.	June.	July.	Aug.	Sept.	Oct.	Nov.	Dec.
Feet.	Feet.	Feet.	Feet.	Feet.	Feet.	Feet.	Feet.	Feet.	Feet.	Feet.	Feet.
0.57	0.82	3.68	8.97	11.47	3.98	4.65	1.05	0.93	0.00	-0.20	1.40
.65	.82	3.80	8.97	11.42	3.67	4.80	.95	.92	0.00	-.25	1.38
.78	.92	3.85	8.78	11.35	3.40	5.00	1.17	.88	0.00	-.20	1.15
1.17	1.15	4.17	8.67	11.28	3.43	5.20	1.32	.77	-0.02	-.15	1.30
1.42	1.17	4.07	8.65	11.17	3.65	5.23	1.30	.68	-.10	-.10	1.12
1.60	1.10	4.10	8.72	11.00	3.92	5.03	1.10	.62	-.10	-.08	1.12
1.65	.95	4.10	8.83	10.87	3.88	4.52	.90	.58	-.05	-.10	1.32
1.57	.82	4.17	8.98	10.70	3.87	4.17	.75	.53	-.05	-.05	1.40
1.55	.88	4.22	9.08	10.50	4.13	3.95	.63	.50	0.00	.05	1.50
1.45	.93	4.27	9.30	10.25	4.55	3.90	.53	.45	0.00	.12	1.55
1.35	.88	4.30	9.57	10.01	4.78	3.92	.55	.40	0.00	.15	1.62
1.40	.87	4.30	9.88	9.80	5.08	3.80	.82	.35	-.05	.15	1.63
1.57	.90	4.27	9.95	9.47	4.83	3.52	1.05	.35	-.05	.10	1.70
1.60	.90	4.23	9.83	9.15	4.17	3.18	1.00	.30	-.07	.05	1.75
1.45	.93	4.20	9.63	8.85	3.75	3.02	.97	.30	-.12	.12	1.82
1.46	.95	4.15	9.55	8.45	3.67	3.08	1.15	.23	-.20	.32	1.85
1.38	.95	4.15	9.87	8.00	3.70	2.92	1.80	.20	-.20	.37	1.85
1.30	.90	4.22	10.57	7.53	3.75	2.83	3.10	.20	-.15	.45	1.92
1.23	.92	4.35	10.75	7.07	3.77	2.48	3.35	.18	-.23	.45	1.98
1.15	1.37	4.50	11.12	6.68	3.92	2.20	3.23	.13	-.25	.53	2.03
1.15	1.95	4.75	11.32	6.67	4.28	1.95	3.05	.12	-.23	.60	2.05
1.10	2.40	5.08	11.43	7.47	4.55	1.88	2.90	.12	-.15	.82	2.05
1.02	2.95	5.40	11.68	7.10	4.87	1.72	2.55	.10	-.15	1.55	2.03
.95	3.30	6.10	11.97	6.75	5.18	1.58	2.15	.10	-.15	1.70	2.02
.90	3.30	6.88	12.13	6.20	5.35	1.35	1.77	.10	-.10	1.85	2.12
.90	3.27	7.31	12.15	5.68	5.28	1.23	1.47	.10	-.05	1.55	2.12
.87	3.42	7.57	11.98	5.33	5.02	1.18	1.20	.10	-.05	1.53	2.00
.90	3.52	7.92	11.87	5.07	4.85	1.05	1.02	.10	-.08	1.48	2.00
.92	8.38	11.73	4.85	4.72	1.07	1.02	.10	-.05	1.35	2.05
.78	8.72	11.57	4.60	4.68	1.17	1.05	.05	-.15	1.20	2.02
.75	8.85	4.25	1.20	1.03	-.15	2.00
1.18	1.54	5.16	10.25	8.36	4.29	2.90	1.48	0.35	-0.09	0.51	1.74

COMMERCIAL STATISTICS.

owing figures are taken from the annual report of the Merchants' Exchange, Mo., for calendar year 1891:

and departures of steamboats and barges at St. Louis, Mo., via Illinois River.

Month.	Arrivals.	Departures.	Month.	Arrivals.	Departures.
.....	August	20	19
.....	9	7	September	11	9
.....	22	19	October	3
.....	15	11	November	9	8
.....	11	10	December	1
.....	14	15	Total	115	98

receipts and shipments, in tons, via Illinois River, at St. Louis, Mo., 1891.

Month.	Receipts.	Shipments.	Month.	Receipts.	Shipments.
.....	August	2,675	675
.....	3,340	550	September	880	490
.....	14,310	670	October	250
.....	3,655	620	November	1,610
.....	1,300	505	December
.....	3,170	795	Total	31,190	4,305

2. PRICES FOR WORK.

work in narrow railway cuttings, where disposal of material is difficult, is done at 70 to 90 cents per yard.

Now being quarried at Lemont for \$2 per cord of 13,000 pounds, or 100 solid. This is at the rate of 54 cents per yard. It is delivered on board barges at 1.08 per yard and has actually been delivered in this city for \$1.62 per yard, cord.

At wharves on the Mississippi River is actually delivered on board barges 75 cents per loose yard for quarries on the banks of rivers.

Amsterdam Canal of Holland (1865-'76) 21,000,000 yards of material were at an average rate of about 4 cents per yard.

St. Petersburg Canal (1878-'85) 63,000,000 yards were moved for about 5 cents per yard.

St. Lawrence has been deepened over a length of 40 miles between Montreal and Quebec, from a depth of 10½ feet to 27½ feet; material clay, sand, hardpan, and some ledge rock. Aggregate cost for last ten years work, 13 cents per yard. The clay has been down at from 3 to 6 cents per yard.

Records of several dredges on harbor works in Europe and Australia gives rates of 4 to 6 cents per yard in free material and at American prices.

Lake Erie and Ohio River ship canal commissioners of Pennsylvania recently reported on a canal 100 miles long from Conneaut on Lake Erie to Beaver on the Ohio River. Total excavation 43,282,475 yards, at \$8,656,495, or an average of 20 cents per yard for all classes of material. Much of the work was estimated at 12 cents per yard.

Manchester Ship Canal involves 47,250,000 yards, one-sixth of which is rock. Total cost of canal complete, \$30,000,000. Less than half this amount is for excavation; the figures are not reported.

North Sea (Holstein) Canal involves 67,000,000 yards of excavation, and the total cost is estimated at \$39,000,000. No details as to cost of excavation are given.

Harbor work in the United States is being done at 10 to 20 cents in limited cases.

Information available indicates that with special machinery and full preparatory work can be done as cheaply as similar work has ever been done, or well below the prices of the Hering Commission, viz, 15 cents for clay, 30 cents for diffcult digging, and 75 cents for rock.

Costs for various channels.—Channel from Ashland avenue to Lockport, 30 miles.

Capacity per minute.	Width.	Depth.	Grade per mile.	Excavation.	Cost of excavation.
Feet.	Feet.	Feet.	Inches.	Cubic yards.	
210,000	160	10	6.84	22,196,000	\$9,917,000
300,000	160	10	15.60	32,231,000	15,817,000
300,000	160	14	4.44	22,882,000	9,721,500
600,000	160	18	7.00	29,685,000	13,136,000
600,000	160	21.6	3.62	29,086,000	12,263,000
600,000	200	18	4.00	30,169,000	14,931,400
600,000	180	18	5.00	37,914,830	14,228,000

* Hering Commission.

† Worthen-Newton.

Cost of rock, 75 cents; hard material, 30 cents; clay 15 cents. All the estimates are on the same basis as to prices per yard.

Between Ashland avenue and Lockport, right of way, bridging, and miscellaneous would add \$5,000,000 to each of the foregoing estimates for the total. One million may be added for passing the volume of water from Lockport to Lake Joliet.

At Joliet the entire value of landed property throughout the Illinois Valley is estimated at less than \$3,000,000. Any damage from overflow will be but a fraction of this.

It will be noticed that a channel 10 feet deep for either 210,000 or 300,000 cubic feet is more expensive than one 14 feet deep. It will also be noticed that a channel to 300,000 cubic feet and 21.6 feet deep is cheaper than a channel 18 feet deep and same capacity. It will also be noticed that to double the capacity from 300,000 cubic feet only adds \$2,541,500, or some 17 per cent to the total between Chicago and

When these results are due to grade and its effect on the amount of rock excavated at the lower end of the channel.

The proportion of channel can only be told after full borings have been made.

That the Hering channel, with a width of 200 feet, cost little

more than the Worthen-Newton channel with a width of 180 feet. This estimate involves more rock than is since found to exist and is accordingly high.

The actual channel is not likely to exceed these estimates, as the actual rock is less than that assumed.

By comparing the quantities with the total for the Manchester Canal, some idea of outside limit of cost may be obtained. That work involves a vast amount of docking, bridging, and locks, and is greater than a channel 25 feet deep carried clear to Lake Joliet, complete with locks and revetment.

4. CAPACITY FOR DILUTION OF SEWAGE.

The best results of investigation up to the time the law was passed placed the dilution at 20,000 cubic feet for each 100,000 inhabitants, as needed for a sanitary condition, and the probable population at 3,000,000 in 25 years.

During the past season the canal carried about 50,000 cubic feet per minute up to December last. The amount of organic matter carried out by the canal at that time was about 250 tons per day. This was not far from the amount going into the Soccer Fork alone at that time, and was probably less than one-fourth of the total organic waste produced by the city as sewerage. It cannot be definitely known until a sanitary survey is carried over the city.

It may be stated that if all the sewage of the city was made tributary to the canal before decomposition had set in, then a volume of 200,000 cubic feet per minute would now found to exist in the canal.

The provisions of the law do not require the necessary dilution, and it may be wise to provide the full amount of 600,000 cubic feet from the beginning.

No engineer of this Board, nor of any previous investigations, have recorded an opinion that the dilution mentioned is not required for a sanitary condition, and no engineer has said that with this dilution, the Illinois River would be fit to drink.

5. AVAILABLE REVENUE.

Assume that the assessed valuation for 1891 will be \$220,000,000, and that the valuation increase at the rate of 5 per cent each year, which is certainly a conservative estimate, at the end of ten years the valuation would be \$341,292,000. This would permit a bond issue of about \$17,000,000 in ten years were it not limited in the law to \$15,000,000.

If these bonds are issued at the rate of \$1,500,000 per year for ten years, and the rate of taxation of one-half of 1 per cent is applied, then the total realized during the ten years from bonds and taxation would be \$25,535,000 after paying interest currently on bonds issued at the rate of 4 per cent.

If one-twentieth of the bonds are retired each year by money from the tax levy, then the available revenue in ten years will be less, or about \$20,000,000 to \$22,000,000, according to the magnitudes of the annual issues and the fiscal policy which may be adopted.

It is well not to lose sight of other sources of revenue which may be made available on a broad range policy.

1. Special assessment will be available for lateral channels which seem a local territory, and perhaps to a limited extent on the main channel.

2. Property values may be created by applying the material excavated. The possibility of large resources are covered in this.

3. Cooperation on the part of the United States. This seems to have been lost sight of.

4. The dockage and water power will be sources of revenue in time, though this may be of slow development.

It is impracticable to construct an economical channel for drainage which is not also a good navigable channel, and the United States should be willing to put into this the cost of the 8-foot channel which its officers have estimated and recommended.

A practical fiscal policy should be possible under this law, and relief can never be had sooner than by making a beginning.

The sanitary project of this city is a necessity, and well-considered investigation has determined that the ship-canal solution is the only one practicable and within the financial resources of this city. The problem must be solved or this city must stop growing.

6. LARGE CITY EXPENDITURES

Cincinnati put from twenty-five to thirty million into the Cincinnati railway. So far as direct revenue is concerned, the capital is largely sunk, though indirectly the city is richly repaid.

Chicago has put the same amount in her park system since 1869, and has not felt the burden, nor does she begrudge the expenditure.

The city expends about twenty-five million each year for all purposes, including money for special assessments. (See last city report.) This is as much as it ever be necessary to raise under this law and is spread over a term of years. Considering the sums spent by other cities for public works, the sanitary project seems the cost ought not to be regarded as serious.

REPORT OF COMMITTEE ON ENGINEERING TO BOARD OF TRUSTEES OF THE SANITARY DISTRICT OF CHICAGO, SUBMITTED JANUARY 9, 1892.

GENTLEMEN: On December 12, 1891 (page 298 of proceedings), your honorable board instructed the engineering committee as follows:

Resolved, That the engineering committee be instructed to carefully examine the plans of the engineering department and any proposed plan of operations and make such recommendations as will expedite the beginning of actual construction upon the main channel between Chicago and Joliet, and secure the completion of the en-
gineering channel at the earliest date and with the greatest economy, and that said committee report as soon as practicable."

This resolution calls for a comprehensive review of the work of the district and the formulation of a policy for the consideration of the board. The committee has had the full use of the time at its disposal, and now submits a general report, referring to detailed recommendations for further consideration.

The committee has consulted maps, diagrams, and reports, conferred with the chief engineer, heard experts upon the relative cost of rock excavation under water by dry quarrying and listened to the objections offered to the Chicago end of the route as adopted, on behalf of the several railway corporations interested. In addition, each member has used every opportunity to inform himself upon the matter at hand.

PAST POLICY OF THE BOARD.

Your committee can not ascertain from the records or by personal inquiry that the action of the board has been guided heretofore by a definite policy or a specific plan of operations. If so, it was not a matter of general information.

The following have appeared to the committee:

(1) The estimates of the engineer contemplated fixed bridges throughout the route from Chicago to Lockport. The requirement of swing bridges and a navigable channel was one not anticipated by the railways, as appeared in the hearings by the committee.

(2) The channel of a capacity of 600,000 cubic feet per minute through the rock bars at Sag, when the law requires that it should begin at Willow Springs, 5 miles nearer Chicago. (See section 23 in regard to "territory with a rocky stratum.")

(3) The dimensions of the channel, its depth at the starting point, and its grade, are not matters of official record, nor are the works proposed for the development of water power below Lockport. In fact there are no proper records which indicate the plans of the board. So far as the committee can infer, one foot only is allowed to fall in a distance of 7 miles from the lake through the river to the stock yards, and the grades are relatively much less than those adopted by Messrs. Worthen and Newton. It is extremely doubtful if the channel as designed will pass muster under section 27 of the law.

(4) Every engineer of the board has suggested that work should begin on the rock cut, between Sag and Lockport, as the completion of this section requires the longest time and is a measure of the period when the entire work may be made available. Statements have been made which led to the inference that condemnation proceedings here would require one year. It has been ascertained that they can be completed for practical purposes in sixty days.

(5) The attention of the Board was largely concentrated on the work between the stock yards and the Desplaines River at Summit, a section that would have been useless for several years until the remainder of the work could be completed. It was intended to erect pumping works at Corwith, and it is inferred that pumping works were also to be erected at Summit to throw water into the Desplaines River, a purpose certainly contrary to the spirit of the law. The report of the engineering committee, July 11, 1891 (page 201 of proceedings), seems to contemplate this section as the only work prior to 1893.

The above, and all that has come to the attention of the committee, is in harmony with the following:

That the work immediately contemplated was a channel by the most direct route from the South Fork to the Desplaines River, near the Ogden-Wentworth Dam, the

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ween Chicago and Lockport is crossed at eight points by ten distinct to six corporations, and seven of these points are within the first Santa Fe Railway is crossed four times, three crossings of the main the throat of their large yard at Corwith.

selected there will be railway crossings, and the number of these on s can not be reduced below seven lines crossing at four points, to sing at six points.

Five cost of the first 9 miles of feasible routes will not greatly difference in cost of right of way will probably be far less than the ills for railway damages, so this factor becomes of prime importance, y be outweighed by considerations which pertain to the efficiency of

tee finds that the railways are not disposed in any way to needlessly work. They regard a drawbridge as a serious matter and insist that shall not be unnecessarily introduced in their lines and that they be ticable, at such points as to occasion the minimum of inconvenience. opted has been characterized as one that inflicts the maximum possi- ge between the waters of the Chicago River and Summit. At Corwith es the throat of the main yard of the Santa Fe system, the canal, and of three railway lines, all in a short distance. The situation is capable ration. The Santa Fe Railway could be relocated for 5 miles to the roposed channel between Corwith and Summit, thus avoiding two sings, but the position of the yards and junctions would not be ma- ed. It is claimed that the value of this yard would be destroyed and other locality necessitated, the entire cost of which would fall upon item in itself stated at over \$1,500,000. This situation may be avoided a route to the south of the yards, by the Illinois and Michigan Canal, e north of the canal.

at Western avenue and Thirty-ninth street is only less serious. Here d the tracks of the Northern Pacific (not yet laid), the Panhandle, Yards Company leading to their railway yard. It is a waiting point bridges would seriously embarrass the business to the stock yards, as w operated. The situation is capable of some amelioration, but not f reducing the point to the status of the main lines without radical s. This may be possible, but the situation must be met in any route t Fork.

lopted was probably the most available from the South Fork to the nit, for the purpose of pumping works at the canal crossing and at River, and was not intended for navigation. It certainly could not e objectionably located for the latter purpose. It is suggested by the that the West Fork, to the junction point near Crawford avenue, zed as the navigable connection with the Chicago River, but no record ion is found in any report or proceeding, and this view is not in har- e estimates for fixed bridges over the entire route and with the chan- ion carried through to the South Fork.

uestion of a route at this end should be reconsidered. The construc- ld be reestimated, the right of way appraised, and the railway dam- r each line that may be available. On any line that may be adopted, mination of the basis of railway compensation is likely to be a matter gation.

ot, however, the only criterion. Within limits of cost, considerations to the efficiency of the channel and its adaptation to future needs vern. Some of these may be stated without further argument at this

anel should effectively remove the sewage without expensive adjuncts s which require large expenditures for operation and maintenance, thus eiciency dependent on the whims of an annual appropriation bill.

n should contemplate the easy development of the water supply in apacity of the Chicago River when the occasion therefor shall arise, ed by the law.

itions should be avoided which will in any way militate against the pment of a navigable route from the lakes to the Mississippi, or the of a deep-water harbor. While these are not issues immediately pend- not be ignored. The only feasible line for a waterway between the

awrence and that of the Mississippi is by the Chicago divide, and a waterway in this generation is through the operations of this is object we may confidently enlist the aid of the United States he deep-harbor question may be best and most cheaply solved he district when the occasion therefor may arise.

not been given to the Chicago end of the route, and for

be sufficient for present consideration. The work in the Chicago certain item, but certainly \$1,000,000 will greatly better the stream the district and for navigation.

es, bonds, and special assessments not less than \$20,000,000 may be es, so there is no present reason to suppose that the resources will be

in works outlined are completed, any further development will come progressively, without entertaining any serious financial question, pro- oral plan is now properly matured.

RECOMMENDATIONS.

ittee has the honor to recommend as follows:

der the route from Sag to Lockport at once and prepare plans for pro- ame from surface water, with a view to beginning work on this section t practicable date.

sideration of this section will require little if any field work prior to the adopted line on the ground. It should be possible to adopt a new repare plans for the same in sixty to ninety days, and actually begin is. Sufficient property can be acquired by that time.

is the route where necessary between Willow Springs and Sag, with a ining work this season. This may be done in sixty to ninety days after kport section.

ider the whole question of route from Chicago to Willow Springs, in the sent needs and the requirements for future development.

for actual construction on this section should be made not later than art of 1893.

ider the route and the treatment of the problem below Lockport.

a here may require one year. Work need not begin until early in 1895. he plans may be matured in harmony with some plan for navigation with peration.

conditions to be met through the Chicago River as soon as practicable, structures and modification in dock line may conform to a general plan. will doubtless cooperate in this policy so far as it has occasion to deal rer in the interests of navigation. The work will be fragmentary and e some tedious litigation, and any opportunity for correcting the river at ould be availed of. The larger part of the work, however, may be post- ld financial considerations render this expedient.

mittee is persuaded that every energy should be bent to secure the actual of work between Sag and Lockport, and that meantime every other con- should be subsidiary. After this is begun ample time will be available reful consideration of other sections of the route, without delaying the of the work as a whole. We are also persuaded that such a course will esources of the district to the best advantage.

CONCLUSION.

of some current misconceptions, your committee would add the following upon the character of the project as a whole:

solution of the sanitary problem was adopted because it was much the nvolved little or nothing for maintenance and operation, and had collateral s as a waterway.

capacity was fixed by two considerations: (a) That the channel should e necessary dilution to maintain a sanitary condition for the probable population during the time for which the bonds are issued, or until the id for. (b) That it should be adequate, in conjunction with other works, snow and rain water in floods, and thus prevent contamination of the lake nes.

not practicable, on account of excessive cost, to make a channel that will requisite volume of water and at the same time be unnavigable. This is l by the physical conditions. A channel flowing at a high velocity regh grade, thus increasing the depth of rock-cutting at the lower end. unnel will be unstable in the clay.

the required capacity a deep channel is less costly than a wide one. It is et to less variation in flow, by changes in the level of the lake, by floods, , and it is more easily regulated at the lower end on account of less fall. re the substantial considerations which determined the present general lined in the law. Fortunately, all these conditions are also in the interest ion. The only incident for actual navigation is proper railway and high- ngs.

This law was matured after long consideration, and is explicit in its provisions. It lays down the conditions which must be met, and definitely prescribes the limitations upon capacity and size of channel. It is no part of the duty of this board to question these provisions, and it has no option other than to carry them out in accordance with their full spirit.

Very respectfully submitted.

LYMAN E. COOLEY,
WILLIAM BOLDENWECK,
JOHN J. ALTPETER,
Committee on Engineering.

The honorable the BOARD OF TRUSTEES OF THE SANITARY DISTRICT OF CHICAGO.

REPORT OF MR. WILLIAMS, CHIEF ENGINEER, TO THE BOARD OF TRUSTEES OF THE SANITARY DISTRICT OF CHICAGO, SUBMITTED FEBRUARY 17, 1892.

CHICAGO, February 17, 1892.

GENTLEMEN: On January 16 you
"Resolved, That the chief engineer
fiable, alternative locations for the
tive estimates in sufficient detail to de
that may be considered, and that he i
and damages.

In complying with this order it is found
and estimate for alternate lines
method of treatment and be con
sideration of the work which was
the Desplaines River from some poi
and that which will be necessary in
Willow Springs. I have hence exce
alternative locations for the route bet
comparative estimates for quantities and
most economical channel of uniform depth.
all the rock excavation on the line of the proposed main drainage channel from
Chicago to the upper basin.

Each of the several lines considered in the comparative estimates have received similar treatment, and the estimates are based upon the same method of working, except so far as differences of location compel variations.

There are three distinct routes from Willow Springs to Lockport and four from a point above Lemont to Lockport covered by these estimates. At or about Lockport all four lines merge into one. These various lines are shown upon the accompanying map, which is made in two sections, the estimates being given in Appendix A. The fifth line shown on the map is considered wholly as regards a different method of treatment and receives separate consideration hereinafter.

Line No. 1 follows the location made by Mr. William E. Worthen to a point above Lockport, where there is a deflection into a line common to all routes. This line is situated almost entirely on the north and west side of the Desplaines River, striking the river only at a very few points.

Line No. 2 is on the location made by Mr. Samuel G. Artingstall, intersecting the common line at Lockport. It follows the bed of the river as closely as is consistent with a satisfactory alignment.

Line No. 3 is a new location throughout. As far as possible it lies on the south and east side of the Desplaines River, between the river and the Illinois and Michigan Canal. It necessarily crosses bends of the river in a few places, but in such a manner that with a comparatively small amount of excavation the river channel may be changed so as to protect the new work. It involves a new location of the Chicago, Santa Fe and California Railway for a distance of 22,200 feet, 6,000 feet of which the company is now engaged in making for the purpose of improving their line, leaving 16,200 feet to be changed should the route be adopted.

Line No. 4 and No. 4 A are still other new locations; the former following line No. 3 to a point nearly $1\frac{1}{2}$ miles above Lemont, where it diverges, crossing the Illinois and Michigan Canal one-half mile below Lemont, and again crossing the canal about 1 mile above Lockport. This line involves a shifting of the tracks of the Chicago and Alton Railroad near Romeo, for a distance of 4,000 feet, and either the abandonment of the Illinois and Michigan Canal below the first crossing, or the carrying of that canal across the new channel in flumes, with a waiver of the navigability of the latter.

board passed an order as follows:
instructed to submit, as soon as prac
sen Sag and Lockport, with compar
the relative cost of the several line
n said estimates all collateral work

to make a proper preliminary locati
nts named that will admit of the best
sanitary district law involves a con
to improve, for the passage of water,
Lockport to the upper basin at Joliet
; the main channel between Sag and
structions to the extent of submitti
low Springs and the upper basin, with
for the whole distance, based upon the
Within these limits is comprised nearly

No. 4 A is a variation of line 4 lying between the two canal crossings. It does not differ materially from the latter.

Line No. 5, as heretofore stated, is considered wholly with reference to a different treatment of the project from that proposed for lines 1, 2, 3, and 4. This difference of treatment is described in another place. This line starts at Willow Springs, incident with line 2, and continues in that line a distance of 7,000 feet, thence in the same course to the west line of section 14, below Sag Junction, crossing the Illinois and Michigan Canal on the way, $3\frac{1}{2}$ miles below Willow Springs. Thence the line deflects northward into the Lemont tangent of the present canal produced; hence it follows the bed of the canal past Lemont, and continues in a straight course to the Romeo point, which it rounds with an easy curve, thence taking a straight course to Dam No. 1, at the lower end of the upper basin at Joliet. It follows close to line 4, east of the canal, and passes east of the wire mill at Lockport, and under the bridge of the Elgin, Joliet and Eastern Railroad. From Willow Springs to Dam No. 1 there are but two curves, the whole distance being accomplished by three tangents.

The adoption of this line will involve the reconstruction of the present canal in three sections aggregating a total distance of 26,000 feet. It involves two canal crossings, change in the Chicago, Santa Fe and California Railroad for one-half mile, and a reconstruction of the Chicago, Alton and St. Louis Railroad in two sections aggregating in all 28,000 feet of double track. Like No. 4, it necessitates the abandonment of the Illinois and Michigan Canal, or the carrying of the same across the proposed new canal in flumes.

Within the range of the above described lines, almost the whole of the Desplaines River Valley is included, that can be said to be available for canal construction.

In the estimates of cost, given in Appendix A, for the first four lines that of the right of way has been excluded, except to state approximately the number of acres. The results are peculiarly notable in the small variation that there is between the lowest and the highest, viz, No. 3 and No. 4, respectively, which is slightly less than one per cent. Indeed, this variation is so small that without considering the question of right of way, facilities for working the various lines present, and the probable hazard from floods in the Desplaines River, one would be in grave doubt as to which line to recommend. This doubt will further be increased when we consider that where the surface of the rock is covered with glacial drift, as it generally is from Willow Springs to Lemont, the profiles of the rock from which the estimates were made are not absolutely reliable, owing to the incompleteness of the borings, and that this unreliability exists probably to a greater extent with lines 3 and 4 than with either of the others.

Probably the surer and more conclusive way to arrive at a decision relative to these lines is by a process of exclusion.

COMPARISON OF 3 AND 4.

Comparing No. 3 and No. 4 it is seen that the estimated cost of the latter line is \$71,752 greater than the former; the estimated cost of No. 3 being \$11,740,678.

No. 4 for a part of its length has some advantage in the matter of exposure to floods of the Desplaines River, but on the other hand is exposed to all drainage from the south of the line for several miles. It is also 1,000 feet shorter. It will require, in order to prevent damages by overflow, the condemnation of 435 acres of land more than No. 3. It crosses the canal twice, which will be an obstruction in working, and involves the raising, complicated questions with the canal commissioners, since the canal will either have to be abandoned or carried in flumes across the new channel. The estimates are based on the proposition to abandon the canal, as to carry it across the channels in flumes would defeat the purpose of the plan.

In this comparison it should be borne in mind that the doubt which exists as to the relative quantities for some of the lines does not exist as between these two, for the reason that throughout that portion of their length where they follow different routes surveys of the surface of the rock and of the ground have been carefully made. It is only in the section common to each that the profile of the rock is not reasonably reliable. Hence, as between these two lines, I have no hesitation in preferring No. 3.

COMPARISON OF 2 AND 3.

Comparing lines Nos. 2 and 3 it is seen that omitting right of way No. 3 is approximately cheaper than No. 2, and that it is 1,600 feet shorter. It is also in the matter of floods, and is likely to be less subject to water ordinary stages of the river. During the construction of a new bed for a considerable part of the way,

which will not be thoroughly puddled and comparatively water tight, as the
is. The lower part must be built along the new channel will for a large part
way fall in the bed of the old river, and the land will slope toward it rather
from it, rendering it more difficult to exclude water. In some places also it
must be forced into very narrow confines between the new canal and the
Santa Fe and California Railway. Indeed, in at least one place, either the
the road will have to be changed.

It has the advantage of saving 800 acres in right of way and in land sub-
overflow, which, at \$200 per acre, makes it cost but little more than No. 1.
alignment, however, is inferior, and if selected should be modified in places.

A careful balancing of the arguments for and against each of these
makes it clear that line 3 is much to be preferred to line 2, though for reasons
than those shown in the estimates. Indeed, estimates are incapable of showing
real differences between lines under conditions such as these.

COMPARISON OF 1 AND 3.

The difference in cost between lines Nos. 1 and 3 is apparently \$539,375, on
right of way. Line No. 1 is also 3,000 feet longer than No. 3. In the facility
which it can be protected from floods in the Desplaines River, line No. 1 is
slightly superior to line 3; but it has the disadvantage of being on the north
west sides of the river, and hence exposed to the drainage from all the creek
ravines and drainage areas tributary to the river. On the other hand, the
takes line 3 by intercepting the drainage from the south and east, which
otherwise reach the new channel.

The lands which it will be necessary to condemn for line 3 are in excess of
for line 1 by 603 acres. Giving these lands an extravagant price will still
balance largely in favor of line 3. This line also has a better alignment
than those that have been considered.

In thus analyzing the advantages and disadvantages of the several routes
described upon which estimates have been made, I am decidedly of the opinion
line No. 3 is the proper one to adopt for the main drainage channel.

The conclusion reached regarding the merits of the four lines under dis-
cuss stands independently of the manner of treating the problem after encounter
shaping ground just above Lockport.

Though the comparison of cost has been made upon the basis of complete
in each case, from Willow Springs to the upper basin, the fact that all the
merge into one at Lockport, the plan below being common to all, still less
relative showing correct for any treatment that may be given.

That there may be a full understanding of what this plan is, and its merits,
it is presented with two other projects that have heretofore been proposed, so
the length of about 23,000 feet lying between the end of the 18-foot channel,
found by the late Chief Engineer, and the upper basin.

Plan 1. Mr. Worthen's plan was to continue the main channel with a
slope along the line shown on the map, to a point opposite the upper basin, and
at this point the water was to pass through Joliet by means of a 24-foot steel pipe
beneath which the water could be allowed to escape into the upper basin and the
channel from the basin down improved to the proper capacity. If this plan
carried out, the upper or lake level would be maintained as far as the upper
and provides an excellent development of water power secured. On the
the river valley would be the high level canal, and nearly parallel thereto
from 10 to 20 feet below it the river bed and the upper basin to act as a tail race.

Plan 2. Mr. Artingstall's plan contemplates an improvement of the river
straightening and retreating the sides, and by building three dams, which will
decrease in at many stages. This plan would allow of a partial utilization
water power, but without extensive subsidiary canals would not give a full
development thereof.

Plan 3. The present plan, the cost of which is given in the estimates, is
consists of the water excavation between the canal between Willow Springs and
to be built a narrow-gauge canal 10 to 12 feet wide on top from a point 90
feet above the upper basin, carrying the water between this bank
to the west side of the river. It is also proposed to waste
water from the upper basin into the channel formed by this embankment
from the point to the west side of the river. This plan will incidentally furnish
a means of waste water from the upper basin to the river.

It is proposed to build the main canal to a height of 5 feet above dat
which will allow the water to pass through 18-inch cast-iron pipe sluice
at each of the dam locations and remove any possible chance
of obstruction to the flow of water.
The plan is a preliminary and to a

at nearly [merely?] suggestive; it can not be worked out properly in detail territory covered by it has been surveyed.

it should be said that in this matter of the terminal, below Lockport, there are modifications which can be made in the plan, and probably will be made on fuller ground which it is premature to enter into here.

A comparison of the cost of covering this stretch of 23,000 feet, by each of the three plans described is given in Appendix B.

The right of way required for plans 1 and 3 is the same, though it is impossible to say how much it is without considering the question with reference to the legal limits of the district to water power created by it. Plan 2 does not require so much as about 670 acres.

Comparing at the results in Appendices A and B, it will be noticed that 11,000,000 cubic yards of waste is estimated as being hauled at 7 cents per cubic yard, and that 3,000,000 cubic yards of rock are allowed at a reduced price, and credit given it in Appendix C.

When waste enters into the bank that confines the water below Lockport, into a channel near the Elgin, Joliet and Eastern Railroad, and into spoil banks west of the proposed canal, thus raising the land and limiting the spread of the water.

At the rate of 7 cents per cubic yard, due to the haul of the material from the excavation to the place of deposit, it should be said that it has been arrived at by thorough investigations and estimates made by Assistant Engineer H. B. Alexander, the results of which are embodied in a report from Mr. Alexander, which is given herewith as Appendix C.

The estimate is shown to be the difference in cost between wasting the material adjacent to the line of the canal, and hauling it into banks an average distance of 10 feet, full allowance being made for cost of track, equipment and operation. In regard to the cost of the road it should be said that the mere building of the roadbed at track and ballast is a part of the excavation of the canal, material from the excavation being used to build the bank for the road. Such a bank, or something its equivalent, is necessary to protect the excavation from the floods in the river, so that the bank serves a double purpose.

A allowance of 10 cents per cubic yard on 3,000,000 cubic yards of rock is based on the theory that if all but this amount of the best quality of rock is permanently removed from the first, that contractors will be found, who in making bids will be in the sake of owning this stone, take the work at a very low price.

When there are large quantities, running into hundreds of thousands of yards, of just stone quarried and sold every year in and about Chicago, and there seems little doubt that the reason why this waste rock has heretofore been considered of no importance arises from the immense quantity which might belong to a great number of owners and thereby become valueless from its very plentifulness.

Incidental advantages which will be derived and the ultimate profits which may be reasonably expected to accrue from carrying out this plan, run up into millions of dollars. Some of these may be summed up as follows:

By permanently disposing of all but about 3,000,000 yards of rock, this amount as heretofore stated, of value, and the contractor, who may own it after the work is done, can dispose of it at remunerative prices, for ballast, for crib work, and for masonry; and will doubtless in submitting a proposition do so with this in mind, making allowances to the benefit of the sanitary district. On the other hand, if the whole amount of rock is wasted, it will have little or no value to anyone, as the market will be glutted with cheap stone.

As far as possible, it should be the policy of the sanitary district to so dispose of the excavated material as to leave the banks clear of spoil. Much value will accrue to the right of way for the canal if it is kept clear of waste banks; but if it is encumbered with a chain of mountains on each side, its value will be almost destroyed. A hundred feet of right of way clear will be of more value than 800 feet cumbered with spoil.

By carrying out the proposed plan below Lockport a channel of full depth will be opened 4 miles further, and whatever the depth of the waterway which the United States Government may build to the Mississippi River, these 4 miles will be a great benefit, and, in dealing with Congress, as the sanitary district will ultimately be able to receive recognition to the financial benefit of the district.

This plan furnishes means for the complete control and measurement of the river which passes down the river at all times, and will doubtless prove of great benefit in preventing actual damages in the river below in the time of excessive floods, and particularly in furnishing a check by which claims for fictitious damages may

be avoided. In carrying out this plan the sanitary district will own the best mill sites, and the most steady and most reliable water power in the Northwest.

As a fact, which will generally obtain 300,000 cubic feet per minute, will furnish the power, and with 600,000 cubic feet per minute it will furnish

25,000 horse power. At the very low price of \$10 per horse power, the first class when leased will bring an annual rental of \$180,000, which capitalized at 5 per cent is \$3,600,000, while the greater quantity will capitalize at \$7,200,000. This power would be readily taken at these rates admits of little doubt. It should be borne in mind that the execution of the proposed plan creates this power with another cent of expenditure other than short flumes and pipes to conduct the water to the wheels.

Line No. 5.—Surveys and estimates are being made of Line No. 5, the location of which is hereinbefore described.

The estimates, when completed, will show the cost of this line by two methods of treatment.

1 The main channel proper will terminate at a point above the lower end of the Illinois and Michigan Canal, at which place controlling gates and waste weirs will be built. Below this point the channel will become a tailrace, and the water will be conducted to the lower basin in the cheapest manner consistent with the requirements.

2 Parallel banks will be built—ones on each side of the line below the crossing of the canal, and as far down as a point just above the Elgin, Joliet and Eastern Railroad Bridge. There they will end in a cross bank forming the main space. At this point waste flumes and controlling gates will be provided for.

The banks of the channel will be carried up to a height of 8 feet above datum. This method will accomplish to a considerable extent the same ends accomplished by the plan heretofore described, and which is included in the estimates for line No. 4 exclusive.

Work was only begun on this line a few days ago, and as the results are not likely to affect the conclusions of this report in any way that can not be properly considered by the engineering committee as a part of the same, it is thought best to submit the whole question in its present stage, and let the new data go to the engineering committee direct and be conveyed by that committee to the Board, rather than to delay another week.

Respectfully submitted,

BENEZETTE WILLIAMS,
Chief Engineer.

The honorable the BOARD OF TRUSTEES OF THE SANITARY DISTRICT OF CHICAGO.

REPORT OF MR. WILLIAMS, CHIEF ENGINEER, TO THE BOARD OF TRUSTEES OF THE SANITARY DISTRICT OF CHICAGO. SUBMITTED MAY 4, 1892.

CHICAGO, May 4, 1892.

GENERAL: My communication of April 20, relative to a board of consulting engineers, has been referred to me with the direction from your honorable board to report in writing at the next regular meeting the specific points which such a board should consider.

It is to be understood, technically construed, contemplate that all issues shall be defined and presented in writing before the appointment of a board of experts; but as the opinion was not obtained that it is fully within the powers of your board to discharge the duties of your capacity which you may see fit to prescribe, the request was made, and if granted it would be upon broad grounds rather than in a narrow one, in compliance with the rules.

When the communication was submitted no issues had arisen that could be defined, and the only question presented in the light of the bids which have been received. The hope was indulged that by the choice of engineers of wide professional standing and ability to consider the subject in all its bearings, it might be possible to avoid the difficulties.

The questions which, before they exist, it is not impossible to state the points which will probably cluster in certain contingencies.

As you are aware, bids have been invited on three propositions, and a comparison of the same will be necessary before awards can intelligently be made.

The first proposition will give the cost of excavating and constructing the main channel from Willow Springs to the upper basin at Joliet, waiving the right of way and the cost of all structures of relatively small importance.

The second will give the cost of excavating, but in order to be comparable with proposition one the cost of drainage and of the transportation of about two-thirds of the excavated material must be added. The determination of this additional cost is a matter of competition, and any conclusion of your chief engineer relative thereto, would likely be twenty universal assent.

position 2" will give the cost of excavating and disposing of a part, and only of the material required to be excavated to furnish a channel for water to upper basin. A tailrace, some 5 miles in length, will have to be located and extended. Where shall this race be, and how much will it cost to excavate it? These important points which your engineer can hardly hope to determine in such a brief time as to be satisfactory to all.

In the work, if carried out in accordance with the proposed plan under either positions 1 or 3, will ultimately develop a water power above Joliet of full horse power, which will be utterly lost under Proposition 2. Will such a power have any value, and if so, how much? And should it be considered in connection with the work of the sanitary district, however small its cost and however great its value?

In carrying out the work under either Proposition 1 or 3, a very large portion of the banks of the channel will be kept clear of spoil, while absolutely none will be left unincumbered under Proposition 2. Does any value attach to this consideration?

A navigable channel under propositions 1 and 3 will extend with deep water 5 miles farther than under proposition 2. Such a channel, or its equivalent, is a necessity for the navigation of the Desplaines and Illinois rivers, and without it the expense being expended by the United States Government on the Hennepin Canal and the improvement of the Illinois River will be utterly thrown away. Should this require any consideration in carrying out this work?

Could it be shown that such a contribution to the work which the Government is now doing can be made at a nominal cost—work which would cost the Government, if carried out independently, many times what it would cost the sanitary district if in connection with the main excavation—may it not be worthy of an effort to secure Government coöperation to the extent of the extra cost? And might not a high consideration of all these questions by a properly constituted board of officers be the initiatory step to such coöperation?

These are mere hints as to the good which may come out of such a board if wisely organized. But in view of the fact that the sanitary district is engaged in a work which is actually contributing immensely toward the project of a waterway which the Government is expending money upon, may it not be the proper time to take a step which may lead to results greatly to the financial benefit of the sanitary district?

I am profoundly impressed with the importance of the questions which must be considered after the bids are received on the 8th of June—questions which it may appear will affect the interests of the sanitary district to the amount of several millions of dollars—and I believe that the only safe way to approach them is in the manner suggested. Should your honorable board reach the same conclusion, then I think all unsettled questions connected with the work below Willow Springs, coming in the domain of the engineering department, should be referred to such board, and your chief engineer shall have made a report upon the same.

It is what, as it seems to me, would be conducive to greater expedition and better results, the consulting engineers might act with the chief engineer in making a report.

Respectfully submitted.

BENEZETTE WILLIAMS,
Chief Engineer.

In honor of the BOARD OF TRUSTEES OF THE SANITARY DISTRICT OF CHICAGO.

REPORT OF MR. WILLIAMS, CHIEF ENGINEER, TO THE BOARD OF TRUSTEES OF THE SANITARY DISTRICT OF CHICAGO, SUBMITTED JUNE 7, 1892.

CHICAGO, June 7, 1892.

SIR: I beg to report hereby, in compliance with the order of your honorable board passed February 17, which is as follows:

Resolved, That the chief engineer is hereby directed to make comparative estimates of three alternative routes (one of which shall be the Illinois River Canal) between the waters of the Chicago River at Ashland avenue and Willow Springs road, and that the route already located be considered in connection with the said routes; and that the estimate on each line include structures, and all supplemental works necessary to make the route navigable; and that he report upon the whole subject as soon as possible, and that he report upon the work below Willow Springs."

at Summit, for the purpose of a uniform comparison, the west line has been used in each case. The footings of the estimates for the various lines begin as follows:

Description of line.	300,000 cubic feet per minute.	600,000 cubic feet per minute.
West Line No. 1.....	\$10,265,095	\$12,099,763
West Line No. 2.....	9,284,944	11,175,070
.....	7,865,963	9,746,018
..... with narrow right of way to Summit.....	5,899,963	7,780,018
..... and Ogden Ditch line.....	9,455,954	11,330,798
..... and canal line.....	8,903,678	10,804,919
..... rock line.....	9,978,153	12,043,016

These amounts include the estimated cost of the main channel, with right of way, locks, changes, bridges, and supplemental works west of Ashland avenue, on the several routes, but give no idea of the relative difficulties that may be encountered in securing right of way, or the relative injury to railroad property, which is not susceptible of estimate; nor do they give any information as to which location will best meet the present and future sanitary needs of the city, nor which one will be the most necessary supplemental works east of Ashland avenue be included. They are, therefore, far from furnishing a conclusive determination as to which route should be selected.

To arrive at a just conclusion, the question should be considered more in detail under the following heads: (1) Facility and expedition with which the respective routes can be worked. (2) Right of way and obstacles to procuring same, excluding railroad properties. (3) Interference with railroad interests. (4) Value of property acquired by the sanitary district upon the various routes. (5) Supplemental works necessary to render the main channel fully effective to the extent contemplated by sanitary district law, and the prospective incidental advantages in the way of a navigable waterway and improved harbor facilities which the various routes afford.

I. FACILITY AND EXPEDITION OF WORK.

The estimates take cognizance of the relative difficulty of executing the work, as far as it is possible to do so, in the matter of quantities and cost per cubic yard; but they show nothing as to the time required to carry out the work on the several routes.

They are based upon the theory that to the west line of section 7, near Summit, each route, respectively, the excavated material will be removed by scows or barges and dumped into the lake, either as filling where needed along the shore or as a cut in deep water as to be no obstruction. This method of disposing of the material is recommended because it is believed to be but little if any more expensive than the process of spoiling the waste, unless the spoiling can be done over a large area by means of hydraulic dredges, and because land that may cost the district \$5,000 to \$10,000 per acre will have too great a value after a navigable channel has been cut through it to be used as dump grounds.

As regards the Ogden Ditch routes, the canal route, and the South Fork and canal routes, they can be worked in this manner with about equal facility as to time and expense. In the case of transportation, the necessity of constructing and using a lock at the mouth of the canal about offsetting what will be, at first, the less commodious method of the Ogden Ditch. On either of these routes with the right of way secured in a reasonable time, and an adequate plant, the work can be prosecuted with sufficient rapidity to insure its completion by the time the rock cut below Willow Point is done.

On the other side of the two remaining routes having their initial point in the South Fork, particularly the southern one, much slower progress must be made because of the more material to be removed for an equal distance, and no channel exists to the west access to the work. Again, to provide temporary crossings for railroads and highways will be more difficult on these lines than in the case of the first four routes.

The material to be excavated on the Ogden Ditch routes, barring the rock at Ashland avenue on route No. 2, can be more easily handled than on any other. It should be said, however, that the weight which is attached to the variation in the nature of the material, and the difference of the manner in which the work will be done on the several lines, finds expression in the rates used in the estimates. The conclusion is that the work can be executed in less time after the right

OF THE CHIEF OF ENGINEERS, U. S. ARMY.

of way in
on any of...

erred on the lines following the Ogden Ditch and the canal line
routes.

2.—RIGHT OF WAY.

In estimating upon the right of a way a strip 800 feet wide has been included, with
some exceptions. A much wider strip for a part of the way between Ashland avenue
and South Fork has been taken for Ogden Ditch, line No. 1, as it is deemed neces-
sary to change the bed of the river to control completely the riparian rights, so far
as they exist.

On Ogden Ditch line No. 2, for a part of the way, only one side of the West Fork
has been included.

In order that the canal line may be at no disadvantage with other lines, a strip has
been included on both sides of the canal reserve as far as summit, giving a clear
width of 1,000 feet, for the most part, between the railroad rights of way as re-
established. By the omission of the strip on the south of the canal reserve there will
be 600 feet remaining for most of the way from Ashland avenue to a point one-quarter
of a mile west of the center of section 8, and the estimated cost of the right of
way will be reduced \$60,000.

On the South Fork a strip east of the canal
course, either minor or
major, any reason why
as the construction of
line there will be a loss
of \$60,000.

Should the various
committees there
until the construction
closed, thus giving the
an increase of time al-
lowing more business
will be no greater per cent
of the South Fork and canal line than
ally but 600 feet wide. There are, of
be noted here. There is not, appar-
ticularly preferred over another, so far
concerned, except that with the canal
lines to deal with.
line with the consent of the canal
within the limits of the canal reserve
ational right of way shall have been
in time of beginning over any other
cost of the work, so that, notwith-
stand, consider that the cost of the work
Ogden Ditch lines.

3.—

INTERESTS.

The constructive cost of railroad changes, the building of bridges, interlocking,
etc., is given in detail in the estimates. These amount in the aggregate for each
route, respectively, to the following sums:

Ogden Ditch, route No. 1.....	\$830,000
Ogden Ditch, route No. 2.....	830,000
Canal route.....	923,000
South Fork and Ogden Ditch route.....	1,035,300
South Fork and canal route.....	1,047,000
South Fork route.....	998,000

As it is impossible to infer from the above figures all the advantages and disad-
vantages which one route may possess over another, from a railroad standpoint, the
following comparison is instituted for each railroad or group of railroads, sepa-
rately:

*Union Stock Yards and Transit Company; Chicago and Northern Pacific; Pittsburg,
Chicago and St. Louis.*—The main tracks of these roads, lying in adjacent and par-
allel lines, are crossed by all the routes in about the same manner, requiring for each
a six-track swing bridge or its equivalent. The Ogden Ditch lines alone cross them
where there are now swing bridges, hence the consequential damages and entailed
expenses to the operating companies will be less along Ogden Ditch than any other
route. It should be said, however, that as yet there are but few vessels passing the
Western avenue bridges, and that should the main drainage channel take some other
route than Ogden Ditch the conditions are not likely to change materially.

All the routes starting from the South Fork strike the Union Stock Yards and
Transit Company's Yards in such a manner as to render rearrangement necessary.
With the plan as projected for this rearrangement it is believed that no permanent
injury would be done to these yards.

Chicago, Madison and Northern; Chicago, Santa Fe and California.—The main line
of these roads would be crossed between Kedzie avenue and Summit by the Ogden
Ditch line, and as now located by the canal line, half way between Ashland avenue
and Western avenue. The canal line also crosses the branch road of the Chicago
Santa Fe and California Railway Company at the throat of the Corwith yards in
such a manner as to be extremely damaging.

Ditch line also crosses the Santa Fe branch tracks north of Corwith, in a manner as to be particularly objectionable. In these cases the crossing is such that these two roads from their present point of crossing eastward, keeping the drainage channel, the new bridge crossing the South Fork thus become a substitute for the one now in use, and saving one bridge to these roads. To be crossing at the throat of the Santa Fe yards it is proposed to remove the crossing to the north of the main line, extending from Corwith to the Western Indiana and to dispense with the present yards. The South Fork and Ogden Ditch line crosses the Santa Fe at the throat of the yards through property beyond in such a manner as to prevent a removal as proposed for the canal line. It also crosses the main line in two places at the Summit. The South Fork and canal line and the South Fork line cross these yards in such a manner as to damage them so seriously that it is considered necessary to provide for the same in the same way as proposed for the canal line. It may be said that various changes made as proposed, the Ogden Ditch route is the only one that will affect the Chicago, Madison and Northern Railway. In this case, however, the injury done would be serious, and a radical change in the alignment, as well as introducing a swing bridge. The Ogden Ditch routes and the South Fork and Ogden Ditch route are the only ones that really damage the Santa Fe branch road, except in crossing the old Grand branch, the latter route being so ruinous in its effects as to practically be prohibitory. The proposed change the Santa Fe system as a whole will be materially benefited by the adoption of the canal route. The Chicago, Alton and St. Louis.—This road is crossed only by the three South Fork routes in these cases the crossing is such that it seriously interferes with the alignment of the tracks in the manner projected but, on the other hand, would ultimately be greatly benefited. The Chicago, Alton and St. Louis.—All the routes cross this road in substantially the same manner, leaving nothing to mark a preference for one route over another, that the Ogden Ditch line comes at a bad point in the grade of the road. The Chicago, Alton and St. Louis.—The road is crossed in the same manner and at the same place by all the lines. A summary of the railroad bridges between Ashland avenue and Willow Springs for the various routes after the proposed changes have been made is given in the following table:

Railroad.	Name of route.				
	Ogden Ditch, 1 and 2.	Canal.	South Fork and Ogden Ditch.	South Fork and Canal.	South Fork.
Chicago, Cincinnati and St. Louis.	Double track	Double track	Double track..	Double track.
Rock Yards and Summit Com.	Four track	Six track	Four track	Four track	Four track.
Chicago and North-Western.
Chicago, Madison and Northern.	Double track
Chicago, Alton and St. Louis.do	Two double track.
Chicago, Alton and St. Louis, old Trunk.	Single track.	Single track	Double track..	Single track..	Single track.
Chicago, Alton and St. Louis.	do	Double track..	Double track.
Chicago, Alton and St. Louis, and Western Indiana Belt.	Double track	Double track	do	do	do
Chicago, Alton and St. Louis, Terminal.	do	do	do	do	do

Inclusion of the railroad problem it may be said that the indirect damages on the "canal route" are at a minimum, while on the "South Fork and Ogden Ditch route" they are at a maximum and next to prohibitory, so far as the Chicago, Santa Fe and Western Indiana Belt is concerned.

4.—VALUE OF ACQUIRED PROPERTY.

The number of feet of available water front which, on the lines and rights of way as proposed, will belong to the sanitary district between Ashland avenue and the west line of section 7, this side of Summit, and to which point it is proposed to keep the banks free of spoil, not including platted streets, railroad crossings, etc., are as follows:

	Linear feet
Ogden Ditch, line No. 1.....	71, 80
Ogden Ditch, line No. 2.....	67, 20
Canal line.....	70, 80
South Fork and Ogden Ditch line.....	73, 70
South Fork and Canal line.....	71, 80
South Fork line.....	69, 90

From this it is seen that there will be but little difference in the value of the acquired property if the wide right of way be taken on the canal line; but should it be narrowed to 460 feet its value on this line would probably be reduced as much as 75 per cent.

5.—SUPPLEMENTAL WORK.

A comparison of the various elements coming under the first four heads affecting the choice of a route renders it apparent that, if nothing else were to be taken into consideration, the choice would inevitably fall upon the canal. Under this head, however, other considerations may affect the problem, but before entering upon them it should be premised that, so far as supplemental works east of Ashland avenue are concerned, there are practically but two routes, the group consisting of the two Ogden Ditch routes and the canal route constituting one and the three South Fork routes another; and, since all but the canal route from the first group and all but the South Fork and canal route from the latter have been eliminated, our further inquiries are concerned wholly with these two routes as affected by the works necessary to render the main channel fully effective.

It is essential to inquire into the sanitary condition and capacity of the Chicago River and its branches and to reach an understanding of the provisions which will be necessary to meet local sanitary needs when the city shall have grown to such magnitude as to tax the full-size channel as required by the sanitary district law.

SANITARY REQUIREMENTS.

Whichever route may be adopted for the main channel, the main river and the south branch will feel the direct effect of the current created thereby, though only the route leading from the South Fork will have a direct effect upon this filthiest of all stagnant pools. The canal route would produce no natural current through it unless supplemented with an inlet channel from the lake connecting with the South Fork through which a flow of water can be established. In lieu of this, resort must be had to intercepting sewers and a flushing conduit with pumping works.

To do the work thoroughly a circular conduit 12 feet in diameter would be required, the conduit and intercepting sewers being located as shown on the accompanying map. The intercepting sewers would collect all the sewage south of Thirty-ninth street, including everything from the stock yards and slaughterhouses, discharging directly into the conduit. In addition to the sewage the capacity of the conduit would be such as to provide for a change of water in the South Fork once every twenty-four hours.

In the Appendix is an estimate of the cost of such works with the cost of operation capitalized, which, including the latter, amounts to \$616,280.

To meet the local sanitary requirements of the North Branch, the method now in use of flushing it by mechanical means will doubtless have to be continued, but in the near future this must be done on a much larger scale by means of an open channel or its equivalent, located farther north than Fullerton avenue.

The volume of water which must be passed through the South Branch, for the proper dilution of the sewage in the Branch itself, is an important element in this inquiry. To determine this some idea must be had of the population whose sewage is and will be, directly or indirectly, tributary to it.

The population whose sewage was, in 1890, tributary to the various branches of the river and that which is estimated will be tributary to it in about twenty years

city shall have reached a population of 3,000,000 and the sewage draining like shall have been turned to the river, are as follows:

	1890.	Population, 3,000,000.
ch.....	345,000	1,000,000
ch to Bridgeport	87,000	
.....	277,000	700,000
.....	152,500	
.....	145,000	700,000
.....	151,000	
.....	48,500	100,000
.....	1,204,000	3,000,000

course impossible to predict with assurance at what points in the river the contamination will occur from sources other than that of population, from present indications it is safe to assume that the stock yards and packeries will remain where they are, so far as they may not be transferred to and this, as is well known, is the one great source of river pollution. These facts it appears that 300,000 cubic feet per minute, or thereabouts, will be required for dilution in the South Branch of the river, with even a less relative than that specified in the sanitary district law.

CAPACITY OF THE CHICAGO RIVER.

to guide to the amount of water which it is possible to draw through the river and its branches, either in its present condition or with modifications, I have had gauging of the river made during the late flood from May 6 to May 10, 1887, similar ones made by the Drainage and Water Supply Commission of Chicago on February, 1887. From these measurements it was found that the maximum discharge through the South Branch was, at 5 p. m. May 6, 1892, 420,000 cubic feet per minute, and on February 10, 1887, 422,000 cubic feet per minute, the former near Eighteenth street and the latter at Kedzie avenue.

The maximum discharge through the North Branch occurred May 5, 1892, and from 11:30 a. m., and probably amounted to 200,000 cubic feet per minute. The discharge at the time measurement was made, 11:30 a. m. of the 6th, was 184,000 cubic feet per minute. For the 1887 flood the maximum was 125,000 cubic feet per minute on February 10, the former measured at Clybourn place and the latter at Illinois.

The maximum discharge from the main river was, approximately, as follows (some being made for water that reached the river below the point of measurement) on May 6, 1892, 584,000 cubic feet per minute and on February 10, 1887, 300,000 cubic feet per minute.

For 300,000 cubic feet per minute through the South Branch with the conditions on May 6, 1892, required a fall for different reaches as follows:

Reach.	Distance.	Total fall.	Slope per 1,000 feet.
	Feet.	Feet.	Feet.
from Lake street to Sixteenth street	9,200	1.04	0.113
from Sixteenth street to Harrison street	5,600	0.53	0.095
from Harrison street to Lake street	4,200	0.98	0.233
from Lake street to Lake street	19,000	2.55	0.134

The velocities which obtained at various places on the South Branch on May 6, at the time of the flood, when 420,000 cubic feet of water per minute was flowing, ranged from 1.96 to 3.91 feet per second in the wider sections of the river to 5.18 feet per second at the cofferdam of the West Division Street Railway Company near to Van Buren street; and for 300,000 cubic feet discharge per minute the velocities at the same places were 1.96 and 3.91 feet per second respectively.

The maximum discharge through the main river was, as above given, approximately 584,000 cubic feet per minute, and occurred on May 6 from 11:30 a. m. to 5 p. m.; which time the average total fall from Lake street to the mouth, a distance of 20,000 feet, was 1.02 feet, or 0.141 per 1,000.

From the above observations and other information, I conclude that to preserve safe

and easy navigation of the Chicago River the velocity should not exceed 14 miles per hour, or 2.2 feet per second.

To get 300,000 cubic feet per minute through the South Branch of the river within this limit of velocity will require material changes. North of Sixteenth street, except at bridges, it can be accomplished by deepening the river to 17 feet in the narrowest places and to a less depth in the wider places. At all bridges, except Adams, Jackson, and Polk streets, and at the Wisconsin Central Railroad bridge, it can be accomplished by deepening to 18 feet. At Adams and Jackson streets it can be done by opening new passages for water west of the center piers. At Polk Street and the Wisconsin Central Railroad new bridges will be required, or some radical modifications giving an increase in the water passages. South of Sixteenth street it will be necessary to resort to a more general widening, deepening, and redocking of the river on one side and the building of new bridges.

In the event of the adoption of the South Fork and Canal route this general widening must be carried to Thirty-fifth street and the deepening made clear through to the entrance to the main channel.

To pass 600,000 cubic feet per minute through the river all the bridges would have to be rebuilt or additional passages provided around their abutments, the main river deepened and perhaps widened in places, and the South Branch throughout radically widened and deepened. Such an undertaking would be formidable and can only be justified as a last resort or on the grounds of a necessary harbor improvement after every other available means of securing such an improvement shall have been shown to be impracticable and unattainable.

ADDITIONAL SUPPLY OF WATER.

The question of securing an inlet for an additional 300,000 cubic feet of water per minute may be considered wholly with reference to sanitary requirements, all collateral benefits and advantages in this way of improved navigation, enlarged harbor facilities, and the possible relief from the downtown bridge nuisance being ignored; or all of these things may be taken into account and that solution of the problem predicted which will most inure to the general good, not only of Chicago in its dual corporate capacities as city and sanitary district, but to the people of the State and the United States.

While it may not be the province of the sanitary district, strictly speaking, to make any provisions for navigation other than that rendered obligatory by law, or to provide harbor facilities for the city of Chicago, it does seem not only within its province, but at least morally binding upon it, so long as these problems are pressing for a solution and all matters pertaining thereto are in a formative stage, to shape the work in hand as to render coöperation between it and other governmental agencies possible in carrying out supplemental works, as only by such coöperation is it possible to secure the best sanitary, financial, and constructive results. Working upon this theory I shall discuss the questions of supplemental channels and inlets in their relation to the two routes under consideration for the main channel, from two stand-points: (a) Strictly from the narrowest sanitary point of view, confined to the sanitary district as such. (b) With reference to the broadest sanitary, commercial, and political interest of this community and of the country at large.

(a) But for the southern boundary of the sanitary district being Eighty-seventh street, thus placing limitations upon the sanitary problem of the city, it would be obligatory to consider a supplemental channel entering the South Fork from the mouth of the Calumet River, which would intercept the sewage that finds its way into the lake from that source, such channel being made to furnish the additional supply of water and to provide main drainage for that part of the city comprising the old town of Lake and the southern part of Hyde Park. But omitting this, because of the omission of the southern part of the city from the sanitary district, we have to consider under this head only the cheapest possible way of obtaining 600,000 cubic feet of water per minute from the lake and the best location of the main channel west of Ashland avenue to adapt it to the lake inlet.

As has already been pointed out, provisions for obtaining 300,000 cubic feet per minute through the South Branch north of Sixteenth street can be made at a comparatively small expense. Another 300,000 cubic feet per minute can be obtained by gravity through a covered conduit laid from the lake to the river along Sixteenth street. Such a conduit, including intake, would be about 4,000 feet long and would need a cross section of about 1,250 square feet. It could probably best be constructed as a twin conduit. To permit of its construction it would be necessary to condemn a strip of property 50 feet wide along the south line of Sixteenth street which property, after the construction of the conduit, would be as valuable for building sites as now.

The South Branch, south of Sixteenth street, would have to be improved to

The three principal considerations justifying this are as follows.

city of Chicago is confronted with the commercial situation of having at low a harbor that will barely admit vessels drawing 14 feet of water. The present harbor is too narrow, too shallow, and too tortuous to admit the class of vessels are fast taking to themselves the carrying trade of the lakes. Vessels nearly 4 long and that now carry from 2,000 to 3,000 tons, with present harbor facilities depths of channels at the St. Clair River Canal and the Lime Kilns, would led to 18 to 20 feet draft and carry from 3,000 to 5,000 tons if sufficient depth aivable. The shallow places throughout the lakes and the Sault Ste Marie are being improved and deepened by the United States Government to 21 feet. this work is completed, which will be done in a few years, Chicago's commercial interests will be seriously threatened if a more adequate harbor is not provided. y cheaper lake freight rates prevail between Duluth and Buffalo on both grain al than between Chicago and Buffalo, notwithstanding 111 miles greater distance Duluth and the necessity of passing the Sault Lock.

n, the United States Government is committed to improved navigation of the River, by reason of the money already expended thereon, and that which is being expended upon the Hennepin Canal. The sanitary district channel will, bly, be the most important link in this chain of waterways, and Government ts will demand a suitable lake connection.

third consideration is found in the urgent need of relieving street traffic in central part of the city from the incubus of movable bridges. The injury that ridges are to business interests is beyond computation and too evident to need an mention here.

ving, as I do, that these considerations will, by the time the main channel is ted from Ashland avenue to Joliet, force the recognition of the necessity of rbor entrances, it is impossible to consider as final any plan for a new inlet ie lake to the river except one that will answer the three-fold purpose of sani-a harbor entrance, and an outlet for what I believe will become a great in-aterway. Hence we have to inquire which of the two routes under consid- is best calculated to fit any possible location for such a channel. If the d from the lake should connect with the South Branch north of Bridgeport, or the river itself be deepened and widened throughout so as to give ample ty for all purposes, then the "canal route" will obviously be the best that can en.

he other hand, should the new channel enter the South Fork at its southern ity, then the "canal route" would not connect so directly with the lake as outh Fork and canal route," but it would cost no more to pass the water ard than southward through the South Fork to the main channel, and in case the intercepting system and flushing conduit would be dispensed with. ould be said that, on the theory herein advanced, the Chicago Harbor will be he terminus of lake commerce on the one hand, and of river and inland com-on the other, and not a mere short cut for vessels passing from the lakes to

My conclusion then is that there is but little choice between the two routes the main drainage channel west of Ashland avenue, as far as any possible connection with Lake Michigan is concerned, and as every other consideration points to "canal route" as the preferable one, I believe there can be no doubt about advisability of adopting it.

The reasons for this conclusion may be briefly recapitulated as follows:

(1) On a like basis as to right of way it is shown to be \$1,058,901 cheaper than next cheapest route, and by sacrificing certain prospective advantages in the work it may be made \$1,963,901 cheaper than the next cheapest possible route.

(2) Work can be begun on it immediately, quite likely saving a full year in time.

(3) With the changes proposed it is open to less objection from a railroad standpoint than any other route.

(4) Should supplemental works to supply water to the main channel be carried out in accordance with the narrowest requirements, it is located in the best position to command the situation. On the other hand, should they be projected on the broadest lines, it still meets the situation as fully as any other location.

FLOOD WATERS.

In order that the estimates for the main channel between Ashland avenue and Willow Springs may cover all collateral works, it is necessary to deal with the flood-water question and its relation to the main channel. This involves a consideration of (1) the sanitary question of Chicago as affected by floods; (2) the disposal of the Des Plaines and Chicago rivers flood waters.

(1) The protection of Chicago's water supply from sewage pollution is by far the most important phase of the sanitary question. The greater or less dilution of sewage, so long as it is confined to the Chicago River or to the main drainage canal, is to the people of this city of little moment compared to the importance of keeping sewage, even in small quantities out of the lake. The reasons for this are too palpable, too well known and acknowledged to need stating here. Hence, from the inception of the project for the sanitation of the city via the Des Plaines and Illinois rivers until now, every fair mind that has intelligently approached the subject recognized that the prevention of all flood-water discharge from the Chicago River into the lake is a *sine qua non* of the plan.

(2) Owing to the relations which the upper Des Plaines River, when in flood, maintains to the Chicago River, and the occupancy of the lower Des Plaines River Valley with the main drainage canal, the plans of the Chicago Drainage and Water Supply Commission provided that the flood water from 440 square miles of the Des Plaines Valley and from 120 square miles of the North Branch of the Chicago River was to be turned to Lake Michigan through cut-off channels uncontaminated with sewage, thereby reducing the flood volume from the combined catchment basins of the rivers, of which 400 square miles would remain, to such proportions that a channel of the capacity proposed, viz, 600,000 cubic feet per minute, would prevent an overflow from the Chicago River into the lake. By such an arrangement the Des Plaines River could be taken into the main drainage channel at Summit with impunity.

This method was then, in the absence of much knowledge which has since been gained of the Des Plaines and Illinois rivers, apparently the best and safest for accomplishing the end in view.

The further study which has been given to the subject in connection with the final location of the main drainage channel has convinced me that a cheaper and perhaps equally safe method is to call into service the combined capacity of the Des Plaines River and the main drainage channel beyond Summit to carry off the flood waters of the Des Plaines and Chicago rivers.

During the late flood of May 6 there was a maximum of 606,000 cubic feet per minute passed through the Des Plaines River below Riverside, and on February 18, 1887, a maximum of 620,000 cubic feet per minute. On April 21, 1881, the water at the place where these measurements were made reached 1.4 feet greater height, which indicated a discharge of 810,000 cubic feet per minute. Of this amount at least one-third, and perhaps a greater proportion, passed down the Des Plaines River below Summit. By the removal of a few obstructions, and a little forcing by raising the height of floods slightly at Summit, there seems no reason to doubt that the Des Plaines River itself can be made to carry 400,000 cubic feet per minute from Summit down, leaving about the same quantity to be taken into the main drainage channel at Summit. If this can be done without setting the water toward the lake at that point in the Chicago River, then the flood waters will have been provided for as furnishing a channel for them across the divide is concerned.

The watershed of the North Branch of the Chicago River comprises 172 square miles, and other portions of the river, including the South Fork, the West Fork

of the surplus flood waters of the Des Plaines River. The combined of the Des Plaines River and the main drainage channel when in flood, lake at mean height, will be upwards of 1,200,000 cubic feet per minute.

It is prepared to say, finally, that it will be permissible to turn such an amount into the Des Plaines River below Joliet, a more extended investigation and a study of the problem being first necessary. The following facts and conclusions, however, seem to indicate the practicability of such a thing.

On the May flood, which was unusually heavy along the river above Joliet, passed through Joliet approximately 625,000 cubic feet per minute; this occurred, however, more than twenty-four hours before the Upper Des Plaines its maximum. At Morris the maximum discharge of the Illinois River, as indicated by gaugings made by the Engineering Department on May 6, was 4,416,000 cubic feet per minute, and it occurred the same day that the Upper Des Plaines discharge was at its highest, showing that at this place the maximum was due to sources of supply than the Upper Des Plaines. From LaSalle to the mouth, at flood times spreads out to such wide proportions that the maximum discharge at Grafton was not attained until about May 23, seventeen days later than at Morris.

Owing to the equalizing tendency which these facts exhibit as the regimen of the Illinois River, it seems evident that the floods from the Des Plaines River do not appear to be likely to affect extreme floods in the Illinois River other than by adding them; the additional amount of water which may be added by the drainage channel and the upper Des Plaines in the manner described having no effect. Hence, I believe it may safely be said that whatever damage done in the Illinois Valley by water discharged from the sanitary district channel occur at intermediate stages with nothing more than the normal flow of the drainage channel combined with medium floods in the Illinois River. But even extreme floods of the Illinois River should be slightly increased, it should be noted that, after land has been overflowed, to add a few inches more or less of incumbent water would do no harm; it is the first flood that does the damage.

In consideration is that the modifications of the river that will be necessary to the normal flow of the drainage channel will be sufficient to prevent any of extreme flood heights, even with the floods of the Upper Des Plaines added. In providing for mean and frequently recurring conditions the maximum of frequently recurring conditions will be amply met.

As it is still an open question, I think the probabilities are all in favor of the proposed being equally safe and cheaper than any northern diversion as a means of diverting a flow of the Chicago River to the lake, the money which such a diversion would cost being better applied in down-river improvements and modifications can also be much more expeditiously put into operation. By the time of completion of the main drainage channel conditions may have been created, in order proposed, that will effectually put a stop to the contamination of Lake Michigan through the Chicago River.

Whatever subsequent investigations may show relative to this matter, there is

REPORT OF THE CHIEF OF ENGINEERS, U. S. ARMY.

Appendix.—Estimates Ogden Ditch, Line No. 1.

Capacity, 600,000 cubic feet per minute:		
25,000,000 cubic yards earth, at 18 cents	\$2,884,320	
7,000,000 cubic yards glacial drift, at 27 cents	2,057,400	
200,000 cubic yards rock, at \$1.25	250,000	
		\$5,1
Bridges and railroad changes:		
One two-track, South Branch, Union Stock Yards and Transit Company, Chicago, and Northern Pacific, Pitts- burg, Chicago and St. Louis Railway	177,000	
One two-track, South Branch, Union Stock Yards and Transit Company, Chicago and Northern Pacific, Pitts- burg, Chicago and St. Louis Railway	109,000	
One single-track, Southern Branch, Chicago, Santa Fe and California Railway	66,000	
One double-track, Southern Branch, Chicago, Milwan- aukee and Northern Railway	146,000	
One double-track, Southern Branch, Chicago and West- ern Railroad	122,000	
One double-track, Southern Branch, Chicago, Santa Fe and California Railway	105,000	
One double-track, Southern Branch, Chicago and Cali- fornia Railroad	105,000	
		83
		18
		4
		15
		5,70
		12,09
Capacity, 600,000 cubic feet per minute:		
15,000,000 cubic yards earth, at 18 cents	1,841,760	
4,000,000 cubic yards glacial drift at 27 cents	1,310,040	
200,000 cubic yards rock, at \$1.25	250,000	
		3,40

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ANNUAL REPORT OF THE CHIEF OF ENGINEERS, U. S. ARMY, ILLINOIS RIVER LOCK AND DAM, ILLINOIS.

The total appropriation for "operating and care of canal locks and locks of navigation" as provided by section 4 of the act of March 3, 1899, the La Grange Lock has been operated for the past fiscal year.

During the year no vessels were navigated or propelled by steam have been navigated through the lockages for the past year.

The number of vessels passing the lock was tripled during the year, showing a notable increase in size of vessels.

During the year repairs were made to lock gates and masonry as needed, an abutment constructed above the lock; 212 line feet of revetment or guide wall were constructed along the land abutment to the lock, and all buildings and wood and iron work above the lock kept well painted.

The lock during the year was engaged in operating the lock, were kept busy in constructing the revetments, grading and improving grounds, and repairing, painting, etc., the appointments of the lock.

tonnage passing La Grange Lock during the year ending June 30, 1892.

Tons.	Passed through the lock.				Passed over the dam.		
	Registered tonnage.	Freight.	Passengers.	Stock.	Registered tonnage.	Freight.	Passengers.
.....	37,585.91	Tons. 2,144	973	Head. 219	23,973.29	Tons. 14	355
.....	47,480.00	33,455	29,681.00	6,900
.....	85,015.91	35,599	973	219	53,654.29	6,914	355

List of steam vessels passing La Grange Lock 1891-'92.

No.	Registered tonnage.	Name.	Registered tonnage.	Name.	Registered tonnage.
.....	466.75	H. W. Longfellow ..	40.00	Polar Wave	150.00
.....	5.00	Rackett	57.80	D. M. Schmoltd.	13.00
.....	77.16	New Idea	176.00	Joliet	75.00
.....	10.00	Dick Clyde.....	76.00	Josie	237.51
.....	20.00	Verona	10.00	Grey Eagle.....
.....	300.00	Borealis Rex	116.13	City of Peoria.....	128.00
.....	10.00	Magnet	42.30	White Eagle.....	312.75
.....	35.00	City of Brunswick ..	77.01	Rescue.....	139.30
.....	358.00	Ida Morse.....	10.29

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ILLINOIS AND MISSISSIPPI CANAL.

object of this improvement is to furnish a navigable waterway like Michigan to the Mississippi River at the mouth of Rock Island connection with the upper Illinois River, and the proposed waterway along the present line of the Illinois and Michigan

river and harbor act of September 19, 1890, contained the first provision for beginning the construction of this canal.

In reference to this work it is to be said that various surveys of routes from the great bend of the Illinois River to the Mississippi River at or above the mouth of Rock River have been made: In 1817 by P. Low (Report Chief of Engineers, 1871, page 303); in 1822 by Benjamin Benyaurd (Report Chief of Engineers, 1883, page 1757), and in 1837 by Major Handbury (Report Chief of Engineers, 1886, page 1707).

There was also a report upon this canal by a Board of Engineers in 1837 under the provisions of the river and harbor act of August, 1836 (Report Chief of Engineers, 1887, page 2125). All of the local engineers and the Board of Engineers of 1887, for engineering reasons, preferred and recommended the Marais d'Osier route, but for commercial reasons the Chiefs of Engineers, Generals Newton and Duane, and Secretaries of War preferred the Rock Island route, and that route has been finally adopted.

The preliminary plans and estimates based entirely upon preliminary surveys under the act of August 11, 1888, were prepared and submitted to Congress June 21, 1890, the report of which location, plans, and estimates (without maps and drawings) were submitted June 21, 1890, and published as House Ex. Doc. No. 316, Fifty-first Congress, first

report was the basis evidently of subsequent appropriations. Under the requirements of the act of September 19, 1890, it was ordered by this office and approved by the Chief of Engineers, reasons given in the report of the Chief of Engineers, 1891, page 1, that work upon this improvement should begin at the mouth of the river and that the appropriation be expended at that point.

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on the enumeration, among works of navigation required for by the act of September 19, 1890, of "bridges" to be constructed it was inferred by the United States.

on the provisions of the river and harbor act of July 5, 1884, making an indefinite appropriation for the "operation and maintenance and other works of navigation."

On a protracted investigation the Secretary of War decided that under the laws of the United States he was not authorized to enter into an agreement, and directed that the lands of this railroad should be condemned for the purposes of the canal.

Onwards on application by the district attorney for the northern part of Illinois that the basis of condemnation be specified, *i. e.*, the character of bridge to be constructed by the railroad company, whether fixed or swing, as well as other description, be indicated, it was ordered by the Secretary of War in effect that the general laws relating to bridges in the river and harbor act of September 19, 1890, and other acts, applied in this case, and that he was not authorized to indicate what bridge would be permitted to be constructed and maintained over the canal by the railroad company.

The matter again came up before the United States district court at St. Louis, Mo., on the 10th term, 1892, but as no basis of condemnation of a right of way across the public highway, except an unconditional crossing by a bridge 10 feet wide at the water surface, and 7 feet deep, or 14 feet below the level of the tracks of the railroad, could be presented under the authority of the Secretary of War by the United States, the court sustained the case, and intimated that until the United States officers specify a basis of joint user over the joint crossing that the case would be indefinitely continued, or that the jury would be instructed to award the land and damages at the full value of the land owned by the Rock Island and Peoria Railroad, less the proved value of their lands as property for other purposes than as a public carrier between Rock Island and Peoria. The case thus stood at the close of the fiscal year ending June 30, 1892, contains a provision that specifically defines the basis of agreement or condemnation in all such cases, and if passed will remove the otherwise insurmountable obstacle to the construction of the canal indicated by the ruling of the Secretary of War and the attitude of the United States district court for the northern district of Illinois, where which court all condemnation suits will be tried.

PROPOSED APPLICATION OF FUNDS ON HAND.

The funds now on hand will be applied to the payment of expenses incurred in connection with the legal proceedings in acquiring right of way around the rapids of Rock River, near its mouth, and to purchasing the land for the plant, and constructing the works of navigation in the vicinity of the rapids.

APPLICATION OF FUNDS REQUIRED FOR THE FISCAL YEAR ENDING JUNE 30, 1894.

Applied, if appropriated, to the definite location of the canal route, to the fixing of landmarks, and to the purchase of lands required for rights of way; and to the construction of the canal and the operation along the line as far as may be deter-

LOCK PIT AND FOUNDATION LOCK 37.

Name and address of bidder.	Earthwork.	Material.	Totals.
in Killeen, New Liberty, Iowa.....	\$49,392.00	\$11,738.00	\$61,130.00
rew J. Whitney, Rock Island, Ill.*	8,643.00	8,975.00	17,618.00
oxon, Quigley & Moore, St. Louis, Mo	14,076.72	10,474.50	24,551.22
Heidenreich Co., Chicago, Ill	22,226.40	14,143.00	36,369.40

* Lowest bidder.

LOCK PIT AND FOUNDATION, LOCK 36.

Name and address of bidder.	Earthwork.	Material.	Totals.
in Killeen, New Liberty, Iowa.....	\$4,083.30	\$10,845.00	\$14,928.30
aroe & Richardson, Portsmouth, Ohio*	2,631.46	9,825.40	12,456.86
lexon, Quigley & Moore, St. Louis, Mo	3,992.56	9,390.10	13,382.66
Heidenreich Co. Chicago, Ill.....	4,537.00	12,114.90	16,651.90

* Lowest bid.

LOCK PIT AND FOUNDATION, GUARD LOCK.

Name and address of bidder.	Earthwork.	Rock.	Totals.
ichael H. King, Des Moines, Iowa*	\$3,107.52	\$5,215.00	\$8,322.52
vid Sears, Rock Island, Ill	1,434.24	7,748.00	9,182.24
in Killeen, New Liberty, Iowa	4,183.20	11,920.00	16,103.20
aroe & Richardson, Portsmouth, Ohio	2,988.00	5,960.00	8,948.00
lexon, Quigley & Moore, St. Louis, Mo	4,661.28	13,410.00	18,071.28
Heidenreich Co., Chicago, Ill.....	2,988.00	6,705.00	9,693.00

* Lowest bidder.

REPORT OF MR. L. L. WHEELER, ASSISTANT ENGINEER.

UNITED STATES ENGINEER OFFICE,
Milan, Ill., June 30, 1892.

AIN: I have the honor to submit the following report upon the work on the
and Mississippi Canal, under my own immediate supervision, for the fiscal
nding June 30, 1892:

he beginning of the year the force was mainly occupied in preparing plats re-
to right of way and in continuing survey of Rock River. Plats of proposed
ng of Rock Island and Peoria Railway were drawn, and general information in
d to ownership of lands compiled for use of United States district attorney.
of witnesses were prepared and evidence given in suit for condemnation of

urvey of Rock River was carried up to the mouth of Green River, but above
e Bridge no topographical work was done on either bank. The notes of the
y between Carrs Island and Moline Bridge were platted, the map completed
aced.

ecognition of the Feeder line was made and an alternative route surveyed
e main line in the vicinity of Wyanet.

measurements of seven bridges over Rock River were taken and estimates made of
of raising four of them to give greater clearance at high water.

eries of measurements of the discharge of the several channels of Rock River
water were made in September, 1891. The method of measurement was by
and rod floats, the water being too shallow to permit the use of a current
comparing this report is a table giving the results for the stream as meas-
Carrs Island, where all of the water flows in one channel.

raphical survey of the Mississippi River in the vicinity of the mouth of
made in September, and a map made showing depths at low water.

ference with a committee representing the Business Men's of Rock Island.

In the afternoon the Board met at Milan, Ill., in accordance with the order, and also examined on the ground the so-called "southern route" for the canal. Milan proved to be an inconvenient place for the meetings of the Board; consequently, authority was given to the office of the Chief of Engineers to hold the subsequent meetings at Rock Island, which was duly granted.

The session of Tuesday, September 8, was devoted to the examination of reports relating to the duties assigned the Board, and to further action by the committee of the Rock Island Business Men's Association, in connection with an inspection of the depression east of the ridge between Pettifers Slough, where it has been proposed to locate a harbor and accommodation of boats while laid up or while transferring cargoes. It is also claimed that a basin at this place is necessary adjunct to the transfer of freight from land to water transportation.

Written invitations were extended to the Business Men's of the cities of Rock Island, of Davenport, and of Moline, W. H. Gest, J. T. Robinson, esq., and to the Sears Brothers, publication in newspapers, to the public generally, to meet the rooms of the first-named organization, on Wednesday the purpose of publicly discussing the questions involved.

The day of Wednesday was occupied in such discussion; everyone was freely invited and everyone present was not only permitted to express his views but encouraged to do so. Rock Island and Davenport were represented at this public meeting, but, so far as Moline was concerned, Moline took no notice of the invitation.

On Friday, September 10, the Board proceeded to Milan, Ill., and examined to the entire extent of the so-called "Southern route," from its entrance into the Mississippi River at the principal mouth near Rock Island.

From that time until the date of adjournment, September 12, 1891, the Board was engaged in considering the matters committed to them, and on Friday some time was allotted to an interview with the owners of property situated along the line of the Northern route, in order to make oral offers as to the prices they would be willing to accept for the right of way.

DESCRIPTION OF THE LOWER REACH OF ROCK RIVER.

About 3 miles from the Mississippi, and just above the lower Rapids, Rock River divides into two channels, which empty into the Mississippi through separate mouths. The lesser channel, known as South Slough, and joins the Mississippi about 2 miles below the mouth of the principal streams. The low-water discharge of the Rock River, as measured for the Board on September 4, 1891, is about 100,000 cubic feet at Rock River.

South Slough and the larger branch of Rock River lies Big Island, which covers a total square miles in extent, parts of which are overflowed at high water from a few inches to 10 feet or more in depth. The center of this island, however, embrace a very considerable area of high ground above high water, and the greater part of the island is used for agricultural purposes.

The island, the foot of which marks the downstream limit of the rapids, the main cause of which causes the lower rapids, the main

stream of the river is much obstructed by sand bars. Turkey Island, doubtless one of these bars originally, subdivides this channel into unequal chutes, the lower junction of which lies some 4,000 feet above the confluence of this branch with the Mississippi.

Lying within the angle between the principal branch of Rock River and the main channel of the Mississippi, on the right bank of the former and east or left bank of the latter, lies Pettifer Island, a triangular formation having sides one-half mile or more in length, and a surface which is overflowed at high water. It is separated from the mainland by a narrow channel, 100 feet in width, more or less, through which the direction of flow is variable, depending upon the relative stages of water in the two rivers; flowing in toward the Mississippi whenever Rock River is the higher and in the contrary direction whenever the conditions are reversed.

MOUTH OF ROCK RIVER.

From the foregoing general description it may be said that Rock River enters the Mississippi through three mouths, two of which, however, are insignificant at low stages. These channels form a delta some 3 miles, more or less, in extent, measured along the bank of the Mississippi. It would therefore seem that any proper location of the western terminus of the Illinois and Mississippi Canal, on the Mississippi front of the delta described, would comply with the requirements of the act of September 19, 1890, which directs that the western terminus be "at the mouth of Rock River."

Considering the law in the light of the plans and estimates submitted to Congress under date of June 27, 1890 (House Ex. Doc. No. 429, Fifty-first Congress, first session), and upon which this item in the act of September 19, 1890, was probably based, the Board is of the opinion that only the principal mouth of Rock River was contemplated by the act referred to. In this the Board is sustained by the opinion of Mr. Gest, as expressed at the public hearing. Moreover, the Board is of the opinion that, whether the principal mouth of Rock River was especially contemplated by the act or not, it is the best for the purpose, because it is easier of approach by Mississippi River boats than any other, has as good depth of water as any other, and, at sufficient stages of water, affords a secure harbor (under the lee of Pettifer Island), and that it should therefore be accepted as the mouth prescribed by the law, unless it should appear that commercial conditions require the establishment of the terminus at another point, or should no controlling reason to divert it therefrom become apparent.

INCEPTION OF THE SOUTHERN ROUTE.

The "location, plans, and estimates," prepared under the direction of the Secretary of War, in accordance with the act of August 11, 1888, provided for attaining the terminus by passing around the lower rapids of Rock River by canal and slackwater navigation along the northern shore.

The dams pertaining to this route were indicated at the lower end of Vandruff Island and extended across all arms of Rock River. The canal portion of this route terminated near the foot of Turkey Island; thence to the mouth of Rock River it was proposed to improve the open river by wing dams and resulting scour.

The memoir, or report, accompanying these plans (House Ex. Doc.

), Fifty-first Congress, first session, page 4) states with reference to

Early in the progress of the present investigation it was seen that the previous surveys were not in sufficient detail to allow definite and final plans.

When the work is definitely undertaken the vicinity of this line must be further examined, to better, if possible, the location shown upon the maps.

After the passage of the act further examinations were made, as suggested in the report, and the fact was then revealed that the plan, referred to above, for passing the lower rapids of Rock River required modification to the extent that the dams across the south arm of Rock River must be placed at the head of the rapids instead of at the foot, as indicated, and still later it became doubtful if the proposed improvement of the last reach of Rock River would prove successful on account of the apparent small discharge.

Then, still further examination discloses a practicable route upon the south side of Rock River that was thought to possess marked advantages over the improvement as planned along the north bank. This route was therefore recommended by Capt. Marshall, under date of March 19, 1891, and it was formally approved by the Secretary of War March 25, 1891.

As far as known to the Board, the location, plans, and estimates accompanying the executive document above referred to (No. 429) were never formally approved by the Secretary of War, and if that be the case, it may be said that the only part of the Rock Island route for the Illinois and Mississippi Canal (which was designated by him as the one to be followed in preparing plans and estimates under the act of August 11, 1888) that has been definitely located with the formal approval of the Secretary of War is the so-called "Southern route" around the lower rapids of Rock River, terminating "at the mouth of Rock River," as prescribed and directed by the act of September 19, 1890.

With reference to the improvement of the lower rapids of Rock River, it appears that as early as 1867 a canal terminating at the Mississippi River south of South Slough, near the lower end of Rock River Delta, was suggested by Gen. J. H. Wilson in his report on the survey of Rock River. Again, in 1885, when surveys of this locality for the Hennepin Canal were in progress, under the direction of Maj. W. H. H. Benyaurd, Corps of Engineers, under the provisions of the river and harbor act of August 2, 1882, Assistant Engineer G. A. M. Liljeurantz, in a letter dated October 28, 1885, mentioned the fact that the south bank of Rock River was more favorable than the north bank for the construction of the canal in the following language:

What is the objection to running the canal into Mississippi River south of the mouth of Rock River?

The ground south appears, from indications on the printed maps, to be lower and more favorable than north of Rock River, to which letter Maj. Benyaurd replied under date of October 30, 1885, as follows:

The survey made under direction of Gen. J. H. Wilson, in 1867, demonstrates the feasibility of running a canal to the Mississippi below the mouth of Rock River, but as the law authorizing the present survey requires the canal to terminate on the Mississippi at or above Rock Island, it is not deemed advisable or necessary to consider anything below Rock River.

The suitability of the south shore for the location of this part of the canal, therefore, was recognized before 1890, but the terms of the laws under which the surveys were made, upon which were based the detailed locations, plans, and estimates submitted to Congress, June 27, 1890

esigns) is the same as submitted to Congress by the Secretary June 27, 1890 (H. R. Ex. Doc. 429), and the second is the one approved by the Secretary of War March 25, 1891.

of Congress of September 19, 1890, appropriating money for the construction of the Illinois and Mississippi Canal, for * * * "a canal to connect the Illinois River, near the Hennepin, with the Mississippi River at the mouth of Rock

It requires in the canal a depth of 7 feet, and provides for the location of * * * "guard gates, waste weirs, locks, lock basins, bridges, and all other erections, and fixtures that may be necessary for safe and convenient navigation of said canal."

It is inferred from the wording of the law and from previous discussions on the subject that the primary object to be attained is the uniting of the navigable waters of the Illinois and Mississippi rivers (as part of a through route of transportation from the Great Lakes to the Mississippi River), furnished with all additional structures that may be necessary for the proper management and control of the work itself, and for handling the cargo that may be subserved by the improvement.

The objections urged against the route as approved by the Secretary of War March 25, 1891, and statements in favor of a line along the north bank of the Rock River, in the vicinity of its mouth, by the Hon. W. H. Hunt, in the document referred to the Board, and by the committee of the Rock Island Business Men's Association, are:

The line has been located by Congress along the north bank of the Rock River, and can not be changed by the Secretary of War.

That the terminus on the south side is not accessible to railroads, and Mississippi boats, but that a terminus on the north side would be easily accessible to the citizens of Rock Island, and would permit the construction of a wharf at a higher level adjacent to the canal, which basin, with other facilities for elevators and other appliances, would be a great benefit between land and water transportations. easily

as upon some line in connection with the basin just named (to be constructed upon the low depression in the mainland north of Turkey Island), and with an entrance into the Mississippi north of Petti-land or by way of a canal constructed through the island.

As already stated by the Board, the low-water discharge of Rock River exclusive of South Slough (1,500 to 1,650 cubic feet per second), is sufficient to maintain a practicable channel for such navigation as is contemplated from the foot of Turkey Island to the mouth of Rock River through moving sands, by the aid of wing dams. If the north of Rock River be followed, therefore, the canal must be carried to the Mississippi River. This must be done either by continuing the section of canal below Sear's Canal at the same level as now contemplated, or by carrying the level of the upper canal at high water (in which case the basin desired by Rock River would be of variable level), or by carrying the level of the upper canal which is practically the same as extreme high water in the Mississippi River, from Sear's Canal to that river.

The former plan would add to the cost of the northern line by the expense of cost of the excavation of the canal and basin over that of wing dams below the foot of Turkey Island.

The second plan would require an embankment above high water, in the bed of Rock River and on Turkey Island, from Sear's Canal to the section before mentioned or a canal cut in the plateau from Sear's Canal to the low ground named; an embankment around the proposed canal; the construction of a short section of canal from the basin to the Mississippi River, with a lock of 17-foot lift to enter the Mississippi River instead of a lock of 8.5-foot lift as now designed. The lock at Sear's Canal would still be required as a guard lock and to enable boats to use Rock River at high stages of water in that stream, all of which modifications would add still more to the cost of the northern line than would the first mentioned.

COST OF THE TWO ROUTES.

The Board has considered the estimates, made by Assistant Engineer Wheeler in his report to Capt. Marshall on routes, dated March 7, 1891, for both routes.

Exclusive of cost of rights of way, which must always be an uncertain element when voluntary purchases are impracticable, these estimates are:

the northern route.....	\$459,055
the southern route.....	393,530

In favor of south line 65,525

In forwarding these estimates Capt. Marshall gave it as his opinion that, while they indicated approximately the relative cost of the two routes, yet they are too low by fully 20 per cent.

The estimates as thus indicated by Capt. Marshall (exclusive of rights of way and legal expenses connected with acquiring the necessary lands, etc.) for the construction of the canal are:

the northern line.....	\$550,865
the southern line.....	472,236

In favor of southern line 78,629

It is to be understood that all costs of rights of way, compensation, and damages to property must be

a canal along the southern route would be easier of access to teams reaching from the south side of Rock River than if the canal were led along the northern route; that the only railroad now actually following Rock River below the head of the Lower Rapids crosses the head of each route, and may therefore be supposed to have equal facilities of access to either; that railroads of the future can not pass Rock Island below the head of the Lower Rapids without crossing any canal that may be located along this reach of the river; that any railroad crossing the Mississippi within a comparatively short distance of the mouth of Rock River would have more ready access to a canal established along the northern route, but such a road could reach a point south of Rock River either by the tracks of the present railroad or by the construction of an independent bridge and an additional length of track not exceeding 2 miles.

That the city of Rock Island exists and it remains to discuss the question whether the benefits which it would derive from a location of the western terminus of the canal on the northern side of Rock River are of such a character as would justify on the part of the United States generally the sacrifice of the advantages of the southern route at the expense of the appropriations made for this public improvement.

The benefits referred to may be summed up as follows: (1) The city would have more easy access to the canal and its terminus both for teams and railroads. (2) Any traffic arising from the transfer of freight from canal boats to Mississippi River craft, or vice versa, and from the storage of boats in the basin would fall to the citizens of Rock Island.

It seems to the Board that the advantages in the first case, so far as teams are concerned, can only be secured at the disadvantage of teams which would desire to reach the canal from the southward of Rock Island; and the latter are, in proportion to their number, entitled to equal consideration with the former. In regard to railroads, it is believed that if the freight to be carried shall warrant the construction of a line other than the one now crossing all lines which have been proposed for the canal, the controlling corporations will not delay in building them. In case transfers to any considerable extent become necessary at or near the western terminus of the canal there would be some local agents to the business men of Rock Island, acting as the intermediaries. It is an argument frequently used by the advocates of the construction of the canal by the General Government was that freight could be carried through it to and from all upper Mississippi River points without transfer, with great resulting benefit to the people of the country westward and northward of Rock Island; that is to say, that the canal could be simply a part of an extended combined canal and river navigation adapted to Mississippi barges and boats, the termini of which systems of navigation are not upon the line of the canal.

Undoubtedly, if a basin at the western terminus of the canal section of this system were to become a storage place for a large number of boats, with their crews, a considerable profit would be derived from the facilities required for subsistence of crews, repair of boats, etc. No estimate of this profit is practicable, but it is very doubtful if it would approximate the expectations of the advocates of the northern route.

The citizens of Rock Island have devoted much time and money in advocating the construction of the canal and they naturally seek some return for both. In this the Board is in entire sympathy with them, and no sufficient reason could be found for recommending the location

APPENDIX K K.

APPENDIX K K.
STATEMENT OF RIVERS AND HARBORS ON EASTERN SHORE OF LAKE MICHIGAN.

OF MAJOR WILLIAM LUDLOW, CORPS OF ENGINEERS, BVT. COL., U. S. A., OFFICER IN CHARGE, FOR THE FISCAL YEAR ENDING JUNE 30, 1892, WITH OTHER DOCUMENTS RELATING TO THE WORKS.

IMPROVEMENTS.

- | | |
|--|--|
| 1. Harbor, Michigan. | 9. Muskegon Harbor, Michigan. |
| 2. Harbor and entrance to Lake Michigan. | 10. Grand Haven Harbor, Michigan. |
| 3. Harbor, Michigan. | 11. Holland (Black Lake) Harbor, Michigan. |
| 4. Harbor of refuge at Portage Lake, Michigan. | 12. Saugatuck Harbor, Michigan. |
| 5. Harbor, Michigan. | 13. South Haven Harbor, Michigan. |
| 6. Harbor, Michigan. | 14. St. Joseph Harbor, Michigan. |
| 7. Harbor, Michigan. | 15. St. Joseph River, Michigan. |
| 8. Harbor, Michigan. | 16. Michigan City Harbor, Indiana. |

EXAMINATION AND SURVEY.

17. Grand River, Michigan, below Grand Rapids.

UNITED STATES ENGINEER OFFICE,
Detroit, Mich., July 11, 1892.

SIR: I have the honor to submit herewith the annual reports of the works of river and harbor improvement in my charge for the fiscal year ending June 30, 1892.

Respectfully,

WILLIAM LUDLOW,
Major, Corps of Engineers,
Bvt. Lieut. Col., U. S. A.

THOMAS L. CASEY,
Chief of Engineers, U. S. A.

Money statement.

appropriated by act approved September 19, 1890.....	\$15,000.00
2, balance unexpended	15,000.00
appropriated by act approved July 13, 1892 (landing pier project)	20,000.00
available for fiscal year ending June 30, 1893	35,000.00
(estimated) required for completion of landing pier project....	35,000.00
that can be profitably expended in fiscal year ending June 30, 1894	35,000.00
expended in compliance with requirements of sections 2 of river and acts of 1866 and 1867.	

K K 2.

IMPROVEMENT OF CHARLEVOIX HARBOR AND ENTRANCE TO PINE LAKE,
MICHIGAN.

Improvements at this harbor include the lower channel leading to Michigan into Round Lake and the upper channel leading to Round Lake into Pine Lake.

The present project, adopted in 1868 and revised in 1876 and 1884, provides for 12-foot navigation, 100 to 150 feet wide in the lower channel and 100 feet wide in the upper channel.

The lower channel is protected on the north side by 302 linear feet of crib work built by the United States, 469 linear feet of crib work built by the local authorities previous to 1873, and 945 linear feet of timber frame revetment built by the United States, with a timber frame revetment 100 feet long covering the gap between the crib work and the timber frame revetment, overlapping the latter about 30 feet. The total length of the revetment is 1,724 linear feet, and the projection of the pier to the present shore is about 740 feet.

The projection on the south side consists of 491 linear feet of crib work, with the 50-foot wing at the Round Lake end, of 1,538 linear feet of timber frame piling and plank-beam revetment. The total length of this revetment is 2,029 linear feet, and its projection beyond the present shore is about 1,000 feet.

The upper channel is protected on the north side by a pile revetment 1,000 feet long and on the south side by a similar revetment 366 feet long. Both revetments are filled with brush and stone ballast.

The dredging done in May and June, 1891, was instrumental in maintaining a navigable depth of water until the end of the season, but the soundings made March 30, 1892, showed a depth of but 10 feet near the outer pier and through the upper channel.

The decrease in depth is partly owing to the extraordinary low lake level at present principally to the insufficient projection of the piers and the decayed nature of the older parts of the piers and revetments. A sounding made on March 30, 1892, showed a depth of but 10 feet in Lake Michigan 170 feet in advance of the South Pier, the shoal extending

to the entrance, and, although the present project provides for a projection of 100 linear feet to the South Pier (the North Pier already projected length), it is evident that a further extension of the North Pier, if even a 12-foot navigation is to be maintained, would require, in all, 470 linear feet of the North Pier was built in 1873, when the Government took charge

of the improvement. This work is a crib structure, rudely built placed on the natural bottom, and, as a result, extends only to a few feet below the water surface. It has been repaired and refilled with stone from time to time, but is now fast disintegrating. It has not been possible to dredge close up to it, because it would have been undermined thereby and consequently wrecked.

Large quantities of sand find their way through it into the channel at every blow, and it is impossible to prevent this so long as the present work remains.

The original project contemplated its removal and the substitution of a more serviceable and durable structure, and the time for this change is close at hand.

The revetments of the lower channel are of a temporary nature. The channel is narrow, with high sand banks on each side, and it is necessary to resort to rapid construction in order to hold the banks while the channel was making.

The portion above water is rapidly decaying, and particularly the south revetment, owing to its exposed position with respect to collisions with vessels, is in great need of strengthening and repairing. Both revetments should be rebuilt above the water surface and provided with a double row of sheet piling and anchor piles in rear.

Owing to the fact that the north revetment has never been fully completed, stopping at the lake side of the highway bridge that spans the entrance, and the bank above the bridge has been built out with concrete slabs as wharfage for the sawmill, the north draw of the bridge is not navigable and passage is confined to the south draw. This matter has not received special attention since the south draw seems so adequate to the commerce of the port, and no complaints of obstruction have been received. It may be well, however, to note these conditions for future reference.

The revetments of the upper channel are also in need of extension to reach into Pine Lake.

The operations during the fiscal year consisted in generally completing the hauling and repairing the revetments of the lower channel. The bridge caps and guard timbers were renewed, additional piles were driven to strengthen the works, and the gap between north pier and revetment at shore line was filled with brush fascines, loaded with large stones picked up on the beach.

A permanent benchmark was established on top of the water tower at northeast corner of the opera-house block and connected by datum lines with the United States gauge, and a careful series of water gauge readings was commenced June 1, 1892, to continue through the months of June, July, and August for the purpose of obtaining data for adjusting the zero of the gauge to a uniform level with the gauges at the other harbors.

The bridge of the Chicago and North Michigan Railroad across the lower end of Pine Lake has been completed and is found to be load-bearing and built in accordance with the plans approved by the Secretary of War under date of September 4, 1891.

The Government dredge *Saginaw* is now on the way to the harbor to deepen the entrance and both channels.

The present working balance is \$2,300, about half of which will be required to pay for dredging and the remainder will be held for repairs.

For 1894 the estimate is, for two cribs in the south pier, to com

ording to the present project, \$9,500; rebuilding 500 feet of inner
f north pier, \$15,000; sheet piling 2,450 linear feet of plank-beam
ment, \$12,500; and extending the revetments of the upper channel
linear feet, \$7,200. The total cost of these items, with \$3,000 for
ging during two seasons and 10 per cent for contingencies, is
00.

harbor is included in the Michigan collection district, Michigan. The nearest
f entry is Grand Haven, Mich.
light-house establishment maintains a fifth-order light on the north pier.

Appropriations for improving harbor at Charlevoix, Mich.

st 14, 1876	\$10,000	August 5, 1886.....	\$10,000
18, 1878	12,000	August 11, 1888.....	12,500
13, 1879	9,000	September 19, 1890	9,000
14, 1880	10,000		
13, 1881	10,000	Total	102,500
st 2, 1882.....	10,000		
5, 1884	10,000		

nal estimated cost of work, 1868, as amended in 1876 and 1884	\$186,000.00
e amount appropriated from 1868 to and including act of Septem- 19, 1890	102,500.00
e amount expended to June 30, 1892	99,944.24

Money statement.

1, 1891, balance unexpended.....	\$6,230.20
30, 1892, amount expended during fiscal year.....	3,674.44
1, 1892, balance unexpended	2,555.76
1, 1892, outstanding liabilities	247.12
1, 1892, balance available	2,308.64
unt appropriated by act approved July 13, 1892	10,000.00
unt available for fiscal year ending June 30, 1893	12,308.64
ount (estimated) required for completion of existing project.....	73,500.00
ount that can be profitably expended in fiscal year ending June 30, 1894	52,000.00
mitted in compliance with requirements of sections 2 of river and arbor acts of 1866 and 1867.	

COMMERCIAL STATISTICS, CHARLEVOIX HARBOR, MICHIGAN.

Entrances and clearances.

Years.	Number.	Revenue collected.	Tonnage.
Year:			
84.....	772	\$355.46	
85.....	599	779.53	
86.....	674	390.46	
87.....	753		151,360
Year:			
.....	526		92,306
.....	473		
.....	532		75,224
.....	534		79,613

Receipts and shipments by vessel, 1891.*

Articles received.	Quantity.	Tons.	Articles shipped.	Quantity.
Beer..... barrels..	75	8	Bark..... cords..	7,764
Coal..... tons..	2,936		Fish..... tons..	
Feed..... bushels..	200	5	Hay..... do.....	
Flour..... barrels..	329	32	Hoops..... bundles..	6,509
Grain..... bushels..	990	1,237	Lath..... M.....	617
Hay..... tons..		508	Lime..... tons..	
Machinery..... do.....		15	Lumber..... M. feet, B. M..	14,899
Merchandise..... do.....		447	Merchandise..... tons..	
Salt..... barrels..	185	26	Pig iron.....	
Steel rails..... tons..		1,642	Posts, cedar, No. 3..... number..	118,476
Shingles..... M.....	1,113	139	Potatoes, bushels..	4,366
Stone..... tons..		1,676	Shingles..... M.....	70
			Ties..... number..	931,476
			Wood..... cords..	3,366
Total.....		8,471	Total.....	

* Compiled from statements furnish

of customs and by the mayor of Charle

IMPROVEMENT OF PORT HARBOR, MICHIGAN.

The project for this harbor was first authorized in 1866 and amended in 1868 and 1879, provides for a channel 70 feet wide and 12 feet deep, protected by piers and revetments.

The original facilities for navigation previous to the commencement of the improvement by the Government in 1866 consisted in a channel 70 or 80 feet wide, with a depth of 3 to 4 feet. These conditions have been greatly improved and at the present time, with the help of occasional dredging, a depth of 10 to 12 feet is maintained.

The survey made March, 1892, shows that the 10-foot contour in the lake is about 250 feet in advance of the end of the north pier, and in consequence of this insufficient development, frequent dredging is required to maintain the needed depth at the entrance. The depth at the outlying bar was found this spring to be 11 feet, but the 15-foot contour is immediately beyond, about 420 feet outside of the pier end of the south pier, and thence outwards the lake deepens rapidly the offshore contour swinging boldly in towards the land.

It is evident that the piers should be built out to at least the 15-foot curve, but the pier development provided for by the present project is 300 feet on the north pier and 100 feet on the south pier, fall 100 feet short of this on the north and 400 feet on the south pier.

At the beginning of the fiscal year the available depth at the entrance was 10 feet and on the 21st of July the Government dredge *inavo* arrived to prepare better water. The channel was dredged throughout, 70 feet wide, from Lake Michigan to about shore line, 40 feet wide from there to Lake Aux Bees Scies, and 14 feet deep, a quantity of sand removed was 22,770 cubic yards. The dredging was completed August 20, 1891.

During the remainder of 1891 no further difficulty was encountered by the vessels frequenting the harbor, but at the opening of navigation this spring a depth of only 11 feet was found on the bar in the middle of the entrance, and the shoal overlapping the end of the north

encroached considerably on the channel between the piers. Towards the end of the fiscal year, however, there was a navigable depth of 13 feet, in great part due to the rise in the lake level.

The required material for making some minor repairs to the piers and revetments, and for sheet piling the latter to make them sand tight, has been purchased, and this work will be done as soon as the land can be spared for use at this harbor.

A permanent bench mark was established at the northwest corner of the water-works building, on top of the water table and connected by duplicate levels with the U. S. gauge, and a careful series of water-gage readings was commenced June 1, 1892, to be continued through the months of June, July, and August, for the purpose of obtaining data for adjusting the zero of the gauge to a uniform level with the gages at the other harbors.

The present working balance is about \$4,400, with which it is proposed to sheet-pile the revetments, make some minor repairs to the existing works, and to pay for the necessary dredging restore the entrance channel to the required width and depth.

The Toledo, Ann Arbor, and North Michigan Railway proposes making Frankfort a terminus for the establishment of a new trans-lake route by means of a system of steam ferries for transportation of cars without breaking bulk. The experiment is one of great importance and its success would be a notable event in the development of lake business.

For 1894 the following estimate is submitted:

To complete the present project by adding 300 linear feet of crib work to the north pier, \$28,500; and 100 linear feet of crib work to the south pier, \$12,000; for additional repairs, \$2,000; and for two seasons of piling, \$5,000, which, with 10 per cent for contingencies, makes \$48,000.

The light-house establishment maintains a sixth-order light near the end of the north pier, and the Life-Saving Service a station at the inner end.

This harbor is included in the Michigan collection district, Michigan. The nearest point of entry is Grand Haven.

Appropriations for improving harbor at Frankfort, Mich.

Bees-Scies:		Frankfort, Mich.:	
June 23, 1866	\$88,541.00	June 18, 1878	\$8,800.00
March 2, 1867	10,000.00	March 3, 1879	4,000.00
July 25, 1868	10,000.00	June 14, 1880	5,000.00
April 10, 1869	29,318.85	March 3, 1881	10,000.00
July 11, 1870	10,000.00	August 2, 1882	15,000.00
March 3, 1871	10,000.00	July 5, 1884	5,000.00
Frankfort, Mich.:		August 5, 1886	7,000.00
June 10, 1872	10,000.00	August 11, 1888	8,000.00
March 3, 1873	10,000.00	September 19, 1890	10,000.00
June 23, 1874	10,000.00		
March 3, 1875	10,000.00	Total	273,659.85
August 14, 1876	3,000.00		

Original estimated cost of work in 1866, as amended in 1868, 1879, and 1892	\$329,655.85
Whole amount appropriated from 1866 to and including act of September 19, 1890	273,659.85
Amount covered into the Treasury (Report 1871, page 133)	5,721.50
Whole amount expended to June 30, 1892	263,511.86

Money statement.

July 1, 1891, balance unexpended.....	89
June 30, 1892, amount expended during fiscal year.....	8
July 1, 1892, balance unexpended.....	4
July 1, 1892, outstanding liabilities.....	
July 1, 1892, balance available.....	4
Amount appropriated by act approved July 13, 1892.....	10
Amount available for fiscal year ending June 30, 1893.....	14
{ Amount (estimated) required for completion of existing project.....	
{ Amount that can be profitably expended in fiscal year ending June 30, 1894.....	
{ Submitted in compliance with requirements of sections 2 of river and harbor acts of 1866 and 1867.	

COMMERCIAL STATISTICS OF PORT HARBOR, MICHIGAN.

Entrances and Clearances.

Years.	Number.	Revenue collected.
Fiscal year:		
1884.....	488	\$404.70
1885.....	261	75.66
1886, 1887, 1888.....	(*)	(*)
Calendar year:		
1888.....	1,342	
1889.....	(*)	(*)
1890.....	443	
1891.....	1,541	

* Not stated.

*Receipts and shipments by vessel, 1891.**

Articles received.	Quantity.	Tons.	Articles shipped.	Quantity
Beer..... barrels.....	80	8	Bark..... cords.....	7,416
Coal..... tons.....		125	Fish..... tons.....	
Flour..... barrels.....	500	50	Fruit..... do.....	
Feed..... tons.....		192	Lumber..... M feet B. M.....	17,571
Grain..... bushels.....	10,460	288	Potatoes..... bushels.....	3,008
Hay..... tons.....		500	Posts.....	8,000
Iron..... do.....		50	Shingles..... M.....	3,660
Lath..... M.....	50	15	Slabs..... cords.....	2,425
Live stock..... head.....	125	22	Ties.....	5,100
Machinery..... tons.....		6	Wood..... cords.....	4,376
Lime and cement..... do.....		100		
Merchandise..... do.....		1,050	Total.....	
Stone..... do.....		100		
Total.....		2,506		

* Compiled from statement furnished by the collector of customs and by the mayor of Fremont.

KK 4.

IMPROVEMENT OF HARBOR OF REFUGE AT PORTAGE LAKE, MICHIGAN.

The official project adopted in 1879 and modified in 1880-8 make this a harbor of refuge with a channel 370 feet wide and deep, protected by piers and revetments.

The north pier consists of 1,240 linear feet of pile work filled with edgings, and 151 feet of crib work, and projects 550 feet beyond present shore line.

outh pier is a pile and edging structure 1,380 feet long, and projected 100 feet beyond the present shore line.

As stated in the previous Annual Report, the revetments are in a poor condition. On the north side the superstructure timbers have almost totally disappeared, or are made unserviceable by the failure of the stay-ties; the filling has washed out for a length of some 400 linear feet, giving the sea a clean sweep through the work and filling the channel with sand from the beach and banks, and the piles of the side walls on a considerable stretch pushed out of position to such an extent that they are useless. The outer half of the south pier is almost abandoned, in short, almost the whole pile work should be removed and replaced by more substantial structures.

Even the present pile piers and revetments, if they were in serviceable condition, would be of some benefit, they would not of themselves afford a navigable depth of water in the channel. At present the 10-foot gage in the lake passes nearly 250 feet outside of the end of the pier.

Soundings made in May, 1892, showed a narrow and crooked channel with less than 7 feet water between the piers, and the Government *Saginaw* was sent to improve the navigation. The dredge arrived June 3, and to June 29 completed a cut 40 feet wide and 13 feet deep from lake to lake, removing 20,284 cubic yards.

No attempt has been made to expend any part of the available balance on the credit of the harbor on repairs to the revetments, for it would have been so much money wasted, inasmuch as nothing but radical repairment will make them useful.

A permanent bench mark was established and connected by duplicate lines with the United States gage, and a series of accurate water-level readings was commenced June 1, 1892, to be continued through the months of June, July, and August for the purpose of obtaining data for adjusting the zero of the gage to a uniform level with the gauges at other harbors.

The present working balance of \$4,100 will be used in keeping the harbor partially accessible by dredging, and to do such repairing as may be practicable.

Notwithstanding this exhibit it still remains, as stated in the reports of the last two years, that the natural conditions at Portage Lake, its size, expanse, ample depth, and central position in the dangerous strait of 50 miles between Point Betsey and Little Point Sable, justify the expense of the original project, and warrant renewed recommendation that this project be carried out.

A harbor of refuge is needed in this locality, as there is no safe shelter or anchorage in westerly gales between Ludington and South Haven.

As recommended in the previous Annual Report are: 1. For 4 cribs, 200 feet, on north pier, \$20,000; for 7 cribs, 200 feet, on south pier, \$33,000 (these constructions being in each case 100 feet of the projected crib piers); for rebuilding 1,240 linear feet of the north pier, \$38,000; sheet piling and filling, \$6,000, and for dredging, \$10,000. The total of these, with allowance for contingencies, etc., is \$107,000.

The Light-House Establishment has recently constructed a fourth-order light-house at the end of the north pier, upon the only portion of the harbor works capable of supporting it.

The harbor is included in the Michigan collection district, Michigan. The nearest town is Grand Haven, Mich.

2322 REPORT OF THE CHIEF OF ENGINEERS, U. S. ARMY.

Appropriations for harbor of refuge at Portage Lake, Michigan.

March 3, 1879.....	\$10,000	August 5, 1886.....	\$15,000
June 14, 1880.....	10,000	August 11, 1888.....	10,000
March 3, 1881.....	10,000	September 19, 1890.....	8,000
August 2, 1882.....	25,000		
July 5, 1884.....	12,500	Total.....	100,000
Original estimated cost of work, 1879, as amended in 1890..... \$267,500.00			
Whole amount appropriated from 1889 to and including act of September 19, 1890..... 100,500.00			
Whole amount expended to June 30, 1892..... 95,655.22			

It has been impracticable to obtain commercial statistics for Portage Lake Harbor.

Money statement.

July 1, 1891, balance unexpended.....	\$7,458.15
June 30, 1892, amount expended during fiscal year.....	2,014.07
July 1, 1892, balance unexpended.....	5,444.08
July 1, 1892, outstanding liabilities.....	1,337.47
July 1, 1892, balance available.....	4,106.61
{ Amount (estimated) required for completion of existing project.....	167,000.00
{ Amount that can be profitably expended in fiscal year ending June 30, 1894.....	125,000.00
{ Submitted in compliance with requirements of sections 2 of river and harbor acts of 1866 and 1867.	

K K 5.

IMPROVEMENT OF MANISTEE HARBOR, MICHIGAN.

The project of 1866, modified in 1870, 1874, and 1890, provides for a channel of navigable width and 15 feet deep, extending from Lake Michigan to Manistee Lake, and protected at the entrance by piers and revetments. With the cribs placed during the season of 1891 the piers and revetments have the following development:

North pier, 1,251 linear feet crib work and 1,304 feet of pile revetment. It projects 850 feet beyond present shore line.

South pier, 1,199 linear feet crib work and 490 feet pile revetment. It projects 650 feet beyond present shore line.

At the beginning of the fiscal year a contract was in force with C. H. Starke to complete the dredging of the river to Manistee Lake, and this work was completed August 28. Before leaving the dredge went again over the work of the previous season so that at the end of August there was a through channel 50 feet wide and 15 feet deep from lake to lake, the width being increased to 75 feet at the bend opposite Canfield & Wheeler's sawmill and to 100 feet at the entrance. The material removed during the fiscal year measured 40,784 cubic yards. An examination made in May, 1892, showed that the depth in the river was diminished to 13 feet, with 15 feet at the entrance, which, however, may be looked upon as very good when the long reaches of unprotected bank along the river and the great number of vessels of all sizes using the navigation are taken into consideration,

The depth at the entrance will continue to be unreliable until the piers shall have been built out to their projected length, which requires addition of 350 feet to each pier.

The contract with George W. Crouter for the construction of four cribs on the north pier, which was under way at the commencement of the fiscal year, was completed in September, 1891. The cribs were each 50 feet long, 24 feet wide, and with the superstructure 22 feet 8 inches high. They rest on a pile foundation at a depth of 16 feet 3 inches below the zero of the gauge, the piles penetrating from 15 to 17 feet to the bottom. The top of the superstructure is at the same level as that of the adjacent old work, viz, 6 feet 5 inches above zero of gauge. At the outer end the new work is protected by a structure of guard piles and horizontal waling timbers, filling the space between the end piers, and a mooring post is built into each crib. The cribs were filled with stone and decked over. The actual addition to the length of the pier by the four cribs is 202 linear feet.

Before final acceptance of the work it was examined by a diver and found to be satisfactory.

The north revetment being in a tumble-down condition, the part from station 3+24 to the life-saving station, a distance of 764 linear feet, was cut down to the water surface and rebuilt with four courses of superstructure. The old filling was taken out and the suitable portion replaced in a substantial manner in the work, reaching as high as the lower cross-ties and ballasted with old stone. The sink and water holes along the rear of the work due to seepage and subsidence were filled up with the old filling.

A passage 8 feet wide for the light-keeper's boat was prepared through the new work at Station 7+61 to 7+69.

The upper part of the north pier, Station 14+90 to 15+50, a distance of 60 feet, where the side walls were crumbling away and the filling exposed to the danger of sliding into the channel, was repaired, the projecting part of the square crib in the south pier (Station 6+16 to 6+46) was repaired and decked, and a plank walk was laid on south pier from the inner end to the fog-horn house, a distance of 800 feet.

All the above repairs were made by day labor and purchased material, commencing the work in September, 1891, and completing it in June, 1892, operations having been suspended during the winter months.

A permanent bench mark was established and connected by duplicate levels with the United States gauge, and a series of accurate water-gauge readings was commenced June 1, 1892, to be continued through the months of June, July, and August for the purpose of obtaining data for adjusting the zero of the gauge to a uniform level with the gauges at the other harbors.

At the present time the existing works are in the following condition:

North pier.—The revetment from the inner end to Station 3+24, a distance of 390 feet, is in urgent need of new superstructure. The north revetment is exceptionally exposed to wear and tear, the sailing vessels frequenting the harbor being in the habit of tying up here during heavy weather on the lake or while waiting for a cargo. The part from station 3+24 to the life-saving station, a distance of 764 feet, has been rebuilt, and the rest should be repaired likewise as soon as possible to prevent it from being totally wrecked.

Part from the life saving station to 12+38, a length of 130 feet, should be treated in the same manner. Oak snubbing posts should be

secured to the rear wall at distances of 50 feet along this whole work to keep vessels from making fast to the cross-ties, which is one of the main causes of the early ruin of the structure.

The north pier from Station 12+38 to 18+83, a distance of 645 feet, is now more than 20 years old, and the part above water, although patched and repaired from time to time, is in urgent need of rebuilding. It is continually giving away in places, and it is getting more and more difficult to repair it, as the old timber is too rotten to hold a bolt. Most of the cross-ties are rotten and broken.

The part from Station 18+83 to the outer end, a distance of 606 feet, is in good condition, but needs some additional filling in the new cribs and some stone riprap along the foot of the lake wall to prevent its being undermined. This should be done before another winter.

South pier.—The inner end consists of some 490 linear feet of pile revetment, built in 1874 and 1875. This work is occupied by the Canfield and Wheeler Lumber Company as a lumber wharf, and in consequence of age and hard usage is completely wrecked. It seems no longer capable of carrying the load placed on it and threatens to cave into the channel. If continued as a part of the lumber yard the company may properly be called upon to rebuild it.

The south pier from Station 0 to 6 + 46, 646 linear feet, is crib work, built previous to 1872. At its inner end it is breaking down, but the remainder is still in tolerably fair condition, and will stand some years yet. From 6 + 46 to the outer end, 451 linear feet, the pier is in good condition.

With the present working balance of \$4,800 it is proposed to purchase about 150 cords of stone for refilling the end of north pier and riprapping the foot of the lake wall, to make minor repairs, and to reserve the rest for dredging the entrance when necessary.

The recommendation for 1894 is as follows: For seven cribs, 350 feet, on the north pier; and seven cribs, 350 feet, on the south pier, to complete the present project, \$84,000; for repairing 645 linear feet of north pier, \$6,450; for repairing 500 linear feet of north revetment, \$3,500, and for two seasons' dredging in the harbor, \$4,000; which, with contingencies, make \$110,000.

The Light-House Establishment maintains a sixth-order light and steam fog signal on the south pier, and the Life-Saving Service has a station near the inner end of the north pier.

This harbor is included in the Michigan collection district, Michigan. The nearest port of entry is Grand Haven, Mich.

Appropriations for improving harbor at Manistee, Mich.

March 2, 1867	\$60,000	June 4, 1880	\$10,000
July 11, 1870	20,000	March 3, 1881	10,000
March 3, 1871	9,000	August 2, 1882	15,000
June 10, 1872	10,000	July 5, 1884	10,000
March 3, 1873	10,000	August 5, 1886	10,000
June 23, 1874	10,000	August 11, 1888	10,000
March 3, 1875	25,000	September 19, 1890	50,000
August 14, 1876	14,000		
June 18, 1878	15,000		
March 3, 1879	10,000		
		Total	<u>298,000</u>

Original estimated cost of works, 1866, as amended in 1871, 1873, and 1875, 1890, and 1892	\$408,000.00
Whole amount appropriated from 1866 to and including act of September 19, 1890	298,000.00
Whole amount expended to June 30, 1892	292,669.31

Money statement.

July 1, 1891, balance unexpended.....	\$35,090.94
June 30, 1892, amount expended during fiscal year.....	29,760.25
July 1, 1892, balance unexpended.....	5,330.69
July 1, 1892, outstanding liabilities.....	510.52
July 1, 1892, balance available.....	4,820.17
Amount appropriated by act approved July 13, 1892.....	50,000.00
Amount available for fiscal year ending June 30, 1893.....	54,820.17
Amount (estimated) required for completion of existing project.....	25,000.00
Amount that can be profitably expended in fiscal year ending June 30, 1894.....	60,000.00
Submitted in compliance with requirements of sections 2 of river and harbor acts of 1866 and 1867.	

COMMERCIAL STATISTICS, MANISTER HARBOR, MICHIGAN.

Entrances and clearances.

Years.	Number.	Revenue collected.	Tonnage.
Fiscal year:			
1884.....	3,609	\$3,181.19	
1885.....	3,560	1,145.29	
1886.....	3,352	634.26	
1887.....	3,412		991,872
1888.....	3,450		889,879
Calendar year:			
1888.....	3,595		966,221
1889.....	3,524		945,329
1890.....	3,691		975,049
1891.....	3,617		1,079,818

*Receipts and shipments by vessel, 1891.**

Articles received.			Articles shipped.		
Quantity.	Tons.		Quantity.	Tons.	
Beer.....barrels..	610	61	Bark.....cords..	7,528	7,528
Brick.....M.....	528	1,325	Coal.....tons..		1,700
Coal.....tons..		6,609	Fish.....do.....		487
Eggs.....dozen..	27,658	17	Fruit.....do.....		300
Flour.....barrels.	13,221	1,322	Furniture.....do..		75
Feed.....tons..		1,181	Lime and cement...barrels	250	33
Gravel.....do.....		6,500	Lumber.....M feet.	283,320	495,810
Grain.....bushels.	129,800	3,570	Lath.....M.....	27,452	8,236
Hay.....tons..		757	Machinery.....tons.		50
Iron.....do.....		200	Merchandise.....do..		1,869
Live stock.....head.	198	35	Posts.....number..	113,140	2,263
Lime and cement....barrels.	1,000	130	Potatoes.....bushels.	86,000	2,580
Machinery.....tons.		250	Salt.....barrels..	1,077,162	150,851
Merchandise.....do..		14,590	Slabs.....cords..	2,849	7,658
Oil.....barrels..	20	4	Shingles.....M.....	154,041	19,255
Pork.....do.....	2,300	230	Stone.....tons..		625
Sugar.....do.....	273	68	Wood.....cords..	665	998
Vinegar.....do.....	123	12			
.....tons..		6,250	Total.....		700,318
Total.....		43,201			

*Compiled from statements furnished by the collector of customs and by the mayor of Manistee.

K K K

IMPROVEMENT OF LANSING HARBOR, MICHIGAN.

The project adopted in 1867 and modified in 1885 and 1889 for a channel 200 to 250 feet wide and 18 feet deep, piers and revetments.

The development of the protection works is as follows:

North pier consists of 1,432 linear feet of crib work, built in 1868 and 1890, and projects 900 feet beyond the present channel; its inner end it connects with an old slab revetment, built at the expense, and now in a tumble-down condition.

South pier consists of 567 linear feet of pile work, built 1870 and of 1,814 linear feet of crib work, built between 1866 and 1889, extends along the entire channel from Pere Marquette Island, Michigan and projects into the latter 1,500 feet.

Considerable portions of the present works are very old and now above the water line, particularly the part of north pier Station 5+88 to 12+36, a distance of 648 feet, the pile structure on south side 567 feet long, and the south crib work from Station 14+23, 856 linear feet.

The mentioned parts of the north pier especially are in need of repairs in consequence of serious abrasions by colliding vessels and undermining action of steamers.

The remaining structures are in good condition and only need additional stone ballast and riprap around the outer ends of the piers.

The dredging by Government plant, which was under way at the beginning of the fiscal year, was continued till July 20, by which the channel 18 feet deep and 40 feet wide, extending from lake to lake has been excavated.

The Government dredge returned to the harbor early in November but the season was too far advanced to allow of much work. However the worst portion of the channel was again deepened, and no trouble was experienced by the vessels frequenting the harbor during winter on account of insufficient water.

The quantity of sand removed during the year was 19,397 cubic yards. The available depth when surveyed in April, 1892, was 14 feet to be nowhere less than 14.5 feet, the lake level being $1\frac{1}{2}$ feet below zero plane.

In order to remove any doubts regarding the stability of the piers in extension of the two piers during 1889 and 1890, the foundations were again examined by a diver in September, 1891, and no changes discovered. During the winter the dredge *Saginaw* received repairs to machinery, etc., as were needed to place her in condition for the coming season's work.

A permanent bench mark was established and connected by duplicate work with the United States gauge. The bench mark is on stone pier on west side of harbor, east face of pump-house of water works, 100 feet above the water level of the gauge. A series of accurate water-level observations were made to be continued during June, July, and August, 1892, for obtaining data for adjusting the zero of the gauge with the gauges at the other harbor. The balance available July 1, \$3,100 for the purpose of making repairs and stone filling and riprap around the piers; Rebuilding above

ted length, the intended depth of 12 feet can not be maintained at repeated dredging, and this is caused partly by the inefficiency inshore revetments to exclude the sand, and partly by the ad- of the shore line since the project was made.

10-foot curve of Lake Michigan is about even with the present the south pier, and the 15-foot curve is 400 feet farther out. It dent that the piers are in need of additional length if the pro- depth of 12 feet is to be maintained between them.

siderable portions of the piers and revetments, from age and ac- , have become inoperative in excluding the sand coming from each and banks, and are in urgent need of repairs.

north pier crib work from Station 1+51 to 3+50 and the pile from 3+50 to 6+72, a total length of 521 feet, and the south pier work from Station 0 to 6+19, a length of 619 feet, are now more 20 years old and should be rebuilt above the water line. The

side revetment has been rebuilt during recent years, as y as the funds permitted, 725 linear feet, Station 6+19 to 13+44 g received new superstructure, but the inner end from Station i to 17+61, a length of 417 feet, still remains to be treated in the manner. In addition most of this pile work needs sheet piling the rear to make it sand tight.

250 feet of railroad bulkhead at the inner end of the south revet- is in a tumble-down condition; the piles are decayed; it is quite d of filling, and large quantities of sand are sucked through it he channel by the action of passing vessels.

inner end of the north revetment has been wrecked by collisions eeds rebuilding.

the beginning of the fiscal year the available depth of water in hannel was 10 feet, and August 18 the Government dredge *Far-* commenced improving the waterway, completing this work Octo-

During this time a 50-foot channel 14 feet deep was dredged Pentwater Lake to Lake Michigan, removing therefrom material amount of 15,395 cubic yards.

further trouble was experienced during the year, but soundings April 11, 1892, showed that the channel had again filled up, a h of about 600 feet near the shore line having barely 10 feet of . It is intended to send a dredge to this harbor as soon as prac- e.

repairs of the south revetment, which were under way on July re completed November 30, the work having been suspended dur- vo and a half months of this time, to utilize the services of the over- m the improvement of the St. Joseph River. The repaired por- xtends from Station 8+88 to 13+44, a distance of 456 feet. The were sawed off, and a new superstructure four courses high in front wo courses high in rear with cross-ties 8 feet apart, built on top; d filling was overhauled to below the water line and new edgings tone placed in the work.

plank walk was laid over the south revetment for a length of feet to connect with the elevated walk to the pier light.

ermanent bench mark was established on top of a large bowlder ating from the foundation of the Sands & Maxwell furniture factory, uthwest corner, and connected by duplicate levels with the tates gauge, and a series of accurate water-gauge readings enced June 1, to be continued through June, July, and e the purpose of obtaining data for adjusting the zero of the n level with the gauges at the other harbors.

It is proposed to use the balance available July 1 (\$2,400) for redressing the entrance and for minor repairs.

The needs of the harbor have been outlined above. To meet the requirements, the following estimate is submitted: For 4 cribs for son pier (to complete the present project), \$24,000; for rebuilding abutment water and refilling 521 feet of north pier, 619 feet of south pier and 1,557 feet of south revetment, 1,557 feet in all, \$15,570; for sheet piling 1,400 feet of north pier and revetment and 780 feet of south revetment total of 1,830 feet, \$5,490; and for dredging, \$3,000; which, with estimate for contingencies, makes \$53,000.

The light-house establishment maintains a sixth-order light, and the Life-Saving Service a station on the north pier.

This harbor is included in the Michigan collection district, Michigan. The nearest port of entry is Grand Haven.

Appropriations for improving harbor at Pentwater, Mich.

March 2, 1867	\$55,000	June 14, 1880	24
April 10, 1869	17,820	March 3, 1881	10
July 11, 1870	10,000	August 2, 1882	10
March 3, 1871	10,000	July 5, 1884	15
June 10, 1872	30,000	August 5, 1886	10
March 3, 1873	20,000	August 11, 1888	8
August 14, 1876	10,000	September 19, 1890	8
June 18, 1878	10,000		
March 3, 1879	6,000	Total	233

Original estimated cost of the work, 1866, amended in 1873, 1884, and 1892	\$286,82
Whole amount appropriated from 1866 to and including act of September 19, 1890	233,82
Whole amount expended to June 30, 1892	231,39

Money statement.

July 1, 1891, balance unexpended	\$7,81
June 30, 1892, amount expended during fiscal year	5,39
July 1, 1892, balance unexpended	2,43
July 1, 1892, outstanding liabilities	1
July 1, 1892, balance available	2,41
Amount appropriated by act approved July 13, 1892	5,00
Amount available for fiscal year ending June 30, 1893	7,41

{ Amount (estimated) required for completion of existing project	24,00
{ Amount that can be profitably expended in fiscal year ending June 30, 1894	48,00
{ Submitted in compliance with requirements of sections 2 of river and harbor acts of 1866 and 1867.	

COMMERCIAL STATISTICS, PENTWATER HARBOR, MICHIGAN.

Entrances and clearances.

Year.	Number.	Revenue collected.	Ton
Fiscal year:			
1884	461	\$400.99
1885	187	69.17
1886	(*)	(*)
1887	(*)	(*)
Calendar year:			
1888	300	
1889	(*)	(*)
1890	27	
1891	1,140	

* Not stated.

*Receipts and shipments by vessel, 1891.**

Articles received.	Quantity.	Tons.	Articles shipped.	Quantity.	Tons.
..... tons.....		100	Brick.....number..	600,000	1,200
..... do.....		115	Carrriages.....	30	10
ulture..... do.....		18	Fruit and vegetables..... tons.....		550
vel..... do.....		300	Fish..... do.....		15
on..... bushels.....	14,851	342	Flour..... barrels.....	721	72
y and feed..... tons.....		342	Furniture..... tons.....		535
s..... do.....		60	Lath.....	905,000	272
lber..... M feet, B. M.....	1,000	2,000	Live stock..... head.....	6	3
restock..... head.....	34	17	Lumber..... feet, B. M.....	5,887	11,774
se and cement..... barrels.....	500	50	Merchandise..... tons.....		41
achinery..... tons.....		45	Posts..... number.....	45,217	1,130
erchandise..... do.....		566	Shingles.....	11,252	1,687
k..... barrels.....	965	144	Ties..... number.....	12,500	1,125
ce..... cords.....	90	570	Wood..... cords.....	250	500
Total.....		4,069	Total.....		18,914

* Compiled from statement furnished by D. C. Wickham, of Pentwater, Mich.

K K 8.

IMPROVEMENT OF WHITE RIVER (LAKE) HARBOR, MICHIGAN.

The present project is for a 12-foot navigation 200 feet wide, and protected by piers and revetments, between Lake Michigan and White Lake.

The north pier is a pile structure filled with edgings and stone, 115 feet long, and projecting about 350 feet beyond the present shore line.

The south pier consists of 356 feet of crib work at its outer end, and 1,498 feet of pile pier and revetment. It projects about 630 feet beyond the present shore line.

Although the project contemplates a channel with a navigable depth of 12 feet, it has not been possible to attain this object, except for short periods of time by means of repeated dredging, for the reason that the piers are not sufficiently extended and the inshore revetments are not tight.

With reference to the existing project, the north pier still needs 250 feet and the south pier 200 feet of crib work to complete them.

The north pier at present reaches only to the 8-foot curve in the lake, and even after its completion will fall some 300 feet short of the 15-foot curve, while the south pier after the addition of the proposed 200 feet will probably be long enough for a considerable time to come. It is evident that large quantities of sand find their way into the channel from the North Beach around the end of north pier, and the latter should be extended as soon as possible. Although on general principles the extension of the north pier requires the first attention, the end of the south pier is in a somewhat unstable condition. The end of the crib is slowly but steadily sinking into the bottom, and at least one pile should be added to the pier at the first opportunity to hold the crib work.

Considerable portions of both piers and revetments have been removed during the last few years, but there still remains 316 feet of the south revetment (Station 7+40 to 10+56) and 367 linear feet of crib work (Station 3+56 to 7+23) which are now

22 years old, and require new superstructure and filling. The mentioned portion of north pier particularly is in urgent need of attention as the superstructure timbers are nearly gone and the filling being washed out permits the sand from the rear to wash into the channel. Part of the south-side crib work also needs repairs very badly, especially the cribs (Station 2+55 to 3+05 and 0+50 to 1+00) which have lost part of their lake wall.

In order to make the pile structures as nearly sand tight as practicable, they must be sheet-piled along the rear, and material for this purpose is now on hand at the harbor. A special land driver was bought last year for this kind of work, but as it was first needed at other harbors it could not be made available for this harbor until now.

Operations have now been begun and the work will be pushed rapidly as circumstances will permit.

Soundings made March 21 showed an available depth of only 9 to 10 feet at the entrance over a distance of some 400 feet and it is intended to send one of the Government dredges to this harbor at the first opportunity.

A permanent bench mark was established on the doorsill, south of the light-house, and connected by duplicate levels with the United States gauge. Its elevation is 20.83 feet above the zero. A series of accurate water-gauge readings was commenced June 1, to be continued through June, July, and August, for the purpose of obtaining data for adjusting the zero of the gauge to a uniform level with the gauge at the other harbors.

The balance of \$11,400, available July 1, will pay for the work, in part under way, of sheet-piling part of the north and south pile structures for building new superstructure on north revetment from Station 7+10 to 10+56 (316 linear feet), for repairing the south-side crib work, and for the dredging required this season.

The amount that can be profitably expended during the next year is: For building five new cribs on north pier, \$23,750, and four cribs on south pier (to complete the project), \$24,000; for 367 linear feet new superstructure and filling on south pier, \$3,670; for sheet-piling 1,000 additional feet of north and south revetments, \$3,000; for dredging, \$3,000, which, with 10 per cent for contingencies, make \$63,000.

The Light-House Establishment maintains a fourth-order flashing coast light on the shore and a sixth-order harbor light on the south pier. The Life-Saving Station has a station on the north pier.

This harbor is included in the Michigan collection district, Michigan. The best port of entry is Grand Haven, Mich.

Appropriations for improving harbor at White River, Mich.

March 2, 1867.....	\$57,000	March 3, 1879.....	\$1
April 10, 1869.....	44,550	June 14, 1880.....	1
July 10, 1870.....	20,000	March 3, 1881.....	1
March 3, 1871.....	20,000	August 2, 1882.....	10
June 10, 1872.....	10,000	July 5, 1884.....	10
March 3, 1873.....	7,000	August 5, 1886.....	10
June 23, 1874.....	10,000	August 11, 1888.....	10
March 3, 1875.....	10,000	September 19, 1890.....	1
August 14, 1876.....	5,000		
June 18, 1878.....	12,000	Total.....	27
Original estimated cost of work, 1866, amended in 1873, 1881, and 1892..	\$337.50		
Whole amount appropriated, 1866, to and including act of September 19, 1890.....	274.50		
Whole amount expended to June 30, 1892.....	262.90		

Money statement.

1891, balance unexpended	\$18,117.94
1892, amount expended during fiscal year.....	6,482.12
892, balance unexpended	11,635.82
1892, outstanding liabilities.....	196.92
1892, balance available	11,439.90
appropriated by act approved July 13, 1892.....	5,000.00
available for fiscal year ending June 30, 1893.....	16,439.90
nt (estimated) required for completion of existing project.....	43,225.00
nt that can be profitably expended in fiscal year ending June 30, 1894	58,000.00
itted in compliance with requirements of sections 2 of river and	
bor acts of 1866 and 1867.	

COMMERCIAL STATISTICS, WHITE RIVER HARBOR, MICHIGAN.

Entrances and clearances.

Year.	Number.	Revenue collected.	Tonnage.
BAR:			
.....	304	\$1,347.23
.....	1,416	476.16
.....	1,623	313.65
.....	1,885	262,440
.....	1,742	184,247
YEAR:			
.....	1,408	147,142
.....	732
.....	579	62,276
.....	405	47,135

*Receipts and shipments by vessels, 1891.**

Articles received.	Quantity.	Tons.	Articles shipped.	Quantity.	Tons.
..... tons.....	82	Hay..... tons.....	20
..... do.....	127	Lumber..... feet, B. M.....	20,395	35,691
..... bushels.....	21,346	587	Lath.....	350,000	105
..... tons.....	5	Slabs..... cords.....	7,119	19,136
..... barrels.....	116	13	Ties..... number.....	17,000	1,530
..... dice..... tons.....	148	Wood and bark..... cords.....	735	919
..... barrels.....	10	2
..... and bark..... cords.....	205	256
..... barrels.....	20	2
.....	1,222	57,401

* Compiled from statement furnished by the collector of customs.

K K 9.

IMPROVEMENT OF MUSKEGON HARBOR, MICHIGAN.

Official project adopted in 1866 and amended in 1880 and 1889, again revised in 1892, provides for a navigation 15 feet deep, ten piers 300 feet apart at the entrance and about 180 feet at the line.

July 25, 1892, a special report was submitted to the Chief of En-

gineers, setting forth the condition and needs of the harbor, and showing with detailed estimates measures required for the construction and maintenance of a reliable 15-foot navigation. The report estimates an extension of both piers 800 feet to reach the 20 foot contour in lake; for dredging from lake to lake, with channel width of 75 feet depth of 18 feet, and for revetting both banks from the inner end of pier to Muskegon Lake. The total estimate for this work is \$260,000, and recommendation was made for appropriation of \$150,000 to be done in two seasons' work. A copy of this report is herewith for incorporation.

The north pier, as now existing, consists of 800 linear feet of crib work, parallel with and distant from south pier 300 feet; of 329.6 linear feet of crib work built in 1891-'92, connecting the outer portion of pier at an angle with the interior portion, in order to close the existing gap in the north pier; of 322 linear feet of old crib work and 392 linear feet of pile work parallel with the south pier, at a distance of 180 feet. It projects about 900 feet beyond the present line. The former end crib, which had been undermined and torn from the rest of the work in 1889, has been removed and is now under contract to be replaced, together with one additional crib, which has been considered necessary to add in order to insure the stability of the restored old crib.

The south pier consists of 800 linear feet of crib work and 380 linear feet of pile work, and projects about 920 feet beyond the present line.

Both piers are in good condition, with the exception of the inner end of the north-side crib work, which is now from 22 to 24 years old and needs rebuilding above the water line, and the north-side pile work, which is somewhat unstable, in consequence of undermining and needs an additional row of piles along the channel wall and other repairs. Besides, there is some additional stone needed to refill the crib work and to riprap the end of the south pier.

From the inner end of the Government works to Muskegon Lake the channel banks are partly revetted with slabs. This work was done in former years by the occupants of the shore, but since the lumber has been removed they have received no care and are now fast being broken down. The project includes the substitution of new sheet pile revetments for the old slab docks, and the material for 1,000 linear feet of this work for the north side is now on hand.

Contractor E. G. Crosby completed the removal of 316 linear feet of the inner north pier and the construction of the new crib work to close the gap between the remaining portion of the old pier and the detached pier in October, 1891. The removal of the old pier was effected by blasting it out with dynamite and dredging up the debris. All the old stone filling was saved and afterwards used again in the crib work. The latter consists of seven cribs, 20 feet wide, the cribs being framed to form close joints with the adjacent structures which they connect. The cribs rest on a pile foundation, and with their superstructure are 20½ feet high.

The total length of the new work is 329.6 feet on the channel line, 2 feet of which overlap the horns of the former detached piers at both ends it is securely connected with the old structures. It is filled with stone to the top and decked over.

Gaylord & Wing, the contractors for the raising and restoration of the outer sunken crib of the north pier, having commenced their operations June 29, continued their work under considerable difficulties. They removed the stone filling by means of divers, and

September had so far completed that portion of the undertaking justified in attempting to raise the crib with chains and jacked up on a specially constructed platform of piles and timber. The long exposure of the crib to the effects of the frequent storms of the season had weakened it, however, to such degree as to withstand the strain and tore apart just above the bottom. The two parts were then raised separately, towed to anchorage in Muskegon Lake, and left for the winter.

The stone removed from the crib by the contractors measured 153 cords of which were placed in the old work as filling, which is needed, the remaining 81 cords being stored on top of the pier for use in filling the restored crib. The 72 cords placed in the crib will be replaced by the contractors at their own cost. They were permitted to deposit it in the pier at their request, as it hindered their operations, and were in consequence willing to replace it at one dollar per cord, which is a much lower price than that at which material can be bought here under ordinary circumstances. As there was no possibility of their completing the work within the time specified by the contract, the contractors requested an extension to June 1, 1892, and this was granted by the Chief of Engineers.

The improbability of the contractors completing the work on the old crib before August 1, 1892, and the impracticability of having other contractors carry on operations at the end of the pier at the same time, eventually led to a recommendation to the Chief of Engineers to let the Gaylord & Wing contract for restoration, by adding to it the construction of the additional crib to be placed outside of the old one, and also to modify the first contract in so far as to the stone foundation, later developments showing that it was entirely practicable to replace the old crib on a pile foundation of more uniform work. This recommendation received the approval of the Chief of Engineers, and a supplementary contract including these provisions was concluded with Gaylord & Wing on May 5, 1892, the work to be completed September 1, 1892.

The contractors resumed operations under the modified contract May 1, 1892. The upper part of the old crib was cut up, saving only the unnecessary ties and the iron for use in the reconstructed crib. A new crib was placed under the recovered lower portion, which was to be restored by new timber blocking and bolting to a sound and sturdy condition, and the crib entirely rebuilt from the third course up. The suitable old and otherwise with new material. At the end of the fiscal year the structure had been completed to the twelfth

The construction of the new crib has progressed to include the twelfth course, which makes the crib ready for sinking in its place when the foundation shall have been prepared.

At the beginning of the fiscal year the navigable depth in the harbor was from 15 to 16 feet at the entrance and between the piers. Soon after the closing of the gap in the north pier this commenced to show itself on the entrance channel. The former gap had acted as a sort of barrier, the large body of water pouring through under the influence of northerly gales assisting by its scouring effect in maintaining a navigable depth in the outer portion of the fairway. The effect of the closing structure, namely, the removal of the dangerous currents at the entrance, had been fully attained, but the channel had become perceptibly narrowed. At the end of September the 15-foot depth had been reduced to 70 feet next to the south pier, and the remainder of the

space between the piers was occupied by a shoal with only 11 feet water over it.

In order, if practicable, to restore the needed depth of water, and the current satisfaction of the harbor interests, the Government dredge *Procyon* commenced work October 2, but the season was so rough that in November 26 but eleven days were partially fit for dredging, and four of these at the end of the pier. The benefit of this dredging to harbor was consequently very slight, and the cost per yard very heavy, 22 cents. Five thousand nine hundred and ten cubic yards the quantity removed during the seven and a half weeks that the dredge was kept in commission.

This endeavor to relieve navigation near the end of the pier and hole in the season was undertaken and continued with two objects in view: First, The attainment, if by chance it were possible, of the best result, as to which, however, no favorable expectations whatsoever were entertained. Second, To prove conclusively the practical difficulties and outlay involved in such an attempt, and thereby meet the facts, in lieu of arguments, the insistence on the part of those interested, both at Muskegon and at several other points, that the Government dredge be kept under pay for the work in question. The demonstration was convincing and worth what it cost to make it.

At the opening of navigation in 1892 great difficulty was immediately experienced by entering and departing vessels; a number of scows grounded in the shoal water and were with difficulty released. There was a depth of scant 12½ feet close to the south pier.

The Government dredge *Sigsbee* began work April 21, dug a canal 100 feet wide and 18 feet deep at the entrance, and removed the shoals in the inside navigation to Muskegon Lake to a depth of 16 feet, completing this work May 31. The quantity of material removed 30,914 cubic yards.

On May 5 the *Sigsbee*, while at work at the harbor entrance, the spuds down, was run into by the City of Racine, breaking two spuds. The dredge was delayed four days making repairs.

A permanent bench mark was established on top of the stone sill the most southerly window on the east side of the light-house. Its elevation is 9.43 feet above zero of gauge. A series of accurate water gauge readings was commenced June 1, to be continued through July, and August, for the purpose of procuring data for the adjustment of the zero of the gauge to a uniform level with the gauges at the other harbors.

Upon completion of the work now under contract both piers lack 800 feet of their full extension according to the revision of July, 1892.

The recommendations for the fiscal year 1894 are: For 322 linear feet new superstructure, north pier, Stations 3 + 16 to 6 + 38, \$3,220; for pairing north pile pier and revetment, \$700; for refilling both piers riprapping end of south pier, \$1,500; for building 1,000 linear feet sheet pile revetment, north side (timber and piles on hand), \$2,000; for additional feet sheet-pile revetment (1,000 on each side), \$12,000; ten new cribs on the north pier and ten on the south pier, \$120,000 for dredging, \$4,000, which, with 9 per cent for contingencies, make \$156,000.

The Light-House Establishment maintains a fourth-order coast light on the shore and a sixth-order harbor light on the south pier. The Life-Saving Service station on the north pier.

This harbor is included in the Michigan collection district, Michigan. The name of the port is Grand Haven, Mich.

Appropriations for improving harbor at Muskegon, Mich.

2, 1867	\$59, 000	March 3, 1881	\$20, 000
3, 1870	10, 000	August 2, 1882	25, 000
3, 1871	15, 000	July 5, 1884	20, 000
3, 1872	10, 000	August 5, 1886	12, 500
3, 1874	10, 000	August 11, 1888	45, 000
3, 1875	25, 000	September 19, 1890	50, 000
3, 1876	15, 000		
3, 1879	5, 000	Total	<u>329, 000</u>
4, 1880	7, 500		

Estimated cost of the work, 1866, amended in 1869, 1873, 1881, 1890, and 1892	\$589, 000. 00
Amount appropriated from 1866 to and including act of September 9, 1890	329, 000. 00
Amount expended to June 30, 1892	<u>317, 796. 53</u>

Money statement.

1891, balance unexpended	\$44, 727. 79
1890, amount expended during fiscal year	<u>33, 524. 32</u>
1892, balance unexpended	11, 203. 47
1892, outstanding liabilities	\$311. 23
1892, amount covered by uncompleted contracts	9, 640. 39
	<u>9, 951. 62</u>
1892, balance available	1, 251. 85
Amount appropriated by act approved July 13, 1892	<u>75, 000. 00</u>
Amount available for fiscal year ending June 30, 1893	<u>76, 251. 85</u>
Amount that can be profitably be expended in fiscal year ending June 30, 1894	156, 000. 00
Amount allotted in compliance with requirements of sections 2 of river and harbor acts of 1866 and 1867.	

COMMERCIAL STATISTICS, MUSKEGON HARBOR, MICHIGAN.

Entrances and clearances.

Years.	Number.	Revenue collected.	Tonnage.
1891:	7, 160	\$4, 987. 06	
.....	6, 112	1, 621. 34	
.....	5, 543	1, 025. 17	
.....	1, 745		232, 007
1892:	2, 685		
.....	4, 626		884, 869
.....	3, 786		649, 540
.....	2, 886		704, 046

REPORT OF THE CHIEF OF ENGINEERS, U. S. ARMY.

Receipt and shipment by canal, 1891.*

Quantity, Tons	Articles shipped.	Quantity, Tons
	Chesters.....barrels..	200
	Flour.....do.....	340
do.....	1,170
	Fruit and vegetables.....tons.....	
packages.....	34,221
tons.....	
M.....	13,747
M feet, B. M.....	274,986
tons.....	
do.....	
barrels.....	5,728
packages.....	11,101
do.....	34,641
barrels.....	21
M.....	3,465
cords.....	152,000
do.....	596
number.....	580
	Total.....	1,123,400

Board of Trade of Muskegon.

REPORT OF MAJOR WILLIAM [REDACTED] CORPS OF ENGINEERS.

ENGINEER OFFICE,

Muskegon, Mich., January 25, 1892.

GENERAL: Pursuant to telegraphic instructions of January 22, I beg to report as follows relative to the present condition of Muskegon Harbor, Michigan, and to what is required for its improvement. The accompanying blue prints show the condition of the harbor at the respective times of examination, viz, September, 1890, and November, 1891. From the depths as shown on the tracing are to be deducted the variation of the lake surface below the harbor zero, constituting the necessary rise or reduction, which vary 6 inches to 2 feet on the 1890 map and 2.7 on the 1891 map. As appears to be the case almost invariably, at this harbor a 13-foot bar exists outside the piers, and this depth is reduced at times by an abnormal wave action during heavy gales in fall and spring. The navigation depth is thus diminished at times to 11 feet, and the entrance becomes unsafe or impracticable for laden vessels of the ordinary lake drafts. The official project for Muskegon Harbor provides for an actual navigation of 13 feet, but it is evident that to secure this reliably three points must be covered.

1) The piers should have such extension into the lake as to cover the entrance to such depth beyond the actual navigable depth as to protect the channel when made, and to allow for the rise and fall of the sea and the send of a heavy vessel. This would indicate that the pier should reach at least the 20-foot contour in order to be sure of having 15 feet clear between them.

2) The fairway between the piers, and thence upward, should be dredged to a depth sufficient to allow for reduction of lake level be the zero plane, and for the inevitable filling in of the dredged channel which follows disturbance of the bottom and ensues from the breakdown of the channel walls and the readjustment of the bottom mater.

indicates dredging to, say, 18 feet in order to be sure of securing a 16 feet.

In order to prevent as far as may be the access of loose material to the banks, these must be revetted throughout the length of the dredged channel of entrance, as the action of currents, of waves, of ice, and of passing vessels tends constantly to the erosion of the channel banks, the seepage of sand into the channel, and its delivery into the fairway.

A complete statement of what is required for the thorough improvement of Muskegon Harbor, in order that its navigation may be fully and properly adapted to the commercial requirements, must include the following items:

(1) Pier extension in such measure as is needful to create and protect a clear 15-foot navigation. This, for the present, is assumed at about 800 feet, although it is by no means certain that ultimately the 25-foot contour in the lake should not be reached.

(2) The dredging of the channel or fairway from lake to lake to a depth, say, of 18 feet, with a width of 75 feet.

(3) The revetment of both banks, whenever unprotected, with as economical a structure as will be substantial. For this purpose the following estimates are made:

(1) Pier extension. To enable both piers to reach the 20-foot contour of the lake an extension of 800 feet is required for both. Estimating the cost of this work from the most recent and reliable data for similar additions, the cost per linear foot will average \$120.

(2) To dredge a channel 75 feet wide and 18 feet deep from lake to lake requires the handling of 54,000 cubic yards, which under contract will cost say 15 cents per cubic yard.

(3) The revetting of the naked banks involves covering some 5,600 square feet on the two banks, and the structure proposed for this will cost \$6 per linear foot. The total estimate therefore is as follows:

800 linear feet pier, at \$120	\$192,000
54,000 cubic yards dredging, at 15 cents	8,100
5,600 linear feet revetment, at \$6	33,600
Total	233,700
Contingencies, 11 per cent.....	26,300
Aggregate	260,000

The funds in hand will suffice to build a single crib on the north pier inside of the damaged crib, which is under contract of restoration, and the total pier extension above noted of 800 feet on each pier is computed from what would be the outer ends of the two piers, supposing the work now under way and immediately in prospect to have been completed. There are at this time the materials for about 1,000 linear feet of revetment on the north pier under contract, and we have funds enough to build it. The present revetment estimate, therefore, omits this from consideration. The dredging estimate is computed on the basis of the most recent survey.

The estimate of \$50,000 given in the annual report 1891, as the basis of appropriations for the fiscal year 1892, was intended to cover 300 feet extension of pier, half on each side; 2,000 linear feet of revetment, half on each side, with an allowance for dredging and contingencies.

Under the system of biennial appropriations for rivers and harbors which has prevailed of late years, the appropriations to be made at this time by Congress must do duty for two years. In the case of Muskegon Harbor work would be about as follows: Say 10 cribs for

per-annum, five on each side, 2,800 feet of revetment, and all dredging, the cost of which in all would be, with contingencies, \$28,000.

During the following season, the remaining half of the revetment would be built, and say, 12 more cribs, which, with some allowance dredging and contingencies, would cost say \$96,000, leaving the remaining ten cribs to be added here. It, therefore, the improvement of harbor were to be planned and prosecuted as an engineering measure without reference to the system of scaling annual appropriations, to grade the work and maintain it in an unfinished condition, the best method of appropriating would be the allotment of the required for the seasons of 1897 and 1898, namely \$176,000.

It is not to be expected that an appropriation of this amount could be had at one time, but it is evident from the data furnished that a proposition of \$176,000 could be advantageously expended in crossing two years.

In explanation of the commercial conditions expending upon access to the harbor of Muskegon, I can hardly do better than to my annual reports of 1880 and 1891, copies of which are herewith respectfully,

WILLIAM LUDLOW,
Major, Corps of Engineers,

Brig. Gen. THOMAS L. CASSET,
Chief of Engineers, U. S. A.

KKK

IMPROVEMENT OF GRAND HAVEN HARBOR, MICHIGAN.

The existing project adopted in 1866 and amended in 1880 and is to secure an entrance channel 18 feet deep between piers and break 400 feet apart.

The present construction is as follows:

On the north side 1,008 linear feet of crib work and 2,128 lines of pile pier and revetment, or a total of 3,187 linear feet, projecting 1,200 feet beyond the present shore line.

On the south side 1,295 linear feet of crib work and 4,272 lines of pile pier and revetment, a total of 5,477 linear feet, projecting feet beyond the present shore line.

No difficulty was experienced during the past year by entering departing vessels, the available depth never having been less than feet between the piers, and 17 to 18 feet on the crossing of the bar, and the steamers of the Detroit, Grand Haven, and Milwaukee Railway made regular trips throughout the winter.

The construction of 6 new cribs, 3 on each pier, commenced the previous fiscal year, under contract with E. G. Crosby, was pleted in October, 1891. The cribs are each 50 feet long and 30 feet wide, and with the superstructure about 27 feet high in the north and about 29 feet high in the south pier. Each crib rests on a foundation of 39 piles, the piles penetrating from 13 to 15 feet in the ground. The work was filled with stone to the top and decked over. A mooring post was built into each crib. The foundation piling of the new

examined by a diver and found to be satisfactory, with a few slight
 tions, which were remedied by driving three additional piles and
 ing them under the cribs, to assist the same number of original
 which had been found to project with part of their diameter out-

the lake bottom, which at the crib site had been 22 to 23 feet below
 ero of gauge previous to placing the cribs, scoured out afterwards
 lepth of 27 to 28 feet at the outer end of the south pier, but no
 onsions were entertained, as this was the greatest depth of
 there was any record, and the foundation piles, which are twelve
 in number under each crib than those under any crib placed here-
 e, still had a penetration of about 10 feet. However, the great
 of December 4 and 5 threatened destruction to the new work.
 t of this under date of February 18, 1892, is herewith. This gale
 h blew continuously from the southwest with a maximum velocity of
 iles on the 4th and 49 miles on the 5th created a head of water in
 angle between the south pier and the beach, causing a powerful off-
 e current along the lake wall of the south pier and around its outer
 and scouring out the bottom to the unprecedented depth of 38 feet
 w zero of gauge, undermining the foundation piles and letting
 cribs sink down. Measurements made after the cessation of the
 n showed that the amount of subsidence at the outer end was 11
 while a lateral displacement had taken place at the same time,
 ards the south, to the extent of 11 feet at the lake end.

he superstructure timbers had been torn apart, there being gaps of
 t in the second crib from the end, and smaller ones at other places.
 he stone filling does not appear to have moved much at first. On
 ember 6 a noticeable settling was apparent only in one pocket of
 of the two outer cribs, both on the lake side, although a subsequent
 r's examination disclosed large holes under the cribs. On Decem-
 14, one pocket of the outer crib had lost all its stone, two pockets
 he second crib were half empty, and nearly the whole lake side half
 he three new cribs had experienced a perceptible settlement of its
 g, while the filling along the channel side underwent almost no
 ge.

he piles under the end crib, where the diver found the bottom acces-
 e, had disappeared.

eps were immediately taken to strengthen the work temporarily,
 s to hold it till spring, when permanent repairs were to be made,
 the pier secured by the addition of new crib work. Extra timbers
 e screw-bolted over the gaps in the superstructure, long drift bolts
 hing down into the cribs proper were driven through the super-
 cture timbers, and a large quantity of stone was cast around the
 outer cribs. About 40 cords of stone were borrowed from St.
 ph Harbor, where it was in store without immediate call for its use,
 the rest, about 80 cords, was taken from the inshore revetments.
 empty pocket of crib 3 and the corner pockets of crib 2 were refilled
 part of this stone. Since the beginning of January no further
 ge in elevation or alignment has been noticed.

circumstances attending the accident to the crib work indi-
 several defects in the general design of the cribs heretofore built,
 modified design was prepared for future use. The new plan
 s for a closed central compartment, of nearly half the total
 of the crib, to hold the permanent load, and a narrower com-
 t on each side, next to the side walls, with open bottoms to

The first part of the work was done in 1880 and 1881 and consisted of the construction of the dam and the establishment of the water gauge. The second part was done in 1882 and 1883 and consisted of the construction of the canal and the establishment of the water gauge.

The dam is a gravity dam and is 100 feet high. It is built of masonry and has a crest width of 10 feet. The water gauge is a simple float gauge and is located on the right bank of the canal.

The canal is 100 feet wide and is 1000 feet long. It is built of masonry and has a bottom width of 8 feet. The water gauge is a simple float gauge and is located on the right bank of the canal.

The dam is a gravity dam and is 100 feet high. It is built of masonry and has a crest width of 10 feet. The water gauge is a simple float gauge and is located on the right bank of the canal.

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The canal is 100 feet wide and is 1000 feet long. It is built of masonry and has a bottom width of 8 feet. The water gauge is a simple float gauge and is located on the right bank of the canal.

500 linear feet and the south pier of 300 linear feet beyond that under contract. The changes made necessary in the general plans and foundation, to prevent a recurrence of the dangers like that of last winter, inevitably enhance the cost of the work, and the risks in view of their location far out in the lake can not be built on a sum considered sufficient heretofore.

The estimate for the fiscal year 1894 is therefore as follows: 800 feet crib work to extend both piers and complete the project, \$10,000; 2,500 linear feet of shore revetment, \$15,000; sand fencing, planting, etc., \$7,000; 2,300 linear feet repairs of south revetment, \$18,400; general repairs of existing works, \$5,000; dredging, which, with contingencies, makes \$180,000.

The Light-House Establishment maintains a fourth-order flashing coast light at the entrance and a sixth-order light on the south pier, with duplicate fog

signals. The Life-Saving Service has a station on the north pier, inside the shore line. The harbor is included in the Michigan collection district, Michigan. Grand Haven is a port of entry.

Appropriations for improving harbor at Grand Haven, Mich.

June 30, 1852 (mouth of Grand River).....	\$2,000.00	August 14, 1876.....	\$15,000.00
June 2, 1867 (mouth of Grand River).....	40,000.00	June 18, 1878.....	15,000.00
June 1, 1866.....	65,000.00	March 3, 1879.....	9,000.00
June 30, 1869.....	1,866.15	June 14, 1880.....	50,000.00
June 1, 1870.....	10,000.00	March 3, 1881.....	50,000.00
June 3, 1870 (allotment)....	500.00	August 2, 1882.....	40,000.00
June 3, 1871.....	6,000.00	July 5, 1884.....	50,000.00
June 1, 1872.....	15,000.00	August 5, 1886.....	30,000.00
June 3, 1873.....	75,000.00	August 11, 1888.....	25,000.00
June 1, 1874.....	50,000.00	September 19, 1890.....	75,000.00
		Total.....	624,366.15

Total estimated cost of the work, 1866, amended in 1880, 1890, and amount appropriated and allotted from 1852 to and including act September 19, 1890.....	\$804,366.15
Amount expended to June 30, 1892.....	604,240.82

Money statement.

1891, balance unexpended.....	\$58,422.18
1892, amount expended during fiscal year.....	38,296.85
	<hr/>
1892, balance unexpended.....	20,125.33
1892, outstanding liabilities.....	\$628.59
1892, amount covered by uncompleted contracts.....	14,058.14
	<hr/>
	14,686.73
	<hr/>
1892, balance available.....	5,438.60
Amount appropriated by act approved July 13, 1892.....	90,000.00
	<hr/>
Amount available for fiscal year ending June 30, 1893.....	95,438.60
	<hr/>
Amount (estimated) required for completion of existing project.....	35,000.00
Amount that can be profitably expended in fiscal year ending June 30, 1894 allotted in compliance with requirements of sections 2 of river and harbor acts of 1866 and 1867.....	90,000.00

of proposals for extension and repair of South Pier, etc.—Continued.

Name and address of bidder.	For repairs.				Total.
	White pine (16,992 feet, B. M.).		Stone in work (250 cords.)		
	Per M feet B. M.	Amount.	Per cord.	Amount.	
Manistee, Mich.....	\$20.00	\$330.84	\$8.00	\$2,000.00	\$2,339.84
Osby, Muskegon, Mich.....	26.00	441.79	6.75	1,687.50	2,129.29
on, Racine, Wis.....	30.00	500.76	7.25	1,812.50	2,322.26
s. & Co., Grand Rapids.....	18.00	305.86	7.40	1,850.00	2,155.86
& Co., Ludington, Mich.....	19.00	322.85	9.00	2,250.00	2,572.85
anvais, Charlevoix, Mich.....	52.00	543.74	17.00	4,250.00	4,793.74
th, Ludington, Mich.....	24.00	407.81	7.00	1,750.00	2,157.81
oper, Manitowoc, Wis.....	17.50	297.36	7.00	1,750.00	2,047.36
rns, Charlevoix, Mich.....	20.00	339.84	7.00	1,750.00	2,089.84

awarded to Messrs. Truman & Cooper, and executed under date of April

COMMERCIAL STATISTICS, GRAND HAVEN HARBOR, MICHIGAN.*

Entrances and clearances.

Year.	Number.	Revenue collected.	Tonnage.
.....	2,331	\$5,221.81
.....	1,634	1,985.43
.....	1,384	1,548.32
.....	1,297	632,159
.....	1,888	1,036,620
.....	1,508	1,405,600
.....	1,110	649,370
.....	1,172	834,089
.....	819	616,422

Receipts and shipments by vessel, 1891.*

Articles received.	Quantity.	Tons.	Articles shipped.	Quantity.	Tons.
barrels.....	3,926	393	Beer..... barrels.....	54	5
tons.....		17,315	Cement..... do.....	100	15
do.....		5,657	Cheese..... boxes.....	200	4
do.....		134	Coal..... tons.....		1,658
barrels.....	725,355	72,535	Fruit..... do.....		980
bushels.....	15,888	437	Fish..... barrels.....	330	36
tons.....		33,773	Gravel..... tons.....		3,250
tierces.....	5,951	982	Iron..... do.....		1,058
bundles.....	38,072	1,904	Lumber, M feet, B. M.....	3,761	6,582
barrels.....	280	31	Leather..... bundles.....	18,739	937
at, B. M.....	360	630	Lime and cement..... tons.....		49
tons.....		7,223	Merchandise..... do.....		21,556
sacks.....	17,131	685	Paper stock..... bales.....	1,291	65
kegs.....	772	39	Rags..... do.....	31	7
barrels.....	60	12	Seeds..... bushels.....	2,282	114
do.....	2,146	215	Staves..... carloads.....	43	753
bales.....	1,734	390	Slabs..... cords.....	1,643	4,416
do.....	5,625	3,655			
cords.....	5,026	31,412	Total.....		40,865
sacks.....	2,282	114			
cases.....	831	8			
bales.....	3,319	290			
tons.....		200			
barrels.....	4,752	475			
		178,499			

ad from statements furnished by the collector of customs and by Col. R. C. Duryea.

REPORT OF MAJOR WILLIAM LUDLOW, CORPS OF ENGINEERS.

UNITED STATES ENGINEER OFFICE,

Detroit, Mich., February 23, 1900.

GENERAL: I have the honor to forward herewith plans and specifications and form of advertisement for the continuation of work on Grand Haven Harbor, Michigan, with request for consideration and approval.

The steps in this direction would have been taken earlier had it been for the damage to the outer three cribs of the south pier, to which reference was made in the monthly report for December. It has been necessary to keep these cribs under observation for a certain period in order to feel sure that the limit of movement had been reached before making arrangements for additional work. The storm of December 4 on Lake Michigan was of exceptional severity, and occasioned notable changes at nearly all the harbors on the east coast. The wind blowing heavily and continuously from the southwest, producing a very heavy sea at the entrance and setting in motion immense quantities of sand in the lake bed.

At Grand Haven, owing to the great projection of the pier into the lake, a formidable scouring action was developed, due to the piling up of the water in the angle between the south pier and the shore line, whence it escaped in a powerful current offshore along the pier and across its outer end northward.

The erosion adjacent to the pier amounted to from 5 feet at about 15 feet from the end to 10 feet at its extremity, in consequence of which the current cut under the three outer cribs, leaving the foundation piles exposed and letting the cribs down by the inability of the piles to support them.

It was at first feared that these three cribs, which were built last season and as well as any on the lake, were practically destroyed, but within two or three weeks after the storm the cribs seemed to reach their ultimate movement, the excess of depth adjacent to them was partly refilled, and observation by transit and level indicate that a permanent position has been reached.

Two results ensue:

1. It had been proposed with the balance to the credit of the appropriation to continue construction by building four new cribs, two on each pier, but this balance must be drawn upon to rebuild the superstructure of the three damaged cribs, to restore the level and integrity of the work. It is estimated that these repairs will cost between \$3,000 and \$4,000, nearly the cost of one 50-foot crib.

The possibilities of construction at this time are therefore reduced to three cribs, two on one pier and one on the other. The construction of a single crib at the outer end of piers so exposed as at Grand Haven is inexpedient for the reason that the precautionary additions to an end crib to secure its safety until further extension can be made, cost about \$400 in excess of that of an interior crib, and this expense is wasted as soon as additions to the pier are made. As it may be expected that further appropriations will presently be made for Grand Haven, it therefore seemed advisable to defer the single outer crib for the near future and to make the construction at this time to the addition of two cribs to the south pier.

It is also apparent that it is advisable to build three new cribs on the south pier because the history of the harbor indicates that the mean action of wind and waves at this harbor is nearly normal to the shore line, and

at the resultant action is directly shoreward and that the best results in navigation have been found when the outer ends of the two piers are kept nearly abreast of each other.

(2) Pile foundations have in general been used for the eastern Lake Michigan harbors with good results but with occasional failure, as formerly at Muskegon and Michigan City and recently at Grand Haven. On the other hand, the riprap foundation used elsewhere on the lake in some cases, as at Chicago, with satisfactory results, when tried two seasons ago at Michigan City in connection with the outer breakwater was not satisfactory, the weight of the cribs, etc., forcing the riprap 5 and 6 feet into the lake bed, and requiring subsequent raising on two occasions. Furthermore, after two years of settlement, a hard northerly storm, at Michigan City last fall, was still found to affect the line and level of the work. The recent experience with the Grand Haven cribs has proved conclusively that at so great a projection into the lake, exposed to powerful currents and seas, a pile foundation alone is not reliable, while the Michigan City experience indicates that a riprap foundation requires for ultimate rest an undue and uncertain period of time.

It is, therefore, necessary in the Grand Haven case to combine the two methods, and, for the attainment of a reliable level as well as security of construction, to use a foundation of piles driven to proper depth and to strengthen these and prevent erosion of the lake bed by free use of riprap.

The accompanying plans and specifications provide for these and for another point in addition. If 100 linear feet of pier are to be built there does not seem to be any sound reason for not building a single crib 100 feet long instead of two cribs of 50 feet each.

The advantage of the single structure in point of strength being continuous at every course from the bottom up instead of being entirely separated below the superstructure would seem obvious, and there should be some economy also perhaps.

It is true that an attempt some years ago at one of the western harbors to sink a 100-foot crib was not entirely fortunate, but this, it is believed, was due to insufficient means of loading the crib promptly, the stone scows being of small dimensions. The stone scows now in use on the lake are of large dimensions, capable of carrying 150 to 200 cords of stone, and the specifications provide that the crib shall be half filled within twelve hours and entirely so within twenty-four. There is no reason to anticipate any difficulty in this direction, and none that will offset the advantages of the single construction, which, it is believed, will in the present case be found entirely advantageous.

It is therefore proposed to contract for a 100-foot crib, and in addition to procure the material, timber, and stone necessary to rebuild and refill the superstructure of the damaged cribs. This work can not well be specified, and, as in similar cases, it is proposed to do the work by hired labor, contracting only for the delivery of the material.

Should the project and specifications be approved, it is requested that the inclosed advertisement and accompanying letter to the chief clerk of the War Department be forwarded, and that authority be given to print 250 copies of the specifications.

Very respectfully,

WILLIAM LUDLOW,

Major, Corps of Engineers, Bvt. Lieut. Col., U. S. A.

g. Gen. THOMAS L. CASEY,
Chief of Engineers, U. S. A.

orth beach and 304 feet on the south beach, with some 40 feet of
al fence on the back wall of the north pier at shore line, to stop
ying sand from blowing into the waterway.

permanent bench mark was established on the stone foundation of
ght-keeper's dwelling and connected by duplicate levels with the
d States gauge. Its elevation is 6.98 feet above the zero of the
e. A series of accurate water-gauge readings was commenced
1, to be continued through June, July, and August, for the pur
of procuring data for the adjustment of the zero of the gauge to
form level with the gauges at the other harbors.

make the revetments subselve their purpose of excluding sand
the channel they should be sheet piled, the north pier from Sta-
5+10 to 12+60, or 750 feet, and the south pier from Station 5+50
+90, or 740 feet. As it may be impossible to drive sheet piles
om the rear of the works on account of the brush and driftwood
om, it may be necessary to drive them along the face of the chan-
wall, and in that case they must be protected by special guard piles
waling timbers.

he remainder of the north revetment from Station 16+22 to 18+50,
28 linear feet, should be rebuilt above water, and some minor re-
smade to the rest of the works.

he available balance, July 1, is \$2,100, and it is proposed to use this
redging and incidental repairs.

he estimate for the fiscal year 1894 is: Sheet piling, north and south
tments, 1,492 linear feet, \$7,460; general repairs of existing works,
ding new superstructure of 228 feet of north revetment, \$5,000;
ging channel, \$3,000, which, with contingencies, makes \$17,000.

addition, provision should be made for further extension of the
, for which purpose the project should be amended to reach the
ot contour in the lake.

Light-House Establishment maintains a fifth-order harbor light, and the Life-
g Service a station on the south pier.

s harbor is included in the Michigan collection district, Michigan. The near-
ort of entry is Grand Haven, Mich.

Appropriations for improving harbor at Black Lake, Michigan.

st 30, 1852.....	\$8,000.00	March 3, 1879.....	\$6,000.00
23, 1866.....	55,615.31	June 14, 1880.....	6,000.00
h 2, 1867.....	51,000.00	March 3, 1881.....	6,000.00
11, 1870.....	10,000.00	August 2, 1882.....	10,000.00
h 3, 1871.....	10,000.00	July 5, 1884.....	15,000.00
10, 1872.....	10,000.00	August 5, 1886.....	5,000.00
h 3, 1873.....	12,000.00	August 11, 1888.....	5,000.00
23, 1874.....	15,000.00	September 19, 1890.....	10,000.00
h 3, 1875.....	15,000.00		
st 14, 1876.....	15,000.00	Total.....	274,615.31
18, 1878.....	10,000.00		

nal estimated cost of the work, 1866, amended in 1873, 1879, 1884, l 1892.....	\$291,615.31
le amount appropriated, 1852, to and including act of September 19, 0.....	274,615.31
le amount expended to June 30, 1892.....	272,242.87
mt carried to surplus fund.....	1.19

2350 REPORT OF THE CHIEF OF ENGINEERS, U. S. ARMY.

Money statement.

July 1, 1891, balance unexpended	86
June 30, 1892, amount expended during fiscal year	4
July 1, 1892, balance unexpended	2
July 1, 1892, outstanding liabilities	5
July 1, 1892, balance available	1
Amount appropriated by act approved July 13, 1892	5
Amount available for fiscal year ending June 30, 1893	1
{ Amount that can be profitably expended in fiscal year ending June 30, 1894 12	
{ Submitted in compliance with requirements of sections 2 of river and	
{ harbor acts of 1866 and 1867.	

COMMERCIAL STATISTICS, HOLLAND (BLACK LAKE) HARBOR, MICHIGAN.

Entrances and clearances.

Year.	Number.
Calendar year 1891	2,674

*Receipts and shipments by vessel, 1891.**

Articles received.	Quantity.	Tons.	Articles shipped.	Quantity.	Tons.
Beck	corals	20,000	Butter	tons	
Brick	M	7,250	Coal	do	
Butter	tons	67	Flour	barrels	70,000
Coal	do	22,000	Fish	tons	
Grain	bushels	170,000	Fruit	do	
Hay and feed	tons	1,320	Farm products	packages	16,802
Iron	do	123	Furniture	do	420,000
Laths	M	200	Grain	bushels	30,000
Lumber	M feet, B. M.	21,000	Hay and feed	tons	
Lime and cement	barrels	3,000	Live stock	head	1,000
Leather and hides	tons	780	Leather and hides	tons	
Logs	do	22,000	Lumber	M feet, B. M.	1,437
Machineroses brought	do	5,611	Machinery	tons	
Machinery	do	1,011	Potatoes	bushels	30,000
Manufacturing	packages	220,000	Stone	corals	3,925
Produce	barrels	1,000			
Shells	barrels	5,200			
Staves	tons	100			
Sticks	corals	2,000			
Stumps	M	2,000			
Total		300,630	Total		38

K K 12.

IMPROVEMENT OF SAUGATUCK HARBOR, MICHIGAN.

The plan for this improvement was adopted in 1869 and revised in 1887. It was originally proposed to create a 12-foot navigable channel to be prepared and maintained by dredging at

*Original data furnished by W. C. Walsh, esq., Holland, Mich.

for a permanent work to cut across this neck of land and make the improvement at that point, but on account of the improvements already made at the present point of the river, which are available, estimates for opening this channel were made, the expenditures being deemed too great.

It is also stated that an additional reason for adhering to the present river channel was the fact that it had already a navigable depth of about 8 feet, which encouraged the belief that it could be deepened effectively to 12 feet.

Efforts to improve the present entrance, however, by remodeling and extending the existing revetments from the lake along the south side of the river to above the bend and revetting the north bank at the bend and at the entrance proved unavailing. The revetment in place was repeatedly undermined, and the sands from the north side of the harbor poured steadily into the channel under the influence of the northwest gales, and repeated dredging insured an average depth of but 10 feet, and for short periods only.

It was concluded that the depth of 8 feet then existing, and which the draft of water had varied little for many years, was amply sufficient for "the present and prospective commerce," and no appropriations should be applied merely to maintaining the channel in their then existing condition.

The last appropriation was made for this harbor in August, 1888, at which time the entrance has been kept partially open by the continual service of a dredge.

The dredging of May, 1891, was instrumental in maintaining a tolerable navigable channel till the fall of the year, but in May, 1892, the depth on the bar, covering the entire width of the harbor, nearly to the light-keeper's dwelling, was found to be only 3 feet; at the ends of the piers the water was 6 feet deep, and on the outlying bar 8

feet. Steamers trading to Saugatuck and requiring from 7 to 8 feet of draft have been in the habit of scouring out a temporary channel with propeller wheels, but it is evident that a channel obtained by such means is but temporary, as the sands are only removed from one place and find lodgment in other parts of the channel.

should be made for the extension and completion of the necessary work. The present condition is as follows:

The pier and rebreast are in urgent need of repairs. The south pier from the outer end to Station 25+24, 2,524 linear feet, is a total wreck. It should be rebuilt above the water surface and refilled with brush and stone. New filling is also needed in the remaining 1,330 feet of the south rebreast and in the north pier 714 feet, and the upper end of the south rebreast should be connected with the shore to keep the river from cutting behind it. A great part of the north rebreast is in the bank & swallowed up by the sand advancing from the north, and the remaining part has become a danger to navigation, as it has canted over into the channel, a considerable portion of the piling being below the water surface. It should be replaced by a more substantial structure.

The recent soundings show the 5-foot curve of Lake Michigan to be 400 feet in advance of the north pier and about 200 feet from the south pier. With the condition of affairs it can not be expected to maintain a navigation of even 7 feet.

A permanent bench mark was established on top of stone foundation southeast corner of Light-keeper's dwelling (the former light-house), and connected by duplicate levels with the United States gauge. It is 22.7 feet above zero of gauge.

A series of accurate water-gauge readings was commenced June 1, to be continued through June, July, and August, for the purpose of obtaining data for adjusting the zero of the gauge to a uniform elevation with the gauges at the other harbors.

To preserve the present structures, pending serious attention to the completion of the improvement, the outer 2,524 feet of the south pier should be rebuilt above the water surface and the remainder of the south rebreast and the outer north pier, altogether 2,050 feet, should be refilled with brush and stone. This is estimated to cost \$25,000. For dredging the sum of \$5,000 is estimated required. The total estimate for 1894 is, therefore, \$30,000.

The light-house establishment maintains a fifth-order light on the south pier.

This harbor is included in the Michigan collection district, Michigan. The nearest post office is Grand Haven, Mich.

Appropriations for improving harbor at Saugatuck, Mich.

Sept. 21, 1866	\$25,000	March 3, 1879	85,000
July 21, 1868	8,000	June 14, 1880	5,000
July 21, 1870	10,000	March 3, 1881	5,000
March 21, 1871	10,000	August 2, 1882	8,000
June 21, 1872	10,000	July 5, 1884	4,000
March 21, 1873	10,000	August 5, 1886	8,000
Sept. 21, 1874	10,000	August 11, 1888	5,000
Sept. 21, 1875	10,000		
Sept. 21, 1876	5,000	Total	140,439
June 21, 1877	2,500		

(Original) estimated cost of the work, 1867, modified in 1869, 1870, 1875.

What amount appropriated from 1868 to and including act of August 11, 1888	\$175,699.46
What amount expended to June 30, 1892	140,439.00
What amount expended to June 30, 1892	139,094.26

Money statement.

By 1, 1891, balance unexpended	\$1,458.30
By 30, 1892, amount expended during fiscal year	113.55
By 1, 1892, balance unexpended	1,344.75
By 1, 1892, outstanding liabilities	10.00
By 1, 1892, balance available	1,334.75
Amount appropriated by act approved July 13, 1892	5,000.00
Amount available for fiscal year ending June 30, 1893	6,334.75
Amount (estimated) required for completion of existing project	30,260.00
Amount that can be profitably expended in fiscal year ending June 30, 1894	30,000.00
Submitted in compliance with requirements of sections 2 of river and harbor acts of 1866 and 1867.	

COMMERCIAL STATISTICS, SAUGATUCK HARBOR, MICHIGAN.

Entrances and clearances.

Years.	Number.	Revenue collected.	Tonnage.
<i>Fiscal year:</i>			
1884	300	\$422.84	
1885	265	118.86	
1886	(*)	(*)	
1887	(*)	(*)	
<i>Calendar year:</i>			
1888	662		132.400
1889	314		76.300
1890	178		42.000
1891	492		120.000

Not stated.

*Receipts and shipments by vessel, 1891.**

Articles received.	Quantity.	Tons.	Articles shipped.	Quantity.	Tons.
Brick	M	15	Flour	barrels	340
Coal	tons	125	Fish	tons	60
Lime and cement	barrels	300	Flags	do	4
Lumber	M feet, B. M.	100	Fruit and vegetables	do	17,513
Merchandise	tons	520	Farm produce	do	20
Machinery	do	70	Household goods		9
Salt	barrels	100	Live stock	heads	16
Staples	M	50	Machinery	tons	35
Wool	M	40	Merchandise	do	11
			Meats	do	93
Total		1,627	Total		17,788

* Compiled from statement furnished by Messrs. Griffin & Henry.

K K 13.

IMPROVEMENT OF SOUTH HAVEN HARBOR, MICHIGAN.

The official project, adopted in 1866 and modified in 1869, 1872, and 1883, provides for a channel 12 feet deep, 180 feet wide, and protected by piers and revetments at the entrance, and extending up Black River to the highway bridge.

The existing works are as follows:

On the north side: 255 feet crib work 32 feet wide, built in 1872 to 1874; 30 feet crib work 30 feet wide, built in 1871; 321 feet crib work 20

feet wide, built in 1868 and 1869; 524 feet pile work built in 1876, a feet plank beam revetment built 1879. The total length is 1,578 feet and the pier projects 700 feet beyond the present shore line.

On the south side: 50 feet cribwork 30 feet wide, built 1888; 1 crib work 32 feet wide, built 1871-1874; 352 feet crib work 20 feet built 1868; 143 feet pile work, built 1878, and 855 feet plank beam ment, built 1878 to 1882. The total length is 1,553 feet, and it projects 520 feet beyond the present shore line.

The dredging, which was in progress at the beginning of the year, was completed August 4, 1891, giving a channel 13 feet deep 50 feet wide. Eleven thousand six hundred and seventy cubic yards were removed after July 1. At the opening of navigation this year the entrance was found to have again shoaled to 10 feet, and the Government dredge reopened the channel to a depth of 14 feet and widened it 50 feet between May 21 and June 23. A few shoals in the river and the revetments were also removed, and the quantity dredged measured 13,830 cubic yards.

The repairs of the outer crib, north pier, were completed. Caissons were driven around the outer half of the crib and connected by means of caps and screw bolts to the old timber work, and the new structure built up on the work thus strengthened. The gaps between the side walls were closed with vertical timbers and screw bolts, and the repaired work was filled with stone. One hundred linear feet of the outer end of the south side pile work, at shore line, where it joins the crib work, was provided with a double row of sheet piling. This place has always permitted large quantities of sand to pass into the channel. Although it was not possible to drive the sheet piling very deep, owing to the mass of brush and driftwood in the bay, it is believed that this part of the work is now practically sand tight.

The north pile revetment, 524 feet long, was also double sheet piling along the rear wall and refilled with new brush, ballasted with stone. The same difficulty was encountered here in driving sheet piling as on the south side, but the indications are that the seepage through the revetment has been stopped.

The conversion of the north plank beam into a sheet pile revetment has been commenced. The work was cut down to 3 feet at water surface, and is to be provided with a new cap over the front and a wale streak along the water line, additional piles along the rear wall, a double row of sheet piles to penetrate to not less than 10 feet below the water, and anchor piles 10 feet in the rear connecting the front walls by iron tie rods. This part of the revetment is 524 feet long, and it is expected to complete the conversion during the month of July. One hundred and ninety-two linear feet of sand fence was driven on the north beach, and the vertical fence on the rear wall of the pier was moved out to the shore line, where it will do better service. The end wall of south pier was repaired.

During the early spring the Government pile-drivers and construction scow received needed repairs to fit them for the season.

A permanent bench mark was established on top of brick foundation at southwest corner of Mr. Hallock's house on the northeast corner of Michigan avenue and St. Joseph street, and connected by double levels with the United States gauge. It is 41.68 feet above zero. A series of accurate water gauge readings was commenced June 1, 1891, continued through June, July, and August, for the purpose of obtaining data for adjusting the zero of the gauge to a uniform level with the gauges at the other harbors.

completion of the repairs now in progress the works will be in a better condition: The north pier, although most of it is in a poor condition, does not need any repairs, except some occasional patchwork, every few years to come.

200 feet of south pier is in a fair state of preservation, and needs no repairs in the immediate future, but the 20-foot crib work between 9 + 98 to 13 + 50, 352 linear feet, which is now 24 years old, needs of rebuilding above the water surface, as the timber is rotten and barely hangs together. As the best water in the harbor is habitually close to the south pier, this part is particularly liable to injury from passing vessels, and its repairs should not be any longer.

It is proposed at this time for the south plank beam revetment. The time is not far distant when it will be necessary to connect it with anchor piles in the rear, and tie rods, and when that time comes it may be found desirable to provide it with sheet piling.

Repairs now being made to the north revetment will help in reserving the needed depth of water in the harbor, but the principal causes of the interfering shoals will still exist unimpaired. The insufficient development of the piers, and the unproportion of the north bank of Black River above the revet-

ment piers do not project beyond the general 10-foot curve in the lake, although there is usually a depth of 14 to 16 feet at the piers, a bar with sometimes not more than 10 feet on its crest across the entrance immediately beyond, and even after the piers have attained their full length as intended by the present plan, it provides for an addition of 150 feet to the north pier and the south pier, they will fall some 300 feet short of reaching the depth in the lake.

The north bank of Black River is in its natural state, although the piers are very close to it, and it is subject to erosion from the current of the river, when in flood, and from the waves and suction of the piers. It would seem to be the duty of the riparian owners to revet the bank, in order to assist in maintaining the desired navigation.

With a working balance of \$900 it is proposed to complete the north revetment, now under way.

For the fiscal year 1894, the estimate is as follows: For repairs to the north pier and general repairs, \$5,000; for completing the existing additional cribs, \$42,000; for dredging, \$3,000, which, with the balance for contingencies, makes \$55,000.

The harbor establishment maintains a fifth-order light on the south pier, and a Service station on the north pier.

The harbor is included in the Michigan collection district, Michigan. The nearest port is Grand Haven, Mich.

Appropriations for improving harbor at South Haven, Mich.

.....	\$43,000	June 14, 1880	\$5,000
.....	10,000	March 31, 1881	5,000
.....	15,000	August 2, 1882	10,000
.....	12,000	July 5, 1884	7,500
.....	20,000	August 5, 1886	5,000
.....	10,000	August 11, 1888	10,000
.....	10,000	September 19, 1890	15,000
3:.....	10,000			
.....	12,000			
.....	7,500			
		Total	<u>207,000</u>

K K 14.

IMPROVEMENT OF ST. JOSEPH HARBOR, MICHIGAN.

acial project, adopted in 1866 and modified in 1874 and 1875, are a channel of entrance from Lake Michigan into St. Joseph 270 feet wide and 16 feet deep, protected by piers and revetment to Benton Harbor it is designed, as later authorized to carry 13 feet through the Benton Harbor Canal, widening the channel to 100 feet.

Channel entrance from Lake Michigan into St. Joseph Harbor is defined on the north side by a pier consisting of 457 linear feet of crib work, 30 to 24 feet wide, and built between the years 1875 and 1877, and 373 linear feet of old crib work built previous to 1866, and of 100 feet of pile revetment built in 1886-'87, and on the south side near feet of pile pier built 1867 to 1872, and of 213 linear feet of crib work built previous to 1866. The inner end of the north pier is connected with the shore and the new light-house depot is defined by a pile wing 179 feet long. The north pier projects 600 feet from the present shore line and the south pier 540 feet.

The 173 feet of old crib work in the north pier has been maintained by means of repeated patching, sustaining piles, tie rods, etc., for many years, but the time has come when this is no longer practicable. The work should be rebuilt above the water line, and as a consequence a portion of the channel wall below the water has been lost, and a pile wing will have to be driven here to support the new superstructure. To rebuild this work in proper shape is estimated to cost \$5,000. The filling in the north revetment has settled considerably, and to avoid the loss of the stone ballast and exclude the sand of the channel behind from the channel it should again be refilled with brush. This is estimated will cost \$600. Some minor repairs to the other side of the north pier and to the south pier, which will not cost above \$1,000 will put the existing channel revetments at the entrance into good repair for some years to come.

Present constructions are not sufficient, however, to maintain the existing entrance depth, owing to their insufficient extension. In the Annual Report for 1891 it is said:

Accumulation of sand from the action of the winds and the currents is against the pier, and this has now occasioned bars and shoals of 10 feet beyond the north pier and overlapping to the southward across the entrance. In 1890 marking the entrance, as established this spring about 1,000 feet a mile west from the end of the north pier, marks the south end of a shoal where there is but 13 feet of water.

Conditions at the entrance went from bad to worse until, in the fall of 1891, a shoal with only 7 feet of water on it overlapped the north pier and projected nearly 100 feet in front of it into the harbor.

Several vessels ran aground on it while trying to enter the harbor, but all escaped without serious damage, as the lake happened to be calm at the time.

Subsequently this shoal was partially scoured out under the combined action of southwesterly gales and the river current, but the marks were distributed farther out and added to the ridges extending north across the harbor entrance, raising the summit of the shoal about 500 feet beyond the end of north pier to 11 feet below the present surface.

December, the company agreed to pay the expenses of
the work on account of the weather. As a matter of fact it
was found fit for work between December 1 and January 1,
become obvious to the company that further dredging
would be futile, the attempt in that direction was discontinued at
once.

The weather conditions of the past winter were more
favorable than had been the case for several years. Ice formed
in quantities in the lake and its beneficial action in protecting
the weight and action of storms was made particularly
evident. With the surface covered, wave action was
prevented and the current of the river had its normal effect in scouring
the available depth nearly in the direct prolongation
of the river. The winter less difficulty was experienced and the winter
work continued with fair measure of regularity.

The freshets this spring have been of unusual volume
and immense amounts of silt have been brought
down the river. Large deposits were made on the bars of
the river, but the material was soft and a vessel could plow
it up without serious hindrance. In time it is
probable that the material will disappear from the channel of
the river and be distributed elsewhere.

The report of the committee does not provide for any further
work on the river. The report of November 19, 1891
states that the work on the river is to be continued
until the river is to be secured.

The report of the committee at and below the main
lock, the river is to be secured by the use of plants
and other material on the side of the canal with a view to
preventing the passage of the waters of the River. The
committee also recommended that the river be secured
by the use of plants and other material on the side of
the canal with a view to preventing the passage of the
waters of the River. The committee also recommended
that the river be secured by the use of plants and other
material on the side of the canal with a view to preventing
the passage of the waters of the River.

an extent that throughout the greater part of its length the depth is 5 to 6 feet deep in it. In this condition it naturally is incapable of serving the purpose for which it was built, and should in part be raised above water and refilled. The plank beam revetment is also in a down condition. There is an agreement with the railroad company whereby the company is to remove this revetment and replace it with a substantial sheet-pile structure about 10 feet farther back, thus giving to the canal the projected width of 100 feet. The railway company, if permitted to use the wing dam, should also rebuild it as their works extend. It is estimated that it will cost \$6,000 to remove and refill the other part of the wing dam.

The remaining part of the north bank of the canal and the winding wharf at Benton Harbor are revetted by the owners, but most of these revetments are badly built and permit the soil from behind to wash into the canal. The south bank has nothing that can be called a revetment, but the city of Benton Harbor has recently passed ordinances requiring all riparian owners to properly bulkhead their fronts.

Dredging in progress at the commencement of the fiscal year was completed until October 28, 1891, and during this time the winding wharf at Benton Harbor and the canal were dredged throughout to a depth of 13 feet; the main channel in the harbor below the canal was dredged to 15 feet, as well as the shoal area between the railroad and the north revetment, and a channel 50 feet wide and 17 to 20 feet deep was made across the bar between the ends of the piers. A pier approach to the life-saving station back of north pier was also completed, and the shoal at the mouth of the Paw Paw in the canal having disappeared, this was again dredged out before the plant left for the City.

The dredge *Farquhar* again arrived at the harbor on December 11, 1891, and, as already stated, was temporarily kept in commission in the harbor to open a passage across the bar in front of the entrance.

During the winter the Government plant was repaired, and on the first of April the dredge resumed operations and again restored the depth in the canal to a depth of 13 feet. The flood in the St. Joseph River, which set in early in May, made further dredging in the harbor impossible for the time being, and the plant was sent to South

The total dredging during the year amounted to 57,520 cubic

feet. At the present time much complaint is made regarding the navigation of St. Joseph Harbor, the high river having brought down large quantities of sand from above, and the available depth above the railroad is only 11 to 12 feet. The service of a dredge will be required as soon as further appropriations shall have been made.

Repairs of the north revetment, by refilling with brush and stone, were completed in July, 1891, and the effect became apparent at once, more sand found its way through this work afterwards. A plank was laid over the revetment from the inner end of the elevated wharf which was rebuilt.

The incomplete portion of the superstructure of the south pier was left. The gap in the end wall of the north pier, under the superstructure, was closed with vertical timbers, placed with the help of a crane, and the crib filled with stone and decked. The end of the pier was subsequently injured by one of the steamers which ran aground on the shoals near it and broke off the end horns. The damage was repaired with extra timbers and tie rods, and a washout at station was provided. The north pier was closed with plank aprons and stone ballast.

The Chicago and West Michigan Railway has commenced dredging the channels through the drawbridge, in accordance with the contract of the Secretary of War. They have been much delayed by the heavy winter in the river, due to the persistent freshets, making dredging difficult and at times impracticable, but they are doing what they can by working hard, and will have it completed in July. The Chicago and West Michigan renewed complaints against the Chief of Engineers, in regard to the obstruction to the harbor at Chicago, in the report and subsequent report submitted under date of 15th July 1891.

A bench mark was established on top of a bonding iron on the south west corner of south abutment of the railway bridge, and connected by a pipe to the United States bench mark on the east side of the river. A series of accurate water levels was run on the line to be continued through the drawbridge, for the purpose of obtaining data for the adjustment of the gauges at the bridge with the gauges at the of

the following estimate is submitted: for the purchase of material, \$3,000; for repairs to the bridge, \$1,000; for repairing the gauges, \$1,000; which, with contingencies, \$5,000.

The estimate also includes extending the pipe to the bench mark on the east side, which would represent a further outlay of \$1,000, which could be done for the year 1892. The estimate for the fiscal year 1894 is the

estimate for the fiscal year 1894 is the

estimate for the fiscal year 1894 is the

estimate for the fiscal year 1894 is the

Money statement.

11, balance unexpended	\$11, 102. 64
1892, amount expended during fiscal year.....	9, 821. 73
12, balance unexpended	1, 280. 91
12, outstanding liabilities	632. 33
13, balance available.....	648. 58
appropriated by act approved July 13, 1892*	60, 000. 00
available for fiscal year ending June 30, 1893	60, 648. 58
that can be profitably expended in fiscal year ending June 30, 1894	85, 000. 00
needed in compliance with requirements of sections 2 of river and	
acts of 1868 and 1867.	

COMMERCIAL STATISTICS, ST. JOSEPH HARBOR, MICHIGAN.

Entrances and clearances.

Years.	Number.	Revenue collected.	Tonnage.
.....	560	\$1, 098. 14
.....	467	361. 57
.....	789	248. 53
.....	580	144, 000
.....	679	208, 797
for 1889	966	359, 925

Years.	Vessels entered.	Tonnage.	Vessels cleared.	Tonnage.
ing Benton Harbor)	948	131, 607	946	131, 395
ing Benton Harbor)	742	215, 334	743	215, 591

Imports and shipments by vessel at St. Joseph and Benton Harbor, Michigan, 1891. a

Articles received.	Quantity.	Tons.	Articles shipped.	Quantity.	Tons.
..... tons.....	508	Cement tons.....	1, 277
..... do.....	41	Carriages do.....	1, 113
..... packages.....	282	12	Canned goods	cases, 3, 515	1, 141
vegetables..... tons.....	75	Fruits and vegetables.....	tons.....	8, 642
..... bushels.....	15, 700	398	Fish	do.....	46
..... barrels.....	18, 525	1, 853	Flour	barrels, 10, 760	1, 078
..... tons.....	4	Hides	bundles, 561	27
..... packages.....	390	59	Iron	tons.....	20
..... tons.....	470	Live stock	head, 40	8
..... head.....	236	42	Lumber..... M feet, B. M.,	359	634
..... M feet, B. M.,	2, 867	6, 417	Merchandise.....	tons.....	36, 510
..... tons.....	12, 494	Nails	kegs, 3, 510	176
..... bundles.....	3, 964	199	Paper	bundles, 52, 478	2, 654
..... barrels.....	100	15	Produce.....	crates, 141	22
..... cords.....	3, 000	60	Pipe, sewer	tons.....	120
..... barrels.....	75	469	Rags	bales, 517	116
..... packages.....	1, 155	283	Starch	boxes, 719	18
.....	2, 960	148	Sugar	barrels, 170	43
.....	Sundries.....	tons.....	1, 139
.....	Tobacco.....	hogsheads, 127	30
.....	Vinegar, cider.....	barrels, 2, 820	282
.....	Wool.....	sacks, 165	17
.....	23, 552	Total.....	55, 141

a Compiled from statement furnished by the collector of customs.

* Of which \$1,000 may be expended on St. Joseph River.

movement and the resulting formation of extensive shoals have greatly expedited by the open winters of the past three years, the northwest gales free sweep on the lake, and at this time are fully aggravated by the abnormal level of the lake, which is about lower even than the extraordinary low water of one year ago. The results are shown in the accompanying tracing from the most recovery.

During the earlier part of the season the course entering was nearly straight in after getting the outer black spar buoy abreast.

There is a small shoal formed close to the end of the north pier, which has developed into the extensive areas inclosed within the 10-foot contour, with depths of $7\frac{1}{2}$ and 8 feet upon them. At present this shoal crosses the prolongation of both piers and has crowded the channel to the southward, toward the 10 $\frac{1}{2}$ and 11 foot shoal lying on the bar, the heavy gales in October having occasioned the rapid accumulations.

The obvious remedy is the westward extension of the north pier past the black buoy to about the 15-foot contour, a distance of some 1,200 feet, which would cost \$100 per linear foot.

It is believed this extension to be indispensable to the maintenance of the channel, and that it should be provided for as early as practicable. As a temporary measure for the relief of navigation, I regret to say that I have of none that I could recommend or suggest at this time. During the fall and winter months the lake is kept in a state of almost constant agitation, such as to make it impracticable to operate dipper boats on the bar, and such scanty work as might be done by awaiting a brief opportunity would be undone in an hour or two. The debit situation balance to the credit of the St. Joseph Harbor is but small, and in any case it is manifestly hopeless for a dredge to contend with the accumulation as has been the case up southward of the line of the north pier. I believe that the present conditions will probably become no worse, as the outflow of water has a certain effect in conserving the channel, and in all probability the lake will rise from heavy rains and continue to do so, so as to be at the level $1\frac{1}{2}$ feet above the present stage.

While the channel has a navigable depth of 13 or 14 feet, which is a fair average, and with judgment in selecting the best water, it may be used in ordinary conditions.

Respectfully,

WILLIAM LUDLOW,
Major, Corps of Engineers,
Bvt. Lieut. Col., U. S. A.

Gen. THOMAS L. CASEY,
Chief of Engineers, U. S. A.

K K 15.

IMPROVEMENT OF ST. JOSEPH RIVER, MICHIGAN.

The project of March, 1889, contemplates securing a depth of 3 to 4 feet in the channel from St. Joseph to Berrien Springs, a distance of 25 miles, by the removal of snags and bowlders and the construction of wing dams where necessary.

Operations were resumed July 13, and continued till the end of September, when the available funds were exhausted. At Twin Springs

OF THE CHIEF OF ENGINEERS, U. S. ARMY.

current was diverted into the right-hand channel of a closing dam between the island and the left bank. A closing dike at the foot of the island was repaired, and subsequently an additional wing dam, 100 feet long, was built at 1 mile below to maintain a navigable channel across the shoal cut by the scour of the concentrated current above. A second dam built at the head of Long Reach, 7 miles from St. Joseph. The river very wide in this locality, and it is probable that additional dams be needed to insure a navigable depth.

Farther down the river at Royalton Island, some 3 miles above St. Joseph, the navigation is affected by the stage of Lake Michigan, and during the last few years has been very low, and in the fall of the year when the river carries a reduced volume of water, the available depth in this locality is usually small. In order to improve matters here as much as the limited means would permit, a number of water-soaked logs were arranged as a kind of dam across the right-hand channel, and the left-hand channel was further constricted at the foot of the island with a dam of the same material.

Before closing operations the entire stretch of river from Belvidere Springs down was cleared of such snags as still remained to obstruct the best water. Altogether 318 snags, 15 overhanging trees and large bowlders were removed, and at the end of September an unobstructed navigation of at least 3 feet was insured for the rest of the season.

It is probable that these rude measures of relief will not suffice to keep the river in good condition for any great length of time. The water channel will need to be restricted in a greater degree and a greater number of points, and snags will be brought down and lodged in the channel at every high water. It is also probable that the banks will need protection at several places to prevent erosion. Heretofore the constructions have been inexpensive, as the materials, such as brush and stone, cost only the labor of gathering them, and the necessary clearing of the plant was done gratuitously by Mr. Graham, the owner of the river steamer. It is probable that in future the cost of similar work will be increased, as the supply of suitable stone is now nearly exhausted, and the required brush may soon have to be paid for.

For further operations it is recommended that an appropriation of \$2,000 be made.

Appropriations for improving St. Joseph River, Michigan.

August 11, 1888	\$2,500
September 19, 1890	1,000
Whole amount appropriated, including act of September 19, 1890...	3,500
Whole amount expended to June 30, 1892	3,500

Money statement.

July 1, 1891, balance unexpended	\$1,000
June 30, 1892, amount expended during fiscal year	1,000
July 1, 1892, balance unexpended*	0
{ Amount that can be profitably expended in fiscal year ending June 30, 1894	2,000
{ Submitted in compliance with requirements of sections 2 of river and harbor acts of 1866 and 1867.	

* The river and harbor act approved July 13, 1892, provides that \$1,000 appropriation for improving St. Joseph Harbor may be expended on St. Joseph River.

COMMERCIAL STATISTICS, ST. JOSEPH RIVER, 1891.

The steamer, *May Graham*, 95 tons, load draft 30 inches, plying between St. Joseph and Berrien Springs.

E. A. Graham & Co., of St. Joseph, Mich., states that the freight carried on the vessel amounted to about 2,600 tons.

K K 16.

IMPROVEMENT OF MICHIGAN CITY HARBOR, INDIANA.

The works of this harbor consist of the improvement of Trail Creek entrance piers 100 feet apart and dredging the creek bed to 13 feet (the local authorities and owners attending to the bank revetting), constituting the "Inner Harbor," and of extensive breakwater and pier constructions in the open lake, constituting the "Outer Harbor."

OUTER HARBOR.

This harbor [extract from Annual Report for 1890] consists of several works connected at different dates and designed for the protection of the general lake commerce in view of the great development of the shipping industries and the exposed situation of the harbor at the head of the lake.

The "Outer Basin," projected in 1870 and completed in 1884, is inclosed on the east by the "east pier" (originally 1,100 feet in length, now reduced by the advance of the shore line to half that), on the north by the "breakwater," 1,400 feet in length, and on the west by the outer 550 feet of the "west pier," prolonged for that purpose. The entrance, 215 feet wide, common to the basin and the creek, is at the northwest angle, fronting about north-northwest down the lake.

It was a part of the project that the inclosed area of this "Outer Harbor" should be dredged to a depth of 14 feet, furnishing some 40 acres of protected anchorage, but this has never been done, and if it were the basin would still be of no commercial value whatever for the reason that the entrance, 215 feet, is too narrow for a harbor of refuge and being directly open to the heaviest storms on the lake, viz., those from the northwest quadrant, admits the sea freely into the basin.

Prior to the completion of these structures it was found that the heavy sea, in connection with the powerful currents sweeping westwardly across the mouth, made the entrance extremely hazardous, for which reason a spur pier, locally termed the "Breakwater Pier," projecting at right angles from the west end of the breakwater 100 feet northward into the lake, was designed in 1880 and completed in 1889. This pier has been found of advantage, but the inherent difficulties of the situation remain. There was, therefore, no "harbor of refuge" for vessels needing shelter from the storms of the lake between Chicago and Grand Haven, nor, in fact, could the "Outer Harbor" at Michigan City furnish it so long as the northward opening in the lake was retained, so that, in 1882, the project for the "outer breakwater" was adopted, in conformity with which work was begun last season by the construction of the first pier. The project provides that the eastern end of the new breakwater shall begin 400 feet west of the north end of the Breakwater Pier, leaving that space open as an entrance; thence extending westerly 1,000 feet to the angle in the work; thence inclining towards the shore at an angle of 135 degrees, and extending another 1,000 feet.

The report points out the disadvantage of the proposed direction of the outer arm of the new breakwater, approaching as it does with its outer end to within 600 feet from the then 18-foot curve, thus making any future extension of the breakwater impracticable, and recommends a modification of the project so as to change the proposed alignment to be nearly parallel with the shore line.

So, as a necessary condition of the utilization of the large expendi-

the "old west pier" and the "old east pier." The west pier, being twenty years old and only needs a little more work in good order, but the outer 700 feet of pier, which is now twenty-four years old, is quite gone, the having disappeared at several feet below the water surface and will soon fall, unless speedily rebuilt. A portion would be maintained upon the completion of the project, but a contract as a settlement for the maintenance of the entire harbor, and as the 14-foot curve in the "outer breakwater" with the outer end, it should be rebuilt unless the "general estimate" that this will cost \$5,000.

The operations during the year have consisted in construction over the 300 feet of crib work in the outer harbor, which were placed on a heavy stone foundation in 1880, and above water surface. When work was commenced on structure in the spring of 1901, they had partly settled, and to 4 feet below water, indicating a total subsidence in harbor of 4 to 7 feet. The submerged portion was leveled to water surface, and built on top, the upper course having been of about 6.5 feet above the harbor zero. The work was done and decked over, the ends of the deck plank being secured with blocking timbers. During the winter a further sink place to the extent of 2.4 feet in the worst place, as set levels on June 16, 1902. In order to avoid such excessive sink work in the future, with its attendant extra cost, it is proposed to place all work of this class on a pile and to secure the structure by heavy stone rip rap against the wind.

Some repairs are needed to the decking, which has been many places during the past winter.

With the working balance of \$13,300, available July 1, it is proposed to pay for the repairs of the old breakwater, which are now for repairing the decking of the outer breakwater, partly re-filling in the west pier, dredging for the maintenance of the harbor, and to reserve the rest to be added to the

Appropriations for improving harbor at Michigan City, Ind.

Date.	Outer harbor.	Inner harbor.	Total.
			\$20,000.00
			30,000.00
			60,783.59
4			25,000.00
6			20,000.00
(claim J. R. Bowes)			470.33
8			75,000.00
8 (allotment)			25,000.00
9 (allotment)			31,185.00
9	\$25,000.00		25,000.00
	15,000.00		15,000.00
2	50,000.00		50,000.00
	50,000.00		50,000.00
4	50,000.00		50,000.00
	50,000.00		50,000.00
6	35,000.00		35,000.00
8	50,000.00	\$25,000.00	75,000.00
ent)	2,500.00		2,500.00
	40,000.00		40,000.00
0	40,000.00	15,000.00	55,000.00
	20,000.00	25,000.00	45,000.00
2	60,000.00	20,000.00	80,000.00
	40,000.00	10,000.00	50,000.00
5	54,375.00	1,875.00	56,250.00
8	90,000.00	5,000.00	95,000.00
0	50,000.00	7,500.00	57,500.00
	721,875.00	109,375.00	1,118,638.92

Estimated cost of project for outer harbor, 1870 \$324,421.40
 by cost of repairs and maintenance to 1882 90,067.10
 for outer breakwater, including dredging of outer basin, 1882.. 587,000.00
 Total estimates 1,001,488.50

Amount appropriated and allotted 1870, to and including act of
 October 19, 1890 721,875.00
 Amount expended to June 30, 1892 706,588.95

Money statement.

1891, balance unexpended \$42,954.88
 1892, amount expended during fiscal year 27,668.83
 1892, balance unexpended 15,286.05
 1892, outstanding liabilities 1,905.71
 1892, balance available 13,380.34
 appropriated by act approved July 13, 1892 30,000.00
 Available for fiscal year ending June 30, 1893 43,380.34
 Estimated (estimated) required for completion of existing project. 249,613.50
 that can be profitably expended in fiscal year ending June 30, 1894 100,000.00
 needed in compliance with requirements of sections 2 of river and
 and acts of 1866 and 1867.

INNER HARBOR.

Available depth having diminished to 13 feet at the entrance
 11 feet in some places in the inner harbor, dredging was com-
 at the beginning of September, 1891, and by the end of the
 10-foot channel 16 feet deep had been excavated from the bend
 inner end of the Government revetments to the entrance, and
 wide and 15 feet deep from the bend to the Michigan Central
 bridge.
 dredging plant was thoroughly overhauled and repaired during
 harbor, and operations were resumed April 22, 1892. Between that
 time the entrance has again been deepened to 18 feet for

REPORT OF THE CHIEF OF ENGINEERS, U. S. ARMY.

of 10 feet, the shoal places in the interior navigation to a depth of 15 feet, and dredging has begun on the extension of the harbor at its inner end towards the railroad crossing in accordance with the project. The quantity dredged during the year was 84,893 cubic yards.

With the working balance of \$2,000 at the beginning of the year it is proposed to maintain a 13-foot navigation and to complete the extension of the channel at its upper end.

To complete this work will require the removal of 135,000 cubic yards of material at an estimated cost of \$15,000, to which should be added for repairs and maintenance, \$5,000. Total estimate with 10 per cent contingencies, \$22,000.

Amount appropriated and expended from 1836 to 1869, inclusive.....	\$287,
Original estimated cost of project for inner harbor, 1870, revised in 1892.....	131,
Whole amount appropriated, 1870, to and including act of September 19, 1890.....	109,
Whole amount expended to June 30, 1892.....	105,

Money statement.

July 1, 1891, balance unexpended.....	\$6,
June 30, 1892, amount expended during fiscal year.....	3,
July 1, 1892, balance unexpended.....	3,
July 1, 1892, outstanding liabilities.....	1,
July 1, 1892, balance available.....	2,
Amount appropriated by act approved July 13, 1892.....	15,
Amount available for fiscal year ending June 30, 1893.....	17,
{ Amount that can be profitably expended in fiscal year ending June 30, 1884.....	7,
{ Submitted in compliance with requirements of sections 2 of river and harbor acts of 1866 and 1867.....	

COMMERCIAL STATISTICS, MICHIGAN CITY HARBOR, INDIANA.

Entrances and clearances.

Years.	Number.	Revenue collected.
Fiscal year:		
1885.....	1,334	\$116.35
1886.....	966	90.10
1887.....	1,099	
1888.....	1,167	
Calendar year:		
1888.....	1,153	
1889.....	795	
1890.....	921	
1891.....	837	

Receipts by vessels, 1891.

[Compiled from statements by John M. Clark, collector of customs.]

Articles received.	Quantity.
Coal.....	tons.....
Lath.....	M.....
Lumber.....	M. feet, B. M.....
Posts.....	number.....
Salt.....	barrels.....
Slabs.....	cords.....
Shingles.....	M.....
Stone.....	tons.....
Total	

K K 17.

[Printed in House Ex. Doc. No. 197, Fifty-second Congress, first session.]

MARY EXAMINATION OF GRAND RIVER, MICHIGAN, BELOW GRAND RAPIDS.UNITED STATES ENGINEER OFFICE,
Detroit, Mich., November 12, 1890.

REAL: The preliminary examination of Grand River, Michigan, and Rapids to Grand Haven, called for by circular letter of November 20, 1890, from the office of the Chief of Engineers, has been completed in pursuance of the requirements of the act of September 1, 1889. I beg to report that in my judgment the river between the above-named points is "worthy of improvement" as a commercial waterway. The main facts upon which this opinion is based are given at length in my report of March 22, 1890, a copy of which is attached and for incorporation herewith.*

A summary of the main points of this report and of other papers of reference relating thereto is as follows:

In the case of *The Daniel Ball* (quoted in 10 Wallace, 557; also in 10 Sup. Ct. Decisions, 69) the United States Supreme Court defined the national character of the navigation as follows:

Grand River, in Michigan, held to be a navigable water of the United States, connecting the north, in Lake Michigan, to Grand Rapids, a distance of 40 miles, being capable of bearing for that distance a steamer of 123 tons burden, laden with coal and passengers, and forming by its junction with the lake a thoroughfare for commerce, both with other States and with foreign countries.

Surveys and examinations of Grand River were authorized by the harbor acts of 1880, 1886, 1888, and 1890, and appropriations amounting to \$50,000 were made for its improvement by the river and harbor acts of 1881, 1882, and 1884. Congress, therefore, has repeatedly recognized the status and importance of the stream as a national waterway.

Following my report of March 22, 1890, and the preparation of a plan for the improvement of the river from a new survey made by the Board of Trade of Grand Rapids, The Board of Engineers in New York, to which the subject was referred by the Chief of Engineers, reported, under date of November 12, 1890, that the river is "worthy of improvement" and may be improved by dredging so as to give 6 or 8 feet to Grand Rapids; that 10 feet or more can be obtained by the same means, although the construction of a lock and movable dam might be more economical. Additional information as to the topography and geology of the river valley was obtained for the determination of this and other important questions. A copy of this report† and the accompanying letters from the

Chief of Engineers; printed in Annual Report Chief of Engineers for 1890, pages 2676-2682.

Chief of Engineers; printed in Senate Ex. Doc. No. 101, Fifty-first Congress, first session, Annual Report Chief of Engineers for 1890, pages 2681-2682.

FIG 92—149

Chief of Engineers* and the Acting Secretary of War* is forwarded herewith.

4. As is shown by the accompanying statement, dated November 1890, from the Grand Rapids Board of Trade, it is evident that commercial interests concerned in the improvement of the river are of great magnitude and that the value of water communication with other lake ports and the country generally would amount to a large sum annually.

5. From the data furnished by the official surveys and reports and the personal examinations made, it is found that the natural features of the stream furnish indications favorable to its improvement; there have any been so far disclosed as to discourage a reasonable expectation of its conversion into a valuable water way.

The low-water navigation is already one practically of 4 feet. The variations in width are moderate; the banks for the greater portion of the upper part are firm and with an elevation of from 4 to 6 feet above low water; the lower portion of the river, for one-half its length practically at the lake level and the slope of the upper portion varies from 3 to 6 inches to the mile. The discharge of the river at low water is moderate, is sufficient to fill a channel of 8 to 10 feet with navigable width.

6. The case is evidently one worthy of thorough investigation to establish the points needed for a final determination of the extent to which navigation can be improved and the best means and cost of effecting the same.

In my report of March 22, 1890, I estimated the cost of a thorough examination at \$8,000, which the survey made by the Grand Rapids people would reduce by about \$3,000. I am now of opinion that, with \$4,000 available for the survey, all the information necessary to a final determination of the matter can be procured, including a project and estimates.

For convenient reference there is forwarded herewith a reduction from the large map of the Grand Rapids survey, showing the main features of the stream.

Respectfully,

WILLIAM LUDLOW,

Major, Corps of Engineers, Bvt. Lieut. Col., U. S. A.

Brig. Gen. THOMAS L. CASEY,

Chief of Engineers, U. S. A.

(Through Col. O. M. Poe, Corps of Engineers, Division Engineer Northwest Division.)

[First indorsement.]

U. S. ENGINEER OFFICE,

Detroit, November 14, 1890

Respectfully forwarded.

As a report of preliminary examination these papers seem to me to be exhaustive. I have personally examined the river, as well as could be done in one day, and concur in the opinion of Major Ludlow that it is worthy of improvement.

I therefore recommend that his project and estimate for a survey examination be approved.

O. M. POE,

*Col., Corps of Engineers, etc.,
Engineer Northwest Division*

* Omitted; printed in Senate Ex. Doc. No. 104, Fifty-first Congress, first session. Not printed.

our factories in that branch of industry being the largest in the world. The enactment of the interstate-commerce law, railroad freight rates to have been largely increased, forcing our manufacturers in sharp and, in instances, ruinous competition with centers situated on our lakes and navigable rivers, compelling some to remove their plants to cities where water communication with Lake Michigan was had, and some to change their line of manufacture. Industries, having their attention called to our city through its well-earned reputation on the wood-working lines, have passed us by, needing cheap water transport, at least for their raw material, and have located at lake ports. The hardest of which we at present use 28,000,000 feet annually, becomes each year a source of access by railroad and the fine oak timber lands in Manistee, Benoni, Charlevoix, and other counties in northern Michigan are rapidly invaded by manufacturers from Chicago, Milwaukee, Kenosha, Racine, Cheboygan, and other thriving cities situated on the opposite shore of Lake Michigan, and we, for want of water communication, are powerless to claim our share of this wealth so needed for our future prosperity.

The jobbing trade with an investment of about \$4,000,000 feels the need of low transportation to meet the favored competitors of Detroit, Chicago, and Mil-

waukee and more jobbing houses to meet the demand of the rapidly growing towns in Michigan and of the upper peninsula. Capital for investment in such a project is ready, provided it can get their wares from the eastern markets and foreign countries by an all-water route.

Grand Rapids is a port of entry and we import direct.

The manufacturing and commercial interest of Grand Rapids is growing, and our many difficulties, is evidenced in the fact that the combined freight of our railroads and one small river steamer for 1889 was 1,120,823 tons as compared with 88,000 tons for 1888, and the year's business from data at hand to date will be a larger amount.

The passenger traffic for 1889 over these lines shows 309,226 persons in and out. Figures are obtained by us from the official records furnished our board by the companies.

The future of our city will depend largely on such improvement of our river as will enable it to receive lake vessels to load and discharge freight at our port. Every business in this city is alive to that project. The counties of Ottawa and of Kent, through their representatives, have indorsed this scheme by suitable resolutions presented to the legislature.

Rapidly growing northern and western Michigan and the upper peninsula are looking for a market, and their newspapers have seconded the effort to that effect. The jobbing interest, with but a small increase of capital, has increased its sales from \$12,489,500 in 1887 to nearly \$16,000,000 in 1889, showing that anything else the growth of such sections of our State as look to Grand Rapids for their supplies. Hard-wood timber lands being cleared becomes fine farm-land and is taken up as fast as it is put on the market. Manufactories of various

With an average saving of only 1 cent per 100 pounds on this traffic it represents the enormous amount of \$200,000 per year that would be saved in freight.

If any other information you may require, I shall be glad to furnish it.

H. D. C. VAN ASMUS,
Secretary

Mr. William Linnell,
Chief of Engineers, U. S. A.

SURVEY OF GRAND RIVER, MICHIGAN, BELOW GRAND RAPIDS.

UNITED STATES ENGINEER OFFICE,
Detroit, Mich., April 11, 1889

GENTLEMEN: I have the honor to submit a report, with accompanying papers, maps, etc., on the Grand River, Michigan, from Grand Rapids to Grand Haven, as authorized by the act of September 19, 1878, in the following terms:

Grand River, below Grand Rapids, Michigan, in a view of determining the existence of a navigable channel of the river, and the detailed topography of the valley subject to a...

Grand River, the largest in the State and the main affluent of Lake Michigan, drains the heart of the Lower Peninsula, with a watershed stretching across to within 40 miles of Lake Erie and covering 8,000 square miles of territory. The mouth of Grand River, on the west shore of Lake Michigan, is the town of Grand Haven, whose harbor has always been an important harbor of refuge by reason of its capacity and depth, due to the large discharge of the stream. For 100 miles up the river, at the head of navigation, is the city of Grand Rapids, an occasionally thriving center of population and industry, whose products, particularly furniture, reach the markets of the world.

Between Grand Haven and Grand Rapids the river is navigable by light draft vessels, the depths diminishing from 4 fathoms in the harbor to 4 feet in the upper reaches, with a few bars of less depth at intermediate stages, and this navigation it is proposed, if practicable, to deep and maintain in order that the full capacity of the stream for water transportation may be developed. The question therefore resolves itself into (1) the extent to which the physics of the stream and its valley will admit of betterments of the existing transportation facilities, the means best adapted to this end, and (2) the cost of securing them.

For a better understanding of the general and special aspects of the matter, its official history may first be summarized.

The status of the navigation in question was adjudicated by the Supreme Court of the United States in the case of *The Daniel Ball* vs. *Woodward*, 10 Wall. 19, 1870.

The Grand River, Michigan, being a navigable water of the United States, the act of March 3, 1879, authorized Grand Rapids and a distance of 40 miles, being a substantial barrier to the passage of a steamer of 125 tons burden, laden with mail, with passengers, and carrying 200 persons with the lake a continuous highway of commerce between other States and with foreign countries.

The act of June 14, 1880, authorized a survey and examination of the navigable section, a report of which was submitted under date of February 17, 1881.

Transect lines were run on both banks from Grand Rapids to the head end of the Ottawa boom, a distance of 33 miles, and a line of level on the north bank from Grand Rapids to Spessville, 29 miles, with

of dredging with the facilities for such work then existing, and at 30 cents per cubic yard the estimate for an 8-foot channel by dredging and wing dams was about \$750,000. An act of March 3, 1881, appropriated \$10,000 toward an improvement of the river; that of August 2, 1882, \$15,000; and that of July 5, 1883, \$5,000; which sums were mostly expended in deepening the channels by dredging; but the contract cost was excessive, ranging from 15 to 25 cents and 30 cents per cubic yard, and the net results were correspondingly reduced.

It is to be noted, as indicating the nature of the river bed and the difficulties of maintaining a dredged channel, that the materials excavated and deposited near the line of the improvement are still in place. Subsequent acts of August 5, 1886, and August 11, 1888, authorized further examinations, that of 1888 for the first time declaring the river a channel of navigation called for, viz: "a channel of navigable width and a minimum depth of 10 feet."

In reference to this, after personal inspection of the river and examination of the records, maps, etc., I made report under date of March 22, 1890, from which the following is taken:

From the data of record a general view of the situation can be had and some of the facts become evident.

It has been on the part of the commercial interests of Grand Rapids a strong and persistent endeavor to secure, if such a thing be possible, adequate water communication with Lake Michigan, and in the absence of the means of forming a channel of what is practicable, their demands have gone to the fullest limit of ability. Grand Rapids has apparently gauged her navigation requirements as the measure of her own development and commercial standing, and insisted that the river should be made to fulfill them, as is indicated in Major Harwood's original report, where reference is made to the desire for means of making shipments of lumber and other products direct to Europe without breaking bulk. In the course of the investigation it is found that, in fact as this there are more obstacles than the inadequate capacity of Grand

Rapids. Commercial statistics for the year 1887 state the number of operatives in the lumber industry at 11,110, the capital employed at \$15,216,400, and the value of the product at \$24,048,800. Such industries as these can not wisely be discouraged, and the difference in cost between rail and water freights—the latter averaging about half the former—be taken into account, the annual value of water commu-



at lowering of the bed of the channel at the upper limits, due opening, the datum slopes being less than half an inch to the mile Grandville down, and about two-thirds of an inch to the mile and Rapids to Grandville.

at prices for material in the estimates are believed to be fairly and contingencies of 15 per cent are included in the totals. The item, of course, is the dredging, which is estimated at 10 cents. In the case of a project of this magnitude there should be no difficulty in doing the work within this figure. The Government dredging plant on the east coast, with plant of small capacity and movable to point, averages less than 10 cents, and in one case a contract for 60,000 cubic yards was made at Portage Lake for that price. In the case of a question of handling 3 or 4 million yards, the price, if the means and a properly equipped plant are available, should be about 7 or 8 cents.

General conclusions derivable from the data now at hand may be summarized as follows:

1. An open 10-foot channel of navigable width can be constructed at a sum the expenditure of which would be warranted by the magnitude of the commercial interests concerned and the economic value of improved navigation.

2. The construction of a dam in the stream in order to diminish the need of dredging needed to secure a 10-foot navigation will not reduce the total cost of the project, and the objection thereto, viz, cost of construction and maintenance and interference with free movement of water and flow of water, as well as possible claims for land damages, are counterbalanced by any economic advantages.

3. The reduced cost of the 8-foot open channel is not sufficient to justify the restriction in navigation facilities.

4. It is probable that at some points permanent works in the bed of the stream may hereafter be found advisable to regulate the movement of water, and an allowance is made in the estimates for these constructions.

5. It is evident, however, that such permanent works should not be adopted until the approximate corrected regimen of the river shall have been established, and prolonged observation and experience under the various conditions of discharge clearly prove their necessity and indicate the sites and methods to be adopted in building them.

6. The estimated cost of the three comparative projects is as follows:

open 8-foot navigation.....	\$163,450
open 8-foot navigation with lock and dam.....	673,880
open 10-foot navigation.....	670,500

The totals in each case include a sum of \$169,000 as a preliminary estimate for wing dams, etc. It is entirely possible that much of this sum may be found unnecessary, but it is considered proper to introduce it in the estimates.

7. We recommend the adoption of the last project and that an appropriation of \$100,000 be made for beginning the work.

8. It is desirable that the total sum required should be placed at the disposal of the Engineer Department, to be drawn against in annual sums as needed, in order that the completion of the work should not be delayed for without suspension from lack of funds, and contracts should be let for the full amount of work to be done. By this means costly delays could be avoided, and advantage could be taken to procure the use of a thoroughly equipped and effective plant, under conditions that would secure the best economic and engineering results.



1. The first part of the document discusses the importance of maintaining accurate records of all transactions and activities. It emphasizes that this is crucial for ensuring transparency and accountability in the organization's operations.

2. The second part of the document outlines the various methods and tools used to collect and analyze data. It highlights the need for consistent data collection practices and the use of advanced analytical techniques to derive meaningful insights from the data.

3. The third part of the document focuses on the role of technology in enhancing data management and analysis. It discusses the benefits of using cloud-based storage solutions and data visualization tools to improve the efficiency and effectiveness of the data analysis process.

4. The fourth part of the document addresses the challenges associated with data management and analysis. It identifies common issues such as data quality, data security, and data integration, and provides strategies to overcome these challenges.

5. The fifth part of the document concludes by summarizing the key findings and recommendations. It emphasizes the importance of ongoing monitoring and evaluation to ensure that the data management and analysis processes remain effective and up-to-date.

three times, the results of which differed somewhat. Before beginning the herefore, an adjusted or most probable value for the elevations of the was determined, and these most probable elevations used.

datum plane of reference in the surveys of 1884 and 1889 is 100 feet below the water table of Rindge, Bertsch & Co.'s store, corner of Pearl and Campau Grand Rapids, the point taken being on the Pearl street side, 1 foot and from the Campau street side." All elevations in the present survey have red to the same datum. The datum plane of reference for the city of Grand 76.08 feet higher than the above datum.

II.—BORINGS.

ing at the Fulton Street Bridge, in Grand Rapids, borings were made in the for about 32 miles. For the remaining 7 miles, to the mouth at Grand more than a 10-foot channel already exists, and no borings were necessary art of the river. Ninety-two borings were made in all, decreasing in fre- down river. For the first 6 miles the average number of borings per mile is the next 9 miles the average is three, and for the remaining 17 miles the is two per mile.

orings were made from a flatboat or raft 20 feet long by 10 feet wide and inches deep. The boat was provided with spuds at the corners for g it and keeping it steady while at work. A well hole about 8 inches was made in the middle of the boat for working the tools through.

ections, from 3 to 5 feet in length, of extra heavy 2-inch (internal diameter) ing with strong couplings were procured. A strong steel annular shoe, larger in external diameter than the pipe and formed with a cutting edge, wed to the lower end of the lower section. On the upper end of the upper was screwed a strong steel driving-head. To make a boring the pipe was ertically through the well-hole in the boat and driven by sledges handled aborers. The blows were transmitted to the steel head through a wooden ound with iron rings and held in place. As the pipe descended under the e blows, it was turned around by a pipe-wrench. When occasion re- se head was unscrewed, another length of pipe introduced, the head screwed p of the new length, and the driving continued. To withdraw the pipe a was placed on the boat with its top vertically over the well-hole. Power ted through a windlass with ordinary wagon wheels attached for applying er, and a 3-sheaved block and tackle for further multiplying the hand force. this arrangement no serious difficulty was experienced in withdrawing the ipe. In most cases it was possible to drive the pipe to the desired depth

withdrawing it in the process, but in hard material in order to reach the l depth it was sometimes found necessary after driving the pipe to refusal draw it, remove the material collected within the pipe, replace it, drive id so on. Sometimes the required depth was reached by introducing within ch pipe a smaller pipe with a drill attached, and the hole continued by g the latter. A large number of specimens of material brought up in the ipe were preserved for record.

epths of the borings vary from 13 to 15 feet below water surface at the time r made, which was at a low stage of the river. As those in the upper part ver have a depth of 15 feet, it will be seen that the borings give a knowledge aterial underlying the river bed for about 10 feet below the water level of chigan. At the time of making the borings their locations were determined uments to stakes on the shore or by ranges. A stake was also set with its ot above water surface. Afterwards the location and elevation of these ere determined by the stadia with reference to points of the survey. The of the borings, and the elevation of their bases, or of the different strata of l found, thus become known.

m Shell Bar, 5 miles below Grand Rapids, very hard material was found at elow water surface, and at 12 feet, or at an elevation of 59.6, plaster rock ck. This rock does not extend for more than 1,000 feet in the direction of r, and as it is almost wholly below any contemplated dredging, and is a k, it is hardly worth noting. At no other place was rock found. In the miles gravel and clay predominate and below this sand.

rat boring was made August 26, and the last on September 23. For want of boat, the most of the borings in the upper reach were made near the shore, o place was especial pains taken to make the borings in the deepest water. ievied that this does not detract from the reliability of the information ob- rom them, as it is not likely that the character of the material would change eet or so either side of the channel. These borings have been plotted on of the survey and a sectional representation of them has been prepared, ich full information of the material met with in the bed of the river may ned at a glance.

Record of the borings—Continued.

Material.	Depth.	Thick- ness.	Eleva- tion of water surface.	Date.	Distance from foot of Graves canal.	Remarks
	Feet.	Feet.	Feet.		Feet. Below.	
Boring 16:						
Water.....	2.3	2.3	73.4	Sept. 4	15,916	Near right bank.
Fine sand.....	15.0	12.7				
Boring 17:						
Water.....	2.4	2.4	73.3	Sept. 5	15,903	Near mouth of creek on right bank.
Yellow clay and sand mixed.....	10.0	7.6				
Sand and coarse gravel.....	15.0	5.0				
Boring 18:						
Water.....	2.5	2.5	72.6	Sept. 5	16,882	Located near right bank.
Sand, shell, and reddish clay.....	11.5	9.0				
Coarse gravel.....	13.6	2.1				
Gravel and white sand.....	14.3	6.7				
Boring 19:						
Water.....	3.0	3.0	71.9	Sept. 5	16,670	Near left bank at mouth of creek.
Mud.....	4.0	1.0				
Sand.....	9.5	5.5				
Very coarse gravel and sand.....	15.0	5.5				
Boring 20:						
Water.....	1.7	1.7	71.9	Sept. 6	16,906	Near right bank a spring creek.
Fine sand.....	5.0	3.3				
Sand, gravel, and shell.....	6.5	1.5				
Blue clay (hard).....	14.8	8.3				
Boring 21:						
Water.....	3.7	3.7	71.9	Sept. 8	21,064	Near right bank.
Sand.....	5.0	1.3				
Blue clay (hard).....	12.0	7.0				
Blue clay (hard), sand, and gravel.....	14.8	2.8				
Boring 22:						
Water.....	4.0	4.0	72.9	Sept. 8, 9	22,965	Located at mouth of creek, right bank. Too hard to drive further.
Sand.....	6.0	2.8				
Yellow clay.....	8.0	2.0				
Gravel and clay (hard).....	12.0	4.0				
Boring 23:						
Water.....	4.0	4.0	72.0	Sept. 9	22,911	6 feet below boring 22.
Sand.....	6.0	2.0				
Yellow clay.....	8.0	2.0				
Clay, sand, and gravel mixed (very hard).....	15.0	7.0				
Boring 24:						
Water.....	3.4	3.4	72.0	Sept. 9	24,006	Near right bank.
Fine sand.....	9.0	5.6				
Gravel and blue clay (soft).....	10.0	1.0				
Blue clay (soft) and stone.....	15.0	5.0				
Boring 25:						
Water.....	3.7	3.7	71.5	Sept. 10	26,382	Near left bank at Clam-shell bar; rock at 10 ft.
Sand.....	8.0	4.3				
Blue clay, sand, and shell mixed (very hard).....	10.0	2.0				
Boring 26:						
Water.....	3.7	3.7	71.5	Sept. 10	26,392	Rock at 10 ft.
Sand.....	8.0	4.3				
Blue clay, sand, and shell mixed (very hard).....	10.0	2.0				
Boring 27:						
Water.....	2.5	2.5	71.6	Sept. 11	26,774	Located at right bank at Clam- shell bar.
Fine sand.....	6.0	3.5				
Clay.....	9.0	3.0				
Clay and stone (hard).....	12.0	3.0				
Plaster rock.....	13.2	1.2				
Boring 28:						
Water.....	3.8	3.8	71.6	Sept. 11	26,960	Near right bank at Clam-shell bar; rock at 10.8 ft.
Fine sand.....	6.0	2.2				
Clay.....	10.8	4.8				
Boring 29:						
Water.....	4.5	4.5	71.5	Sept. 11	27,090	Near right bank at Clam-shell bar; rock at 10.02 ft.
Sand.....	6.0	1.5				
Blue clay (stiff).....	8.0	2.0				
Blue clay and gravel.....	10.2	2.2				
Boring 30:						
Water.....	4.5	4.5	71.3	Sept. 12	29,257	Near left bank.
Sand.....	14.0	9.5				
Boring 31:						
Water.....	5.0	5.0	71.3	Sept. 12	31,105	136 ft. from left bank at Grand- ville.
Sand.....	7.0	2.0				
Yellow clay (stiff) and stone.....	14.0	7.0				

Record of the borings—Continued.

Material.	Depth.	Thick- ness.	Eleva- tion of water surface.	Date.	Distance from foot of Ganoes canal.	Remarks.
	<i>Feet.</i>	<i>Feet.</i>	<i>Feet.</i>		<i>Feet.</i>	
.....	7.8	7.8	69.3	Sept. 18	Below 71,786	125 ft. from right bank.
.....	14.4	6.6				
.....	5.0	5.0	69.3	Sept. 18	72,018	113 ft. from left bank.
d.....	13.5	8.5				
and.....	14.5	1.0				
.....	3.8	3.8	69.3	Sept. 18	74,420	124 ft. from left bank.
and.....	13.3	8.5				
d.....	14.3	1.0				
.....	4.0	4.0	69.2	Sept. 19	75,977	Near mid-channel.
and.....	12.0	8.0				
coarse sand.....	14.0	2.0				
.....	4.0	4.0	69.2	Sept. 19	77,538	Near left bank.
and.....	12.5	8.5				
.....	14.0	1.5				
.....	4.0	4.0	69.2	Sept. 19	79,468	Located near left bank.
.....	14.0	10.0				
.....	5.0	5.0	69.0	Sept. 19	81,833	Near left bank.
.....	14.0	9.0				
.....	7.0	7.0	69.0	Sept. 19	84,018	37 ft. from left bank.
.....	8.0	1.0				
and.....	14.0	6.0				
.....	3.8	3.8	69.0	Sept. 19	86,498	Located 78 ft. from left bank.
and.....	8.8	5.0				
and (soft).....	11.0	5.2				
.....	3.8	3.8	68.75	Sept. 19	89,687	140 ft. from left bank.
and.....	6.8	3.0				
and.....	13.0	6.2				
.....	14.0	1.0				
.....	11.3	11.3	68.7	Sept. 21	91,125	Near center bridge pier at Lamont.
and.....	14.3	3.0				
.....	6.5	6.5	68.7	Sept. 21	92,610	145 ft. from left bank.
.....	14.0	7.5				
.....	5.0	5.0	68.6	Sept. 21	95,360	60 ft. from right bank.
and.....	14.0	9.0				
.....	6.2	6.2	68.6	Sept. 21	97,853	Located near mid- channel.
and.....	7.2	1.0				
id.....	14.0	6.8				
.....	4.2	4.2	68.6	Sept. 21	100,218	150 ft. from right bank.
and.....	14.0	9.8				
.....	3.0	3.0	68.6	Sept. 21	102,750	140 ft. from left bank.
id.....	14.0	11.0				
.....	7.0	7.0	68.5	Sept. 22	104,698	Near right bank.
.....	13.5	6.5				
.....	7.5	7.5	68.5	Sept. 22	106,604	Do.
and, fine gravel and.....	13.5	6.0				
.....	7.0	7.0	68.5	Sept. 22	108,843	Near lower end of dock at East- manville.
and.....	10.0	3.0				
.....	13.5	3.5				
.....	4.0	4.0	68.4	Sept. 22	110,755	Located 96 ft. from right bank.
and.....	12.0	8.0				
.....	13.5	1.5				
.....	5.0	5.0	68.4	Sept. 22	112,325	82 ft. from right bank.
and.....	12.0	7.0				
l clay mixed.....	13.0	1.0				

the wire on the upper section was so adjusted the soundings of this section directly upstream from the corresponding lower section.

ity of that portion of the stream lying between any two soundings on section and the corresponding two on the upper section was determined by

Before dropping in a float it was so weighted as to pass down through section with its lower end close to the river bed. The place for drop-

floats was determined by stretching the wire across the river about 20

the upper sounded section so as to bring the tags directly above the mid-

points determined by the corresponding down and up stream soundings. The

then dropped at these tags from a small boat. The time at which each

d the upper and lower sounded sections was taken to the nearest second.

se observations the mean area and velocity per second of each 10-foot

the river was determined, which multiplied together gave the discharge

tion, and the sum of these the total discharge. The total discharge di-

se total area gives the mean velocity. From the care taken to have the

h near the bottom, and from other considerations, it has been thought

y to make any correction or deduction from the discharge as computed

erved quantities. A temporary gauge for reference during the sounding

bservations was set and afterwards referred to a permanent bench.

ct of setting the four posts on the bank was to preserve the exact loca-

gaged section in the hope that rises in the river would occur during

is of the survey and enable new gaugings to be made at the same place

t river stages, for the purpose of comparison and more accurate deter-

f relative velocities and discharges due to different depths.

level and tape the form of the sounded sections from the water's edge to

t the top of the bank was determined, so that a complete section of the

sen the tops of banks could be plotted, and the area and wetted perimeter

pth could be computed. With this data such a section of the river bed

etween the sounded sections has been prepared at each of the places of

is.

e of the river remained almost stationary during the survey, and no op-

ffered for further gauging. The slopes, velocity, and discharges, as

se accompanying table for other stages than that of the observation, must

be taken as near approximations only. The result of gauging near Grand

en in the first line of the table.

comparison of the conditions of Grand River with similar conditions in

rs where gaugings have been made, we are led to select for the coefficient

as in Kutter's formula a value of 0.026. Substituting this value, the ob-

an velocity and the hydraulic mean radius in that formula, and solving,

0065 as the value of the slope. From the observed low water of 1889, the

water surface in the 9,880 feet next above Grandville is 0.33 feet, giving a

e of 0.0000334. Considering the fact that the water surface at the time

of gauging was about 0.45 feet lower than the observed low water of 1889

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The following table gives the results of the observations and computations derived from Kutter's formula, which is more fully elaborated on page 2

Stages on the river below the dam	Stage of discharge section	Mean depth of water	Sectional area	Wetted perimeter	Hydraulic mean radius	Slope	Mean velocity, feet per second
			A	b	$r = \frac{A}{b}$	i	$v = \frac{f}{A}$
74	1.25	2.95	1,275.81	320.0	3.988	.00005	0.5410
75	1.26	2.96	1,275.96	320.5	3.980	.00004603	1.2168
76	1.27	2.97	1,276.11	321.0	3.972	.00004206	1.9742
8	1.28	2.98	1,276.26	321.5	3.964	.00003809	2.7316

An examination of the tabulated gauge readings herewith will show the time of year and duration of the stages for which discharges have been computed.

Statement of the highest, lowest and average monthly discharges of river surface above Grand Rapids, Grand Haven, and Lamont, and of the lake surface at Grand Rapids.

Months	Means			Highest observed.			Lowest obs.		
	Grand Rapids	Grandville	Lamont	Grand Rapids	Grandville	Lamont	Grand Rapids	Grandville	Lamont
Jan	1,000	1,000	1,000	1,000	1,000	1,000	1,000	1,000	1,000
Feb	1,000	1,000	1,000	1,000	1,000	1,000	1,000	1,000	1,000
Mar	1,000	1,000	1,000	1,000	1,000	1,000	1,000	1,000	1,000
Apr	1,000	1,000	1,000	1,000	1,000	1,000	1,000	1,000	1,000
May	1,000	1,000	1,000	1,000	1,000	1,000	1,000	1,000	1,000
Jun	1,000	1,000	1,000	1,000	1,000	1,000	1,000	1,000	1,000
Jul	1,000	1,000	1,000	1,000	1,000	1,000	1,000	1,000	1,000
Aug	1,000	1,000	1,000	1,000	1,000	1,000	1,000	1,000	1,000
Sep	1,000	1,000	1,000	1,000	1,000	1,000	1,000	1,000	1,000
Oct	1,000	1,000	1,000	1,000	1,000	1,000	1,000	1,000	1,000
Nov	1,000	1,000	1,000	1,000	1,000	1,000	1,000	1,000	1,000
Dec	1,000	1,000	1,000	1,000	1,000	1,000	1,000	1,000	1,000

STATE OF MICHIGAN

IN SENATE,
 January 1, 1888.
 REPORT OF THE CHIEF OF ENGINEERS,
 U. S. ARMY,
 ON THE
 PROGRESS OF THE RECONSTRUCTION OF THE
 GRAND RAPIDS DAM,
 ON THE GRAND RAPIDS RIVER,
 IN THE STATE OF MICHIGAN,
 UNDER THE ACT OF MARCH 3, 1879,
 CHAP. 108, SECT. 1034, U. S. STATUTES AT LARGE,
 VOL. 21, P. 372.

ALBION: PUBLISHED BY THE STATE OF MICHIGAN, AT THE OFFICE OF THE CHIEF OF ENGINEERS, U. S. ARMY, GRAND RAPIDS, MICHIGAN, 1888.

U. S. ARMY
 OFFICE OF THE CHIEF OF ENGINEERS
 GRAND RAPIDS, MICHIGAN
 1888

PROFILE.

A profile of the river from Grand Rapids to Grand Haven has been prepared showing the low-water surface of November, 1889, which is nearly as low as that of 1891, and the rise of the water at the time the soundings were taken, the probable water rise for each 1 foot rise on the Grand Rapids gauge from a 75 to an 83 foot stage, the location and depth of borings, and a mean profile of the river bed averaged for a distance of 100 feet.

The line along which this profile is made is shown on the maps, and, approximating the present channel, is the line along which it is thought the proposed improvement should be made. A reduced profile of the entire river has also been made. The drawings of the bottom of proposed dredging are also shown on these profiles.

GAUGE READINGS.

Gauge readings were taken at four places on the river from December 22, 1890, to December 13, 1891, namely, at Grand Rapids, Grandville, Lamont, and Grand Haven. The readings have been plotted on profile paper for convenience of comparison.

The following maps and profiles of Grand River, below Grand Rapids, submitted herewith.*

1. A map of upper 12 miles of river.
2. A map of middle 12 miles of river.
3. A tracing of lower 15 miles of river.
4. A tracing of reduced map and profile of river.
5. A profile of river.
6. Gauge reading.
7. Sections of borings.

The following table shows the low-water (1889) slopes, total fall, and fall in inches of different portions of the river. It will be seen from it that the fall from Grand Rapids to Grandville, a distance of 6 miles, is 2.85 feet, or 5.73 inches per mile; from Grandville to Lamont, a distance of 11.5 miles, it is 2.92 feet, or 3.05 inches per mile; and that from Lamont to Grand Haven, a distance of 21.64 miles, the fall is 1.04 foot, or 0.24 inch per mile; also that from Grand Haven to the mouth of Grand Creek, a distance of 27.26 miles, the fall is only 1.04 feet, or 0.46 inch per mile, leaving 5.16 feet fall in the upper 12 miles. Any contemplated improvement therefore be principally concerned with these upper 12 miles.

Low water (1889) slopes and total fall.

Place.	Distance from canal.	Elevation.	Place.	Distance from canal.	Elevation.	Difference of distance.	Difference of elevation.	Slope.	Fall per mile.
a Canal	0	74.47	B. M. 2	4,320	74.09	4,320	.38	.0009877	Ins.
1	4,320	74.09	B. M. 3	8,260	73.64	3,940	.45	.0001142	5.56
2	8,260	73.64	B. M. 4	13,442	72.42	5,182	1.22	.000235	7.24
3	13,442	72.42	B. M. 5	16,956	72.39	3,514	.03	.0000085	14.89
4	16,956	72.39	B. M. 6	21,624	71.95	4,668	.44	.0000904	.54
5	21,624	71.95	B. M. 8	31,504	71.62	9,880	.33	.000033	5.90
6	31,504	71.62	B. M. 9	32,605	71.57	1,101	.05	.000012	2.09
7	32,605	71.57	B. M. 11	38,453	71.01	5,768	.56	.000097	2.66
8	38,453	71.01	B. M. 12	43,735	70.14	5,282	.87	.0001647	6.15
9	43,735	70.14	B. M. 13	46,805	70.04	3,070	.10	.0000325	10.44
10	46,805	70.04	B. M. 14	50,295	69.95	3,490	.09	.000026	2.06
11	50,295	69.95	B. M. T	55,636	69.70	5,341	.25	.0000468	1.65
12	55,636	69.70	B. M. J	62,570	69.31	6,934	.39	.0000562	2.97
13	62,570	69.31	B. M. 15	77,530	69.18	14,960	.13	.0000087	3.66
14	77,530	69.18	B. M. I	84,961	69.00	7,431	.18	.0000242	5.55
15	84,961	69.00	B. M. S	84,165	68.84	3,204	.16	.0000430	1.48
16	84,165	68.84	B. M. 19	92,235	68.70	4,070	.14	.0000344	.66
17	92,235	68.70	B. M. 11	97,586	68.58	5,351	.12	.0000253	.72
18	97,586	68.58	B. M. 111	105,165	68.50	7,579	.06	.00001056	.83
19	105,165	68.50	B. M. IV	110,582	68.44	5,277	.06	.00001137	.52
20	110,582	68.44	B. M. V	119,297	68.36	8,715	.08	.00000819	.19
21	119,297	68.36	B. M. VI	126,081	68.34	6,783	.02	.00000295	.34
22	126,081	68.34	B. M. VII	135,305	68.29	9,225	.05	.00000542	.24
23	206,500	68.27	B. M. 19 Lamont (B. M. J.)	92,235	68.70	114,265	.43	.000065763	.46
24	206,500	68.27	Sand Creek	62,570	69.31	143,930	1.04	.000007225	.83
25	92,235	68.70	do	62,570	69.31	29,665	.39	.000013147	3.05
26	92,235	68.70	B. M. 8, Grandville	31,504	71.62	60,731	2.92	.000048008	4.71
27	62,570	69.31	do	31,504	71.62	31,060	2.31	.000074358	5.73
28	31,504	71.62	Foot of Gauges Canal.	0	74.47	31,504	2.85	.0000904647	

* Not printed.

For the purpose of showing the relation between quantity of discharge and section of channel, the following table has been prepared by the aid of the latter and Kutter formula:

$$v = \left(\frac{a + \frac{l}{n} + \frac{m}{i}}{1 + a + \frac{m}{i}} \right)^{1/2} \sqrt{ri}$$

in which $a=41.66$, $l=1.81132$, and $m=0.0028075$,
 v =mean velocity, r =the hydraulic mean radius,
 i =sine of slope and n =coefficient of roughness of perimeter.

In the table n is taken = .026.

The form of a section of the channel is assumed to be trapezoidal.

z = width of base.

ϕ = angle of inclination of sides to the horizon.

d = depth.

A = area.

b = wetted perimeter.

Hence,

$$A = dz + \frac{d^2}{\tan \phi} = \frac{dz \tan \phi + d^2}{\tan \phi}$$

and

$$b = z + \frac{2d \sec \phi}{\tan \phi} = \frac{\tan \phi z + 2d \sqrt{1 + \tan^2 \phi}}{\tan \phi}$$

Table to ascertain dimensions of channel to be filled.

z	$\tan \phi$	d	A	b	v	n	i	Fall per 1000	r
		6	918	176.83	5.1944	0.26	0.00015	Inches.	
		8	752	110.77	6.6143	0.26	0.00015	2.2275	1.660
		8	912	150.63	6.5873	0.26	0.00015	2.2275	1.40
		8	848	125.77	6.7157	0.26	0.00015	2.2275	1.000
		10	600	90.72	7.1026	0.26	0.00015	2.2275	0.915
		10	1100	134.72	8.1650	0.26	0.00015	2.2275	1.285
		10	1340	167.24	7.9657	0.26	0.00015	2.2275	1.000
		10	1500	184.72	8.1953	0.26	0.00015	2.2275	1.000
		10	1660	163.24	7.9657	0.26	0.00015	2.2275	1.000
		12	1220	146.7	8.7766	0.26	0.00015	2.07575	1.000

of discharge, q , taken in the table in all except the last three cases is the observed low water discharge at the 1891 gauging above Grandville. In the last three cases q is the discharge at the same place or a similar one at 20 ft. points, which is 1.53 feet above the low-water stage.

Assuming $v = 0.880035 = 2.2176$ inches per mile, which is the velocity of a nearly 2 miles above Grandville, a 6-foot channel with a base of 150 feet will have a base of 153 feet; if the depth be increased to 8 feet, the width on the bottom is reduced to 78 feet; and if the depth be increased to 10 feet, the width is 49 feet. Again, for a 100 ft. channel with a base of 100 feet, it will be seen from a preceding table to be necessary to increase the width to 107 feet at the lower 27.25 miles, and $q = 988.1$ at 100 ft. above Grandville, the bottom and have a side slope of 5 to 1.

TABLES FOR OPEN CHANNELS.

For the purpose of showing the relation between quantity of discharge and section of channel, the following table has been prepared by the aid of the latter and Kutter formula: In the table n is taken = .026. The form of a section of the channel is assumed to be trapezoidal. z = width of base. ϕ = angle of inclination of sides to the horizon. d = depth. A = area. b = wetted perimeter. Hence, $A = dz + \frac{d^2}{\tan \phi} = \frac{dz \tan \phi + d^2}{\tan \phi}$ and $b = z + \frac{2d \sec \phi}{\tan \phi} = \frac{\tan \phi z + 2d \sqrt{1 + \tan^2 \phi}}{\tan \phi}$

bridges at Grand Haven, it is proposed to pass up the left channel to the channel, and thence up the middle channel to the main river channel. of this middle channel is about 4,400 feet. It already has a maximum depth of more than 10 feet throughout its whole length, and a mean depth of 10 feet in width in the lower 2,700 feet, and a mean depth of 8 feet for the same in the upper 1,700 feet. The upper end of this channel should be widened and deepened. The expense would be small and is fully covered by the estimates. Following the middle channel the line follows approximately the existing one. The "Cawa Boom" has been removed, and all logging interests in the river are abandoned. With the exception of the few hundred feet at the head of the middle channel mentioned above, a more than 10-foot channel already exists for 6½ miles above Grand Haven. Above this point as far as the mouth of Bass River, or rather, a 7 to 8 foot channel exists. Above Bass River the depth of channel is less than 10 feet excepting at the shoalings and bars given in the following table a channel exists to Grand Rapids:

Statement of the location of shoalings and bars with less than 4 feet of water.

Distances below foot of Ganoc Canal, Grand Rapids.		Length.	Depth at crossing.	Location.
Beginning.	End.			
Feet.	Feet.	Feet.	Feet.	
0	270	270	3.0	
800	1,200	400	3.6	
2,900	3,400	500	3.0	
4,300	4,750	450	3.9	
5,550	5,950	400	3.5	Mouth of Plaster Creek.
8,200	9,600	1,400	3.0	Opposite Lower Plaster Mill.
13,500	15,100	1,600	2.5	Just below Lake Shore and Michigan Southern Railroad Bridge.
16,500	17,350	850	2.7	
17,800	21,300	3,500	2.0	
24,750	25,000	250	3.8	
27,600	28,050	450	2.9	Clam Shell Bar.
36,100	36,600	500	3.8	
39,350	40,000	650	3.6	At Weatherwax's.
41,400	43,200	1,800	3.0	At Boynton's.
49,000	50,100	1,100	3.0	
56,400	56,900	500	3.8	At Haire's.
74,600	77,100	2,500	2.6	At Bridge Street Ferry.
77,450	77,700	250	3.6	
79,500	80,700	1,200	3.2	
85,050	89,700	4,650	2.3	Just above Lamont.
95,700	96,500	800	2.7	Do.
100,400	103,000	2,600	2.8	
113,900	114,400	500	3.0	Just below Eastmanville.
		27,120		

and Haven to Grandville, about 33 miles, the slope assumed for a 10-foot channel is 0.000007, or 0.44352 inch per mile, or about the mean existing for a river in the lower 27½ miles. From Grandville to Grand Rapids the slope is 0.000011, or 0.69696 inch per mile. For the low-water discharge rate, as obtained at the gauging above Grandville, the former gives a channel 90 feet wide on the bottom and side slopes of 3 to 1 and the latter a channel 90 feet wide on the bottom and side slopes of 2 to 1. From the nature of the material determined by the borings, it is thought that these side slopes will in time be quite permanent. The 90 and 100 foot width of bottom and corresponding width of 130 and 160 feet are thought to be sufficient. To dredge this channel will require the removal of 4,000,000 cubic yards, which at 10 cents per cubic yard will cost \$400,000.

The dredged channel would lower the water at Grandville about 2.1 feet, and at Grand Rapids about 4.6 feet at low-water stages, this effect decreasing with higher stages in the stream.

At Grand Rapids a basin large enough to accommodate boats in passing and turning is required by the excavation of about 70,000 cubic yards of material, which excavation would cost \$14,000.

At Eastmanville a similar basin can be obtained by the excavation of about 70,000 cubic yards at a cost of \$8,000.

of the proposed dam estimated for is formed of two rows of piles and about four feet apart, between which the earth is excavated and the in with concrete. Below the core two rows of piles are placed, one at the downstream slope of the over fall and the other at the lower edge of the between the piles of this lower row and a similar row above the core wall is driven. Estimates are made for gravel and stone filling with a suit- ing of planking and rock filling below the apron.

Estimated cost of dam.

Material.	Unit.	Price.	Quantity.	Cost.	Total.
One.....		\$6.25	160	\$1,000.00	
do.....		4.00	240	960.00	
M. B. M.....		40.00	92,160	3,686.40	
do.....		40.00	62,400	2,496.00	
do.....		60.00	80,000	4,800.00	
do.....	Cubic yard	4.50	680	3,060.00	
stone filling.....	do	.75	1,616	1,212.00	
below dam.....	do	1.75	1,112	1,946.00	
					\$19,160.40
ation.....	do	.50	364	182.00	182.00
paving bank at end of dam.....	Cubic yard.....	3.00	278	834.00	834.00
					20,176.40

hat similar dam, but with only one central row of piles and sheet piling, t the concrete coré, will cost \$15,200.

ESTIMATE FOR 8-FOOT CHANNEL BY DREDGING.

s are made for an 8-foot channel by dredging. From Grand Haven to of Sand Creek (27.26 miles) the slope taken is .000007 (0.44352 inches per ee to Grandville (5.88 miles) it is .000019 (1.20384 inches per mile). One at at the base and side slopes of 3 to 1 is the section assumed for the above From Grandville to Grand Rapids the slope is .000026 (1.64736 inches per the section has a 90-foot base, with side slopes of 2 to 1. The sur- water at Grand Rapids would be lowered about 1.8 feet. The removal of cubic yards of material would be required, costing, at 10 cents per yard, A winding basin at the upper end would cost \$8,000.

WING DAMS AND TRAINING DIKES.

ining the inclination and the dimension of the section of a channel that rge the low-water flow of the river, the assumption of course is that the rity of water discharges through that section. A dredged channel of the ize selected would not be more than 130 to 160 feet wide at the surface. t width of the river varies from about 350 to about 600 feet at low water. herefore to confine the flow to a sufficiently narrow channel and at the to prevent the dredged material from finding its way back into it, wing aining dikes, or both, may be required at various places to maintain a requisite depth. Such works would especially be needed at crossings and ide and straight portions of river where shoaling occurs. In the whole he river there are about twenty crossings. The lower seven of these, being 2 to 3 feet of dredging will be required to open a 10-foot channel and in e river where there is little current even at high water, will probably re- rticular works, as it will be less expensive and more satisfactory to re- dredging from time to time any slight shoaling that may occur at these

the upper 5 miles of river, where five crossings occur and where the project a 10-foot channel contemplates the lowering of the bed of the stream at from 2 to 4 feet, and where, too, the material to be dredged is com- sly of clay, coarse gravel, cobblestone, and boulders, it is thought that a osition of the dredgings, supplemented by the encouragement of a willow growth on the dredge dumps and portions of the river bed which for low be out of water, may secure a practically permanent channel without the ms or dikes. At and in the vicinity of the remaining eight crossings, istributed over a distance of about 18 miles of river and where the large fraction of this distance is largely sand, works more or less

ikes or to contract the channel width, and that some of them are from 2
 above low water, and have young willows growing upon them, strongly
 that a permanent channel when once formed can be maintained with small

arison of soundings taken in 1884 previous to dredging, and again in 1889,
 s after dredging had been done, indicate that, except at crossings, the fill-
 he excavated channel has not been great.

ated first cost of improvement with lock and dam being somewhat greater
 of the open channel, the inconvenience to navigation of a lock, added to
 l cost of maintaining and operating it, make the open channel preferable
 more economical. There is also the following consideration in favor of the
 ot channel: The lowering of the river bed 4.5 feet at Grand Rapids, as
 onemplates, would make a water power there which might be utilized in in-
 he already large manufacturing interests of that city. Taking the minimum

discharge of the river, viz, 980 cubic feet per second=61,152 pounds per
 d 4.5 feet head, we have a theoretical horse power = $\frac{61,152 \times 4.5}{550} = 500.3$,

h an efficiency of 80 per cent, gives 400.2 as the actual horse power at-

g 5 pounds of coal to the horse power per hour, if this power be used 24
 ay for three hundred and ten days in the year, the quantity of coal to give
 power will be 7,440 tons, worth, at \$2.25 per ton, \$16,740, yearly. If the
 used only 10 hours a day for 310 days in the year, it will be equivalent to
 of coal, worth \$6,975 yearly. For a large part of the year the available
 uld be considerably larger, but even this amount, if capitalized at 5 per
 ld justify the expenditure of \$334,800, or \$139,500 for the open channel in
 the cost of a channel with lock and dam.

owing gentlemen assisted in the field work: E. C. Dunbar, assistant engi-
 bert L. Sackett, E. L. Allor, F. A. Sager, H. Baldwin, and H. B. Beecher.
 Joddard, assistant engineer, and R. Stierle assisted in part of the office

y respectfully, your obedient servant,

FRED MORLEY,
Assistant Engineer.

WILLIAM LUDLOW,
Chief of Engineers, U. S. A.

Statement of the jobbing business of Grand Rapids for 1891.

Nature of business.	No.	Capital employed.	Amount of business of 1891.	No. of employes.
and rubbers	4	\$200,000	\$325,000	56
stationery	1	100,000	300,000	49
	2	150,000	275,000	168
produce	7	118,000	1,000,000	174
	2	165,000	345,000	56
	4	800,000	1,400,000	295
	2	165,000	500,000	27
	6	900,000	5,000,000	143
	2	500,000	908,000	117
and furs	2	25,000	60,000	12
and wool	4	525,000	800,000	22
	1	10,000	25,000	5
ment	4	50,000	200,000	16
	4	200,000	500,000	31
	1	35,000	150,000	6
	2	73,000	375,000	41
	2	92,000	350,000	23
oils, etc	3	200,000	500,000	31
supplies	1	10,000	25,000	6
edders' supplies	3	10,000	50,000	24
rdware	2	100,000	350,000	29
	2	25,000	150,000	23
	3	10,000	25,000	11
	67	4,463,000	13,613,000	1,365

THE UNIVERSITY OF CHINA

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1912

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1912



ent of the manufacturing industries of Grand Rapids for 1891—Continued.

Nature of business.	No.	Capital employed.	Amount of business of 1891.	Number of employes.
.....	1	\$9,000	\$11,000	13
.....	1	8,000	35,000	7
st.....	2	350,000	385,000	54
fluid.....	2	25,000	50,000	7
.....	4	13,000	76,000	27
.....	3	30,000	52,000	35
.....	1	4,000	12,300	6
.....	1	65,000	215,000	60
and truck.....	1	10,000	25,000	15
.....	1	6,000	25,000	10
tract.....	2	55,000	75,000	15
.....	1	25,000	75,000	10
ad feed mill.....	10	950,000	2,200,000	152
.....	46	6,160,300	10,010,000	6,613
lamp.....	1	2,500	8,500	4
wood trimmings.....	8	176,000	238,500	106
iron, etc.....	25	72,400	357,750	152
.....	1	600,000	175,000	41
.....	2	2,000	4,800	7
.....	1	25,000	25,000	10
.....	1	10,000	25,000	20
.....	14	102,150	342,150	116
.....	1	1,000	6,000	4
ornament.....	1	2,000	5,000	4
.....	1	10,000	10,000	14
.....	16	778,000	1,116,200	507
granite.....	8	50,100	141,000	91
rifle.....	1	25,000	65,000	13
aper.....	1	3,000	5
.....	7	34,500	172,200	29
icine.....	1	15,000	25,000	25
.....	2	2,500	6,200	27
l, sash, door, etc.....	27	2,807,000	4,950,000	1,554
l.....	5	450,000	215,000	141
rust.....	1	5,000	21,000	7
.....	1	2,000	8,000	4
.....	2	8,000	27,000	23
.....	1	5,000	10,000	7
.....	1	5,000	18,000	12
.....	1	10,000	15,000	9
and windows.....	3	2,500	24,500	16
.....	1	1,500	2,700	4
.....	5	25,400	233,000	127
n fastener.....	1	25,000	25,000	15
.....	2	3,000	14,500	17
headings.....	2	70,500	292,000	76
cell.....	3	12,200	22,000	9
.....	5	35,000	146,000	40
.....	2	345,000	470,000	102
.....	2	18,400	53,000	22
.....	1	2,000	6,500	2
.....	1	6,000	18,000	9
.....	1	100,000	200,000	100
ng compound.....	1	225,000	575,000	174
estern brick.....	2	2,300	11,600	12
ow.....	1	17,000	51,000	32
d rattan (not furniture).....	1	7,000	21,000	15
ade.....	1	200	1,000	2
.....	1	15,000	40,000	12
.....	2	4,200	13,500	9
ing.....	8	3,000	32,000	16
.....	3	3,450	27,000	17
are (not enumerated).....	17	78,000	172,000	61
.....	1	10,000	15,000	12
ig.....	1	2,000	5,000	4
.....	498	18,238,000	33,555,900	14,900
ents included by the U. S. Census Bureau in 1890, chedule of manufactures, but which are local in their ill add to this table with a corresponding increase..	465	347,000	1,642,650	2,898
.....	963	18,575,000	35,198,550	17,888

H. D. C. VAN ASMUS,
Secretary Board of Trade, Grand Rapids, Mich.

APPENDIX L L.

IMPROVEMENT OF CERTAIN RIVERS AND HARBORS IN EASTERN MICHIGAN.

REPORT OF COLONEL O. M. POE, CORPS OF ENGINEERS, OFFICER IN CHARGE, FOR THE FISCAL YEAR ENDING JUNE 30, 1892, WITH OTHER DOCUMENTS RELATING TO THE WORKS.

IMPROVEMENTS.

Marys River, Michigan.	11. Black River at Port Huron, Michigan.
Rating and care of St. Marys Falls Canal, Michigan.	12. Mouth of Black River, Michigan.
Stock at St. Marys Falls Canal, Michigan.	13. St. Clair Flats Canal, Michigan.
St. Clair Lake Channel, St. Marys River, Michigan.	14. Operating and care of St. Clair Flats Canal, Michigan.
Work at Cheboygan, Michigan.	15. Clinton River, Michigan.
Work at Thunder Bay, Michigan.	16. Grosse Pointe Channel, Michigan.
Thunder Bay River, Michigan.	17. Rouge River, Michigan.
Work at Au Sable, Michigan.	18. Detroit River, Michigan.
St. Ignace River, Michigan.	19. Removing sunken vessels or craft obstructing or endangering navigation.
Work of refuge at Sand Beach, Lake Huron, Michigan.	

UNITED STATES ENGINEER OFFICE,
Detroit, Mich., July 16, 1892.

SIR: I have the honor to transmit herewith the annual reports relating to the works of river and harbor improvements under my charge, for the fiscal year ending June 30, 1892.

* * * * *

I am, sir, very respectfully, your obedient servant,

O. M. POE,
Colonel, Corps of Engineers,
Bvt. Brig. Gen., U. S. A.

Gen. THOMAS L. CASEY,
Chief of Engineers, U. S. A.

L L 1.

IMPROVEMENT OF ST. MARYS RIVER, MICHIGAN.

The project for obtaining a navigable channel of 16 feet depth between Lakes Superior and Huron had been barely completed when the demands of commerce so enormously increased that the work of obtaining a depth of 20 feet throughout was undertaken, with the full sanction of both legislative and executive authority.

A necessary part of the project is the construction of a new lock upon the site of the old State locks, to have a length of 800 feet between gates, a width of 100 feet throughout, a depth of 21 feet on the miter sills, and a single lift approximating 18 feet. The canal is to be deepened to correspond. The estimated cost of this enlargement of the canal system is \$4,738,865, for the details of which see page 2220 of vol. of the Annual Report of the Chief of Engineers for 1887.

Prior to June 30, 1890, a total of \$2,500,000 had been appropriated for the work. The river and harbor act of September 19, 1890, appropriated an additional \$900,000 for continuing the improvement with the provision "that such contracts as may be desirable may be entered into by the Secretary of War for materials and labor for the entire structure and approaches, or any part of the same, to be paid for as appropriations may from time to time be made by law;" and the sundry civil act of March 3, 1891, appropriated \$600,000 for continuing the improvement during the fiscal year ending June 30, 1892. The total amount appropriated up to date, therefore, is \$2,750,000, with authority to contract for all or any part of the work.

For the time being the work is confined to the improvement of St. Marys Falls Canal and its approaches. On June 30, 1891, its condition was as follows, viz: The cofferdam surrounding the site of the 800-foot lock was in good condition; the excavation of the lock pit had been carried below grade, except a small area at the eastern incline, and the contractors were engaged in removing a considerable amount of loose and broken stone below grade; and a considerable amount of filling yet remained to be done behind Fort Brady Pier.

The contractors for the masonry of the lock walls had a considerable portion of their plant in readiness for use; had begun quarrying face stone at Kelly Island, Lake Erie, and backing stone at Drummond Island, Michigan; were receiving cement and had begun laying concrete in the lock foundations, but no estimate had yet been made for payment therefor.

The following contracts were in force during the year, viz:

Contractor.	For—	Entered into.	Remarks.
Collins & Farwell	Excavating	Mar. 1, 1889	In force.
H. D. Edwards & Co.	Ship chandlery	Mar. 7, 1891	Closed October, 1891.
R. G. Ferguson & Co.	Zinc and galvanized iron	June 15, 1891	Closed July, 1891.
Hughes Bros. & Bangs	Building masonry	Feb. 9, 1891	In force.
George Kemp	Coal	Mar. 7, 1891	Closed December, 1891.
Jno. H. Killmaster & Co.	Timber and plank	May 1, 1891	Closed September, 1891.
King Iron Bridge & Mfg Co.	Gate anchorages	Oct. 29, 1891	Closed June, 1892.
Chas. Hebard & Son	Lumber and shingles	Mar. 7, 1891	Closed January, 1892.
P. M. Church & Co.	Ship chandlery	Mar. 25, 1891	In force.
James R. Ryan	Delivering clay	Nov. 28, 1891	Closed February, 1892.
John P. McGuire	Valve frames and valves	Mar. 10, 1891	In force.
Richards & Co., limited	Chemicals	Mar. 25, 1891	Closed May, 1892.
R. G. Ferguson & Co.	Hardware	do	Do.
James Strachan	Machine work	do	In force.
Chas. Hebard & Son	Lumber and shingles	do	Do.
George Kemp	Coal	do	Do.

MADE DURING THE FISCAL YEAR ENDING JUNE 30, 1892.

the Collins & Farwell contract but little was done. The con- topped the work of excavation on July 8, 1891, and made no ither to make good with concrete the excessive excavation, or te the filling behind Fort Brady, both of which were required ms of their contract.

the contract of Hughes Brothers & Bangs work on the ma- the lock walls has been pushed with vigor. They have a fine use at the canal and at the Drummond Island Quarries. A n of it is given in detail in the appended report of Assistant E. S. Wheeler. Although the information is not at hand for ration of a similar description of their plant at the Kelly's arries, yet I know, from personal inspection, that it is quite g with the rest of it.

the fiscal year the following quantities represent the work

	Cubic yards.
pit.....	3,161
up well.....	2,603
.....	5,764
ndations of lock walls.....	7,970
n backing and rock face of excavation.....	244
f pump well.....	264
.....	8,478
aid.....	2,242
one laid.....	7,305

backing stone laid the contractors furnished 6,591 cubic yards Government furnished 714 cubic yards, the latter being stone in the walls of the old State locks (of 1855).

	Pieces.
ad stone in "the rough"—	
ed July, August, September, and October, 1891.....	1,912
ed April, May, and June, 1892.....	3,033
received during fiscal year.....	4,945

ere carried in thirty-seven cargoes.
 ggest stone contained 183 cubic feet, and weighed about 14½ ter being cut its volume was 105.26 cubic feet and the weight ed to about 8½ tons.

	pieces..	cubic yards..
id stone "cut"—		
fiscal year.....	2,414	
ning.....		3,229½

horages—all have been delivered at the canal, the weight being about

	Barrels.
eived and used—	
id.....	2,159
l.....	19,200
l.....	21,358

ditional cement house was built.

PRESENT CONDITION OF THE WORK.

am.—The process of "stock-ramming" the clay wall of the was continued throughout the year with great confidence in its The operation is described in detail in the appended report ant Engineer E. S. Wheeler. The party engaged upon this



cofferdam, excluding, however, the site occupied by the mov-
which it is not intended to disturb at present.

WATER LEVELS.

Water gauge readings above and below the locks were continued
year, and the annual means for the calendar year 1891 and
ly means for the various months in the fiscal year have been
and added to the water-level tables appended to my annual
1890. In order that these published tables may be kept up
the various monthly and annual means are submitted herewith,
follows:

Elevations above sea level.

Month.	Lake Superior.	St. Marys River.	Lake Huron.
	<i>Feet.</i>	<i>Feet.</i>	<i>Feet.</i>
	601.292	582.780	590.43
	601.304	582.792	590.31
	601.238	582.617	590.14
	601.294	582.516	579.77
	601.185	582.354	579.46
	600.920	582.208	579.40
mean	601.042	582.604	590.03
	600.815	582.518	579.42
	600.457	582.465	579.36
	600.277	582.475	579.42
	600.429	581.794	579.50
	600.940	582.637	579.61
	601.300	582.530	580.16

Difference in elevation.

Month.	Between Lake Superior and Lake Huron.	Between Lake Superior and St. Marys River.	Between St. Marys River and Lake Huron.
	<i>Feet.</i>	<i>Feet.</i>	<i>Feet.</i>
	20.86	18.532	2.33
	20.97	18.512	2.46
	21.10	18.621	2.48
	21.52	18.775	2.75
	21.72	18.831	2.89
	21.52	18.711	2.81
mean	21.01	18.348	2.66
	21.40	18.297	3.10
	21.10	17.992	3.11
	20.86	17.802	3.06
	20.93	18.635	2.29
	21.31	18.903	2.41
	21.14	18.770	2.37

gs showing the water-level curves for lakes Superior and
e former platted from observations taken above the locks at
from January, 1889, to date, the latter from observations
and Beach, Mich., during same time, are also submitted.*

aneous.—The series of characteristic photographs of the work
is has been kept up. The negatives become the property of
nment and are preserved with the records.

rk is under local charge of Assistant Engineer E. S. Wheeler,
ided by Assistant Engineers Joseph Ripley and J. L. Callard,

* Not printed.

part of the work does nothing else, because the dam is so vital to other operations that it demands and receives unremitting attention. About 6 cubic yards of clay are rammed into the dam each working. The dam is now as tight as it ever was, and the leakage through or immediately under it is remarkably small.

Pumping plant.—The water which is now pumped from the lock comes almost entirely through seams in the rock, and from that water in the operations connected with the construction of the lock. The pumping plant in position for the purpose of keeping the lock pit has not been required to operate one-fourth of the time during the year. It is not liable to get out of order, and the greatest danger to be apprehended is from fire which might consume the buildings which house all but the 8-inch piston pump. For greater precaution this has been placed in the pump well of the new lock, at a distance from buildings containing the other pumps.

There are four cement houses, and in excellent condition, with a capacity for the storage of 8,200 barrels of cement at one time.

Masonry.—The first stone was laid in the lock September 2, 1892. The stonecutting for the first five courses is practically completed, with the exception of the piers under the miter wall of the upper lock gate and the stone for the main miter wall. Work on the sixth course is well advanced.

The greater portion of the south face wall of the lock is built up to the top of the fifth course high and the backing completed to correspond. In like manner the greater part of the opposite wall has been carried four courses high. Two courses in the bottom of these walls are laid through nearly their whole length, but portions are not yet backed. The courses are uniformly 2 feet high, except the lower course downstream of the "drop" in the lock floor, which course is 1½ feet thick. Part of the two lower courses of the upper guard gate miter wall have been laid.

Pump well and pumping plant.—The concrete walls of the present well have been carried up to the level of the lock floor and the masonry thereto from the lock chamber is completed. The preparation of plans for the pumping plant has been intrusted to Mr. Julian Kennel of Pittsburg, Pa., and he now has them under consideration.

Lock gates.—The designs for the lock gates are being prepared in this office, and are now well advanced.

Gate anchorages.—The anchorages for the ten gates have been determined at the canal. The anchor plates and the lower series of eyes for the lower guard gates have been set in place. They are in keeping with the massive character of the whole work. The anchorages have a total weight of about 275 tons, costing, at the contract prices, \$20,768.

Valves and valve frames.—These are under construction in accordance with a contract entered into March 10, 1892, with John P. McGuire of Cleveland, Ohio. The contract provides for 14 valves and 12 valve frames, the estimated weight of the whole being nearly 136 tons, will cost, at the contract price, \$26,862.96, subject to a slight variation however, in case any modifications are made during the process of manufacture.

Valve and gate engines.—The designs for these are in course of preparation in this office, and the drawings are well advanced.

Deepening the canal prism.—In accordance with advertisement dated June 2, 1892, proposals will be opened on July 2, 1892, for deepening the canal prism from a cross section about 275 feet west of the west

the cofferdam, excluding, however, the site occupied by the mow, which it is not intended to disturb at present.

WATER LEVELS.

Water gauge readings above and below the locks were continued the year, and the annual means for the calendar year 1891 and monthly means for the various months in the fiscal year have been added and added to the water-level tables appended to my annual report for 1890. In order that these published tables may be kept up, the various monthly and annual means are submitted herewith, as follows:

Elevations above sea level.

Month.	Lake Superior.	St. Marys River.	Lake Huron.
	<i>Feet.</i>	<i>Feet.</i>	<i>Feet.</i>
1891	601.292	582.760	580.43
1891	601.304	582.792	580.23
1891	601.288	582.617	580.14
1891	601.294	582.516	579.77
1891	601.185	582.354	579.46
1891	600.920	582.209	579.40
Annual mean	601.042	582.694	580.03
1892	600.815	582.518	579.42
1892	600.457	582.463	579.36
1892	600.277	582.475	579.42
1892	600.429	581.794	579.50
1892	600.940	582.037	579.63
1892	601.300	582.530	580.16

Difference in elevation.

Month.	Between Lake Superior and Lake Huron.	Between Lake Superior and St. Marys River.	Between St. Marys River and Lake Huron.
	<i>Feet.</i>	<i>Feet.</i>	<i>Feet.</i>
1891	20.86	18.532	2.33
1891	20.97	18.512	2.46
1891	21.10	18.621	2.48
1891	21.52	18.775	2.75
1891	21.72	18.831	2.89
1891	21.52	18.711	2.81
Annual mean	21.01	18.348	2.66
1892	21.40	18.297	3.10
1892	21.10	17.992	3.11
1892	20.86	17.802	3.06
1892	20.93	18.635	2.29
1892	21.31	18.903	2.41
1892	21.14	18.770	2.37

Drawings showing the water-level curves for lakes Superior and Huron, the former plotted from observations taken above the locks at Sand Beach, Mich., from January, 1889, to date, the latter from observations at Sand Beach, Mich., during same time, are also submitted.*

Miscellaneous.—The series of characteristic photographs of the work has been kept up. The negatives become the property of the Government and are preserved with the records.

The work is under local charge of Assistant Engineer E. S. Wheeler, assisted by Assistant Engineers Joseph Ripley and J. L. Callard,

* Not printed.

APPENDIX L L—REPORT OF COLONEL POE.

2403

Appropriations for improving St. Marys River, Michigan.

1886	\$250,000
1, 1888	1,000,000
er 19, 1890	900,000
1891, sundry civil bill	600,000
Total	2,750,000

of bids for furnishing ten gate anchorages for the 800-foot lock at St. Marys Falls Michigan, received and opened on October 19, 1891, in accordance with advertisement dated September 19, 1891.

Name and address of bidder.	Wrought steel, rolled shapes, etc. (210,240 pounds), price per pound.	Cast iron (332,188 pounds), price per pound.	Total.	Remarks.
King Iron Bridge and Manufacturing Co., Cleveland, Ohio.	5 Cents.	3 Cents.	\$20,477.64	Recommended for acceptance.
Walker Manufacturing Co., Cleveland, Ohio.	6 ¹ / ₁₀	2 ¹ / ₂	21,129.34	
Stone Bridge Co., Chicago, Ill.	6 ³ / ₁₀	3	23,336.90	Informal as to signature to bid; not witnessed.
Six Iron Co., Trenton, N. J. . . .	6 ⁷ / ₁₀	2 ⁷ / ₁₀	23,475.64	Informal as to certificate; not signed.
Wheel and Foundry Co., Detroit, Mich.	7 ⁷ / ₁₀	2 ¹ / ₁₀	25,589.40	
Hopkins, St. Louis, Mo.	7 ³ / ₁₀	2 ¹ / ₁₀	26,474.94	
Passaic Rolling Mill Co., Paterson, N. J.	6 ¹ / ₂	4 ¹ / ₂	28,614.05	
Dock Engine Works, Detroit, Mich.	8 ¹ / ₂	3 ¹ / ₂	28,666.51	
ard S. Pope, Detroit, Mich.	10 ¹ / ₂	3 ¹ / ₂	32,345.71	Informal as to modification of clause 15 of specification.

of bids for delivering clay for improving St. Marys Falls Canal, received and opened on November 21, 1891, in accordance with advertisement dated October 23, 1891.

Name and address of bidder.	Strip and dispose of 300 cubic yards, more or less, bank measure (price per cubic yard B. M.).	Deliver 5,000 cubic yards, or more, clay (price per cubic yard).	Approximate total.
	Cents.	Cents.	
R. Ryan, Sault Ste. Marie, Mich.	\$0.94	\$0.47	*\$2,632
R. Whiteman, Sault Ste. Marie, Mich.23	.57	2,919
Sam Howlett, Sault Ste. Marie, Mich.43	.62	3,229
Armstrong, Sault Ste. Marie, Mich.74	.64	3,422
Eye, Sault Ste. Marie, Mich.42	.72	3,726
Small & Wynn, Sault Ste. Marie, Mich.47	.73	3,791
T. & Nelson Eagle, Sault Ste. Marie, Mich.50	.85	4,400
D. Eaina, Sault Ste. Marie, Mich.75	.85	4,475
McC & Murphy, Sault Ste. Marie, Mich.59	.89	4,627
McC & Burgess, Sault Ste. Marie, Mich.50	1.00	5,150

* Recommended for acceptance.

has been used continuously during the year, keeping the lock pit dry at all times and never being used to more than one-fourth its capacity. Three men have been employed, each standing a watch of eight hours per day; 480,000 tons of coal have been used.

PHOTOGRAPHS AND MODELS.

Photographic views showing the progress of the work have been taken each month as work was in progress. Models of the lock of 1881 and the 800-foot lock are being prepared for the Columbian Exposition. The woodwork of these models is advanced.

COFFERDAM.

The cofferdam has remained without any leak of any magnitude during the entire year. No repairs have been necessary except replacing two broken washers on the dam. In November the dam was covered with brush so as to hold the snow to a depth of 3 or 4 feet during the winter, which effectually prevented the freezing of the clay. When the brush was removed in May the clay was found unfrozen.

STOCK RAMMING.

The stock-ramming apparatus has been somewhat improved and perfected since last year. The accompanying plates, Nos. 1 and 2, show it as it is now used. Plate 1 shows the apparatus at work; the 3-inch pipe driven down to the bottom in the clay is shown in section. In the figure it is represented full of cylindrical molds of clay rammed into it; the rammer is withdrawn; the workmen are upon the point of pulling the rammer in the tube and forcing the clay out.

An attempt has been made to show the form that the clay assumes after being driven to the bottom of the pipe, since no part of the dam, which has been stock rammed, has yet been examined.

The pipe is composed of several short sections, which are coupled together in the ordinary manner of pipe coupling; this enables the pipe to be shortened or lengthened so as to deposit the clay at any required depth. The manner of making the cylinders of clay is shown in sketch "A," Plate 2. An ordinary shovel has its blade cut and rolled over to form a ring a little less than 3 inches in diameter and about 3 inches in length; some of these shovels are shown in the sketch. The workman pushes the shovel into the clay and forces the cylinder of clay through the ring; when the cylinder is about 1 foot long and lying on the upper part of the blade it is taken off and placed in a wheelbarrow and is then wheeled to the stock rammer and put in the tube by hand, as shown in the sketches.

The rammer is an iron rod 30 feet long and about 2½ inches in diameter; about 3 feet from the lower end is enlarged to 2½ inches in diameter, so that it will work easily in the 3-inch pipe.

The weight which pushes the rammer down is an ordinary pile-driver hammer and weighs about 1,900 pounds; the derrick which is used is an ordinary pile-driver. A block is so arranged at the top of the derrick that it is slipped under the hammer and holds it when it is up; this block is moved with lines from the ground; a workman in these lines in his hands is shown in Plate 1. The rammer is operated by steam engine for this purpose is shown in the small house in Plate 1. Seven men are required to work this apparatus.

The system which is being used this year is as follows:

The pipe is first pushed down to the bottom of the dam; about one-third of a cubic yard of clay is driven in the pipe; it is then raised about 10 feet and another third is driven in; it is again raised 10 feet and the operation repeated, making 1 yard in a hole. The apparatus is then moved forward 5 feet and the operation repeated. When soft spots are found in the clay intermediate holes are sometimes driven.

This does not lift the clay, crack it, or strain the cribs any appreciable amount. Over the entire length of the dam has been gone over in this way the operation is repeated a second time and even a third time; in this way the clay is constantly compressed and no injurious strains are created at any time. The total amount so rammed during the season is about 1,000 cubic yards.

LOCK-PIT EXCAVATION. CONTRACT DATED MARCH 1, 1889.

The time of completion of this contract was extended from June 1, 1890, to June 1, 1891, and on account of break in cofferdam was again extended to June 1, 1891, and extended a third time to June 15, 1891.

The contractors, Messrs. Collins & Farwell, stopped work on July 3, 1891, with having completed the required excavation, filling, etc., as per terms of contract. A careful survey was made in June, and levels taken every 10 feet over the pit, dumping ground, and the fill back of Fort Brady Pier. The levels were reduced to cross sections plotted, and check computations made for each square of the amount of—

- Rock excavation above grade and within specified side slopes.
 - Rock excavation below grade.
 - Rock excavation outside of side slopes.
 - Earth excavation above grade and within side slopes.
 - Earth excavation outside of side slopes.
 - South wall removed outside of side slopes.
 - Earth remaining between certain cross sections.
 - Filling back of Fort Brady Pier.
 - Earth necessary to complete back of Fort Brady Pier.
- A report was submitted, giving all written orders issued to the contractor, a statement of methods of doing work, and amounts of final estimate, of extra amounts claimed.

RANGE TARGETS, BUOYS, ETC.

... to Mud Lake. All the range targets placed by the California... also, all the channel buoys were located by transit... more triangulation stations. The locations were first plotted... St. Marys River survey of 1879 and then transferred to the...

LOCATION OF WRECKS.

... of *Pontiac*, sunk at head of Little Lake... the Schooner *Helena*, at Park Hole...

STREVEY OF CANAL.

... through the ice from the head of the canal... of the Southwest pier.

CONCLUSION.

... completed. There are 27... 24 sheets... by 72 in. long... from time to time... stones will... have been cut.

DETAILS OF THE MACHINERY.

... 4,000 gallons capacity... The water is pumped... steam from the machine... on... attachment... about... the work... double... are... related in... by the... these are... operated... The other four are of 5

city each and are operated by 9-inch by 12-inch double-drum, double-cylinder steam engines. They are of 16-foot gauge and travel on 5-inch rails laid on trestles the level of the top of the first course of masonry.

Steam derricks.—Four steam derricks are used for unloading stones from the scows and loading them on the cars for shipment into the lock pit. Two derricks at each end of the work, are provided with turntable and steam swing-plate. Each derrick is operated by an 8 $\frac{1}{2}$ -inch by 10-inch double-drum, cylinder Lidgerwood engine. They are all of the same size and make; booms 40 feet long; mast, 68 feet high; two stiff legs, each 66 feet long, supported by A-frames 40 feet long.

Power derricks.—There are now being assembled on the north side of the lock at the east end four stiff-legged derricks, each having a 35-foot mast and boom, and will be operated by single-drum horse-power hoists. These are used for the traveling cranes in laying backing stones on the wide wall. A derrick is placed at the pile of Government backing stone (face stone taken from the old State lock) for the purpose of loading them on the cars for shipment into the lock pit.

Rock-crushing plant.—The rock-crushing plant consists of two Gage rock-crushers, model No. 4. They are operated by two horizontal boilers and engines. The plant is on the north side of the lock pit.

Sand scow.—The sand scow is 100 feet long and 34 feet wide and is provided with a sand pump and a rotary engine.

A derrick is used for unloading the sand near the head of the west incline. It is operated by a small rotary engine.

Sand scows.—The tug *Arthur*, of the Moiles line, is chartered by the season. She is employed in towing sand scow, pumping sand, towing scow laden with cut stone to the stone yard to the place of unloading for shipment into the lock pit, and for necessary work. She also makes a weekly trip to the Drummond Island wharf. Three scows are used in carrying out stone.

Miscellaneous plant.—Besides the above enumerated plant there is a large miscellaneous plant, engine houses, shops, tool houses, duplicates of various parts of machines, harnesses, dump carts, wheelbarrows, tools, etc.

LOCK CONSTRUCTION.

Hughes Bros. & Bangs began grading and laying tracks, assembling machinery, and other preliminary work on May 1, 1891, but the work of actual construction was not begun until June 24, when the first batch of concrete was laid.

The stone used for the concrete was the sandstone from the lock-pit quarry. An inspector was stationed at each crusher to see that good stone was used, leaving the crusher the broken stone passed through a revolving screen having 2 feet in diameter, the openings in the screen being about three-fourths of an inch square, so that not the dust alone, but a considerable portion of the larger pieces of stone were removed. The crushed stones were thoroughly washed when loaded into dump carts to be taken to the mixing boards.

Wash Pins sand is used both for the concrete and for the laying of the masonry; it is washed from the bottom at a short distance from shore, and is a good quality of even-grained, coarse, sharp, and clean.

The concrete for the foundations of the lock walls is laid to a line 3 feet out from the face of the masonry, to insure against crushing the edges. The average thickness of the concrete for the foundations of the lock walls is about 3 feet. The greater thickness of the concrete was laid in the following manner: Three mixing boxes for concrete were placed side by side on the completed concrete, their forward ends abreast with the front edge of the concrete. An area about 15 feet long and the width of the foundation to be laid was then prepared by having all loose and unconsolidated rock removed, the surface washed with a stream of water from a hose, and a thin coating of mortar applied and rubbed in with a broom to infill of all cracks in the rock and a good bond between the concrete and

the concrete was of one part Milwaukee natural cement, one and one-half parts of four parts broken stone. A batch consisted of 5.25 cubic feet of cement, 10 cubic feet of sand, and 21 cubic feet of broken stone. The sand was spread over the bottom of the mixing box and the cement spread evenly on top of the sand and sand were first thoroughly mixed dry and then into a rather stiff mortar. The mortar was spread evenly over the bottom of the mixing box and the stones spread over it. The stones and mortar were then mixed by four shovel-men working in and then casting out again. When carefully done the two casts were sufficient to insure a thorough mixing, but this largely depended upon the skill of the shovel-men. The concrete was then cast direct from the boards into place,

ed in this contract the removal of some soft material and shattered rock in the upper guard-gate miter wall and the west end of the lock pit. Total excavated under this contract up to date, 3,161 cubic yards. This work is completed.

stone quarry.—The backing stone is brought from Drummonds Island. Messrs. Hughes Bros. & Bangs leased the ground from Mrs. Johnson and developed the quarry. It is located about midway between the quarries from which was obtained the backing stone for the old State lock and the lock of 1881. It has a face 700 feet long and 100 feet high, of which 1 foot is earth stripping, 16 feet is of mixed stone, and 83 feet of which can be used, and 13 feet of good stone.

The stone is a compact, bluish gray, crystalline limestone, having a conchoidal fracture and weighs about 170 pounds per cubic feet.

quarry.—Hughes Bros. & Bangs have built a good substantial pile dock 700 feet long and 75 feet wide in 14 feet of water, and an approach to the dock 700 feet long and 30 feet wide.

At each end of the dock are two steam derricks to unload the stone from the cars and load the scows. These derricks are of the same make and size, and are operated by a 20-horse-power engine as those used for unloading stone at the lock.

There are also nine derricks for handling the stone in the quarry. These are all operated by drum horse-power hoists.

A track is laid from the dock and extends the whole length of the quarry, with necessary side tracks, crossings, and switches. There are also all tracks for disposing of earth strippings and refuse stone. There are in use 15 reform cars of 10 tons capacity each and thirty dump cars. Five barges are required to transport the stone from the quarry to the lock. There are also built five boarding camps, one hospital, one ice house, and one black-pit.

storage.—The gate anchorages were furnished by the King Iron Bridge and Manufacturing Company, Cleveland, Ohio, and consists of 40 anchor castings, 112 anchor bolts, 240 eyebars, 40 eyebolts, 160 pins, 4 wrenches, and 1 box of washers. The anchor castings for the lower guard gates, together with the two eyebolts to each, have been placed in position in the lock walls.

CUT STONE.

Stone-cutting has all been done on the canal lands. The stone used was transported from Kelly Island in the rough, the first cargo arriving in the schooner *Fred* on July 17. The area occupied by contractors for stone yard was a strip of 1,100 feet long and 150 feet wide, lying just back of the Fort Brady Pier. It used consists of two stiff-leg derricks of 15 tons capacity each, one McElroy crane, four stone planers, manufactured by the Rutland Iron Works, Vermont; a 100 H. P. automatic cut-off engine, and boiler for driving the same. The derricks are placed at each end of the yard, 18 feet from the face of the pier and have booms about 50 feet long. The booms can make a complete revolution and are raised and lowered by steam. Three-fourths inch wire cable, spring swinging gear are used on each. The track for the traveling crane is about 100 feet on the face of the pier and 1,000 feet long. It runs between the two stiff-leg derricks. A 5-inch rail and 16-foot gauge are used. The McMyler crane can lift 10 tons per minute, although its usual speed is about 400 feet. Its reach is 100 feet in the center line of track, at which point it is able to raise 4 tons, which is the weight of the ordinary header or stretcher in the rough. It is supposed to lift a load of 10 tons when situated not more than 30 feet from the center of the track. The machinery for planing stone is situated in the center of the yard.

The four planers are completed and two of them in operation, but the setting, and minor fittings will not be completed before July 7. At the present time the entire plant will be ready for operation by that date.

The construction of these machines is similar to that of an iron planer. Work on the stone yard was begun April 1. The first machine was completed May 11. The second was completed June 6, and the third and fourth June 30. The yard is lighted by electricity, and the crane and planers being worked night and day. The capacity of the stone yard cannot be determined at this date, as they are not yet fully equipped. All the machinery is suitably housed. The office, pattern room, storeroom, and workshop are in one building, situated near the center of the yard. The engine house in the yard has been erected during the fiscal year.

The contractors began cutting July 30 with a force of 25 cutters, and continued until September 30, with the exception of a delay from the 9th to 12th of September, when the work was practically suspended for want of stone, one of the vessels having been damaged. The number of cutters employed did not exceed 70 at any time, the work being suspended on August 27. The work of cutting was resumed April 5 with a force of 31 cut-

with a
 of 45
 was
 one ft
 assist
 and

Date				Date
1891				1892
August				
September				
October				
November				
Total				

Total number of pieces cut
 Total number of cubic yards cut

The total number of pieces registered will be about 172 different patterns. Patterns of sheet zinc one inch thick are used for all stones that are cut in form. The cut stone was sent to lock pit August 20. About 400 pieces of cut stone were cut during the winter.

They were piled four deep and three abreast. The space between was calked with hay, the lewis holes being covered with of that thickness. The lewis holes in the top layer were filled covered with a thin layer of cement. The top of the pile had a manure, the sides were banked lightly with same, while the north sides were protected by boards.

The following table shows the amount of stone in the rough rock

Date	Number of loads	Number of pieces	Date
1892			1892
	2	286	April
	3	377	May
	4	436	June
	5	510	
	14	1,902	Total

contained in the rough 183 cubic feet, weighing 16,503 pounds. The results were computed in order to

of various
 results
 of various
 results

to compare them with that for 800-foot lock. The following table shows the results of these tests:

Classification.	Number of samples tested.	Weight per cubic foot.
		<i>Pounds.</i>
800-foot lock, from Kelly Island.....	38	157.36
800-foot lock, from Drummonds Island.....	2	170.31
800-foot lock, of 1881. Samples taken from the pile of rejected stone.....	3	154.79
800-foot lock, old State lock.....	3	153.96
800-foot lock, from excavation for 800-foot lock.....	3	157.50
800-foot lock, at Sault Ste. Marie, Ontario, a limestone from Amherstburg.....	3	147.71
800-foot lock, at Sault Ste. Marie, Ontario, a limestone from Manitoulin.....	2	175.50

from old State lock one sample was very light, weighing only 143.12 pounds per cubic foot. Rejecting this the weight of the other samples would average 157.50 pounds per cubic foot.

The samples of the Potsdam sandstone from lock pit were close grained and hard. The sample sent from the Watertown Arsenal shows the ability of the face of the 800-foot lock to resist compression to be as follows:

No.	Marks.	Dimensions.			Sectional area. <i>Sq. in.</i>	First crack. <i>Pounds.</i>	Ultimate strength.	
		Height.	Compressed surface.				Total. <i>Pounds.</i>	Per square inch. <i>Pounds.</i>
			<i>Inches.</i>	<i>Inches.</i>				
0		2.98	2.98	2.98	8.88	105,800	11,910	
1		2.99	2.99	2.99	8.94	115,280	12,890	
3		2.98	2.99	2.98	8.88	112,490	12,670	
4		2.95	3.00	2.98	8.94	58,000	8,290	
6		3.00	2.97	2.97	8.82	89,000	10,980	
17		2.98	2.98	2.98	8.88	111,260	12,530	
22		2.97	2.96	2.99	8.85	111,590	12,610	
27		3.00	2.98	2.98	8.88	117,200	13,200	
30		2.98	2.98	2.99	8.91	128,200	14,510	

ultimate strength 12,177 pounds per square inch. The compressed surface is finished with plaster of Paris.

The masonry for the first five courses of masonry is practically completed. The stone for the piers and main miter wall. A few stones for the piers have been set for the main miter wall. Work on the sixth course is well advanced.

The masons leave a very smooth surface, and all work coming from them in contact with cement is afterwards bush hammered in order to insure a rough surface.

Mr. L. Fleming has prepared the following estimate of the cost of the construction to the close of the season of 1891:

Name of plant.	Cost.
Lock plant.....	\$3,715.44
Track.....	602.50
Line houses.....	8,723.24
.....	403.39
.....	2,311.57
.....	11,655.02
derricks.....	202.62
.....	2,159.01
.....	3,939.50
.....	26,000.00
.....	12,354.80
.....	72,066.89

FINENESS.

culty has been experienced in obtaining sieves suitable for use in sifting sand. Five wire cloth sieves were first received marked Nos. 20, 30, 50, 80, and 100. Upon counting, the meshes per linear inch each way were found to be as follows:

TABLE 1.

Number of sieve.	Meshes per linear inch.	
	Across web wires.	Across wool wires.
.....	20	19½
.....	30	28½
.....	50	37½
.....	80	67
.....	100	89

These meshes should be more nearly accurate to count were asked for to repeat four numbers. The manufacturer said no complaint had ever been received, but sent four sieves of the same make whose meshes counted as given below:

TABLE 2.

Number of sieve.	Meshes per linear inch.	
	Across web wires.	Across wool wires.
.....	30	28½
.....	50	34½
.....	80	67
.....	100	90

No. 100 was a trifle better as to count, but the meshes appeared to be more irregular than those of the first No. 100. The second lot received the last three were of a different style. It was explained that they had been procured of a "party in New York who claimed to have had the sieves especially for such work." Table 3 gives the results for this lot.

TABLE 3.

Number of sieve.	Meshes per linear inch.	
	Across web wires.	Across wool wires.
.....	30	28½
.....	50	47
.....	80	80
.....	101	88½

The diameters of the wire in some of the sieves were measured with a microscope and a micrometer attachment. Results are given in Table 4.



...the wool and ... be de- ...
... was tested by ...
... mesh was fou- ...
... and in the ...
... It should ...
... become

If we assume that the No. 28's are of table 2
across the wool wires, to a 4 1/2 type having
stead of being .012 in. square, will be .012 in.
as wide as long. Then the number of meshes, p
thus seen to be of greater frequency than would

The committee of the American Society of Civ-
-Uniform system for tests of wool, recommen-
-7, and 10, 11, 12, 13, 14, 15, 16, 17, 18, 19, 20, 21, 22, 23, 24, 25, 26, 27, 28, 29, 30, 31, 32, 33, 34, 35, 36, 37, 38, 39, 40, 41, 42, 43, 44, 45, 46, 47, 48, 49, 50, 51, 52, 53, 54, 55, 56, 57, 58, 59, 60, 61, 62, 63, 64, 65, 66, 67, 68, 69, 70, 71, 72, 73, 74, 75, 76, 77, 78, 79, 80, 81, 82, 83, 84, 85, 86, 87, 88, 89, 90, 91, 92, 93, 94, 95, 96, 97, 98, 99, 100, 101, 102, 103, 104, 105, 106, 107, 108, 109, 110, 111, 112, 113, 114, 115, 116, 117, 118, 119, 120, 121, 122, 123, 124, 125, 126, 127, 128, 129, 130, 131, 132, 133, 134, 135, 136, 137, 138, 139, 140, 141, 142, 143, 144, 145, 146, 147, 148, 149, 150, 151, 152, 153, 154, 155, 156, 157, 158, 159, 160, 161, 162, 163, 164, 165, 166, 167, 168, 169, 170, 171, 172, 173, 174, 175, 176, 177, 178, 179, 180, 181, 182, 183, 184, 185, 186, 187, 188, 189, 190, 191, 192, 193, 194, 195, 196, 197, 198, 199, 200, 201, 202, 203, 204, 205, 206, 207, 208, 209, 210, 211, 212, 213, 214, 215, 216, 217, 218, 219, 220, 221, 222, 223, 224, 225, 226, 227, 228, 229, 230, 231, 232, 233, 234, 235, 236, 237, 238, 239, 240, 241, 242, 243, 244, 245, 246, 247, 248, 249, 250, 251, 252, 253, 254, 255, 256, 257, 258, 259, 260, 261, 262, 263, 264, 265, 266, 267, 268, 269, 270, 271, 272, 273, 274, 275, 276, 277, 278, 279, 280, 281, 282, 283, 284, 285, 286, 287, 288, 289, 290, 291, 292, 293, 294, 295, 296, 297, 298, 299, 300, 301, 302, 303, 304, 305, 306, 307, 308, 309, 310, 311, 312, 313, 314, 315, 316, 317, 318, 319, 320, 321, 322, 323, 324, 325, 326, 327, 328, 329, 330, 331, 332, 333, 334, 335, 336, 337, 338, 339, 340, 341, 342, 343, 344, 345, 346, 347, 348, 349, 350, 351, 352, 353, 354, 355, 356, 357, 358, 359, 360, 361, 362, 363, 364, 365, 366, 367, 368, 369, 370, 371, 372, 373, 374, 375, 376, 377, 378, 379, 380, 381, 382, 383, 384, 385, 386, 387, 388, 389, 390, 391, 392, 393, 394, 395, 396, 397, 398, 399, 400, 401, 402, 403, 404, 405, 406, 407, 408, 409, 410, 411, 412, 413, 414, 415, 416, 417, 418, 419, 420, 421, 422, 423, 424, 425, 426, 427, 428, 429, 430, 431, 432, 433, 434, 435, 436, 437, 438, 439, 440, 441, 442, 443, 444, 445, 446, 447, 448, 449, 450, 451, 452, 453, 454, 455, 456, 457, 458, 459, 460, 461, 462, 463, 464, 465, 466, 467, 468, 469, 470, 471, 472, 473, 474, 475, 476, 477, 478, 479, 480, 481, 482, 483, 484, 485, 486, 487, 488, 489, 490, 491, 492, 493, 494, 495, 496, 497, 498, 499, 500, 501, 502, 503, 504, 505, 506, 507, 508, 509, 510, 511, 512, 513, 514, 515, 516, 517, 518, 519, 520, 521, 522, 523, 524, 525, 526, 527, 528, 529, 530, 531, 532, 533, 534, 535, 536, 537, 538, 539, 540, 541, 542, 543, 544, 545, 546, 547, 548, 549, 550, 551, 552, 553, 554, 555, 556, 557, 558, 559, 560, 561, 562, 563, 564, 565, 566, 567, 568, 569, 570, 571, 572, 573, 574, 575, 576, 577, 578, 579, 580, 581, 582, 583, 584, 585, 586, 587, 588, 589, 590, 591, 592, 593, 594, 595, 596, 597, 598, 599, 600, 601, 602, 603, 604, 605, 606, 607, 608, 609, 610, 611, 612, 613, 614, 615, 616, 617, 618, 619, 620, 621, 622, 623, 624, 625, 626, 627, 628, 629, 630, 631, 632, 633, 634, 635, 636, 637, 638, 639, 640, 641, 642, 643, 644, 645, 646, 647, 648, 649, 650, 651, 652, 653, 654, 655, 656, 657, 658, 659, 660, 661, 662, 663, 664, 665, 666, 667, 668, 669, 670, 671, 672, 673, 674, 675, 676, 677, 678, 679, 680, 681, 682, 683, 684, 685, 686, 687, 688, 689, 690, 691, 692, 693, 694, 695, 696, 697, 698, 699, 700, 701, 702, 703, 704, 705, 706, 707, 708, 709, 710, 711, 712, 713, 714, 715, 716, 717, 718, 719, 720, 721, 722, 723, 724, 725, 726, 727, 728, 729, 730, 731, 732, 733, 734, 735, 736, 737, 738, 739, 740, 741, 742, 743, 744, 745, 746, 747, 748, 749, 750, 751, 752, 753, 754, 755, 756, 757, 758, 759, 760, 761, 762, 763, 764, 765, 766, 767, 768, 769, 770, 771, 772, 773, 774, 775, 776, 777, 778, 779, 780, 781, 782, 783, 784, 785, 786, 787, 788, 789, 790, 791, 792, 793, 794, 795, 796, 797, 798, 799, 800, 801, 802, 803, 804, 805, 806, 807, 808, 809, 810, 811, 812, 813, 814, 815, 816, 817, 818, 819, 820, 821, 822, 823, 824, 825, 826, 827, 828, 829, 830, 831, 832, 833, 834, 835, 836, 837, 838, 839, 840, 841, 842, 843, 844, 845, 846, 847, 848, 849, 850, 851, 852, 853, 854, 855, 856, 857, 858, 859, 860, 861, 862, 863, 864, 865, 866, 867, 868, 869, 870, 871, 872, 873, 874, 875, 876, 877, 878, 879, 880, 881, 882, 883, 884, 885, 886, 887, 888, 889, 890, 891, 892, 893, 894, 895, 896, 897, 898, 899, 900, 901, 902, 903, 904, 905, 906, 907, 908, 909, 910, 911, 912, 913, 914, 915, 916, 917, 918, 919, 920, 921, 922, 923, 924, 925, 926, 927, 928, 929, 930, 931, 932, 933, 934, 935, 936, 937, 938, 939, 940, 941, 942, 943, 944, 945, 946, 947, 948, 949, 950, 951, 952, 953, 954, 955, 956, 957, 958, 959, 960, 961, 962, 963, 964, 965, 966, 967, 968, 969, 970, 971, 972, 973, 974, 975, 976, 977, 978, 979, 980, 981, 982, 983, 984, 985, 986, 987, 988, 989, 990, 991, 992, 993, 994, 995, 996, 997, 998, 999, 1000

... the number of meshes, p
... thus seen to be of greater frequency than would

... the number of meshes, p
... thus seen to be of greater frequency than would

of this labyrinth in indetermination it is impossible to know, except by the size of holes in sieves that have been used and are being used. An experimenter cannot measure and determine the dimensions of the holes in his own sieve, nor certainly know within a large percentage the sizes of the holes in sieves used by other experimenters with whom he may wish to compare.

It seems out of place, then, to endeavor to formulate what is desirable in a sieve, and to inquire how it may best be obtained. It is thought that the following specifications define an ideal sieve: (1) holes of uniform size and shape, (2) sides of the holes very smooth, (3) space between holes of such size that particles will not easily rest there.

Sieves that deviate from the ideal and still be a good one. As regards uniformity, the character of the sieve is determined by the size of the holes. For example, a sieve with half its holes 0.01 inch in size and the other half 0.02 inch in size, would, if used to separate the cement exactly the same as it would if all the holes were 0.02 inch in size. If even a very small percentage of the holes are larger than normal, it seriously impairs the accuracy of the sieve by introducing an inaccuracy. On the other hand, holes smaller than the normal have no greater effect on the sifting process than do larger holes, and as such do not fulfill the third specification. Therefore, while smaller holes may be allowed to the extent of, say, 10 per cent, larger holes are not admissible, and the sieve still be good and accurate. Concerning the second specification, the sides of the holes must be so smooth as not to allow particles of cement to lodge in the hole. The third requirement is for convenience, but might be changed.

A metal plate has been suggested as a substitute for wire cloth, and is discussed in Engineering News for September 26, 1891. An objection is raised in the shape of the hole. I do not see the validity of this objection. A sieve of quartz, recommended by the American Society of Civil Engineers' and which is coming into use for sand tests in this country, has practically round grains, and it is not probable that the shape of the grains passing round holes will be materially different from those passing square holes. If it is argued that it will be difficult in adjusting the size of openings to correspond with the holes in current use, it may be returned that sieves now in use purport to be of the same size of mesh differ so widely that they are hardly worth serious consideration in any changes which may seem desirable. A punched metal plate could be made to fulfill the first specification almost perfectly, and would be less likely to be damaged than a wire sieve, but the former might not so fully comply with the second and third requirements. On the other hand, a wire-cloth sieve readily made to fulfill the first specification almost perfectly, and would be less likely to be damaged than a wire sieve, but the former might not so fully comply with the second and third requirements.

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The question of sieves might be put upon a better basis if the following specifications were done, viz:

Manufacturers of wire cloth should be encouraged to make cloth which will conform to the foregoing specifications; and
 Users should be urged to designate their sieves by the size and shape of the holes.

TIME OF SETTING.

When these wires are used for this test, the apparatus being as simple as it can be, the neat cement is gauged with the same percentage of water for this test as for making briquets. Care is taken that the water and cement and the room in which the test is made shall vary only slightly, if at all, from 65° F. The effect of changes in temperature on the setting of cement is marked.

CHECKING, ETC.

Tests that are made for determining the time of setting are immersed, when they are made, in water, and tend to expand. This test is of the highest importance, as it is better to use a cement that is sound, even if it is not very strong, than to use one that will ultimately "blow." Immersed in water of the same temperature, a pat may not show signs of unsoundness until it has been immersed in water for some time. It is therefore desirable that some method be used which will give certain and quick results.

MOLDS.

the form recommended by General Gilmore's committee of the American Civil Engineers, and now in general use in the United States. Fifty molds fasten with an iron clamp and the other two halves are completely dry are hinge molds with a fastening at one end. The latter are neater and occupy less space on the slab when in use, but are more difficult to use is a matter of some moment when making a large number of briquets.

MOLDING.

The practice to put about one-sixth of the mortar into the mold at a time and to pat it in firmly with the fingers, finishing the top with a small pointing trowel. The method I have had this method compared with the method specified by the committee, which is to use the trowel for pressing the mortar into the mold. Two neat natural cement briquets were made from the same mixing, and the results of one hundred briquets at seven days show no difference in the two methods. The briquets are covered with a damp cloth when made and are in general removed from the molds after twenty-four hours. Natural briquets may generally be removed the same day, but slow setting briquets are left in the molds over night, the cloth being well dampened. From the molds the briquets are placed on metal covered as before and are ready for use twenty-four hours after molding.

TEMPERATURES.

The temperature of the room is kept as nearly as may be at 65° to 70° F. For this purpose a watchman was employed all winter. Hourly readings of the thermometer were recorded during the night and a reading recorded for each set of briquets during the day. Four readings a day were also made of the temperature of the air where some special test briquets are exposed. The temperature of the water used in gauging was also at 65° F. except in some special cases where a very quick setting cement it was found necessary to use colder water. The briquets to be moulded before signs of setting appeared. The briquets were immersed in water seven feet high in the test room, but it was found necessary to immerse in temperature between the top and the bottom boxes was enough to make the use of the bottom three was discontinued. All the boxes are on the same level and have a nearly uniform temperature.

TESTING MACHINE AND CLIPS.

The briquets are broken on a Riehle 1,000-pound machine. It has worked well, the only objection being that it requires considerable space. A pair of pulleys with a rope thrown in and out of gear by the feet facilitates working considerably. Briquets, having an average strength of 100 pounds, can be broken without exceeding the rate of applying strain of 400 pounds, per minute. The clips used are the Riehle style, furnished with the machine, and are shown in Fig. 6 and are often broken at the gripping points of the clips. It is not known exactly when a clip break is due. It may be, and probably is, due to the briquet breaking to compression at that point, but other theories are also advanced. The tests were not usually accompanied by an exhaustive series of tests, as this is difficult to make. With the hope of obtaining more truthful results in the future a pair of even clips, designed by Mr. S. Bent Russell, of the St. Louis Works extension, were purchased. These clips are shown in Fig. 6, and are described in an Engineer News for July 3, 1891.

It is thought by some experimenters that the insertion of a rubber strip between the mold and briquet would be advisable. There is no doubt that this does prevent some breaks, but the results are lower than without the rubber. Two sets of clips have been made to compare the three styles of clip mentioned. (Call the clip with rubber cushion a style by itself.) These will be reported when they have been reduced. At present it is sufficient to say that the rigid clips are better.

The rate at which the strain is applied is 400 pounds per minute, as has previously been mentioned. This rate is maintained constant by means of a pendulum, in accordance with which the hand wheel is turned that moves the weight applying the strain.

Sand is from Point aux Pins, about 3 miles above the lock, in fresh water. In general it contains but little gravel and the percentage of the grains are quartz, but they are not sharp. 11 per cent is retained on the No. 20 sieve (mentioned in the report) and 9 per cent passes the No. 30 sieve. When a measure is loosely filled the voids occupy 35 to 40 per cent of the measure. A barrel of Portland cement weighing about 380 pounds net is mixed with 100 bushels of sand. This amount of sand dry would weigh about 1000 pounds and the amount of sand varies greatly with the moisture contained. Experiments made in France the presence of 1 per cent of water in sand is equivalent to 19 per cent of sand by measure to obtain the same cement. When the sand is perfectly dry the proportions as given are about one to one by weight. Similar results for a different cement. Nos. 12 and 13 show experiments with 100 parts Point aux Pins sand. Nos. 14 to 18 are the results of experiments with sand which could be used for pointing. The sandstone used below the screen of the stone crusher which is crushing sand for use in making concrete. Considerable of this sand is excluded by the No. 30 sieve (Table 4), 35 to 40 per cent passes the No. 100 sieve. It is seen that the results are about 20 per cent of the finest portions are removed. The Water Power sand was used, is clean, but too fine for general use. Sand A is a very fine sand with rounded grains. It is believed a similar sand was used for the lock of 1881.

TABLE 6.—Sand tests—Portland cement.

No.	Molder.	Sand.		Mortar.		Age.	Number averaged.	Tensile strength.	
		Kind.	Fineness.	No. of parts of sand to one of cement by weight.	Per cent of water.			Mean in pounds per square inch.	Average of the error in per cent of breaking strain.
17	L	Point aux Pins.	Pebbles removed.	1	12.5	7 days	5	318.4	6.0
17	L	do	do	1	12.5	28 days	10	424.9	3.9
17	L	do	do	1	12.5	3 mos.	5	549.4	3.5
17	L	do	do	1	12.5	9 mos.	10	604.0	5.6
18	L	do	do	3	11.1	7 days	5	145.4	5.7
18	L	do	do	3	11.1	28 days	10	213.6	5.4
17	L	do	do	3	11.1	9 mos.	9	345.7	3.0
22	N	do	do	3	11.4	7 days	5	147.4	7.6
22	N	do	do	3	11.4	28 days	10	213.4	4.3
22	N	do	do	3	11.4	6 mos.	5	291.2	2.1
21	N	Standard	20-30	3	11.1	6 mos.	7	379.2
6	N	Point aux Pins.	Pass 10	2	10.0	6 mos.	5	479.0	4.6
6	N	do	Pass 10	2	10.3	6 mos.	5	477.6	3.2
9	N	Sandstone screenings	Pass 40	2	18.5	6 mos.	5	388.0	8.0
9	N	do	40-100	2	17.5	mos.	5	478.2	1.0
9	N	do	Pass 40	2	13.3	mos.	5	433.4	4.2
9	N	do	Pass 40	2	12.1	6 mos.	5	382.0	2.7
9	N	do	Pass 40	2	13.3	6 mos.	5	398.4	3.7

Grife too stiff.

TABLE 7.—Sand tests—Natural

The barrel every barrel sample is marked to supposed to a core also in a series-sampled, for a break breaking, then said whether that it is. Of the sand considered cements of:

Briquets record book ment, when bers of barrels briquets, the temper the tank weighing and age tensile strength.

The result book. One book. In the record of an

If the tests barrel book of. Each sample rough tests of (page number) they, with to is afterwards

the oil

the oil

the oil

the oil

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sequence the results are somewhat fragmentary. During the winter more available, and some of the most important work is done at that time. Series started as follows: To show the comparative strength of about fifteen Portland cement when mixed neat and with one and three parts of sand t. A similar series for seven brands of natural cement, ages being seven or years. Several series from different barrels of the same brand for long time tests, made to aid in the interpretation of the regular short-time tests the brands. Series to show the effect of salt, the strength of brine required at injury from frost, effect of heating of materials, and tests to illustrate points which have come up in the practical use and testing of cements. Some rigquets of a few of these series have been broken, but the results are not they would now be incomplete and might be misleading.

OFFICE WORK.

dition to the routine office work the following special work has received at-

letter books used during the construction of the lock of 1881, most of which indexed, have been completed in this respect, and these records are now in pe for reference.

report on claims of Collins & Farwell was submitted to you under date of ember, 1891. A special report by Subinspector L. Fleming on cost of work s. Hughes Brothers & Bangs for the season of 1891, was submitted. This is d as a part of the report of Clerk Common.

ems upon "excavation" and "surveys" in this report have been prepared ant Engineer Joseph Ripley. The items concerning "lock" construction n prepared by Assistant Engineer J. L. Callard. The items "office work" racts" have been prepared by Clerk Richard Common. The item "cut as been prepared by Inspector F. H. Reed. The item "cement" has been y Inspector L. C. Sabin, and the item of "cost" of contractors' plant prepared by Subinspector L. Fleming.

have been employed upon this work during the year, Assistant Engineers nd Callard, Clerk Common, Inspectors F. H. Reed, Rohnert, Sabin, Sheneon, and Birton Reed, Draughtsman Mangelsdorf, Subinspectors Fleming, Spencer, Porter, Pomeroy, St. John, Barnes, Reals, Fowle, and Johnson.

ry respectfully, your obedient servant,

E. S. WHEELER,
Assistant Engineer, etc.

O. M. POE,
Chief of Engineers, U. S. A.

REPORT OF COLONEL O. M. POE, CORPS OF ENGINEERS.

UNITED STATES ENGINEER OFFICE,
Detroit, Mich., November 10, 1891.

I have the honor to submit the following report upon the recent de of shipping in St. Marys River, Michigan, caused by the g of the steamer *Susan E. Peck* in the natural channel about 550 below the point known as "the Elbow" at the lower end of the al channel dredged across Lake George Flats, about 21 miles t. Marys Falls Canal. The exact location of the wreck is shown accompanying Lake Survey chart (No. 1) of river Ste. Marie. accident occurred about 1:10 p. m. on Saturday, October 10, 1891, son of a collision between the *Susan E. Peck*, bound down, laden rain, and the schooner *G. W. Adams*, upward bound, light. sumed to fix the blame for the deplorable event, yet * if the tow of which the *G. W. Adams* formed a ard instead of the windward side of the chan- ot have happened.

g less than 6 feet forward at the time, while
In consequence of these conditions the in-

The result was almost universal satisfaction upon the part erected.

Mr. Lynch, superintendent of St. Marys Falls Canal, with his assistants, proceeded to the point of obstruction. Two tugs were used, and by visiting all waiting vessels obtained from their masters the time of their arrival respectively, and an arrangement was determined upon for the passage of boats through the wreck as soon as the channel should become available.

The distance above and below where the wreck was lying the channel was so narrow for vessels to safely pass each other, and it would have been very unwise to run the chance of another collision which might have completely undo all that had already been done. It was, therefore, not advisable not to try to pass the vessels alternately up and down, but instead to arrange them in groups of ten in the order of their arrival, and then pass these groups alternately—first one group up and the next one down. This arrangement having been made, all the masters of vessels were informed that a tug would notify them when their turn had come. The great majority of masters were contented with this arrangement, and quietly awaited their turn, and did not use their influence to preserve order. There were a few discreditable exceptions, and these made the question of the maintenance of order critical for one day only. In one case a master of a vessel passed through before his turn, and great disquiet arose among the others, who felt that if it was to be a scramble they must get to get an early passage. For a short time it seemed as if control would be lost and discipline destroyed. But when it was known that the offending vessel had gained nothing, but had to wait at the canal until her turn came, in accordance with the arrangement, all signs of dissatisfaction disappeared.

The narrow and curved channel around the bow of the wreck and the current made it advisable to use tugs in passing large boats, and accidents should happen, and it was mainly due to this that none of any consequence occurred. The operation was naturally a slow one, and it was not until noon of the 21st that the last tug vessel had passed. Mr. Lynch and his assistants remained on duty until the 22d.

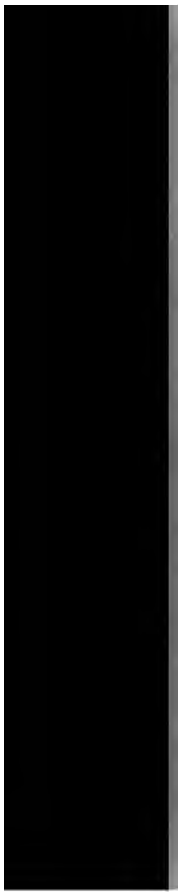
The time of each vessel delayed was recorded, together with the name of the vessel, the registered tonnage, and value of each.

The time of delay was not as accurately determined as in the case of the other locks, because the time of arrival of many of the vessels at the wreck was not noted until after they had been stopped for three days, and then it was obtained from their masters upon their recollection. There were no large errors, however, and the results are believed to be quite trustworthy.

The statistics have been tabulated in a manner similar to those reported in the annual report of 1890, but the interruption of navigation by the breaking of a lock valve in 1891 is so long and are therefore not given in detail, but can be supplied if desired. The following is a summary of the statistics derived from them:

Vessels delayed	275
Tonnage same	\$23,294,000
Time lost	827 days 5 hours and 12 minutes.

In the annual report for 1890 I estimated the loss caused by the breaking of the lock valve at \$56,000. The value of the delayed fleet was \$16,489,000 and the total time lost by delayed vessels



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s charged for the dredges and tugs, as stated above, include private plant and all supplies therefor.

In addition to the sum of \$6,455 there was an indirect cost to the Government, due to the use of the United States tug *Myra*, but it is not necessary to estimate it.

I was asked why we dredged a channel around the wreck when it could have been entirely removed in as short a time. The answer is simple. The laws of the United States require thirty days' notice before any of their agents can proceed to remove a wreck. This provision was quite sufficient to prevent any attempt in that direction and would limit us to the only course left to our discretion.

The dredging companies and others interested in the wreck were notified, however; yet it was five days after the collision before the dredging outfit reached the ground, and it was not until nine or fourteen days after the accident, that the wreck was swung around and partially clear the channel. In this position it remained for some time longer, causing more or less annoyance to passing vessels, and on one occasion, for half a day completely blockading the passage to the channel.

Great credit is due to vesselmen for the patience and forbearance displayed during a period which must have severely tried them. In three instances have come to my knowledge of violation of the regulations made for the orderly passage of the vessels, and of these, two were due to pure selfishness, one appears to have resulted from a misunderstanding, and another from an attempt upon the part of the vessel to obey a telegraphic order from the owner of the boat. In the first case, the offending vessel, being upward bound, was stopped and detained there thirty-six hours by the canal authorities. Her return came exactly in accordance with the time when she would have passed the obstruction. In this case there was no mitigation. The second vessel would also have been stopped, but she had passed through before notice was received. The third vessel appeared determined to disregard the arrangement made to secure orderly passage, and paid no attention to the canal officer's directions, who then called upon the vessel captains to appear, and to the number of 25 they accompanied him on a tug to the scene of the accident for a decisive interview with the offender. After this there was no further trouble.

In this report I take occasion to again invite attention to the services of Mr. Martin Lynch, superintendent of St. Marys Falls, who possesses in a high degree the confidence of the vesselmen. Mr. Lynch, who so freely gave his time and experience to the Government, with tireless energy pushed it ahead when he might, without any compensation, have left it to his employes, I desire to express my thanks. Engineer E. S. Wheeler acted with his usual judgment and skill in conducting the operations to my entire satisfaction.

It is to be regretted that criticism has been indulged in by persons who were themselves guilty of some of the bad features of the matter. I can only think that it is to be deplored by this course to divert attention from their own de-

faults. The part taken by the representatives of the Government I have no fault to find, and, after the event, with all the light we now have, it is difficult to see where they could have done better.

The annoyance to shipping interests were great, but the accident was not at a point where no improvement has ever been made. The depth of water was ample and the width of the channel was sufficient.

ORGANIZATION.

ization remains practically the same as last season, only a having been made among the lockmen. The regular of 1 superintendent, 3 assistant superintendents, 1 clerk, enginemen, 7 watchmen, and 27 lockmen, a total of 45. A party averaging 10 men, during the working season, has d in making current repairs to buildings and piers and in ds. The regular force is divided into three watches of 8 nd this arrangement has proved very satisfactory.

ACCIDENTS AND DELAYS.

have lost their lives in the canal during the year: July 5, y the name of Malcolm McDonald was found drowned in ve the locks. April 24, 1892, a sailor was drowned near e canal. April 30, a sailor was crushed against the pier er *Pasadena* and instantly killed.

ater in the canal has caused an unusual amount of ground- atforms. On this account the lock has been delayed on one-half a day per month during the season of navigation. elbow was broken in the pipe connecting with the engine emptying valve, and in order to repair it it was necessary lock out; this was done and the pipe repaired, making a f twenty-five hours and thirty-six minutes. In addition en the usual number of small delays caused by obstruct- the miter sill, foul tow lines, jamming of boats in the lock, 26th of May the *S. R. Kirby* (bound down) in passing out appeared to ground on the inner platform. When her nearly stopped her stern rose about 12 inches and then again, and she passed on without any further difficulty. ars from the platform came up. A diver was immediately determine the cause. A piece of iron 2 inches thick, 2½ l 14 inches wide was found embedded in the platform so was necessary to saw a number of the timbers before it ed out. This iron was evidently broken off the shoe of This incident, though only causing a delay of one hour and utes, shows the importance of the platform, for without sill would undoubtedly have been carried away and the until a new sill could have been put in.

STAGE OF WATER.

in the lock during the season has averaged lower than ever . On account of this lower stage it has been impossible sels to load to their full capacity; for this reason, light- ave been returned to this route, and the apparent falling istered tonnage of boats using the canal is probably due

ber 16 and 17 there were unusual fluctuations, the water lling 3.65 feet in two hours.

elevation of the surface of the water below the lock for the ril was 581.79 feet above mean tide at New York, which ; lower than ever before during the season of navigation.

MOVABLE DAM.

n has been tested monthly and kept in perfect order. e winter on the 30th of December. The wickets were

ESTIMATES.

The project for operating and care of the canal for the fiscal year ending June 30, 1893, contemplates maintaining the present organization, purchasing the requisite supplies, moving any buildings belonging to the canal that may stand in the way of the work in progress in connection with the construction of the proposed new lock, possibly constructing a new set of lower gates for the existing lock (of 1831), purchasing the timber necessary for repairing canal piers, adding two mules to the present machine house, and employing such labor as may be required in making current repairs and policing the grounds. The estimated cost of the foregoing is as follows:

of regular lock force.....	\$30,000
of labor party.....	6,000
of material purchases.....	5,000
for repairs and additions to machine house.....	10,000
for material for repairing canal piers, etc.....	4,000
for installation of electric-light plant.....	4,000
Total.....	59,000

This estimate is included for extraordinary repairs which may be rendered necessary by accident. Such can not be foreseen, and, therefore, can not be estimated for.

In case of injury or damage to the canal beyond that due to ordinary wear and tear, it must be promptly made good at whatever cost.

All expenses of operating and care are provided for by indefinite appropriation, under section 4 of the river and harbor act approved March 5, 1884.

Total expenditure to June 30, 1891.....	\$316,015.34
ended during fiscal year, exclusive of liabilities outstanding June 30, 1892.....	57,870.99

Total expenditure to June 30, 1892.....	373,886.33
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The Marys Falls Canal is in the collection district of Superior, Mich. The nearest point of entry is Marquette, but Sault Ste. Marie is a subport. Two beacons stand on the piers at the western end of the canal and Fort Brady is within half a mile.

Amount required for fiscal year ending June 30, 1893.....	\$59,000.00
Balance remaining from allotments of preceding year, exclusive of outstanding liabilities.....	\$9,559.01
Outstanding liabilities.....	4,876.53
	4,682.48

Amount required for fiscal year ending June 30, 1893.....	54,317.52
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Appropriations for operating and care of St. Marys Falls Canal, Michigan.

ended during fiscal year ending June 30—

1882.....	\$31,207.48
1883.....	35,509.70
1884.....	31,212.93
1885.....	27,242.45
1886.....	25,400.95
".....	22,138.92
".....	29,898.72
".....	30,749.45
".....	34,323.85
".....	48,330.89
".....	61,389.74
.....	377,405.08

1. The mean of the data is 1000.

2. The standard deviation is 100.

3. The variance is 10000.

4. The coefficient of variation is 10%.

5. The range is 2000.

6. The mode is 1000.

7. The median is 1000.

8. The quartiles are 800, 1000, and 1200.

9. The deciles are 700, 800, 900, 1000, 1100, 1200, 1300, 1400, and 1500.

10. The percentiles are 600, 700, 800, 900, 1000, 1100, 1200, 1300, 1400, and 1500.

11. The mean deviation is 100.

12. The harmonic mean is 900.

13. The geometric mean is 950.

14. The arithmetic mean is 1000.

15. The harmonic mean is 900.

16. The geometric mean is 950.

17. The arithmetic mean is 1000.

18. The harmonic mean is 900.

19. The geometric mean is 950.

20. The arithmetic mean is 1000.

21. The harmonic mean is 900.

22. The geometric mean is 950.

23. The arithmetic mean is 1000.

24. The harmonic mean is 900.

Freight relates solely to tonnage. If we consider the value of tonnage freight we find a very large increase over any other. The average yearly increase in the value of the freight transported in 1891 was about 15 per cent, but for the season of 1891 it was about 25 per cent. The cause of this great increase in valuation is plainly the very large wheat crop. Since we are not likely to be unduly favored with another such crop, a decrease in valuation may probably be predicted for next year.

The failure to maintain the record of annual increase in freight tonnage is due, in a great degree, to three causes other than the decrease in iron-ore tonnage.

The first of these is the fact that the stage of water in the lakes, and consequently the depth available at the canal, was the lowest of which there is any record. In 1889 the average available depth was 15.14 feet, in 1890 it was 15.06 feet, and in 1891 it was 14.42 feet. The average available depth in 1891 was, therefore, 0.64 foot, or 7.6 inches lower than in 1890, at a reasonable estimate of 20 tons to an inch of draft, corresponds to an average of, say, 150 tons for each registered vessel; that is, the available depth of water in the canal during 1891 had been less than that of 1890 (the next lowest recorded), the same 9,744 vessels which carried 8,888,759 tons in 1891, would have carried 1,461,359 tons more than they did, or an aggregate of 10,350,118 tons, or, say, 10,000,000 tons. This is no violent estimate, but is, I think, quite within the truth.

The next cause was the intentional delay of the vessel men in putting their vessels in commission in the spring. This amounted to quite a long delay in the case of many of the largest carriers. No attempt is made to estimate the effect of the delay, but it must have been consid-

The third cause was due to the sinking of the steamer *Susan E. Peck*, "Elbow," in St. Marys River, by which navigation was totally interrupted from 1:10 p. m., October 11, to 3 p. m., October 15, a period of five days. Two hundred and seventy-five vessels were delayed

1. The first part of the document is a list of names and addresses of the members of the committee.

2. The second part of the document is a list of names and addresses of the members of the committee.

3. The third part of the document is a list of names and addresses of the members of the committee.

4. The fourth part of the document is a list of names and addresses of the members of the committee.

5. The fifth part of the document is a list of names and addresses of the members of the committee.

6. The sixth part of the document is a list of names and addresses of the members of the committee.

7. The seventh part of the document is a list of names and addresses of the members of the committee.

8. The eighth part of the document is a list of names and addresses of the members of the committee.

Valuation based on estimates of 1885.

for—	\$53,413,472.13
.....	69,080,071.95
.....	79,031,757.78
.....	82,156,019.97
.....	83,732,527.15
.....	102,214,948.70
.....	128,178,208.51

as open to navigation during season of 1890, 228 days; 1891, 225 days.

COMMERCIAL STATISTICS.

Statement of traffic through St. Marys Falls Canal for fiscal years ending June 30, 1891, and June 30, 1892.

Items.	Fiscal years.		Increase.		Decrease.	
	1891.	1892.	Amount.	Per cent.	Amount.	Per cent.
..... numbers	9,541	11,557	2,016	21		
..... do	4,614	5,615	1,001	22		
ed net tons	7,431,423	9,828,874	2,397,451	32		
..... do	8,338,981	10,107,603	1,768,622	21		
..... number	26,226	25,697			529	2
..... net tons	2,298,487	2,717,029	418,542	18		
..... barrels	3,460,238	4,184,612	724,374	21		
..... bushels	18,184,989	48,466,240	30,281,251	167		
..... (than wheat) do	1,291,084	952,891			338,193	26
ed and pig iron net tons	108,441	73,047			35,394	33
..... barrels	188,292	296,169	47,877	25		
..... net tons	48,816	65,667	16,851	35		
..... do	3,873,044	4,142,797	269,753	7		
..... M feet, B. M.	383,461	399,985	16,524	4		
..... net tons	3,076	1,330			1,746	57
ne do	45,506	38,678			6,828	15
freight do	354,375	443,212	88,837	25		

of the number of vessels passed through St. Marys Falls Canal, with number and cost of lockages for fiscal year ending June 30, 1892.

of vessels	11,557
of lockages	5,615
of tonnage	9,828,874
of charge	10,107,603
	Hrs. Min.
time during which lock was operated	3,860 8
time occupied in making a lockage	41
time spent by vessels in the lock	6,833 20
time spent by vessels in passing lock	36
of lockage	\$10.93
of vessel	\$5.31
	Mills.
of registered ton	6.25
of freight ton	6.07

Cost includes all repairs and improvements made by the operating and labor party, and purchases therefor.

The amount of tonnage which might exceed that of any other fiscal year by 1,768,622 tons. The total for the season of 1891, that is, during that calendar year, was about 2 per cent more than the preceding season; but the traffic so far in 1892 has been so much more than made up, and the traffic for the fiscal year is more than ever before.

APPENDIX L L—REPORT OF COLONEL PO

season of 1891. Cost of carrying freight transported through ... ys Falls Canal.

Items.	Unit.	Quantity.	Price per unit.	Amount.
.....	Net tons.....	2,507,532	\$0.43	\$1,078,238.76
.....	Barrels.....	3,780,143	.15	567,021.45
.....	Bushels.....	38,816,370	.04	1,785,562.22
er than wheat.....	do.....	1,032,104	.03	36,123.64
ared iron.....	Net tons.....	42,560	2.50	106,400.00
.....	do.....	27,181	1.17	31,801.77
.....	Barrels.....	234,528	.18	42,215.04
.....	Net tons.....	69,190	2.00	138,380.00
.....	do.....	3,500,213	.98	3,489,068.74
.....	M feet.....	366,305	2.70	989,023.50
and bullion.....	Net tons.....	1,731	2.25	3,894.75
stone.....	do.....	44,080	2.00	88,160.00
ous merchandise.....	do.....	417,093	3.58	1,493,192.94
al.....	9,849,022.81

In this table "tons" means net tons, or tons of 2,000 pounds.

total amount of freight paid, \$9,849,022.81, divided by the total mile tons, 7,269, gives the cost per mile ton as 1.35 mills.

average distance freight was carried was 820.4 miles, which was 23.2 miles than in 1890.

nature of the data from which the preceding result was found is such that it the cost of loading and unloading. results obtained were as follows:

ile tons.....	7,269,462,269
ight paid.....	\$9,849,022.81
mile ton.....	1.35
distance freight was carried.....	820.4

total number of registered craft which used the canal during the season was—	
s.....	396
sels.....	256
total.....	652

American craft.

Class.	No.	Registered tonnage.	Freight tonnage.	Number of passengers.	Valuation.
.....	358	325,697	6,129,521	13,817	\$26,879,100
.....	237	142,667	2,396,557	5,068,200
al.....	595	468,364	8,526,078	13,817	31,947,300

Eleven thousand seven hundred and twenty-nine net tons of freight were also carried by unregistered craft in 447 passages, the average cargo being 26.8% tons.

Canadian craft.

Class.	No.	Registered tonnage.	Freight tonnage.	Number of passengers.	Valuation.
.....	38	18,489	291,418	12,373	\$1,771,500
.....	19	8,255	59,534	348,000
al.....	57	26,744	350,952	12,373	2,119,500

Summary.

umber of registered craft.....	652
essages by unregistered craft.....	447
ight carried by registered craft..... tons.....	8,877,030
ight carried by unregistered craft..... do.....	11,729
passengers carried.....	26,190
of craft (registered).....	\$34,066,800

The following is a list of the supplies and materials used in the construction of the building at No. 1000 Broadway, New York City, during the year ending March 31, 1917.

The supplies and materials used in the construction of the building at No. 1000 Broadway, New York City, during the year ending March 31, 1917, are as follows:

The following is a list of the supplies and materials used in the construction of the building at No. 1000 Broadway, New York City, during the year ending March 31, 1917.

Name of material	Quantity
Cement	1000
Bricks	5000
Lumber	10000
Iron	500
Glass	1000
Paint	100

zed statement of expenditures incurred on account of appropriation for operating and of canals and other works of navigation, indefinite, applied to operating and care St. Marys Falls Canal, Michigan, for the fiscal year ending June 30, 1892.

No. of voucher.	From whom purchased.	Articles.	Amount.
<i>Part of first quarter, 1892.</i>			
24	3 Morris Machine Works, per William F. Morris, sole owner and proprietor.	For furnishing and delivering at St. Marys Falls Canal, Michigan, 2 No. 12 centrifugal pumps with engines, boilers, boiler feed pumps, foot valves, flap valves, ejectors, elbows, pipe, gaskets, bolts, etc., complete for an agreed price of	\$7,905.00
18	4 Emery D. Weimer.....	Furnishing and delivering at St. Marys Falls Canal, Michigan— 106 cedar fence posts, at 20 cents each. 102 pieces round white oak timber 8 feet long, 16 inches in diameter at one end, at \$4.45 each.	21.20 453.90
18	5 Emery D. Weimer.....	Furnishing and delivering at St. Marys Falls Canal, Michigan— 10,000 shingles, at \$2.50 per M..... 10,200 feet, B. M., pine lumber, common stock, 1 inch by 12 inches by 16 feet, at \$16.75 per M. 1,045 feet, B. M., pine lumber, common stock, 2 inches by 14 inches by 16 feet, at \$14.50 per M. 6,112 feet, B. M., pine lumber, common stock, 2 inches by 12 inches by 16 feet, at \$16.50 per M. 4,116 feet, B. M., pine lumber, common stock, 4 inches by 4 inches by 16 feet, at \$15.50 per M. 72,284 feet, B. M., square timber, 12 inches by 12 inches by 16 feet, at \$18.25 per M. 269,376 feet, B. M., square timber, 12 inches by 12 inches by 24 feet and upwards, at \$21 per M.	25.00 170.85 15.15 100.85 63.80 1,319.18 5,056.90
28	6 O. M. Poe, Colonel Corps of Engineers, etc.	Mileage from Detroit, Mich., to Grand Rapids, Mich., and return, being 316 miles, at 8 cents per mile.	25.28
31	7 Frank M. Dunlap.....	Services as draftsman from July 1 to July 31, 1891 (both days inclusive), being one month.	125.00
3	8,9 Pay rolls, July, 1891.....	1 superintendent..... 3 assistant superintendents, at \$100 per month. 1 clerk..... 1 engineman..... 1 engineman..... 1 engineman..... 4 foremen, at \$75 per month..... 1 watchman..... 3 watchmen, at \$50 per month..... 2 watchmen, at \$45 per month..... 3 lockmen, 2 months and 28½ days, at \$60 per month. 3 lockmen, 2 months and 21 days, at \$50 per month. 21 lockmen, 20 months and 28 days, at \$45 per month. 10 laborers, 9 months and 27½ days, at \$45 per month. 2 divers, 10½ days, at \$7.50 per day..... 1 scrubber, 10 days, at \$1.20 per day..... 1 carpenter, 25½ days, at \$2.50 per day..... 1 tinner, 14½ days, at \$3 per day..... 1 machinist, 8½ days, at \$3 per day..... One kitchen sink with waste pipe and fixtures.	150.00 300.00 150.00 90.00 80.00 75.00 300.00 75.00 150.00 90.00 177.00 135.00 942.00 446.25 78.75 12.00 63.00 43.75 24.15 9.30
4	10 John Hickler & Son.....		
31	11 Frank M. Dunlap.....	Services as draftsman, from August 1 to August 31, 1891 (both days inclusive), being 1 month.	125.00
3	12,13 Pay rolls, August, 1891.....	1 superintendent..... 3 assistant superintendents, at \$100 per month. 1 clerk..... 1 engineman..... 1 engineman..... 1 engineman..... 1 engineman..... 4 foremen, at \$75 per month..... 1 watchman.....	150.00 300.00 150.00 90.00 80.00 75.00 300.00 75.00

2418 REPORT OF THE CHIEF OF ENGINEERS, U. S. ARMY.

Detailed statement of expenditures incurred on account of appropriations for works of
 care of canals and other works of navigation, etc.—Continued.

Date.	No. of voucher.	Particulars purchased.	Articles.	Am't.
		Part of first quarter, 1891— Continued.		
Sept. 1	11, 12	Pay rolls, August, 1891.	2 watchmen, 2 months and 22 days, at \$50 per month. 2 watchmen, at \$60 per month. 2 lockmen, at \$60 per month. 2 lockmen, 2 months and 19 days, at \$50 per month. 22 lockmen, 10 months, and 6 days, at \$45 per month. 20 laborers, 3 months and 24 days, at \$45 per month. 1 diver, 10 days, at \$7.50 per day. 1 carpenter, 22 days, at \$2.50 per day. 1 watchman, 124 days, at \$1.25 per day. 1 tinner, 12 days, at \$5 per day.	
	14	Thomas H. Forsyth.	1 month, 12 days, at \$5 per day. Holding and delivering at South St. Louis, Mich., 10,000 feet, S. M. white timber, at \$60 per M feet, S. M.	
	15	C. W. Farr, propagator.	Rent of telephone, 7 months, from June 1 to August 31, 1891 (both dates inclusive), at \$40 per year.	
	16	Russell Wheel and Foundry Company, by George H. Russell, president.	Contracting and delivering at St. Marys Falls Canal, Michigan, four (4) valve frames for the sum of	
	17	Justus E. Smith.	Rent of 1 storage room at Detroit, Mich., from July 1 to September 30, 1891 (both days inclusive), being 3 months, at \$10 per month.	
	18	Frank M. Dunlap.	Services as draftsman from September 1 to September 30, 1891 (both days inclusive), being 1 month.	
		Second quarter, 1891.		
Oct. 1	1, 2	Pay rolls, September, 1891.	1 superintendent. 3 assistant superintendents, 2 months and 29 days, at \$100 per month. 1 clerk. 1 engineman. 1 engineman. 1 engineman. 4 foremen, at \$75 per month. 1 watchman. 2 watchmen, at \$50 per month. 2 watchmen, at \$45 per month. 2 lockmen, 2 months and 24 days, at \$50 per month. 3 lockmen, 2 months and 24 days, at \$50 per month. 21 lockmen, 29 months and 25 days, at \$45 per month. 9 laborers, 8 months and 124 days, at \$45 per month. 2 divers, 1 day. 1 scrubber, 10 days, at \$1.50 per day. 1 tinner, 44 days, at \$3 per day. 2 machinists, 24 days, at \$3 per day. 1 carpenter, 26 days, at \$2.50 per day. One 4-inch tap from city water-main to curbstone.	
June 9	3	Board of Public Works, by F. M. Taylor, deputy secretary.		
Oct. 1			Water-rent from July 1 to September 30, 1891 (both dates inclusive), at \$9 per year.	
	31	4 Frank M. Dunlap	Services as draftsman from October 1 to October 31, 1891 (both days inclusive), being one month.	
Nov. 3	5, 6	Pay rolls, October, 1891.	1 superintendent. 3 assistant superintendents, at \$100 per month. 1 clerk. 1 engineman. 1 engineman. 1 engineman. 4 foremen, 3 months and 27 days, at \$75 per month. 1 watchman. 2 watchmen, at \$50 per month. 2 watchmen, at \$45 per month.	

red statement of expenditures incurred on account of appropriation for operating and care of canals and other works of navigation, etc.—Continued.

No. of voucher.	From whom purchased.	Articles.	Amount.
<i>Second quarter, 1892—Cont'd.</i>			
3	6, 6	Pay rolls, October, 1891.....	3 lockmen, at \$60 per month..... \$180.00 3 lockmen, at \$50 per month..... 150.00 21 lockmen, 20 months and 28 days, at \$45 per month..... 942.00 9 laborers, 8 months and 25 days, at \$45 per month..... 397.50 1 scrubber, 12½ days, at \$1.20 per day.... 15.00 1 carpenter, 27 days, at \$2.50 per day..... 67.50 4 calkers, 37½ days, at \$2.50 per day..... 93.75 2 teams, 2½ days, at \$4 per day..... 9.00 1 tinner, 1½ days, at \$3 per day..... 4.50 1 diver, 3 days, at \$7.50 per day..... 22.50
5	7	George Kemp.....	5 tons anthracite coal (chestnut), at \$5.60 per ton..... 28.00 45 tons anthracite coal (stove), at \$5.50 per ton..... 247.50 20 tons bituminous coal (Massillon), at \$3.75 per ton..... 75.00 6 tons cannel coal (West Virginia) at \$5.25 per ton..... 31.50
7	8	James Strachan.....	1 suction pipe, 5 feet 9 inches long, 15 inches in diameter, of boiler plate ½-inch thick, with cast-iron flanges..... 38.50 One tank, 10 feet long, 8 feet wide, and 4 feet high, of ½-inch boiler iron..... 287.00
3 16	9	The Richmond and Backus Company, by Charles F. Backus, secretary and treasurer.	2,000 ¼-sheet paper, plain, unruled, at \$2 per M..... 4.40 25 sheets paper, carbon, at \$4 per hundred..... 1.00
3 10, 11	10, 11	Pay rolls, November, 1891....	1 superintendent..... 150.00 3 assistant superintendents, at \$100 per month..... 300.00 1 clerk..... 150.00 1 engineman..... 90.00 1 engineman..... 80.00 1 engineman..... 75.00 4 foremen, at \$75 per month..... 300.00 1 watchman..... 75.00 3 watchmen, at \$50 per month..... 150.00 2 watchmen, at \$45 per month..... 90.00 3 lockmen, at \$60 per month..... 180.00 3 lockmen, 2 months and 29½ days, at \$50 per month..... 149.17 21 lockmen, 20 months and 29½ days, at \$45 per month..... 943.50 9 laborers, 6 months and 25 days, at \$45 per month..... 307.50 1 diver, ½ day, at \$7.50 per day..... 3.75 1 scrubber, 8½ days, at \$1.20 per day..... 10.50 1 carpenter, 29½ days, at \$2.50 per day..... 58.75 3 calkers, 11½ days, at \$2.50 per day..... 28.14 1 tinner, 5½ days, at \$3 per day..... 16.80 1 machinist, 14½ days, at \$3 per day..... 44.10 1 team, ½ day, at \$3 per day..... 1.50
-24	12	O. M. Poe, Colonel Corps of Engineers, etc.	Mileage from Detroit, Mich., to Sault Ste. Marie, Mich., and return, being 774 miles, at 8 cents per mile..... 61.92
30	13	C. W. Farr, proprietor.....	Rent of three (3) telephones from May 1 to November 30, 1891 (both days inclusive), at \$90 for the season of navigation (or 7 months)..... 90.00
31	14	Justin E. Smith.....	Rent of one storage room at Detroit, Mich., from October 1 to December 31, 1891 (both days inclusive), being 3 months, at \$10 per month..... 30.00
<i>Third quarter, 1892.</i>			
5	1, 2	Pay rolls, December, 1891....	1 superintendent..... 150.00 3 assistant superintendents, at \$100 per month..... 300.00 1 clerk..... 150.00 1 engineman..... 90.00 1 engineman..... 80.00 1 engineman..... 75.00 4 foremen, at \$75 per month..... 300.00 1 watchman..... 75.00 3 watchmen, at \$50 per month..... 150.00 2 watchmen, at \$45 per month..... 90.00

ized statement of expenditures incurred on account of appropriation for operating and care of canals and other works of navigation, etc.—Continued.

No. of voucher.	From whom purchased.	Articles.	Amount.
<i>Third quarter, 1892—Cont'd.</i>			
11 and 12	February, 1892.....	3 lockmen, 1 month and 23 days, at \$45 per month. 1 carpenter, 17 days, at \$2.50 per day..... 1 scrubber, 5 days, at \$1.20 per day 1 blacksmith 18½ days, at \$3 per day..... 1 tinner, 1½ days, at \$3 per day..... 4 machinists, 96 days, at \$3 per day.....	\$79.50 42.50 6.00 56.40 3.75 288.00
13	James Strachan	283½ hours' work with steam hammer, at 70 cents per hour. 11 hours' work with planer, at 35 cents per hour. 34½ hours' work with steam-power drill, at 35 cents per hour. 112½ hours' work with bolt cutter, at 35 cents per hour. 16½ hours' lathe work, at 40 cents per hour.	268.62 3.85 12.07 39.73 6.60
14	James Strachan	4 sets of slides for valve engines, cast, planed, and fitted.	119.50
15	The Richmond and Backus Company, by Charles F. Backus, secretary and treasurer.	50 sheets paper, carbon, at \$4 per hundred.	2.00
16	Justin E. Smith	Rent of one storage room at Detroit, Mich., from January 1 to March 31, 1892 (both days inclusive), being 3 months, at \$10 per month.	30.00
<i>Fourth quarter, 1892.</i>			
1 and 2	Pay rolls, March, 1892	1 superintendent	150.00
		3 assistant superintendents, at \$100 per month.	300.00
		1 clerk	150.00
		1 engineman	90.00
		1 engineman	80.00
		1 engineman	75.00
		4 foremen, at \$75 per month.....	300.00
		1 watchman.....	75.00
		3 watchmen, 2 months and 29½ days, at \$50 per month.....	149.17
		2 watchmen, 1 month and 8 days, at \$45 per month.....	57.00
		3 lockmen, at \$60 per month	180.00
		3 lockmen, at \$50 per month	150.00
		21 lockmen, 20 months and 21 days, at \$45 per month.....	931.50
		1 scrubber, 6½ days, at \$1.20 per day	7.50
		1 team, 2 days, at \$4 per day	8.00
		1 machinist, 23½ days, at \$3 per day.....	71.60
3 and 4	Pay rolls, April, 1892	1 superintendent	150.00
		3 assistant superintendents, at \$100 per month.....	300.00
		1 clerk.....	150.00
		1 engineman.....	90.00
		1 engineman.....	80.00
		1 engineman.....	75.00
		4 foremen, at \$75 per month	300.00
		1 watchman	75.00
		1 watchman, 4½ months, at \$55 per month.....	22.00
		4 watchmen, 3 months, at \$50 per month.....	150.00
		2 watchmen, 1 month and 12 days, at \$45 per month.....	63.00
		3 lockmen, 2 months and 25½ days, at \$60 per month.....	171.00
		3 lockmen, 2 months and 25½ days, at \$50 per month.....	142.50
		21 lockmen, 20 months and 26½ days, at \$45 per month.....	939.37
		11 laborers, 3 months and 16½ days, at \$45 per month.....	160.13
		2 divers, 9 days, at \$7.50 per day.....	67.50
		1 carpenter, 4 days, at \$2.50 per day.....	10.00
		1 scrubber, 12 days, at \$1.20 per day.....	14.40
		3 machinists, 54 days, at \$3 per day.....	162.00
5	Board of Public Works, by J. F. [illegible] for, deputy re-	Water rent for first quarter, 1892, from January 1, to March 31, 1892 (both dates inclusive), at \$9 per year.	2.25

L L 3.

DRY DOCK AT ST. MARYS FALLS CANAL, MICHIGAN.

position to locate a dry dock in immediate proximity to the stem is as objectionable as ever; but if it should be decided when the location heretofore referred to, at the eastern end of transferred from the Fort Brady military reservation to the reservation, is the least objectionable.

construction of a pier in front of the Fort Brady reservation completed, and a portion of this work would have to be re-estimated increasing by \$20,000 the probable cost of a dry dock, and the cost therefor should be increased accordingly.

estimated) required for the construction of a dry dock at the point

..... \$343,872
 can be profitably expended in fiscal year ending June 30, 1893. 150,000

should be added to the \$65,000 (more or less) which it is estimated the State of Michigan holds in readiness to transfer to the State for the purpose of constructing a dry dock at St. Marys Falls.

It is not improper to add that I am strongly opposed to the construction of a dry dock to be operated in connection with the canal.

L L 4.

IMPROVEMENT OF HAY LAKE CHANNEL, ST. MARYS RIVER, MICHIGAN.

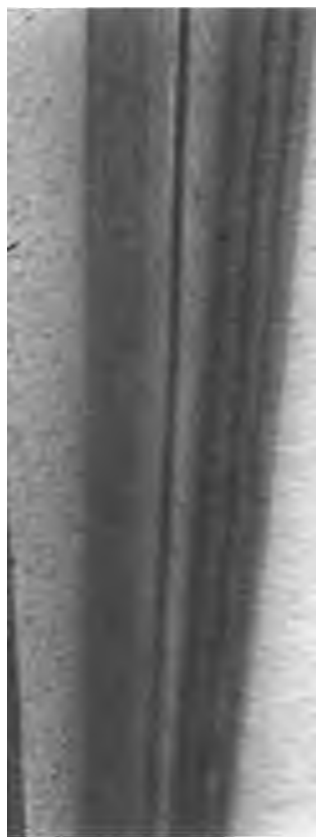
Improvements were commenced in the channel through Hay Lake which is restricted in depth at Sugar Island Rapids and at Middle Island.

At these two places a maximum draft of but 8 or 9 feet is carried, if a very irregular course was taken, and, practically, drawing more than 6 feet of water would not attempt the passage.

In addition to the places above mentioned there were some places in Hay Lake requiring removal to make the channel available for navigating St. Marys River.

Original estimates for this improvement were based upon a channel 300 feet wide and 17 feet deep, leaving the present channel of St. Marys River at Sugar Island Rapids (about 1 mile below the canal) through those into Hay Lake, and then by Middle Neebish, rejoining the present navigable channel at the Sugar Island, thus saving a distance of 11 miles and obtaining a channel which can be so marked by lights as to be navigable by night, and which is impracticable with the present channel, except by the use of lights.

The estimated cost of this project was \$2,127,292. The project was subsequently modified to increase the navigable depth to 20 feet, the cost being \$2,659,115, subject to change, however, in case any difficulties are developed during the progress of the work. On June 30, 1890, \$975,000 had at various times been appropriated for the work. The river and harbor act of September 19, 1890, provided an additional \$400,000 for continuing the improvement. In provision, "That such contracts as may be desirable may be made by the Secretary of War for materials and labor for the same, or any part of the same, to be paid for as appropriations



THE
STATE OF
NEW YORK
IN SENATE,
January 12, 1909.

REPORT

OF THE

COMMISSIONERS OF THE LAND OFFICE

IN ANSWER TO A RESOLUTION PASSED

AT THE SESSION OF THE SENATE, APRIL 11, 1907,
AND AT THE SESSION OF THE ASSEMBLY,
MAY 11, 1907,
RELATIVE TO THE LANDS BELONGING TO THE STATE.

ALBANY:

EXCAVATION UNDER CONTRACTS STILL IN FORCE.

During the fiscal year excavation has been carried on under six contracts divided into February 6, 1891. Each of these contracts covers sections into which the work has been divided, as shown on appended to this report. A depth of 20 feet was required except where rock occurred, where a depth of 21 feet was called for to establish a 20-foot navigation. At the close of the fiscal year the condition of work on the different sections was as follows:

Section 286 and 490.—The material to be removed consisted of about 190,000 cubic yards of sand, gravel, bowlders, hardpan, and sandstone bed of unknown proportions, and a great deal of blasting was necessary. A depth of 21 feet is called for by the specifications; a rough dike, 2,040 feet in length, is required to be constructed on the top of the channel between cross sections 286 and 490, and the price for excavation is \$1.20 per cubic yard, bank measure, the cost of the dike being included without additional charge. The contractors began operations May 6, 1891, and, except for interruption by the winter, work on this section has been in progress ever since. The total to June 30, 1892, of the estimates for excavation is 150,696 cubic yards, bank measure, of which 150,696 cubic yards have been excavated during the fiscal year. The dike construction required for this section has been entirely finished, and the total length of the dike is now 6,140 feet.

Section 287.—The material to be removed consisted of about 66,875 cubic yards of sand, gravel, bowlders, hardpan, and sandstone of unknown proportions, and some blasting was necessary. On this section a depth of 21 feet is required; elsewhere 20 feet, and the price is 57 cents per cubic yard, bank measure. The contractors, Messrs. Dunbar & Sullivan, began operations May 15, 1891, and, except for interruption by the winter, work on this section has been in progress ever since. The total to June 30, 1892, of the estimates for excavation is 66,875 cubic yards, bank measure, of which 57,028 cubic yards have been excavated during the fiscal year. The actual amount of work done is really much greater than this, but owing to certain conditions of the specifications the additional amount can not yet be paid for.

Section 288.—The material to be removed consisted of about 303,846 cubic yards of silt, sand, clay, and gravel. The depth of excavation required is 20 feet, and the contract price is 14½ cents per cubic yard, bank measure. The contractors, Messrs. Carkin, Stickney & Cram, began operations July 27, 1891, and, except for interruption by the winter, work on this section has been in progress ever since. The total to June 30, 1892, of the estimates for excavation is 303,846 cubic yards, bank measure, of which 288,399 cubic yards have been excavated during the fiscal year.

Section 289.—The material to be removed consisted of about 329,163 cubic yards of sand, clay, gravel, and bowlders. The depth of excavation required, and the contract price is 13 cents per cubic yard, bank measure. The contractors, Messrs. Carkin, Stickney & Cram, began operations July 16, 1891, and, except for interruption by the winter, work on this section has been in progress ever since. The total to June 30, 1892, of the estimates for excavation is 329,163 cubic yards, bank measure, of which 329,163 cubic yards have been excavated during the fiscal year.

Section 290.—The material to be removed consisted of about 710,000 cubic yards of sand, gravel, bowlders, hardpan, and sandstone of unknown proportions, and a great deal of blasting was necessary. A depth of 21 feet is called for by the specifications; a rough dike, 2,040 feet in length, is required to be constructed on the top of the channel between cross sections 286 and 490, and the price for excavation is \$1.20 per cubic yard, bank measure, the cost of the dike being included without additional charge. The contractors began operations May 6, 1891, and, except for interruption by the winter, work on this section has been in progress ever since. The total to June 30, 1892, of the estimates for excavation is 710,000 cubic yards, bank measure, of which 710,000 cubic yards have been excavated during the fiscal year. The dike construction required for this section has been entirely finished, and the total length of the dike is now 6,140 feet.

ing reports, and are so universally admitted, that it is not necessary to repeat them. It is an important part of the system of improvement of the navigation of the great lakes, and if it were available during the fiscal year just closed, it would have been carried by vessels carrying 10,107,603 tons of freight and 25,697 pas-

sengers. The estimate for the work is \$2,659,115, of which \$1,675,000 has been expended, leaving \$984,115 to be provided. To pay the various claims of the employes and contractors, \$500,000 will be required for the ending June 30, 1894. This amount should be appropriated, and it is proposed that any portion of it not required for Hay Lake would be expended elsewhere between lakes Superior and Lake Michigan, in places where excavation is needed to obtain the navigable depth of 12 feet. This entire amount can be expended to advantage during the fiscal year ending June 30, 1894.

Balance available to June 30, 1891	\$828,823.08
Less: amount expended during fiscal year (exclusive of outstanding liabilities).....	280,983.01

Balance available to June 30, 1892	1,109,806.09
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Channel is in the collection district of Superior, Mich. The nearest light is Marquette, but Sault Ste. Marie is a subport. The nearest light-beacon on the pier at the western end of St. Marys Falls Canal.

Money statement.

Balance unexpended	\$849,695.80
Less: amount expended during fiscal year	284,370.24

Balance unexpended	565,325.56
Less: outstanding liabilities	\$84,820.07
Amount covered by uncompleted contracts	480,505.49
	<u>565,325.56</u>

Appropriated by act approved August 5, 1892	115,000.00
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Estimated amount required for completion of existing project	869,115.00
Amount that can be profitably expended in fiscal year ending June 30, 1894	500,000.00

in compliance with requirements of sections 2 of river and harbor acts of 1866 and 1867.

Estimate for improving Hay Lake Channel, Sault Ste. Marie River, Michigan.

Section 82	\$200,000
Section 86	125,000
Section 88	150,000
Section 388	500,000
Section 3, 1890	400,000
Section 1, sundry civil bill	300,000
	<u>1,675,000</u>

work is in the Michigan collection district, Michigan. The nearest port of Grand Haven, Mich., and the nearest light-house stands on the pier head of the outer end of the channel.

Money statement.

1891, balance unexpended	\$18,340.43
1892, amount expended during fiscal year	12.70
1892, balance unexpended	18,327.73

Appropriations for improving harbor at Cheboygan, Mich.

1871	\$10,000	June 14, 1880	\$6,000
1872	15,000	March 3, 1881	6,000
1873	15,000	August 2, 1882	10,000
1874	15,000	July 5, 1884	5,000
1875	15,000	August 5, 1886	15,000
14, 1876	10,000	August 11, 1888	15,000
1878	8,000		
1879	3,000	Total	148,000

COMMERCIAL STATISTICS.

Commerce of the harbor at Cheboygan, Mich., for the season of 1891.

[Compiled from the report of Mr. Jno. W. Loucks, deputy collector of customs.]

Articles.	From custom-house books.		From warehouse books.		Total tons.
	Quantities.	Tons.	Quantities.	Tons.	
<i>Shipments.</i>					
..... foot B. M.	115,974,607	173,961	3,000,000	4,500	178,461
wood	1,723,000	3,876			3,876
e	15,988,550	3,997	2,500,000	625	4,622
ties, cedar	236,600	11,830			11,830
sts	313,619	7,840			7,840
pine	5,136,000	898	35,000,000	6,125	7,023
a poles, cedar	1,900	285	1,500	225	510
, hemlock	1,442	1,081			1,081
seous				7,000	7,000
al shipments		203,768		18,475	222,243
<i>Receipts.</i>					
..... bushels..	11,700	351			351
..... do.	5,000	140			140
..... do.	41,500	664			664
d and soft		5,861			5,861
sacks	1,513	75			75
..... barrels.	500	100	1,200	240	340
..... tons.		537			537
..... barrels.			2,000	300	300
..... do.			500	75	75
ster			300	45	45
..... do.			2,000	200	200
seous				15,000	15,000
al receipts		7,728		15,860	23,588
regate					245,831

that the depth of 16 feet that has been obtained in the river rendered available, the next appropriation should be expended in deepening the channel across the bar to the full depth of 16 feet for its full width. The river and harbor act of September 19, 1890, which separate appropriations for the river and the harbor, seemed to make an absolute division between the two, and this division was the result of the light-house crib. It is evident, however, that the intent of the law was to render the 16-foot depth required in the river available for commerce, and should no such distinction be made again between the two portions of the work, future appropriations should be made with this understanding.

The river and harbor act of September 19, 1890, although separating the work into two portions—permitted the grouping in one contract of work of a similar character situated in the same region. A contract could therefore be used on this work to the best advantage by utilizing the harbor during fair weather and in the river during bad weather. By being thus prevented from lying idle the cost of the work would be reduced nearly one-half.

On account of the commercial importance of the city of Alpena, which is situated at the mouth of the river, the river and harbor are well worthy of a special appropriation of \$20,587.48, which, if made at one time and not restricted as to the portion of the work on which it must be expended, will probably suffice to make a continuous depth of 16 feet. The entire amount can be profitably expended during the fiscal year ending June 30, 1894.

Expenditure to June 30, 1891	\$20,785.81
During fiscal year	3,403.69
<hr/>	
Expenditure to June 30, 1892	24,189.50

Thunder Bay River is in the collection district of Huron, Mich. The nearest port is Huron, and the nearest light-house is at the work.

Money statement.

Balance unexpended	\$5,372.50
Less, amount expended during fiscal year	4,548.87
<hr/>	
Balance unexpended	823.63

Estimated) required for completion of combined projects for Thunder Bay River and harbor	10,587.48
Amount that can be profitably expended in fiscal year ending June 30, 1894	10,587.48
Amount in compliance with requirements of sections 2 of river and harbor acts of 1866 and 1867	

Appropriations for improving harbor at Thunder Bay, Michigan.

1876	\$4,500
1882	15,000
1890	5,500
<hr/>	
	25,000

COMMERCIAL STATISTICS.

It is not practicable to obtain statistics for the calendar year 1891, but they are substantially the same as for the preceding year, when the commerce of Michigan, at the mouth of Thunder Bay River, was estimated at a value at over \$5,000,000.

River is in the collection district of Huron, Mich. The nearest port Huron, Mich., and the nearest light-house is at the mouth of the

Money statement.

ance unexpended	\$9,608.50
mount expended during fiscal year.....	8,328.80
<hr/>	
ance unexpended	1,279.70
riated by act approved July 13, 1892.....	10,000.00
<hr/>	
le for fiscal year ending June 30, 1893.....	<u>11,279.70</u>
<hr/>	
ated) required for completion of existing project*.....	
an be profitably expended in fiscal year ending June 30, 1894*	
compliance with requirements of sections 2 of river and	
of 1866 and 1867.	

appropriations for improving Thunder Bay River, Michigan.

890.....	\$10,000
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COMMERCIAL STATISTICS.

practicable to obtain statistics for the calendar year 1891, but they be substantially the same as for the preceding year, when the com- Mich., at the mouth of the river, was estimated at 500,000 tons and r \$5,000,000. A large portion of this commerce enters Thunder Bay impossible to determine its amount.

L L 8.

ROVEMENT OF HARBOR AT AU SABLE, MICHIGAN.

beginning of improvements the mouth of Au Sable River wide, with a depth of 5 feet over the bar. Above the mouth of a mile 7 to 10 feet of water was found, and above this or 6 feet.

t project for the improvement of the harbor was adopted notified in 1879, the object being to obtain a channel of 10 feet in depth for a width of 100 feet from Lake Huron Road Bridge at Au Sable.

heretofore expended on this improvement has given tem- s only, and a permanent channel can not be secured except cost as to be disproportionate to the benefits to be ob-

ents from this port are large and important, but are prin- from private piers built into the lake entirely outside of Were there a fair prospect of securing a permanent int- t a reasonable cost I would take pleasure in recommending opriations for this harbor, but can not clearly see my way

are to June 30, 1892.....	\$114,786.12
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in the collection district of Huron, Mich. The nearest port of entry and the nearest light-house is at the work.

* See Thunder Bay Harbor.

of Engineers recommended that a channel be made 200 feet a depth of 14 feet from Saginaw Bay to Bay City and with a depth of 12 feet thence to the head of the river, a total distance of 4 miles.

Plans for this project have been made at various times. These plans are for the repair, extension, and reconstruction of the revetment, the Carrollton Channel, and for the construction of similar works at the head of Crow Island for the improvement of Zilwaukee

On August 30, 1891, dredging was in progress on the bar at the mouth of the river and at Zilwaukee Bar, and repair work at Carrollton and extension of wing dam.

During the fiscal year the following contracts were in force, all under the appropriation of September 19, 1890:

Contractor.	Contract for—	Entered into.	Remarks.
Abbott & Co.	Dredging above Bay City	Feb. 21, 1891	Closed Dec. 7, 1891.
Abbott & Co.	Timber and plankdo.....	Closed Nov. 6, 1891.
Abbott & Co.	Iron bolts and nailsdo.....	Closed Oct. 5, 1891.
Abbott & Co.	Dredging bar at mouth	Apr. 9, 1891	In force.

Work done during the fiscal year was as follows:

Work done on the bar at the mouth, 75,982 cubic yards, scow measure, at the contract price of 34 cents per cubic yard, \$25,833.88. Work yet completed.

Work done at Zilwaukee Bar, 25,368 cubic yards, scow measure, at the contract price of 36 cents per cubic yard, \$9,132.48 and in a channel 200 feet wide and 12 feet deep for the whole extent of the bar.

Work done at Bristol Street Bridge, in the city of Saginaw, 13,019 cubic yards, scow measure, costing, at the contract price of 36 cents per cubic yard, \$4,686.84, resulting in a navigable channel 12 feet deep and 12 feet wide nearly half a mile in length.

Work done from the channel of Saginaw River below Melbourne, 36,000 cubic yards, scow measure, costing, at the contract price of 36 cents per cubic yard, \$12,960.00.

The superstructure of the Carrollton revetment was repaired, the wrecked portion of the lower end of the old wing dam at the head of Crow Island, and the latter was extended downstream, and a wing dam was built upon the west shore of the island. The original length of the dam was 900 feet. It is now 1,685 feet long, with a shore arm 120 feet long.

The present condition of the improvement is as follows:

at mouth of the river.—A channel 14 feet in depth and about 100 feet wide has been made across the crest of the bar from deep water to the mouth of the river to a point on the light-house range—or axis of the old channel—12,300 feet from the front light, terminating in 13 feet of water; thence northward one dredge cut, 14 feet deep and about 100 feet wide, nearly on range and east of axis, extends 3,800 feet farther to a sharp curve in Saginaw Bay.

A recent survey of the channel was made in January and February, 1891, and was then found that in order to complete the width of the 14-foot channel to the projected 200 feet, and extend it to the 14-foot curve, at a distance of 16,100 feet from the front light, it will be necessary to remove about 180,000 cubic yards of material, two-thirds

2 feet along the track followed by tugs in
 are not connected. The bottom consists
 so far made to secure a channel of the
 been successful. It is estimated that a chan-
 wide will involve the excavation of about
 . Whether such a channel would be per-

Shoal.—The improvement of this reach of
 during the season of 1891. A survey made in
 the channel had not deteriorated during the
 Based upon the survey referred to, it is esti-
 channel 12 feet deep and 200 feet wide will in-
 12,300 cubic yards of sand. Whether it will be
 is very doubtful.

Nothing has yet been done toward improving
 immediately above Mackinaw Street Bridge. In
 channel 12 feet deep and 200 feet wide at this locality
 of about 100,000 cubic yards of material, and
 ce made it will require frequent dredging to

along West Bay City.—This is not a part of the origi-
 of \$15,000 has been expended in dredging it and
 latest survey (in January, 1891) it was found in much
 expected. It can not be regarded as permanent,

amount of material yet to be removed to complete
 at about 640,000 cubic yards, at an estimated
 but it is perfectly well understood that considerable
 can never be considered as permanently completed,
 less dredging will always be required after each
 probable cost of not less than \$5,000 per year.

report by Assistant Engineer B. H. Muehle, gives
 in detail necessarily omitted from this. Particular
 to his schedule of dredging heretofore done, and
 required to complete the original project; including
 West Channel along West Bay City, and \$18,500 for
 ville Bar, neither of which were comprised in the origi-
 will be observed that the total exceeds the balance
 original estimates; but if the amounts already ex-
 West Channel along West Bay City, the amount
 for its further improvement, the amount of the esti-
 exville Bar, the amounts of the annual expenditures
 and hereafter to be made for dredging—referred to in
 part as necessary, and estimated at \$5,000 per year—and
 expended in keeping the several revetments and wing
 and the sum of the incidental expenses pertaining to
 regated and the total deducted (as it should be done)
 required as shown by the schedule, the remainder will
 within the proportion of the original estimates yet re-
 credit.

al year ending June 30, 1893, the sum of \$150,000, in-
 for West Channel along West Bay City and \$18,500
 r, can be profitably expended upon the improvement.

o June 30, 1891	\$503,331.78
al year (exclusive of outstanding liabilities).....	52,326.00
res to June 30, 1892	555,657.78

Summary statement

1. The Commission has conducted a comprehensive study of the
United States Fish and Wildlife Service and its operations.

2. The Commission has found that the Service is a highly
efficient and effective organization.

3. The Commission has identified several areas where
improvements can be made.

4. The Commission has recommended several changes
to improve the Service's operations.

5. The Commission believes that these changes will
result in a more efficient and effective Service.

Recommendations for improving operations

- 1. The Commission recommends that the Service should
conduct a comprehensive study of its operations.
- 2. The Commission recommends that the Service should
improve its financial management.
- 3. The Commission recommends that the Service should
improve its personnel management.
- 4. The Commission recommends that the Service should
improve its public relations.
- 5. The Commission recommends that the Service should
improve its information management.

Summary statement

The Commission has conducted a comprehensive study of the
United States Fish and Wildlife Service and its operations.
The Commission has found that the Service is a highly
efficient and effective organization.

The Commission has identified several areas where
improvements can be made.

The Commission has recommended several changes
to improve the Service's operations.

The Commission believes that these changes will
result in a more efficient and effective Service.

The improvements which were made under these
recommendations are as follows:

1. A dam was built across the bar at the mouth of the river. The
dam dug, and two dump scows, continued the four
arms of the proposed 200-foot channel, northward to t



12,300 feet from the front range light; brought the unfinished first, and cuts to the same terminus, and made a complete second and partial of the range, closing operations for the season of 1891 on November 19, 1891, the authority of the Chief of Engineers—the contract having been extended to the end of the fiscal year; completing the third cut east of range light, and extending the fourth cut to a point 11,200 feet distant from light. All excavated material was dumped in Saginaw Bay, about the outer end of the dredged channel. The result of the work of this contract may be summarized as follows:

Date.	Linear feet.	Cubic yards.
July 1-Sept. 21, 1891	5,859	17,821
Aug. 29-Sept. 21, 1891	1,332	3,734
Aug. 15-26, 1891	1,371	2,792
Aug. 22-25, 1891	364	578
Sept. 26-Nov. 7, 1891	8,540	16,550
Nov. 7-21, 1891	2,610	5,846
Apr. 21-June 8, 1892	4,724	13,661
June 8-30, 1892	4,320	15,000
		*75,982

which cost, at the contract price of 34 cents per cubic yard, the sum of \$25,832.88.

above Bay City.—Under contract with Thomas M. Hubbell, dredging at the beginning of the fiscal year with one dredge in the channel auker Bar at the head of Crow Island. This work was completed a channel 200 feet wide and 12 feet deep having been made in accordance with the project, by excavating 25,368 cubic yards of sand, at a cost of \$8,640. The same dredge was then employed in commencing the improvement across a shoal at the Bristol Street Bridge, in the city of Saginaw, for the allotment was made from the appropriation for Saginaw River improvement. Between September 17 and November 2, 1891, 13,019 cubic yards were excavated at an expense of \$4,686.84, resulting in a navigable channel 120 feet wide, and nearly half a mile in length. On the 17th of November, 1891, the same contractor furnished a second dredge, as he was required by the terms of the specifications. This dredge was used in improving the Saginaw River below Melbourne, and worked there continuously until November 27, 1891, when the season's operations were terminated on account of ice, and the contract was closed. On the 2d of November, 1891, the contractor joined the plant at Melbourne, and worked there to the end of the season. The operations at Melbourne resulted in making a navigable channel 120 feet wide, and about 2,575 feet long across the crest of the shoal immediately above Melbourne lumber docks, this partially removing the most serious obstruction to navigation in this locality. The dredges excavated 23,623 cubic yards, at a cost of \$8,044.28.

The amount of dredging done, under contract with Thomas M. Hubbell during the fiscal year, was 62,010 cubic yards, and its cost \$22,323.60.

Of the excavated material, principally clay, was put into dump at a distance of about 14 miles to the dumping ground in Saginaw Bay. A portion was transferred to the City Hall Dock and grounds, adjacent to the Bristol Street Bridge, in the city of Saginaw, and the balance dumped in the slips, and with the aid of an elevator dredge, redredged and used for the improvement of the wharves, and swamp land. The following schedule shows the disposition of:

	Cubic yards.
Dumping ground.....	19,948
City Hall, Saginaw.....	13,355
City Hall, at Crow Island.....	11,408
.....	9,601
.....	7,401
at Carrollton.....	297
.....	62,010

It is appropriate in this connection to note the fact that the selection of a place of deposit for the excavated material is a difficult problem to solve, and the operations for improving Saginaw River "above Bay City" are being made.

h was used almost daily, transferring materials and men engaged on the improvement, during examinations and surveys incident thereto, and on section by the assistant engineer between the city of Saginaw and the river. At the close of the season of 1891 the launch was laid up, and, United States property, placed in charge of a watchman at Zilwaukee. It was slightly damaged by the ice, and frequent pumping did not keep her at she was allowed to settle to the river bottom, in shoal water. Partly of this damage and partly of her having been found unsuitable and inadequate service required, owing to insufficiency of speed, it was deemed advisable of her at the opening of the season of 1892, and her sale at public order by the Chief of Engineers, under date of June 6, 1892. June 28, at was sold, accordingly, for the sum of \$250.

the months of January and February, 1892, ice surveys were made in accordance with your instructions dated January 11, 1892, at the following places in order:

at Bristol Street Bridge, in the city of Saginaw.

in front of Saginaw (east side) below the Genesee avenue and the Flint and Marquette Railroad bridges.

at the New York Works.

near the mouth of the river, between the light-houses and the village of

these surveys were made by me during the closing months of the fiscal year with computations of quantities of materials yet to be removed by order to complete the present approved project for improving Saginaw

the latest examinations of the different localities, I have prepared a detailed report showing the present condition and brief history of all the obstructions to navigation on Saginaw River, for the purpose of showing how complicated, difficult, and how the river improvement has been and will continue to be until such time as the project is considered satisfactorily and finally completed.

The approved project consists in providing a dredged channel 14 feet deep and 200 feet wide from Saginaw Bay to the Portsmouth Bridge at South Bay City, and thence a channel 12 feet deep and 200 feet wide to the head of navigation, a distance of 25

miles. The named reach of river has been entitled the "General improvement" and "above Bay City." This separation is due to the fact that the Saginaw River is divided into two Congressional districts, the Representatives of which have transferred a division of the appropriation for Saginaw River, and have the particularly specified in every river and harbor bill passed by Congress; so although there is but one general project for the improvement of the river, it is regulated by the terms of the appropriations, and forms two separate projects of the work.

A. GENERAL IMPROVEMENT.

Mouth.—This is unquestionably the most important part of the improvement, as it does for the passage of all vessels navigating the Saginaw River from Saginaw Bay to and from Saginaw Bay and Lake Huron. A channel of 14 feet deep and about 160 feet wide has thus far been made across the crest of the bar at the mouth of the river, to a point on the light-house range, the proposed channel, 12,300 feet from the front light, terminating in 13 feet depth; thence northward one dredge cut 14 feet deep and about 22 feet wide, and east of the axis, extends 3,800 feet farther to the 14-foot curve in Saginaw Bay.

The survey of the channel was made in January and February, 1891; it was in order to complete the width of the 14-foot channel to the projected 200 feet to the 14-foot curve—which is at a distance of 16,100 feet from the front light—it will be necessary to excavate 180,000 cubic yards, two-thirds of sand and one-half of this with only 1.5 feet depth of excavation. The examination also developed the fact that the crest of the bar along the side of the channel is slowly but constantly being raised by accumulation of sand and even sewage brought down by the current, causing the channel to cave in within a linear distance of about 100 feet.

The dredging records show that the second, third, and fourth dredge cuts range line have been and the last-named is now being dredged to 14 feet depth for the third time.

Ute Bar.—This is a shoal, located between the village of Essexville and Saginaw near the mouth of the river, and was not included in the original survey. It has come into notice within the last two years, during the prevailing low water. The first, last, and only examination (since the lake survey

mate of the probable cost of the improvement was \$1,442,500. If \$1,155,000 has been appropriated for the work and it was completed in 1885 at a cost of about \$975,000. Few improvements have resulted in greater benefit to the lake commerce, as by the infrequency of disasters in the vicinity since it became the following statement shows the various contracts in force during the year ending June 30, 1892, and the present condition of each:

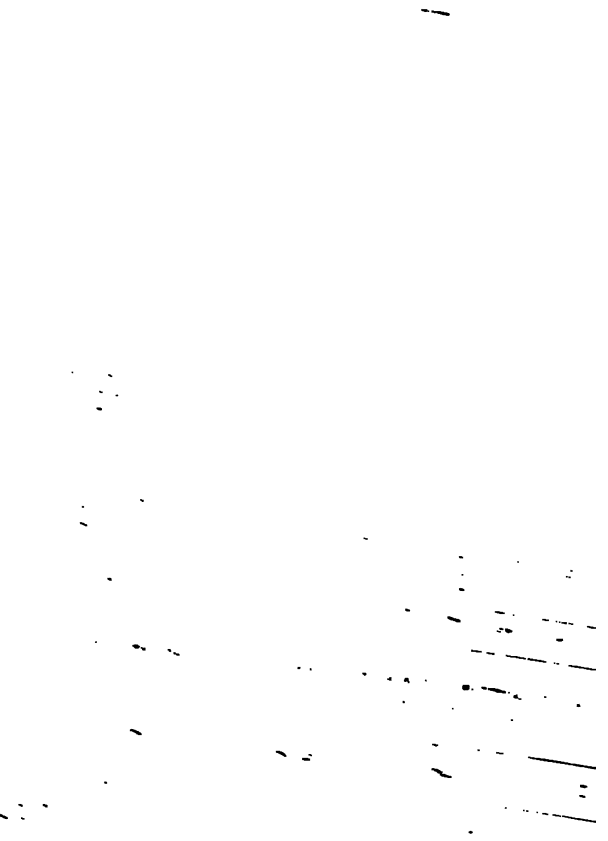
contractor.	Contract for—	Date of contract.	Remarks.
Mitchell	Hire of dredging plant	June 16, 1890	Closed June 30, 1892.
	Coal	Mar. 7, 1891	Closed Dec. 31, 1891.
& Co.	Ship chandlery	Mar. 26, 1892	Closed Apr. 30, 1892.

Contract with Chauncey E. Mitchell, dated June 10, 1890, for dredge and outfit by the hour, dredging was continued until the season of 1891 and resumed upon the opening of the 1892. The work done consisted in removing shoals in and about the harbor, the material to be excavated being composed of bowlders, and sandstone bed rock, very similar in character to the material removed in the previous season. All the dredging contemplated for the fiscal year was completed, except upon the shoal at the main entrance to the harbor, where additional dredge cuts were made across this from west to east. The work was stopped on June 30, 1892, because the limit of the excavation was reached on that day, and the dredge was then discharged. The amount of material removed during the fiscal year was 25,383 cubic yards measure, in 1,338 hours, 25 minutes, amounting to \$10,573.49 contract price of \$7.90 per hour of work, or nearly 41.7 cents per hour, or scow measure, for the material removed. The total number of hours worked under the contract from its beginning was 1,899½; the amount of material removed was 31,547 cubic yards, scow measure, and the total amount earned by the dredge was \$15,004.07, and the cost for the use of the dredging plant was a little more than 40 cents per cubic yard, scow measure. The apparent cost per hour for the excavation was increased by the time spent during the year in removing the wrecked barge *Col. Brackett*, for which it would be no equivalent in cubic yards.

By a diver were made to the main, west, and south piers during the season of 1891, prior to October 1. During the early part of 1892, the diver made a thorough examination of the pier foundations and reported the result in detail. The damage done by the worms was much less than usual, doubtless due to the measures taken during the season and to the unusual immunity from severe storms during the year. In this latter respect the winter was quite exceptional, and this fact, more than to any other, may be attributed to the present condition of the structures and freedom from necessity for extensive repairs.

The diving crew was reorganized June 1, 1892, and from that time to the close of the fiscal year was employed in repairing the damaged pier foundations, using for this purpose bowlder stone purchased in the market for the low price of \$5 per cord.

The pier now appears to be in a less satisfactory condition than in the past, and extensive repairs must soon be made to the portion of the pier. The timbers are badly decayed, and the whole super-



vessels taking shelter in the harbor of refuge, Sand Beach, Lake Huron, Michigan.

calendar year.	Steam.	Sail.	Tow.	Total.	Total number of vessels.	Average tonnage.
	<i>Tons.</i>	<i>Tons.</i>	<i>Tons.</i>	<i>Tons.</i>		
.....	63,966	27,699	50,954	142,619	493	289
.....	104,025	39,699	99,282	243,006	781	311
.....	133,060	45,750	100,096	278,926	921	303
.....	158,720	55,630	147,260	361,610	1,317	275
.....	144,645	55,960	127,855	328,460	1,176	279
.....	146,132	26,504	114,067	286,703	1,022	280
.....	177,122	32,713	114,091	323,926	1,139	284
.....	156,518	34,724	122,980	314,222	1,142	275
.....	196,364	29,426	151,607	377,397	1,158	325
.....	196,355	33,790	140,862	370,987	1,304	284
.....	271,327	33,689	153,087	458,103	1,447	317
.....	271,917	39,756	202,191	513,864	1,624	316
.....	289,719	37,922	165,896	493,537	1,512	326
.....	325,852	38,826	169,104	533,783	1,575	339
.....	292,917	27,076	171,067	491,059	1,341	366
.....	2,928,639	559,164	2,080,399	5,518,292	17,952	307

L L II.

IMPROVEMENT OF BLACK RIVER AT PORT HURON, MICHIGAN.

Improvements were commenced the lower reach of Black River, mouth to the Grand Trunk Railroad Bridge, a length of nearly varied in width from 120 to 150 feet, and had a navigable depth of from 10 to 14 feet in depth, except at a few points where it obstructed by shoals or bars having only 8½ feet of water over

the river and harbor act of August 11, 1888, called for an examination of Black River at Port Huron, to deepen channel from mouth to Grand Trunk Railroad Bridge to depth of 18 feet." After a preliminary examination of the locality had been made I reported that the channel within the limits indicated, while not worthy of improvement to a depth of 18 feet, was worthy of improvement to a depth of 15 feet and a survey was consequently authorized to obtain data on which to base a project for this latter depth. As a result of this survey an estimate of \$55,110 was made on May 9, 1889, for improving the channel of the river in question to a depth of 15 feet.

The river and harbor act of September 19, 1890, appropriated \$25,000 for improving Black River at Port Huron, Mich., to deepen channel from mouth to Grand Trunk Railroad Bridge to depth of 16 feet," 1 foot greater depth than that for which the estimate had been made. A project for the improvement of the river to the depth named in the appropriation was therefore submitted in January, 1891, and was approved. It contemplates excavating a channel 16 feet deep from mouth of the river to the Grand Trunk Railroad Bridge, and varying between 160 and 75 feet, according to locality. On the 1st of the greater depth the estimate for the work was increased to \$100,000.

At the beginning of the fiscal year a contract with Chauncey E. Smith for dredging was in force, under a project for the expenditure of \$100,000 appropriated for the work by the river and harbor act of September 19, 1890. The work proposed to be accomplished was to

OF THE CHIEF OF ENGINEERS, U. S. ARMY.

el two dredge-cuts wide and 16 feet deep from the
er upstream as far as the funds available would per-
were more than sufficient to carry the two cuts to the
broad Bridge, then to widen the lower reach of the
funds were exhausted, all in furtherance of the gen-
ject. This contract has been extended to December 1, 1892.
; has been in progress throughout the fiscal year and is not
ted. It was stopped for the season on November 28, 1891.
resumed April 16, 1892. On June 30, 1892, a channel 16 feet deep
ended from the mouth of the river towards the Grand Trunk Rail-
d Bridge, the whole length being about 6,200 feet. Its width from
mouth of the river to Military Street Bridge, a distance of about
0 feet, was 75 feet, and it was 50 feet wide for the further distance
-bout 4,500 feet.

		Cubic yards
Amount of material removed prior to 1891	1891	18,225
Amount of material removed during 1892	73,048
Total amount of excavation to June 30, 1892	1892	91,273
Amount of examination and survey prior to June 30, 1891, and expenditures and obligations during 1892	Provision for improvement	\$731.50
.....	2,389.11
.....	12,965.38
Total cost to June 30, 1892	16,665.99
Estimated cost of the improvement authorized by act of September 19, 1890	75,000.00
.....	25,000.00
Amount to credit of estimate	50,000.00

Of the balance remaining to the credit of the estimate the sum of \$25,000 can be advantageously expended during one fiscal year.

Black River, at Port Huron, is in the collection district of Huron, Mich. The nearest port of entry is Port Huron, through the midst of which the river runs, and the nearest light-house is Fort Gratiot Light, distant about 2 miles.

Money statement.

July 1, 1891, balance unexpended	\$23,831.25
June 30, 1892, amount expended during fiscal year	9,456.75
July 1, 1892, balance unexpended	14,374.50
July 1, 1892, outstanding liabilities	\$2,309.88
July 1, 1892, amount covered by uncompleted contracts	12,064.62
	14,374.50
Amount appropriated by act approved July 13, 1892	10,000.00
{ Amount (estimated) required for completion of existing project	40,000.00
{ Amount that can be profitably expended in fiscal year ending June 30, 1894	25,000.00
{ Submitted in compliance with requirements of sections 2 of river and harbor acts of 1866 and 1867.	

Appropriation for improving Black River, Michigan, at Port Huron, etc.

September 19, 1890

COMMERCIAL STATISTICS.

Entered during calendar year, 1891.

Commodities.	Amounts.	Tons.
..... feet, B. M.	18,000,000	37,000
..... pieces	2,908,000	285
..... do	1,800,000	100
..... do	6,000	125
..... do		12,000
..... cords	7,000	14,500
..... do		180
..... cords	700	4,700
..... do	5,000	10,000
..... pieces	400,000	9,800
.....		80,885

is were made from the river.

L L 12.

IMPROVEMENT OF MOUTH OF BLACK RIVER, MICHIGAN.

er empties into St. Clair River, at Port Huron, Mich. At its mouth, extending beyond the middle of St. Clair River, bar, or Middle Ground. Under former appropriations this dredged to a clear depth of 15 feet. The main channel of St. Clair is found between the Middle Ground and the Canadian

deposit of material brought down by the current of Black River. The depth on the bar had been reduced an average of about 6 feet. In conjunction with a low stage of water, became an impediment to navigation along the front of the city of Port Huron in

conjunction with projects approved in 1889 and 1891 it was proposed to improve the bar by making a series of dredge cuts, of a uniform width of 16 feet, along the dock front of Port Huron, from the wharf above to the same curve below the mouth of Black River, at a distance of about 50 feet from the wharves, and the work to continue outward as far as the available funds would

allow. In the project of 1889 (act of August 11, 1888) the bar was dredged to the full depth of 16 feet, for a width of 300 feet, the amount of material removed being 46,239 cubic yards, scow measurement.

In the project of 1891 (act of September 19, 1890) a contract was made on November 11, 1890, with the Bay City Dredging Company, for dredging with a dredge at the beginning of the fiscal year, and work was continued until November 14, 1891, when it ceased on account of the expiration of the appropriation. The amount of material removed during the fiscal year was 40,011 cubic yards, scow measurement, and the total under this contract was 57,515 cubic yards. The result was a clear depth of 20 feet to the width of the improvement, thus making it a

total amount of material removed under the two projects was 96,250 cubic yards, scow measure. The dredging should be continued



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Tonnage of vessels taking shelter in the harbor of refuge, Sand Beach, Lake Huron, Michigan.

Calendar year.	Steam.	Sail.	Tow.	Total.	Total number of vessels.	Average tonnage.
	<i>Tons.</i>	<i>Tons.</i>	<i>Tons.</i>	<i>Tons.</i>		
.....	63,966	27,699	50,954	142,619	493	289
.....	104,025	39,699	99,282	243,006	781	311
.....	133,080	45,750	109,096	278,926	921	303
.....	158,730	55,630	147,290	361,619	1,317	275
.....	144,645	55,960	127,855	328,460	1,176	279
.....	146,132	26,504	114,067	286,703	1,022	280
.....	177,122	32,713	114,091	323,926	1,199	284
.....	156,518	34,724	123,980	314,222	1,143	275
.....	196,264	29,426	151,607	377,397	1,158	325
.....	196,335	33,790	140,862	370,987	1,204	284
.....	271,327	33,689	153,087	458,103	1,447	317
.....	271,917	39,756	202,191	513,864	1,624	316
.....	289,719	37,922	165,896	493,537	1,512	326
.....	325,852	38,826	169,104	533,783	1,575	339
.....	292,917	27,076	171,067	491,059	1,341	366
.....	2,928,639	559,164	2,030,399	5,518,202	17,952	307

L L II.

IMPROVEMENT OF BLACK RIVER AT PORT HURON, MICHIGAN.

The improvements were commenced at the lower reach of Black River, from the mouth to the Grand Trunk Railroad Bridge, a length of nearly 1½ miles, varied in width from 120 to 150 feet, and had a navigable channel of from 10 to 14 feet in depth, except at a few points where it was obstructed by shoals or bars having only 8½ feet of water over

the river and harbor act of August 11, 1888, called for an examination of "Black River at Port Huron, to deepen channel from mouth to Grand Trunk Railroad Bridge to depth of 18 feet." After a preliminary examination of the locality had been made I reported that the depth within the limits indicated, while not worthy of improvement to a depth as 18 feet, was worthy of improvement to a depth of 16 feet, and a survey was consequently authorized to obtain data on which to base a project for this latter depth. As a result of this survey an estimate of \$55,110 was made on May 9, 1889, for improving the river to the depth of 15 feet.

The river and harbor act of September 19, 1890, appropriated \$25,000 for "improving Black River at Port Huron, Mich., to deepen channel from mouth to Grand Trunk Railroad Bridge to depth of 16 feet," 1 foot greater depth than that for which the estimate had been made. A project for the improvement of the river to the depth named in the appropriation was therefore submitted in January, 1891, and was approved. It contemplates excavating a channel 16 feet deep from the mouth of the river to the Grand Trunk Railroad Bridge, and of a width varying between 160 and 75 feet, according to locality. On account of the greater depth the estimate for the work was increased to \$100,000.

At the beginning of the fiscal year a contract with Chauncey E. L. L. for dredging was in force, under a project for the expenditure of \$100,000 appropriated for the work by the river and harbor act of September 19, 1890. The work proposed to be accomplished was to

OF THE CHIEF OF ENGINEERS, U. S. ARMY.

for operating and care of Saint Clair Flats Canal, Michi

cal year ending June 30—

1890
1891
1892

Total

Itemized statement of expenditures incurred on account of appropriation for operating and care of canals and other works of indefinite, applied to operation of Saint Clair Flats Canal, Michigan for the fiscal year ending June 30

Date.	No. of voucher.	From whom purchased.	Articles.
1891.		<i>Part of first quarter, 1891.</i>	
Aug. 1	2	Payroll, July, 1891	1 custodian
Sept. 1	3	Payroll, August, 1891	1 oarsman
			1 custodian
			1 oarsman
		<i>Second quarter, 1891.</i>	
Oct. 2	1	Payroll, September, 1891	1 custodian
Nov. 2	2	Payroll, October, 1891	1 oarsman
Dec. 2	3	Payroll, November, 1891	1 custodian
			1 oarsman
			2 carpenters, 10 days, at \$2 per day
31	4	Payroll, December, 1891	1 custodian
			1 laborer, 28 days, at \$50 per month
1892.		<i>Third quarter, 1892.</i>	
Jan. 31	1	W. H. Mott, custodian	Services as custodian at St. Clair Flats Canal, Michigan, from January 1 to January 31, 1892 (both days inclusive), being one month.
Feb. 29	2	W. H. Mott	Services as custodian at St. Clair Flats Canal, Michigan, from February 1 to February 29, 1892 (both days inclusive), being one month.
Mar. 31	3do	Services as custodian at St. Clair Flats Canal, Michigan, from March 1 to March 31, 1892 (both days inclusive), being one month.
		<i>Fourth quarter, 1892.</i>	
May 4	1	Payroll, April, 1892	1 custodian
June 10	2	Payroll, May, 1892	1 custodian
		<i>Month of July, 1892.</i>	
July 1	1	Payroll, June, 1892	1 custodian
			2 sub-inspectors, 24 days, at \$90 per month.
			Total

L L 15.

IMPROVEMENT OF CLINTON RIVER, MICHIGAN.

In 1870 the channel over the bar at the entrance to this river afforded a depth of only 3½ feet, while the depth in the river for some distance above was 10 feet.

A project for dredging a channel across the bar was approved and carried into effect in 1870. A project for the general improvement of the river from its mouth to the city of Mount Clemens was submitted in 1880 and renewed and approved in 1889. It contemplates a channel of deep and of navigable width for the entire distance of about 2½ miles from the mouth of the river to Mount Clemens. Involved in this project is the closing of a gap opposite Mount Clemens and of Catfish (or Red) Channel; also closing the main channel at, and making a straight cut across, Shoemakers Bend, constructing a revetment on the north side of the mouth from the shore to the requisite depth in Lake St. Clair, and dredging wherever necessary to attain the desired depth, the estimated cost of the improvement being \$32,926.

Prior to the beginning of the fiscal year the work at Shoemakers Bend had been completed; the gap opposite Mount Clemens had been closed, and some dredging had been done at the mouth of the river and at various shoals between there and Mount Clemens.

At the beginning of the fiscal year the only contract in force was one with Mr. George Lockerbie, dated February 13, 1891, approved March 1, 1891, for dredging. Under this contract operations had been begun on May 1, 1891, and were continued until October 4, 1891, when the funds were exhausted and the work ceased.

The result was a channel 8 feet deep and 75 feet wide or more, from the mouth of the river to the bridge at Mount Clemens. Although the channel can not be considered permanent, yet no complaint of its deterioration has reached this office up to the date of this report.

	Cubic yards.
Volume of material removed during the fiscal year was	23,374
and the total volume under this contract was	38,401
	<hr/>
Actual expenditures to June 30, 1891	\$45,405.06
expended during fiscal year	6,006.54
	<hr/>

Total expenditures to June 30, 1892

51,411.60
Eight thousand five hundred and sixty-four dollars and fourteen cents still remain to be appropriated to complete the improvement in accordance with the existing project, and in view of the relief already afforded the river is worthy of the additional appropriation. This amount can be profitably expended during the fiscal year ending June 30, 1893.

But the good effect of the dredging already done is liable to be destroyed by deposits of material brought down by annual freshets in the river, for which reason the foregoing estimate can only be considered as approximate for the fiscal year, and is probably too low if the work be deferred beyond that time.

The proposed navigable depth having been once obtained, no charges for maintenance should be made against it. The balance named is expended only for the construction of the revetment on the north side of the river at the mouth. The good effect of the dredging already done is liable to be impaired by deposits of material brought down by annual

freshets in the river, and the cost of restoring the depth shoal chargeable to future estimates and appropriations thereunder.

Clinton River is in the collection district of Detroit, Mich. The nearest entry is Detroit. The nearest light-houses are those at St. Clair Flats.

Money statement.

July 1, 1891, balance unexpended	\$7
June 30, 1892, amount expended during fiscal year	7
July 1, 1892, balance unexpended	
Amount appropriated by act approved July 13, 1892	8
Amount available for fiscal year ending June 30, 1893	8

Appropriations for improving Clinton River, Michigan.

August 30, 1852	\$5,000	August 5, 1886	
July 11, 1870	5,000	August 11, 1888	
March 3, 1871	1,500	September 19, 1890	
March 3, 1881	8,000		
August 2, 1882	6,000	Total	

COMMERCIAL STATISTICS.

Articles entered and cleared during the season of 1891.

Articles entered.	Amount.	Tons.	Articles cleared.	Amount.
Lumber	7,200,000	14,000	Flour	1,800
Coal	10,500	10,500	Oats	5,000
Salt	800	110	Staves	6,000,000
Cement	500	50	Headings	6,000,000
Stone	1,600	9,000	Total	
Iron		650		
Logs	8,000,000	16,000		
Shingles	4,000,000	500		
Laths	6,000,000	520		
Heading bolts	5,000	10,000		
Total		61,330		

Number of vessels cleared during the season of 1891.

Steam	
Sail	

L L 16.

IMPROVEMENT OF GROSSE POINTE CHANNEL, MICHIGAN.

Between the lower end of St. Clair Flats and the deep water troit River, the only known obstruction to navigation is the shoal off Grosse Pointe, known as Grosse Pointe Flats. At or stages of water vessels drawing 16 feet can cross this obstructi

When the water is as low as it has been during the last few years, vessels drawing more than 15 feet can pass only with great care and difficulty.

The river and harbor act of August 11, 1888, appropriated \$75,000 "improving St. Clair Flats Ship Canal, * * * all or any portion of which may, in the discretion of the engineer, be expended in dredging Grosse Pointe Channel." Five thousand dollars was consequently reserved from this appropriation for the removal of any small and well-defined obstruction that might be found at Grosse Pointe, as well as for making such surveys as might be necessary before making a definite project. A small shoal was removed in July, 1889, but no other such obstructions have since been found.

As the improvement of other connecting channels of the Great Lakes begins to approach the 20-foot depth, which it is recognized that they would ultimately have, the annoyance to shipping at Grosse Pointe must increase, and the necessity for a channel at this point is urgent. The number of vessels annually crossing these flats is enormous, and to insure a thoroughly satisfactory result a channel 800 feet wide and nearly 5½ miles long should be dredged. The material to be excavated consists of sand, gravel, and clay, and the approximate estimate of the cost of this work is as follows:

Dredging 2,889,472 cubic yards, at 30 cents	\$869,841.60
and 10 per cent for contingencies	86,984.16
Total	956,825.76

In an estimate submitted February 10, 1888, I placed the cost of a channel 19½ feet deep at \$553,000, but remarked that to "gain an additional half foot would materially increase the cost." That estimate was based upon a price of 20 cents per cubic yard for dredging. For a channel 20 feet in depth the length of the channel is increased nearly 5 miles, over which the work would be merely "scraping," and therefore disproportionately costly.

But the full depth of 20 feet should be obtained at whatever cost. The commerce which passes this point already exceeds 20,000,000 tons annually, and steps can not be taken too soon for its accommodation.

To accomplish valuable results a large appropriation will be required at first, and unless this be granted the results will be unsatisfactory, and the eventual cost will be greatly increased. With \$300,000 available, a narrow channel of the proposed depth can be opened, and will afford immediate though insufficient relief. Its width can subsequently be increased to such an extent as may be necessary.

This improvement is both necessary and urgent, and will benefit largely the entire commerce of the lakes.

Grosse Pointe Flats are in the collection district of Detroit, Mich. Windmill Point Light-house and range lights are in close vicinity.

Money statement.

July 1, 1891, balance unexpended	\$3,844.05
July 1, 1892, balance unexpended	3,844.05

Amount (estimated) required for completion of existing project	956,825.76
Amount that can be profitably expended in fiscal year ending June 30, 1891	300,000.00
Submitted in compliance with requirements of sections 2 of river and harbor acts of 1866 and 1867.	

L L 17.

IMPROVEMENT OF ROUGE RIVER, MICHIGAN.

Improvements were begun Rouge River had a channel of 100 feet over the bar at its mouth, and from 10 to 17 feet thence to the mouth of the St. Louis and Wabash Railroad, a distance of nearly 15 miles. In earlier days vessels had ascended it to Dearborn, a distance of about 15 miles.

The approved project for improvement contemplates dredging the river to a depth of 16 feet and width of 240 feet at the mouth, gradually narrowing to 100 feet at a distance of about 1,150 feet above, and in continuing this width to the bridge of the St. Louis and Wabash Railroad.

On June 30, 1891, the dredged channel was 16 feet deep, 240 feet at the mouth, gradually narrowing to 100 feet at a distance of about 1,150 feet above; thence to the bridge of the Michigan Central Railroad bridge, a total distance of about 1,500 feet, it averaged 70 feet in width, and was 100 feet wider at the curves. The length of the dredged channel was about 12,650 feet.

No funds were available during the fiscal year ending June 30, 1892, for which reason no work was done, and some shoals have since been formed in places where the channel already dredged.

To complete the improvement, it is estimated that an appropriation of \$1,039, that being the difference between the original estimate and the amount appropriated, will require the sum of \$1,039, that being the difference between the original estimate and the amount appropriated. In view of the fact that the aggregate of the sums thus appropriated for the improvement of the river is \$1,039, and the aggregate number of manufacturing establishments along its banks, and its availability as a winter harbor for vessels, the river is well worthy of the additional appropriation necessary to complete the project for its improvement. The whole amount can be profitably expended in one fiscal year.

Amount of original estimate.....	\$31,68
Amount expended to June 30, 1892.....	\$20,483 28
Less cost of surveys and examinations prior to making the estimate.....	616 45
Amount actually expended upon improvement.....	19,86
Balance to credit of estimate.....	11,82
Balance on hand.....	15
Amount to be appropriated.....	11,66

This work is in the collection district of Detroit, Mich. The nearest light-house is Grassy Island, and the nearest fort is Fort Wayne, each of which is in sight of the work.

Money statement.

July 1, 1891, balance unexpended.....	13
July 1, 1892, balance unexpended.....	13
Amount appropriated by act approved July 13, 1892.....	11,66
Amount available for fiscal year ending June 30, 1893.....	11,88

Appropriations for improving Rouge River, Michigan.

August 11, 1888.....	\$1
September 19, 1890.....	1
Total.....	2

COMMERCIAL STATISTICS.

Commercial statistics have not been obtained for the calendar year 1891, but they are practically the same as those reported in the preceding Annual Report for 1890, wherein they were stated in detail. The freight tonnage for 1890 was 311,000 tons.

L L 18.

IMPROVEMENT OF DETROIT RIVER, MICHIGAN.

Originally the channel at Limekiln Crossing, Detroit River, could be depended upon for more than 13 feet of water, the ordinary depth being much affected by the direction of the wind. As originally projected in 1874 the improvement at this point was to consist of a dredged channel 300 feet wide, with a uniform depth of 20 feet, and the original estimate was based upon this project.

In 1883 it was wisely determined to so modify the project as to secure a straight channel, the least width of which should be 300 feet, with a somewhat greater width at each end, utilizing the work already done.

In 1886 this was further modified to the end that the width of the channel should be increased to 400 feet by removing an additional 100 feet from the western (American) side; and in 1888 a further additional width of 40 feet on the western side was authorized, as the low bid under the final appropriation was so low that the money in hand would pay for the increased excavation.

The estimated cost of a 400-foot channel was \$1,374,500. The total amount expended up to June 30, 1891, was \$702,122.04, and the result has been a channel 440 feet in width, thus accomplishing one-tenth more work than was estimated for at a cost of but little more than half the estimate. Gratifying as this is, the benefit to commerce is far beyond the amount of the original estimate, and since the excavation is through rock the improvement will be permanent.

The only work in progress during the fiscal year was the preparation of a map of the water front of Detroit River, from its head, at Windmill Point, to a point about 2 miles below the River Rouge, a total distance about 13 miles. For this purpose the total sum expended to June 30, 1892, was \$64.48.

Water-gauge observations were made during the months of May and June, 1892, at the light-house depot, Detroit, and at Windmill Point Light-House. Their cost is included in the above statement of expenditures.

Total expenditure to June 30, 1891.....	\$702, 122. 04
Expended during fiscal year	64. 48
Total expenditure to June 30, 1892	702, 186. 52

The project for Limekiln Crossing having been completed, no further estimate is submitted for work at that point. The extreme low water of the last few years, however, has caused vessels to strike on a number of shoals in different parts of the river, whose existence had not previously been known; and, in view of the magnitude of the commerce on the river, all such shoals should be removed as soon as possible. As no information on this subject is at hand, an accurate estimate of work can not be made, but \$50,000 would probably suf-

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for the removal of the more serious of the shoals between the of Detroit and Lake Erie, and an estimate of that amount is there submitted and strongly recommended.

The work is in the collection district of Detroit, Mich. The nearest port of is Detroit. The nearest United States light-houses are Mammy Judy and the lights at the head of Grosse Isle, about 5 miles distant.

Money statement.

July 1, 1891, balance unexpended.....	\$98
June 30, 1892, amount expended during fiscal year.....	0
July 1, 1892, balance unexpended.....	92
Amount appropriated by act approved July 13, 1892.....	30, 00
Amount available for fiscal year ending June 30, 1893.....	30, 92
{ Amount (estimated) required for completion of existing project.....	20, 00
{ Amount that can be profitably expended in fiscal year ending June 30, 1894.....	20, 00
{ Submitted in compliance with requirements of sections 2 of river and harbor acts of 1866 and 1867.	

Appropriations for improving Detroit River, Michigan.

June 23, 1874.....	\$25, 000	July 5, 1884.....	\$200
June 18, 1878.....	100, 000	August 5, 1886.....	35
March 3, 1879.....	50, 000	August 11, 1888.....	130
June 14, 1880.....	50, 000		
March 3, 1881.....	50, 000	Total.....	705
August 2, 1882.....	60, 000		

COMMERCIAL STATISTICS.

Commerce of Detroit River during the season of 1891, comprising staples only and such as cleared from United States ports.

Commodities.	Amounts.	Tons.	Commodities.	Amounts.	To
Iron ore and finished iron.....		6, 855, 097	Stone.....		2
Coal.....		6, 335, 841	Provisions..... hhd.	60, 000	
Wheat..... bushels.....	79, 184, 912	2, 363, 180	Merchandise..... pkgs.	6, 618, 296	1, 8
Flour..... barrels.....	9, 847, 343	984, 734	Salt..... barrels.....	408, 361	1
Corn..... bushels.....	40, 288, 760	1, 151, 167	Lumber..... feet. B. M.	1, 122, 683, 000	2, 2
Oats..... do.....	19, 403, 828	336, 398	Laths..... pieces.....	22, 462, 000	
Barley..... do.....	12, 193, 053	292, 633	Shingles..... do.....	82, 462, 000	
Flaxseed..... do.....	8, 665, 303	236, 295	Silver ore.....		
Malt..... do.....	73, 000	1, 460	Cement..... barrels.....	692, 956	
Oil cake..... sacks.....	24, 000	2, 400	Telegraph poles..... pieces	51, 639	
Copper ore.....		69, 000	Total.....		23, 3

Number of vessels, 34,251; registered tonnage, 22,160,000, exclusive of Can. vessels. If the latter could be included, and the list of commodities made to in all that were carried, whether cleared from a custom-house or not, the total v be much greater.

Total number of vessels and tonnage cleared from all the collection districts on the of lakes.

Vessels.....	5
Registered tonnage.....	32, 48

Number of loaded cars that crossed Detroit River during 1891.

Not bound.....	199, 173
At bound.....	120, 956
Total.....	320, 129
Wreckage at an average of 12 tons per car.....	3, 841, 548
Wreckage at an average of 15 tons per car.....	4, 801, 956

L L 19.

MOVING SUNKEN VESSELS OR CRAFT OBSTRUCTING OR ENDANGERING NAVIGATION.

Wreck of scow Hannah Moore.—The unladen wooden scow *Hannah Moore* became water-logged and sunk in St. Clair River on July 3, 1891. She lay off Port Huron, Mich., in about 16 feet of water, nearly at right angles to the channel, on her beam ends, her two masts pointing upstream. She was about 450 feet from the Flint and Pere Marquette Railroad Dock, and 1,250 feet below Black River. Her dimensions were approximately as follows: Gross tonnage, 74.23; length, 88 feet; breadth 20 feet; depth, 6 feet 4 inches. She was built, in 1868, at St. Clair, Mich., and hailed from Detroit, Mich.

When it became apparent that the owner of the scow did not propose to remove her, a project for her removal at an estimated cost of \$1,000, accompanied by the necessary specifications, was submitted to the Chief of Engineers. This project having been approved, the thirty days' notice required by law was published on August 22, 1891, and specifications were issued.

Proposals for removing the wreck were opened on September 21, 1891, and on September 28, 1891, a contract was duly entered into with the lowest bidder, Mr. Chauncey E. Mitchell, for the lump sum of \$880.

The work of removal was begun October 15. The scow was broken into two pieces by dynamite, and on October 21 and 23 the two portions were raised and towed to a point near the shore, about 2 miles below Black River. On November 9 all the remaining portions of the wreck were removed satisfactorily, and on November 16 the contract was closed. The total cost to the Government, including superintendence, printing, advertising, etc., was \$902.10. No articles of value were recovered.

This work was in the collection district of Huron, Mich. The nearest port of entry was Port Huron, and the nearest light-house was Fort Gratiot Light, at the end of St. Clair River.

Abstract of bids for the removal of the wreck of the scow Hannah Moore from the St. Clair River, opposite the Flint and Pere Marquette Railroad Dock, Port Huron, Mich., received and opened on September 20, 1891, in accordance with advertisement dated August 22, 1891.

Name and address of bidder.	Price.	Remarks.
Chauncey E. Mitchell, Detroit, Mich. *	\$880	Will use dynamite. Dredge and lighters to be used when wreck will not float, after blasting.*
Enoch Townsend, Somers Point, N. J. . .	980	Remove said vessel by dynamite, and pieces if necessary will lighter.
Elijah Dunbar, Detroit, Mich.	1, 490	Will use dynamite to break the vessel in pieces, and land same on bank of river with tug.

* Recommended for acceptance.



APPENDIX M M.

IMPROVEMENT OF RIVERS AND HARBORS ON LAKE ERIE WEST OF ERIE, PENNSYLVANIA.

REPORT OF LIEUTENANT-COLONEL JARED A. SMITH, CORPS OF ENGINEERS,
OFFICER IN CHARGE, FOR THE FISCAL YEAR ENDING JUNE 30, 1892,
WITH OTHER DOCUMENTS RELATING TO THE WORKS.

IMPROVEMENTS.

- | | |
|--------------------------------|--|
| 1. Lake Erie Harbor, Michigan. | 8. Black River Harbor, Ohio. |
| 2. Lake Erie Harbor, Ohio. | 9. Cleveland Harbor, Ohio. |
| 3. Clinton Harbor, Ohio. | 10. Fairport Harbor, Ohio. |
| 4. Sandusky City Harbor, Ohio. | 11. Ashtabula Harbor, Ohio. |
| 5. Sandusky River, Ohio. | 12. Removing sunken vessels or craft ob- |
| 6. Sandusky Harbor, Ohio. | structing or endangering naviga- |
| 7. Sandusky Harbor, Ohio. | tion. |

EXAMINATIONS AND SURVEYS.

- | | |
|--|----------------------------|
| 1. Sandusky River, Ohio, between Rich- | 14. Conneaut Harbor, Ohio. |
| mond and the mouth. | |

UNITED STATES ENGINEER OFFICE,
Cleveland, Ohio, July 9, 1892.

SIR: I have the honor to transmit herewith, in duplicate, annual reports for the year ending June 30, 1892, upon the improvement of rivers and harbors in my charge.

Very respectfully, your obedient servant,

JARED A. SMITH.
Lieutenant-Colonel, Corps of Engineers.

2. Gen. THOMAS L. CASEY,
Chief of Engineers, U. S. A.

M M I.

IMPROVEMENT OF MONROE HARBOR, MICHIGAN.

This improvement was commenced in the year 1835, at which the Raisin River was considered an important stream, and Monroe a place of some prominence. The plan of improvement consisted in straightening the river and making direct connection with Lake St. Clair by a canal 4,000 feet long and 100 feet wide, through a sand peninsula.

A description in detail of the earlier operations heretofore carried on for the improvement of this harbor will be found in Annual Report 1880 and 1881.

Some minor repairs to the piers and revetment of canal were made in the fiscal year ending June 30, 1891, but nothing has been done during the last fiscal year, and the project for repairs remains untouched.

A project for a general repair of the piers and revetment of canal was adopted in 1886, the cost at that time being estimated at \$20,000. The general repairs have been undertaken owing to lack of funds for such a purpose. Of course the condition of the superstructure of piers and the canal revetment is constantly becoming worse. The amount estimated as necessary for the general repairs in 1891 was \$26,000. This estimate appears to have been a general one only, as it is not easy to determine in advance the exact extent to which the work must be renewed.

As the officer in charge of the work has had no opportunity to make a critical examination of the situation, the last estimate is here repeated.

The annual report of last year gave the least depths of water at Monroe, June, 1890, as 9 feet to lower docks and thence to upper docks 2 feet. No examination for channel depths has been made since the one mentioned.

The following is a statement of the amount and date of all appropriations for this improvement:

February 24, 1835.....	\$30,000.00	August 14, 1876.....	\$5.00
July 2, 1836.....	15,000.00	June 18, 1878.....	2.50
March 3, 1837.....	30,000.00	March 3, 1879.....	2.00
July 7, 1838.....	15,000.00	June 14, 1880.....	2.00
June 11, 1844.....	20,000.00	March 3, 1881.....	1.00
August 30, 1852.....	14,000.00	August 2, 1882.....	1.00
June 23, 1866.....	31,015.27	August 5, 1886.....	2.00
June 10, 1872.....	10,000.00	August 11, 1888.....	5.00
March 3, 1873.....	15,000.00	September 19, 1890.....	5.00
June 23, 1874.....	10,000.00		
March 3, 1875.....	10,000.00	Total.....	225.50

The commercial statistics are not sufficiently reliable to make year to year comparisons.

Apparently the tonnage for last fiscal year was about 25 per cent less than in the year previous. The principal imports were telegraph poles and the exports a small amount of fish and grapes. These are not of a kind to promise a great increase in the future.

The harbor of Monroe is in the collection district of Detroit, Mich. There is a light-house on the outer end of the west pier.

No new lines of transportation have been established in the last fiscal year.

Money statement.

Amount appropriated by act approved July 13, 1892..... \$10,

{	Amount (estimated) required for completion of existing project.....	16.
	Amount that can be profitably expended in fiscal year ending June 30, 1894.....	16.
	Submitted in compliance with requirements of sections 2 of river and harbor acts of 1866 and 1867.....	

COMMERCIAL STATISTICS.

Following statistics for the year 1891, relative to the commerce of Monroe Michigan, were compiled from information furnished by the collector of and others:

telegraph poles	tons..	18,000
fish and grapes	do...	22
entering		83
departing		84
value not known.		
value for 1891 (estimated).....		20,000
value given in last annual report		27,000

depth of the largest vessel using harbor is 12 feet.

depth of water in the harbor prevents the largest vessels from loading to full

number of lines of transportation have been established during the year.

M M 2.

IMPROVEMENT OF TOLEDO HARBOR, OHIO.

The city and harbor of Toledo, Ohio, are near the mouth of the Maumee River. The harbor occupies some distance on the river, and the depth of water at the mouth of the river is about 5 miles from the mouth, where it empties into Maumee Bay. From the mouth of the river to 17 feet depth of water at Toledo is a little more than 7 miles.

The history of the earlier operations carried on in past years for the improvement of this harbor will be found in the Annual Reports of 1881, and 1883.

The project for the "old channel" has been amended from time to time since 1866. The latest plan provides for a channel 200 feet wide at the bottom and 16 feet deep through Maumee Bay to Lake Erie.

A considerable part of the cost is due to the deposits of silt from the river and the filling caused by seas in the bay.

The old channel is now about 100 feet wide on the bottom and 260 feet deep at the bends, with a least depth of 15.6 at the mean level of the lake. This depth has been obtained by dredging in a long course of years, with a total expenditure to June 30, 1892, amounting to \$724,332.61.

The project of the old channel is now included in the straight channel project from Maumee Bay to deep water of Lake Erie.

By an act of July 5, 1884, appropriated \$25,000 to commence the work on the project of a straight channel for the Maumee River from its mouth to Toledo, Ohio, and the act of August 5, 1886, provided for continuing the project "by a straight channel along such line as may be approved by the Secretary of War."

The project for the work was approved by the Secretary of War April 1, 1887.

The project consisted in dredging a straight channel through Maumee Bay 100 feet wide at the bottom and 17 feet deep, referred to the mean level of the lake. The line was so located as to utilize the old channel as far as possible. The estimated cost of the work was \$1,875,000. The amount expended upon this project to June 30, 1891, was \$286,470.55. The amount expended to June 30, 1892, including liabilities, \$471,707.

incur any further expense upon the old channel projec

STRAIGHT CHANNEL IMPROVEMENT.

Dredging was continued from July 1, 1891, to Ji from December 1 to April 30, inclusive, when it was s winter.

The amount dredged during the fiscal year is as fol

Under contract with James Rooney.....	
Under contract with L. P. and J. A. Smith.....	
Total.....	
Amount done by James Rooney under his contract is.....	
Amount dredged by L. P. and J. A. Smith	
Total under the contract with L. P. and J. A. Smith	

As a result of this dredging the channel is now op with a depth exceeding 16 feet throughout, and a wid the botton, save in one section of 2,400 feet in the I where it is but 175 feet -wide, and 1,300 feet of Turr where the width is but 170 feet.

It should be observed that the surface level of the l: ble, and there seems to have been more than one plane in different years have been referred.

For further details of work during the fiscal yea report of Lieut. William V. Judson, Corps of Engine pended as part of this report.

It will be noticed that the channel is not 17 feet e planned. This is primarily due to the fact that the d has been limited to a depth of 17 feet, and it is pract to obtain a perfectly uniform depth by dredging. As for less depth, there has been a constant deposition of dredged channel.

owing the fill since dredging. Copies of these sections and map of Maumee River and Bay are forwarded to accompany report, and I invite attention to Lieut. Judson's discussion of the

fill in the channel from sliding in of the banks of the cut seems small, but it will be observed that the fill from all causes in two years is a little more than eight-tenths of a foot.

It is evident that the dredged channel through Maumee Bay can not be maintained by natural or ordinary conditions.

An alternative is then presented of permitting the channel to fill or being obliged to incur a considerable expense for its preservation. It is obvious that the method which offers most of certainty in maintaining the channel is dredging as necessities may require, and it is my opinion that dredging can be done most promptly when needed and most satisfactorily and economically at all times with a dredge and scows owned by the United States and operated by hired labor. I believe that one large dredge, with the necessary scows, would be capable of removing any sedimentary material which may be required for maintenance of the channel, and the services of a tug, which can probably be hired to best advantage, may be needed.

The subject for this channel left the question of its protection and maintenance open for determination after observation and experience. A stone or concrete revetment to retain the sides of the cut, whether by a low wall, by piling, or by other means, would be of little or no value, and would neither prevent the sedimentary deposit nor the movement of mud and sand under the action of the seas.

Whether any system of piers or dikes, submerged or otherwise, will be sufficient to prevent the filling of the channel is doubtful, unless they are so constructed as to confine the river at its high stages sufficient to maintain a strong current entirely across Maumee Bay to

the system would be attended with many disadvantages in addition to the great cost. It would bring great inconvenience to many small vessels and boats of light draft which navigate the bay, and the sediment which is now deposited over all of Maumee Bay would be cleared to the end of the dikes, and sooner or later make further excavation necessary.

The location of the channel in the lake subdivision was cut through a sand bar, and the conditions would indicate the probability of a more rapid filling than at other places under the action of the seas. It may be therefore, to protect the channel over this bar by dikes, entirely enclosed at first, as an experiment. The tops should be so arranged as to provide a wide side against injury from ice.

The dikes should be placed at considerable distance from the dredged channel. In determining that distance, the régime of the river should be considered. In other words, the distance between the dikes should be the same as would be used if the river were to be conducted between

the dikes as the dikes are to be used only to protect the channel from the action of the waves, probably the cheapest and best construction may be a compact dump of stone upon mattresses

These would break up the seas which wash across the channel and would permit any material washed over them to settle on the shore before the channel is reached.

As to the deposit in the channel, there now remains to be dredged the Turn-out Division on the south side of the light-house cribs

in addition to amount required to complete the width at place be mentioned.

No revision of former estimate has been made.

The estimate for completion of project is the same as was submitted in last annual report.

The following is a list of the appropriations made for the old channel from 1866 to present time:

June 23, 1866.....	\$20,000.00	March 3, 1879.....	\$20.00
March 2, 1867.....	20,000.00	June 14, 1880.....	20.00
April 10, 1869.....	29,700.00	March 3, 1881.....	40.00
July 11, 1870.....	50,000.00	August 2, 1882.....	50.00
March 3, 1871.....	50,000.00	July 5, 1884.....	20.00
June 10, 1872.....	15,000.00	August 5, 1886 (see note).....	9.63
March 3, 1873.....	100,000.00	August 11, 1888.....	5.00
June 23, 1874.....	75,000.00	September 19, 1890.....	5.00
March 3, 1875.....	75,000.00		
August 14, 1876.....	60,000.00	Total.....	724.33
June 18, 1878.....	50,000.00		

Appropriations have been made for straight channel as follows,

July 5, 1884.....	\$25.00
Deduct amount appropriated for old channel (see note).....	9.63
	15.37
August 5, 1886.....	112.50
August 11, 1888.....	150.00
September 19, 1890.....	200.00
Total.....	477.87

NOTE.—By act of August 5, 1886, the balance then available of the \$25,000 appropriated July 5, 1884, for straight channel, was made available for clearing old channel.

Toledo is in the collection district of Miami. There is a fixed white light on fourth order on Turtle Island, and three sets of range lights for parts of the channel.

The tonnage of Toledo Harbor, as furnished for 1891, shows a considerable increase over previous years.

Money statements.

OLD CHANNEL.

July 1, 1891, balance unexpended.....	\$4.00
June 30, 1892, amount expended during fiscal year.....	4.00
July 1, 1892, balance unexpended.....	
July 1, 1892, outstanding liabilities.....	

STRAIGHT CHANNEL.

July 1, 1891, balance unexpended.....	191.
June 30, 1892, amount expended during fiscal year.....	176.
July 1, 1892, balance unexpended.....	15.
July 1, 1892, outstanding liabilities.....	9.
July 1, 1892, balance available.....	5.
Amount appropriated by act approved July 13, 1892.....	200.
Amount available for fiscal year ending June 30, 1893.....	205.

{	Amount (estimated) required for completion of existing project.....	1,200.
	Amount that can be profitably expended in fiscal year ending June 30, 1894.....	400.
	Submitted in compliance with requirements of sections 2 of river and harbor acts of 1866 and 1867.	

REPORT OF LIEUTENANT W. V. JUDSON, CORPS OF ENGINEERS.

TOLEDO, OHIO, June 30, 1892.

have the honor to submit the following report upon operations at this harbor the fiscal year ending June 30, 1892:

to clearer what is to follow, I would first explain that the straight channel Maumee Bay diverges from the old channel at a point near the mouth of the River. From this point to the 17-foot contour of Lake Erie it is 40,270 feet line of the channel. Between the river mouth and the "Black Can" inner section, 12,670 feet in length. Between the "Black Can" and a point southwest of the main crib light lies the middle section, 10,600 feet in length. From the last-named point to a point 2,400 feet beyond (*i. e.*, northeast of) the crib light lies the turn-out subdivision, 3,400 feet in length. Beyond the turn-out subdivision, and extending 5,600 feet toward the lake, lies the crib subdivision from the outer end of the crib subdivision to the 17-foot contour of the lake the lake subdivision, 8,000 feet in length.

The above-mentioned divisions of the straight channel are mentioned in reports made for the past three years. At the beginning of the fiscal year dredging being done under two contracts, one with James Rooney, and one with L. A. Smith.

contract.—Two dredges worked steadily upon this contract from July 1 to October 9, 1891, when the work of the contractor was completed. During this time 165,138 cubic yards (scow measurement) were removed from the crib subdivision, completing the excavation within the limits to a least depth of 16.4 feet; average depth being 16.9 feet.

contract.—Four dredges worked steadily upon this contract from July 1 to October 30, 1891, and these dredges resumed operations May 1, 1892, and worked steadily thereafter until the completion of the contract, June 7, 1892.

The angle between the old channel and the new, and at the inner end of the new channel situated a clay bank, which formerly deflected the water descending the River into the old channel. This clay bank constituted the river division, and it was removed 111,967 cubic yards, whereby the approach to the straight channel has been widened, promoting ease of navigation, and tending to introduce the river current into the straight channel for purposes of scour. The least depth of this division at completion of dredging was 16.2 feet, with an average depth of 16.8 feet.

At the turn-out subdivision there was removed 106,640 cubic yards. About 100 feet of this subdivision is parallel to a pile-protection work that embraces the inner end of the crib lights in Maumee Bay, and occupies a space 80 feet wide in the center of the straight channel. The north side of this pile work was chosen for the approach, and at this locality the channel has a width of 170 feet, measured from the center of the work.

The turn-out has slanting approaches to the straight channel at either end each 100 feet long. The remainder of the turn-out subdivision is uniform in width with the rest of the straight channel, and was excavated to a least depth of 16.4 feet; average depth being about 16.8 feet.

The lake subdivision there was removed 232,327 cubic yards. This yielded a depth of about 16.4 feet with an average of 16.8 feet, but the full width of 200 feet was not obtained throughout; 2,400 linear feet of this subdivision is but 175 feet wide.

At the old channel there was removed 20,356 cubic yards. Several shoal spots were opened and the channel was made wider at the elbows. The result was a depth of 15.6 feet minimum depth throughout, the width varying from 100 feet in the straight reaches to 260 feet at the turning points.

protection work.—The main and east crib lights of the Maumee Bay range are situated in the axis of the channel. To prevent vessels from colliding with these, a protection work was in progress at the beginning of the fiscal year. Repeated requests of time were granted the contractor, who was not prepared to prosecute the work rapidly in the face of the serious difficulties that were consequent upon the location of the work. The work was finally satisfactorily completed December 31. The crib lights are about 1,080 feet apart. Two rows of piles, each 40 feet apart, in the axis of the channel and parallel thereto, inclose the interval between the lights, and prevent vessels from getting between them. At either end the protection work is extended to an apex reaching 170 feet beyond the cribs. In bulkheads, extreme apices stone and brush have been deposited to the water line. About 100 tons of stone and brush have also been deposited about each crib to maintain the protection. Large oak piles were used throughout. In the parallel rows these piles are 6 feet from center to center, bearing three heavy waling pieces and bound together with iron tie-rods and wooden pieces supported upon intermediate piles. At the apices the piles are but 3 feet from center to center, and an additional

waling strip is used. Cross pieces strongly bind the converging sides of the apron together.

An inspector employed upon this work by the United States was paid \$85 per month. *Material dredged.*—No rock and no boulder were encountered. In the river division was found a stiff clay. In the turn-out subdivision was found clay and sand. The crib sub-division yielded principally sand, as did also the lake subdivision. At the outer end of the lake subdivision the sand was singularly hard and difficult to dig.

Table showing areas and quantities dredged, etc.

Contractor.	Locality.	Length of channel dredged.	Area dredged.	Approximate face.	Depth immediately after dredging.	Remarks
			<i>Square feet.</i>	<i>Feet.</i>		<i>Cubic yards.</i>
James Rooney	Crib subdivision.....	4,000	800,000	6	16.9	16
L. P. & J. A. Smith.....	River division	3,300	373,000	2 to 10	16.8	11
Do	Lake subdivision	8,000	152,800	1 to 5	16.8	22
Do	Turn-out subdivision	3,400	419,500	5 to 6	16.8	19
Do	Old channel	5,000	300,000	2	17.	3
Total		23,700	3,420,500			65

The result of the dredging.—The straight channel has now been cut through the lake. In several places, as above mentioned, the channel has not its full width nor does the full depth of 17 feet obtain throughout, because 17 feet was the minimum depth to which, according to their specifications, contractors have been allowed to dredge, and because the work has been several years in progress and annual fill has been going on over the dredged portions. At the present time the channel is practicable for vessels drawing 15.6 feet with the water at the mean of 1860–1875. This is at least as good water as obtains in the old channel. The Light House Establishment has not yet buoyed the straight channel, but when they do and the proper lights are established, the channel may be regularly opened to navigation.

Recent condition of channel.—During May and June, 1892, an examination has made to determine with all possible accuracy the fill that has taken place over different parts of the channel bottom since the soundings were taken that immediately followed the dredging. Over the inner and middle sections this would be the fill for two years, and over the turn-out subdivision, crib subdivision, and subdivision the fill would be shown for one year. About 10,000 soundings taken and plotted on five charts, each chart representing one of the above sections or subdivisions. The same charts show also the depths immediately after dredging and therefore contain the data for obtaining the fill. An inspection of these charts shows that—

- (1) The fill is not far from uniform over the whole length of the channel.
- (2) The banks have held very well. The greater part of the fill is not from the banks.
- (3) The fill is very soft and of a sedimentary character. A large part of Maum Bay is covered to a depth of a foot or more with this soft mud or slush, which lies the hard sand or clay, and which appears to have been brought by the Maum River. This slush is easily washed about, and the fill in the channel appears principally derived from this source, and directly by deposit from the river water.
- (4) The fill over the whole channel averages a little over eight-tenths of a foot depth. I transmit herewith a tracing, which is intended to show the extent of fill and the present condition of the channel in characteristic sections. Each section drawn is the mean of from 9 to 12 actual cross sections.

Cross section A is typical of the manner of fill and present condition of the channel in the outer slope of the bar.

Cross section B is typical of the same through the bar.

Cross sections C, D, and E, represent the conditions respectively on the inner side of the bar, in the middle section, and in the inner section near the river mouth. The middle and inner sections are the only parts of the channel that have yet been extensively by vessels. In cross sections D and E the path of vessels is seen near the center. The fill is so soft that it is readily pushed aside by the screws of steam vessels.

Straight channel work in the future.—From the amount of fill mentioned above will be seen that as at present constituted the straight channel can not maintain itself. Either extensive bank protection must be built or dredging to the extent of 200,000 cubic yards or more must be done annually. Considering the magnitude

and the continuing charges for repairs and interest, it seems probable that dredging would be most economical to the United States. To accomplish annual dredging I would recommend the purchase of a plant by the Government. The work demands the continuous employment of one good dredge, the running of which would be about \$30,000 per season. More would be accomplished than contractors would do for the same money, and Government work done more cheaply at neighboring harbors if the contractors hereabouts, if well together, should see that the Government is not altogether dependent upon them.

River.—A considerable shoal exists at present about 1 mile below the Maumee Railroad Bridge. A preliminary examination of the river between the Maumee and Maumee Bay shows that about 40,000 cubic yards should be removed to form a 17-foot channel throughout. The current prevents any rapid accumulation here, and this dredging will probably maintain the channel for several years.

1.—For purposes of inspection, survey, etc., the steamer *Swansea* was employed by the Government for a period of 222 days during the fiscal year. The average expense of this steamer to the Government was about \$19. The crew of the steamer assisted in laying out the work, taking soundings, and making surveys. During the year, in addition to her regular work, the *Swansea* removed eighteen barges from the river. An inspector was employed upon each dredge, whose duty it was to attend and supervise its work. These inspectors received from \$80 to \$100 per month, according to their efficiency and length of service. During the past fiscal year the extended examination of the harbor has been in progress, in addition to the above employes there has been a principal inspector on the work, at a salary of \$120 per month.

I am, sir, respectfully, your obedient servant,

W. V. JUDSON,
Second Lieut., Corps of Engineers.

L. J. A. SMITH,
Corps of Engineers, U. S. A.

COMMERCIAL STATISTICS.

The following statistics for the year 1891, relative to the commerce of Toledo Harbor, were compiled from information furnished by the collector of customs and

Articles.	Tons.	Articles.	Tons.
<i>Imports.</i>		<i>Exports.</i>	
.....	200,893	Flour and grain.....	578,079
.....	89,310	Coal.....	518,182
Cement.....	17,268	Timber.....	25,350
.....	11,943		
.....	643		
	314,657		1,111,617

Shipping.	No.	Tonnage.
Shipping.....	2,189	968,810
Shipping.....	2,185	985,310
Shipping.....	4	922,56

Shipping for 1891.....	1,954,120
Shipping given in last Annual Report.....	1,542,617
Shipping.....	411,503

The depth of the largest vessel using the harbor is 21 feet. The depth of water in the harbor prevents the largest vessels from loading to full capacity. Lines of transportation have been established during the year.

M M 3.

IMPROVEMENT OF PORT CLINTON HARBOR, OHIO.

Port Clinton, Ohio, is situated at the mouth of the Portage River, a stream which rises in the northwestern part of Ohio and empties into Lake Erie.

A history of the earlier operations heretofore carried on for its improvement will be found in Annual Reports of 1880 and 1881.

The present project, adopted in 1875, consists of a pile revetment 967 feet long, running from the north shore of Portage River opposite the town out into the lake, in a direction north 57 degrees east. This revetment then inclines toward the north and extends 301 feet farther, when a pile dike commences, which will be prolonged a total distance of 1,200 feet out to a depth of 10 feet at the ordinary level of the lake.

Parallel to this and 200 feet from it is an east pile pier, which will be about 2,600 feet long, its inner end resting on the south shore of the river. This east pier will be a simple pile structure of 2,450 feet; the outer 150 feet will be a strong pile dike 12 feet wide. A channel 10 feet deep will be dredged between the piers.

At the date of the last Annual Report, an agreement had been made with Messrs. Carkin, Stickney & Cram, of East Saginaw, Mich., to dredge through the bar and between the piers to obtain a channel depth of 10 feet.

Work under the agreement was commenced July 28 and completed in August. The amount of material dredged and removed from the channel was 11,705 cubic yards, measured in scows. The price paid was 25 cents per cubic yard.

The deepening of the channel causes no permanent improvement. Nearly the same conditions return annually, and with the present conditions annual expenditure for dredging will be required if the channel is to be maintained.

The project of 1875 was estimated to cost \$90,000. The sum of \$56,000 has been appropriated and expended since the adoption of the project, but a part has been for repairs and for dredging, which did not advance the constructions proposed.

The amount estimated in last Annual Report for completion of project was \$37,000.

As the officer now in charge of the work has had no opportunity for a revision of the estimate, it is here repeated.

The following is a statement of the amount and date of all appropriations for this improvement:

June 10, 1872	\$8,000	March 3, 1881	\$5,000
June 10, 1872 (allotment)	2,000	August 2, 1882	6,000
March 3, 1875	5,000	August 5, 1886	2,000
August 14, 1876	5,000	August 11, 1888	5,000
June 18, 1878	10,000	September 19, 1890	3,000
March 3, 1879	10,000		
June 14, 1880	5,000	Total	68,000

Port Clinton is a port of entry in the collection district of Sandusky, Ohio. The nearest work of defense is Fort Wayne, Mich., 30 miles distant, and the nearest light-house is at Green Island, 10 miles distant.

The commercial statistics for the year 1891 indicate a slight decline in numbers from the previous year. The tonnage of vessels entering has not been ascertained.

Money statement.

July 1, 1891, balance unexpended	\$3,000.00
June 30, 1892, amount expended during fiscal year	3,000.00
Amount appropriated by act approved July 13, 1892	10,000.00

t (estimated) required for completion of existing project \$27,000.00
 t that can be profitably expended in fiscal year ending June 30, 1894 27,000.00
 ted in compliance with requirements of sections 2 of river and
 or acts of 1866 and 1867.

COMMERCIAL STATISTICS.

Following statistics for the year 1891, relative to the commerce of Port Clinton Ohio, were compiled from information furnished by the collector of customs:

Shipping.	No.	Tonnage.
bering.....	34	(*)
parting.....	32	Do.
lit.....	1	13

* Not known.

—Fish and miscellaneous merchandise Tons. 3,753
 ts—Lumber, flour, and miscellaneous merchandise 4,022
 gregate tonnage of vessels given in last annual report was 5,149.
 :ggest vessels entering the harbor draw from 9 to 12 feet.

M M 4.

IMPROVEMENT OF SANDUSKY CITY HARBOR, OHIO.

usky Bay empties into Lake Erie about 40 miles from its western
 ty. It is a natural harbor, containing an area of about 22½
 depth of from 8 to 12 feet, protected on the north and northwest
 ie gales of the lake by a long narrow peninsula, and on the
 st by what is known as Cedar Point.
 roject adopted in 1880 provides for a channel 200 feet wide and
 deep through the outer bar and in the bay, up to within 50 feet
 ine of docks, and then parallel to the docks, with a width of 100
 l depth of 15 feet.
 evised project, adopted in 1888, proposes to improve the present
 l by a straight channel cut from the north end of Cedar Point to
 end of the existing channel in front of city.
 history of this project for making a straight channel and de-
 stimate of cost, see Annual Report of the Chief of Engineers for
 ages 2303 and 2304, and pages 2335 to 2341.
 l description of the operations carried on in earlier years for the
 ment of this harbor will be found in Annual Reports of 1880
 1.
 e close of the last fiscal year work was in progress under a con-
 ith Messrs. L. P. & J. A. Smith, of Cleveland, Ohio, for dredg-
 straight channel. Work under the contract was completed
 : 17, 1891.

The amount of material removed after June 30 was 148,638 cubic yards, and the total amount under the contract was 212,962 cubic yard measured in scows.

Other works of improvement in this harbor have been carried simultaneously with Straight Channel, having office and other contingent expenses without a definite allotment or set of accounts for each work, so that the officer recently assigned to charge of the work find it difficult to determine the exact amount which should be charged to the cost of the Straight Channel alone. As nearly as can be determined the amount expended on Straight Channel in last fiscal year was \$27,786.22; the amount reported as expended on same to June 30, 1892, \$41,199.07; total, \$69,185.29.

The cost of Straight Channel originally estimated was for dredging 628,000 cubic yards, scow measurement, at 14 cents per cubic yard, \$87,920; contingent expenses, \$8,792; total, \$96,712.

The prices by contract have been 17 and 22 cents per cubic yard instead of 14 cents, as estimated.

At the close of last fiscal year, Messrs. Carlin, Stickney & Co. of East Saginaw, Mich., were dredging in the old channel. Work under this agreement was completed July 23. The amount dredged in last fiscal year was 5,872 cubic yards, making a total of 9,500 cubic yards under the agreement. The cost was 25 cents per cubic yard.

The dredging was reported as giving a channel depth of 16½ feet throughout the dock channel and 15 feet in old channel.

The bar outside of Cedar Point is the worst obstruction encountered in entering the harbor.

The water over the bar is only 12 to 14 feet deep and the channel quite narrow, the least being but about 50 feet.

The following is a statement of the amount and date of all appropriations for this improvement.

May 20, 1826 (survey).....	\$400	June 18, 1878	\$2
June 11, 1844	15,000	March 3, 1879	1
August 30, 1852	15,000	June 14, 1880	1
June 28, 1864 (allotment)	10,000	March 3, 1881	1
June 23, 1866	38,580	August 2, 1882	1
July 11, 1870	10,000	July 5, 1884	2
June 10, 1872	13,000	August 5, 1886	4
March 3, 1873	25,000	August 11, 1888	4
June 23, 1874	25,000	September 19, 1890	4
March 3, 1875	25,000		
August 14, 1876	25,000	Total	36

Total expenditures to June 30, 1892, \$365,391.46.

Sandusky City Harbor is in the collection district of Sandusky, Ohio. There is a light-house on Cedar Point with a fixed white light of the fifth order, and three lights within the bay. Fort Wayne, below Detroit, is the nearest work of defense.

Money statement.

July 1, 1891, balance unexpended	\$40,3
June 30, 1892, amount expended during fiscal year.....	38,2
July 1, 1892, balance unexpended	2,0
Amount appropriated by act approved July 13, 1892.....	41,7
Amount available for fiscal year ending June 30, 1893	43,8

COMMERCIAL STATISTICS.

Following statistics for the year 1891, relative to the commerce of the harbor of Sandusky City, Ohio, were compiled from information furnished by the collector of customs and others.

	Tons.
Number	150,000
.....	120,300
Total	270,300
Tons, coal	164,789

Shipping.	No.	Tonnage.
Entering	3,664	758,691
Departing	3,699	764,819
Total		1,523,510

Aggregate tonnage given in last annual report was 1,051,108; increase, 472,402.
 Depth of the largest vessel entering harbor, 15½ feet.
 Shortage of water prevents largest vessels from loading to full depth.
 New lines of transportation have been established during the year.

M M 5.

IMPROVEMENT OF SANDUSKY RIVER, OHIO.

Sandusky River rises in Richland County, Ohio, and after a very tortuous course empties into Sandusky Bay about 14½ miles from Cedar Point, where the bay empties into Lake Erie.

At present, the head of navigation, is 17 miles from the mouth of the river.

It is a city of about 9,000 inhabitants, and the market place of a large and productive surrounding country.

The history of the work carried on in earlier years for the improvement of this river will be found in annual report for 1881.

The present project, adopted in 1880, provides for dredging a channel 100 feet wide and 9 feet deep through the various bars between the city and the depth of 9 feet in Sandusky Bay.

At the beginning of the fiscal year a contract had been made with Messrs. Carkin, Stickney & Cram, of East Saginaw, Mich., for dredging to the extent of available funds under the appropriation of September 30, 1890.

The work under the contract was commenced September 1 and completed on November 15, 1891.

The amount of material dredged and removed was 5,808 cubic yards, which was piled in scows. The price paid was 25 cents per cubic yard.

The channel was reported to be in fairly good condition, with a least depth of 7 feet in September, 1891.

The estimated cost of the project for 9 feet depth was \$44,000, but the State was doubtless intended to cover the first cost only of obtaining the channel if the work were done in one or two seasons. In a river like this there is a large amount of silt and other drift, which will fill the dredged cuts unless the conditions of the channel be changed by reclamation works.

In the ten years since the commencement of work on the project for 9 feet depth the drift has probably obliterated nearly all the dredging which has been done, and the expense would therefore not be materially reduced by the work already done.

It is the opinion of the officer now in charge of this improvement that dredging will afford no really permanent improvement. It would follow that unless the commerce be such that the benefits of a deeper channel are sufficient to justify a constant expense for dredging, or a very considerable outlay for contraction works, the improvement of the river will not pay as a business proposition.

The estimate here submitted for amount required to complete the present project is nearly the original estimate reduced by amount already appropriated, there being no present means of making the amount more definite, and no apparent necessity for so doing.

The following is a statement of the amount and date of all appropriations for this improvement.

March 2, 1867	August 2, 1882	\$4,000
June 10, 1872	September 19, 1890	1,000
June 14, 1880		
March 3, 1881	Total	\$5,000

Fremont, the head of navigation in the collection district of Sandusky, Ohio. The nearest light-house is at Cedar Point, the entrance to Sandusky Bay. Fort Wayne, near Detroit, Mich., is the nearest work of defense.

No definite statistics have been received for Sandusky River. Vessels for the river are included in the statistics for Sandusky Harbor.

Money statement.

July 1, 1891, balance unexpended	\$1,500
June 30, 1892, amount expended during fiscal year	1,500
Amount appropriated by act approved July 13, 1892	5,000
{ Amount (estimated) required for completion of existing project	16,000
{ Amount that can be profitably expended in fiscal year ending June 30, 1894	16,000
{ Submitted in compliance with requirements of sections 2 of river and harbor acts of 1866 and 1867.	

M M 6.

IMPROVEMENT OF HURON HARBOR, OHIO.

The Huron River rises in the northern part of Ohio, and after a circuitous course empties into Lake Erie about 10 miles east of Sandusky city.

The project for the improvement of this harbor, adopted in 1866 when the mouth of the river was closed by a sand bar, and which project has been amended from time to time, as the demands of the commerce called for an increased depth of channel, consists of parallel piers, 140 feet apart, extending to the depth of 16 feet in the lake.

For a history of the earlier operations carried on in past years the improvement of this harbor, see Annual Reports of 1880 and 1881.

The project approved in 1890 consists in extending piers to the depth of 16 feet in the lake and dredging channel to same depth.

This project was in compliance with the requirements of the act

er 19, 1890, appropriating \$16,000 for "improving harbor at Ohio; continuing improvement in amended project to give 16 feet low water."

end of the last fiscal year work was in progress under a contract Mr. John Stang, of Lorain, Ohio, for extending west pier a of 120 feet. This work was completed September 23, 1892. In

to the construction, minor repairs have been made to superstructure of old piers.

project of 1890 is about one-half completed. The old piers are, in a very bad condition from decay and injury by storms, and their superstructure should be renewed.

end of June, 1891, the channel between piers was reported at a depth exceeding 16 feet, but on the bar about 150 feet out-ends of piers the depth ranged from 14.5 to 15 feet.

Following is a statement of the amount and date of all appropriations for this improvement:

26.....	\$5,000.00	June 23, 1874.....	\$1,500.00
28.....	4,413.35	March 3, 1875.....	1,000.00
1829.....	5,935.00	June 18, 1878.....	1,000.00
830.....	1,880.36	June 14, 1880.....	3,000.00
831.....	3,480.00	March 3, 1881.....	3,000.00
82.....	1,500.00	August 2, 1882.....	2,500.00
334.....	6,700.00	July 5, 1884.....	7,500.00
38.....	4,300.00	August 5, 1886.....	3,000.00
837.....	2,565.00	August 11, 1888.....	6,000.00
18.....	5,000.00	September 19, 1890.....	16,000.00
344.....	5,000.00		
1852.....	10,000.00	Total.....	199,273.71
366.....	39,000.00		

Amount expended upon Huron Harbor to June 30, 1891, was.....	\$128,469.43
res in last fiscal year.....	10,804.28
Amount to complete project of 1890.....	13,600.00
Cost of repairing old piers.....	12,000.00

ould be remarked that in harbors like this the woodwork of piers is constantly decaying and receiving more or less injury from storms.

At end of piers forms to a greater or less extent during each winter and spring, and as a result a part of each appropriation is necessary for maintenance of piers and channel and the remainder is carrying out the project of improvement.

Number of vessels which entered and cleared during the last year was considerably less than in the preceding year, but the aggregate in freights was considerably more than the registered tonnage of the previous year. This indicates that a larger class of vessels is used at this port.

Huron Harbor is in the collection district of Sandusky, Ohio. Fort Wayne, Mich., 100 miles distant, is the nearest work of defense. There is a fixed white light on the outer end of the west pier.

Lines of transportation have been established during the year.

Money statement.

Balance unexpended.....	\$10,804.28
1892, amount expended during fiscal year.....	10,804.28
Appropriated by act approved July 13, 1892.....	15,000.00

That can be profitably expended in fiscal year ending June 30, 1894, in compliance with requirements of sections 2 of river and harbor acts of 1866 and 1867. 10,600.00

COMMERCIAL STATISTICS.

The following statistics for the year 1891, relative to the commerce of Huron Harbor, Ohio, were compiled from information furnished by the collector of customs and others:

Vessels entering	137
Vessels departing	137
Freight receipts	tons.. 17, 00
Freight shipments	do.. 134, 00
Total	do.. 151, 00

The aggregate tonnage of vessels was stated in last Annual Report as 73,464.

M M 7.

IMPROVEMENT OF VERMILION HARBOR, OHIO.

The Vermillion River rises in the northern part of Ohio and empties into Lake Erie about 20 miles to the westward of Sandusky City.

The project of improvement, which was adopted in 1836, when there was a depth of less than 2 feet of water over bar at entrance, and which project has been amended from time to time as the requirements of commerce demanded deeper water, consists of parallel piers, 125 feet apart, running out to a depth of 12 feet in the lake.

A history of the earlier operations carried on in the past years for the improvement of this harbor will be found in Annual Reports of 1880 and 1881.

The act of September 19, 1890, appropriated \$2,000 for preservation of piers. The amount of repairs being small, the most economical and advantageous method of doing the work was by hired labor. Other and more extensive works being in progress, it was not convenient to make the repairs in last fiscal year, as was contemplated.

The repairs will be completed as far as available funds may permit during the season of 1892.

The commerce of this harbor is not sufficiently extensive or general at present to justify any large expenditure further than may be required for preservation of piers heretofore constructed, save possibly a small amount of dredging to remove small obstructions.

It is therefore proposed to apply such funds as may be appropriated to the purpose indicated.

The following is a statement of the amount and date of all appropriations for this improvement:

July 2, 1836	\$10,000.00	June 18, 1878	\$4,000.00
March 3, 1837	20,000.00	June 14, 1880	2,000.00
July 7, 1838	23,626.57	March 3, 1881	2,000.00
June 28, 1864 (allotment)	5,758.97	August 2, 1882	3,000.00
June 23, 1866	15,315.74	August 5, 1886	3,000.00
June 10, 1872	5,000.00	August 11, 1888	1,000.00
March 3, 1873	12,000.00	September 19, 1890	2,000.00
June 23, 1874	3,000.00		
March 3, 1875	10,000.00	Total	126,701.36
August 14, 1876	5,000.00		

Vermillion Harbor is in the collection district of Sandusky, Ohio. There is a light of the fifth order on the west pier. Fort Wayne, Mich., 80 miles distant, is nearest work of defense.

Money statement.

July 1, 1891, balance unexpended.....	\$2,000.00
July 1, 1892, balance unexpended	2,000.00
Amount appropriated by act approved July 13, 1892.....	2,000.00
Amount available for fiscal year ending June 30, 1893.....	4,000.00
Amount (estimated) required for completion of existing project.....	6,000.00
Amount that can be profitably expended in fiscal year ending June 30, 1894	6,000.00
Amount submitted in compliance with requirements of sections 2 of river and harbor acts of 1866 and 1867.	

COMMERCIAL STATISTICS.

The following statistics for the year 1891, relative to the commerce of Vermillion Harbor, Ohio, were compiled from information furnished by the collector of customs and others:

Tons entering	34
Tons departing.....	33
	Tons.
Exports: Lumber, pound poles, and fishing tackle	2,000
Imports: Timber, fish, and fishing tackle.....	1,000

The draft of the largest vessel entering the harbor is 15½ feet. As the depth of water in the harbor is only 10 to 12 feet, the largest vessels can not load to full depth. The chief industry of the port is fishing. No record is kept of the amount brought in.

M M 8.

IMPROVEMENT OF BLACK RIVER HARBOR, OHIO.

Black River, Ohio, is formed by two branches nearly equal in size, each rising in Lorain County, Ohio, and following northward, unite about 8 miles from the town of Lorain, where the river empties into Lake Erie.

A history of the operations carried on during the past years, whereby a depth at the entrance to this harbor has been increased from about 10 feet to at least 16 feet, will be found in Annual Reports of 1880 and 1881.

The project of improvement submitted in 1828, and amended from time to time as the demands of commerce called for an increased depth channel, provides for parallel piers 200 feet apart, running out from the shore on each side of the mouth of the river to a depth of 16 feet into the lake.

At the beginning of the fiscal year work was in progress under a contract with Mr. John Stang, of Lorain, Ohio, for extending the east pier a distance of 102 feet.

The date of expiration of the contract was originally August 1, 1891. Owing to failure to receive materials for the construction, the contractor was granted an extension to August 30, and subsequently a second, extending the time for completion of contract to September 30, 1891. Work under the contract was completed September 12, 1891.

In addition to work of pier extension some minor repairs were made to the superstructure of old piers. The superstructure of old piers is in a bad condition from decay and injury by storms and ice.

In the last annual report the officer then in charge of the improvement recommended that provision be made for 17 feet depth of water at an estimated cost of \$40,000 in addition to the project previously adopted. The estimate is here repeated with the remark that the officer now in charge of the work has had no means of judging of the necessity for the work or the accuracy of the estimate.

2502 REPORT OF THE CHIEF OF ENGINEERS, U. S. ARMY.

At the close of the last fiscal year the channel was reported to be feet deep. No examinations of the channel have been made since the date.

The following is a statement of the amount and date of all appropriations for this improvement:

May 23, 1828.....	\$7,500.00	March 3, 1873.....	\$20,000.
April 23, 1830.....	8,559.77	June 23, 1874.....	20,000
March 2, 1831.....	9,275.00	March 3, 1875.....	10,000
July 3, 1832.....	8,000.00	August 14, 1876.....	6,000
March 2, 1833.....	2,400.00	June 18, 1878.....	1,000
June 28, 1834.....	5,000.00	June 14, 1880.....	1,000
March 3, 1835.....	4,400.00	March 3, 1881.....	7,000
July 2, 1836.....	6,660.00	August 2, 1882.....	7,000
March 3, 1837.....	6,410.00	July 5, 1884.....	10,000
July 7, 1838.....	5,000.00	August 5, 1886.....	10,000
August 30, 1852.....	5,000.00	August 11, 1888.....	10,000
June 28, 1864 (allotment).....	20,000.00	September 19, 1890.....	12,000
June 23, 1866.....	10,000.00		
June 10, 1872.....	20,000.00	Total.....	232,200

Black River is in the collection district of Cuyahoga, Ohio. There is a fixed light of the fourth order at the outer end of the west pier. The nearest work of defense is Fort Wayne, Mich., 80 miles distant.

The number of vessels reported as entering and leaving the harbor in the year 1891 is less than statistics for previous years. The later figures were furnished by the collector of customs and are presumed to be approximately correct.

On the other hand the tonnage in freight reported is greater than in the previous year.

Money statement.

July 1, 1891, balance unexpended.....	\$9,27
June 30, 1892, amount expended during fiscal year.....	9,27
Amount appropriated by act approved July 13, 1892.....	20,00
{ Amount that can be profitably expended in fiscal year ending June 30, 1894.....	38,00
{ Submitted in compliance with requirements of sections 2 of river and harbor acts of 1866 and 1867.	

COMMERCIAL STATISTICS.

The following statistics for the year 1891, relative to the commerce of Black River Harbor, Ohio, were compiled from information furnished by the collector of customs and others:

Articles.	Tons.	Articles.	Tons.
<i>Imports.</i>		<i>Exports.</i>	
Ore.....	270,240	Coal.....	450
Lumber.....	9,143	Lumber.....	
Railroad ties.....	3,774	Miscellaneous.....	
Telegraph poles.....	5,000		
Stone.....	46	Total.....	450
Total.....	288,203		

Shipping.	No.	Tonnage
Vessels entering.....	288	Not known
Vessels departing.....	305	Do.
Vessels built.....	1	

Total tonnage for 1891 (estimated), 800,000.

The draft of the largest vessel entering the harbor was 21 feet.

The depth of water in the harbor prevents the largest vessels from loading to depth.

No new lines of transportation established.

M M 9.

IMPROVEMENT OF CLEVELAND HARBOR, OHIO.

Cleveland, Ohio, is situated at the mouth of the Cuyahoga River. The river rises in the northern part of Ohio and after a very circuitous course empties into Lake Erie.

The original project of improvement, adopted in 1825, when there was a depth of only 3 feet in the narrow and crooked channel at the entrance, and which project has been amended from time to time as the demands of commerce called for an increased depth of water, provides two parallel piers 200 feet apart, running out to a depth of 16 feet in the lake.

This project is completed.

In 1875, in accordance with an act of Congress, a plan was submitted to provide a harbor of refuge at this place.

The amended plan for this outer harbor consists of two breakwaters. The shore arm of the west breakwater starts from a point about 700 feet west of the extremity of the old bed of the Cuyahoga River and runs out into the lake in a direction nearly due north a distance of 3,130 feet.

The lake arm, which is about parallel with the main shore, is 4,030 feet long, and at a point 200 feet from its eastern extremity a spur 100 feet long runs out at right angles so as to break the force of the heavy waves rolling along the breakwater during westerly and northwesterly gales. All this portion was completed in December, 1883.

The proposed east breakwater, under the latest plan, begins at a point on the prolongation of the lake arm of the west breakwater and 100 feet from it, extends eastward on this line about 3,500 feet, then inclines toward the shore, and extends 2,000 feet in a depth of 26 feet of water, and having between its eastern end and the curve of 14 feet depth of water an entrance 2,300 feet wide.

For a history of this change in plan for harbor of refuge, see Annual Reports of 1884, 1885, 1886, and 1888.

At the beginning of the fiscal year work was in progress, extending the east breakwater, under a contract with Messrs. L. P. & J. A. Smith, Cleveland, Ohio.

The contract was completed November 23, 1891. By this work the west breakwater was extended eastward a distance of 452½ linear feet, making the total length completed of breakwater 9,308½ feet. The amount remaining to complete the breakwater, as planned, is 3,350 feet.

Work under contract with Messrs. Carkin, Stickney & Cram was continued from July 1 to July 15, when it was completed to extent of available funds.

The amount of material removed in July was 4,786 cubic yards, making a total under the contract of 18,286 cubic yards, measured in scows. In the spring of 1892, the breakwaters were found to be considerably injured by ice and winter storms.

Repairs, which could be undertaken, were necessarily limited by funds available for the purpose, which amounted to but \$1,500. These repairs were commenced in June, but were delayed owing to the difficulty of procuring certain pieces of oak lumber which were necessary for the purpose.

The repairs will be completed early in July.

In April, 1892, soundings were taken to ascertain the condition of the channel from the railroad bridge to the opening in breakwater. The soundings show that the channel depths in the river have been

fully maintained to a point about 400 feet inside the ends of piers, where a shoaling is perceived. From the end of the piers outward a distance of 400 feet the channel has an average of about 2 feet less depth than in July, 1891, referred to the mean lake level.

The actual present depth is, however, nearly the same as last year as the mean surface level of the lake in June, 1892, was 1.73 feet higher than in June, 1891. The least depth in the channel on the bar in April, 1892, was 15.2 feet, referred to mean lake level.

At a point 50 feet below the bridge the channel is 165 feet wide; greatest depth exceeds 20 feet and the average is about 18 feet.

The piers are not parallel, but diverge so that they are 250 feet apart at the outer ends. The result of the divergence is plainly shown in the shoaling of the river.

It is apparent that no very deep channel can be relied upon with a considerable annual expenditure for dredging.

The mud and drift brought down by the river during freshets is forming a bar across the entrance to the anchorage basin behind the west breakwater. It is also reported that the anchorage basin is gradually becoming filled from various causes, such as sewerage, mill dumping, drift, etc. How far the statements are correct can only be determined by careful examination, and it is therefore proposed to make a good hydrographic survey of the harbor in the summer of 1892. The area of the outer harbor, which was planned as a harbor of refuge, is now more than ample for all the present demands upon it, and it seems to be more need of greater depth in the west basin than of increasing the area for anchorage on the east side.

All the business of the harbor is done in the river. About one-third of the lake front on the west side of the river is fully protected from the sea by the breakwaters, and harbor lines were established some years ago, but no part of it has thus far been utilized for business purposes.

The following is a statement of the amount and date of all appropriations for this improvement:

March 3, 1825 (survey).....	\$5,000.00	March 3, 1875.....	\$
March 2, 1827.....	10,000.00	August 14, 1876.....	1
March 3, 1829.....	12,179.00	August 14, 1876 (repair of pier).....	
April 23, 1830.....	1,786.56	June 18, 1878.....	1
March 2, 1831.....	3,670.00	March 3, 1879.....	1
July 3, 1832.....	6,600.00	June 14, 1880.....	1
June 28, 1834.....	13,315.00	March 3, 1881.....	2
July 2, 1836.....	15,006.59	August 2, 1882.....	1
March 3, 1837.....	10,000.00	July 5, 1884.....	1
July 7, 1838.....	51,856.00	August 5, 1886.....	
June 11, 1844.....	25,000.00	August 11, 1888.....	1
August 30, 1852.....	30,000.00	September 19, 1890.....	
March 3, 1853.....	145.69		
June 28, 1864 (allotment)....	20,000.00		
June 23, 1866.....	59,806.00		
July 25, 1868 (allotment)....	17,000.00		
April 10, 1869 (allotment)....	13,380.00		
July 11, 1870.....	20,000.00		
March 3, 1871 (allotment for repairs).....	*636.77		
March 3, 1873.....	1,000.00		
June 23, 1874.....	30,500.00		

Total appropriated since adoption of present project..... \$1,167.

Total previous to adoption of project for harbor of refuge..... 346,881.61

Amount expended to June 30, 1892..... \$1,167.

* Difference between \$1,000 and the amount which reverted to the Treasury.
 † About \$50,000 of this amount has been expended in maintenance of the old and channel.

Cleveland harbor is in the collection district of Cuyahoga, Ohio. There is a fixed light of the third order on the shore and a beacon on the outer end of each pier, and a beacon with flash light and fog-whistle on the independent crib, just inside the east end of the lake arm of the breakwater. The nearest work of defense is at Fort Wayne, Mich., 110 miles distant.

Money statement.

July 1, 1891, balance unexpended	\$56,350.15
June 30, 1892, amount expended during fiscal year	54,718.40
July 1, 1892, balance unexpended.....	1,631.75
July 1, 1892, outstanding liabilities	233.87
July 1, 1892, balance available.....	1,397.88
Amount appropriated by act approved July 13, 1892	100,000.00
Amount available for fiscal year ending June 30, 1893	101,397.88
Amount (estimated) required for completion of existing project	344,250.00
Amount that can be profitably expended in fiscal year ending June 30, 1894	344,250.00

Submitted in compliance with requirements of sections 2 of river and harbor acts of 1866 and 1867.

COMMERCIAL STATISTICS.

The following statistics for the year 1891, relative to the commerce of Cleveland harbor, Ohio, were compiled from information furnished by the collector of customs and others:

Articles.	Tons.	Articles.	Tons.
<i>Imports.</i>		<i>Exports.</i>	
Wool	1,375,144	Coal and coke	1,398,896
Lumber	1,107,038	Iron and steel	99,428
Flour and grain	123,983	Oil and grease	29,658
Stone	118,701	Stone	18,731
Fish	84,721	Lumber	16,540
Woolen	49,331	Flour and grain	2,763
Wool	18,250	Brick	3,424
Salt	3,595	Plaster, cement, and lime	954
Wool and cement	2,844	Miscellaneous	46,478
Coal	2,987		
Miscellaneous	9,770	Total	1,616,871
Total	2,895,894		

Shipping.	No.	Tonnage.
Vessels entering.....	3,039	(*)
Vessels departing.....	3,151	(*)
Vessels built.....	17	12,792.08

* Not known.

Total tonnage for 1891 (estimated), 5,000,000, varying little from amount given in last annual report.

The draft of the largest vessel entering the harbor was 21 feet.

The depth of water in the harbor prevents the largest vessels from loading to full depth.

The following lines of transportation were established during the year:

Lakewood Transportation Company of Rockport, Ohio.

Cleveland and Lakeside Steam Navigation Company of Cleveland, Ohio.

Hawgood & Avery Transit Company of Mantua, Ohio.

M M 10.

IMPROVEMENT OF FAIRPORT HARBOR, OHIO.

Grand River rises in the northeastern part of Ohio, and after a very circuitous course empties into Lake Erie at a point about midway between the eastern and western extremities.

A full description of the earlier operations carried on for the improvement of this harbor during past years will be found in Annual Reports of 1880 and 1881.

The project of improvement adopted in 1825, when the mouth of the river was closed by a sand bar so hard and dry in summer that teams could drive across, and which project has been amended from time to time since that date, as the demands of commerce called for an increased depth in the channel, provides for parallel piers, 200 feet apart, running out from each side of the lake to a depth of 16 feet in the lake.

Revised project of 1890 provided for extension of piers to depth of 18 feet in the lake, and dredging to a depth of 8 feet depth in channel.

At the close of the last fiscal year a contract was outstanding with the American Transportation Company of Painesville, Ohio, for dredging in the channel of the river and for the removal of the sand bar at its mouth.

Work was continued under the contract until July 30, when the time expired by limitation, and the contract was annulled in accordance with its terms.

The amount dredged in the channel under the first contract was 9,291 cubic yards. The total amount dredged under the second contract was 51,292 cubic yards, and at the time of its annulment there remained about 13,500 cubic yards to complete it.

Proposals for dredging were invited by circular letter, and the proposals were opened September 5. The lowest bid was from J. R. Irwin, of Painesville, Ohio, and a contract was entered into with him, dated September 21, 1891, to do the dredging at 17½ cents per cubic yard.

Work under the contract was commenced October 5 and completed November 23, the total amount dredged under the contract being 42,062 cubic yards.

The reason for the large excess in amount done over that required to complete the former contract was that the forfeitures of amounts due under the first contract increased the amount of available funds which could be expended under the second.

By this dredging the channel was given a depth of 17 feet between the piers and through the bars in the lake near end of piers.

In the spring of 1892 the bars had again formed so that the channel depth did not exceed 14 feet. The necessity for immediate relief by dredging through the bars was urgently represented, and the amount of dredging required being too small to attract any competition in doing the work by contract, an agreement was made with the American Transportation Company, which owns the only dredging plant at Fairport Harbor, to furnish an outfit of dredge, scows, and tug, with their crews, and to do the work at the rate of \$75 per day. The commencement of work as well as its completion was greatly delayed by the windy weather which was phenomenally bad. Dredging was commenced April 21 and completed on the 4th of June, the number of days in which the dredge could work on the bar during that time being but ten and a half.

The channel was opened through the bar to a depth of 17 feet. No examination has been made between the piers to ascertain the condi-

of the channel at close of fiscal year, but no complaint has been
ved.

work of pier extension was continued from July 1 to August 31,
it was completed. The entire amount done under the contract
isted in extending the east pier a distance of 120 feet and the west
80 feet. General repairs were also made to old piers.

commerce of this harbor is rapidly increasing, and it is probable
additional facilities of increased depth in channel will be required
near future.

ppropriations have been made for Grand River and Fairport Har-
from 1825 to 1890, inclusive, as follows:

h 3, 1825	\$1,000.00	June 18, 1878	\$5,000.00
20, 1826	5,620.00	June 14, 1880	3,000.00
19, 1828	9,135.11	March 3, 1881	10,000.00
23, 1830	5,563.18	August 2, 1882	10,000.00
h 2, 1831	5,680.00	July 5, 1884	10,000.00
3, 1832	2,600.00	August 5, 1886	18,750.00
28, 1834	10,000.00	August 11, 1888	10,000.00
2, 1836	6,000.00	September 19, 1890	30,000.00
7, 1838	10,000.00		
11, 1844	10,000.00	Total	320,873.53
st 30, 1853	10,000.00		
28, 1864	24,453.24	Amount expended to June 30,	
23, 1866	24,072.00	1891	299,469.82
h 2, 1867	60,000.00	Amount expended in last fis-	
23, 1874	20,000.00	cal year	20,033.26
h 3, 1875	15,000.00		
st 14, 1876	5,000.00	Total to June 30, 1892 .	319,503.08

irport Harbor is in the collection district of Cuyahoga, Ohio. There is a fixed
e light of the third order on the shore and a beacon on the east pier.

Money statement.

1, 1891, balance unexpended	\$21,403.71
30, 1892, amount expended during fiscal year	20,033.26
1, 1892, balance unexpended	1,370.45
Amount appropriated by act approved July 13, 1892	35,000.00
Amount available for fiscal year ending June 30, 1893	36,370.45
Amount (estimated) required for completion of existing project	79,400.00
Amount that can be profitably expended in fiscal year ending June 30, 1894	79,400.00
Submitted in compliance with requirements of sections 2 of river and harbor acts of 1866 and 1867.	

act of proposals for dredging at Fairport Harbor, Ohio, received and opened by Maj.
Cooper Overman, Corps of Engineers, at Cleveland, Ohio, at 5 o'clock p. m., Septem-
5, 1891, in accordance with circular letter dated August 22, 1891.

[Net amount available, \$7,500.]

Name and address of bidders.	Price per cubic yard, measured in scows (30,000 cubic yards, more or less).	Total.
Irwin, Painesville, Ohio	Cents. 17½	*85,250
Mooney, Toledo, Ohio	25	7,500
Stang, Lorain, Ohio	23	6,900

* Lowest bid received.

commended that contract be awarded to J. R. Irwin, Painesville, Ohio, at the
of 17½ cents per cubic yard, scow measurement.

ABSTRACT OF CONTRACT FOR IMPROVING HARBOR AT FAIRBANKS
DURING THE FISCAL YEAR ENDING JUNE 30, 1891.

(1) Contract with J. R. Irwin, of Painesville, Ohio, for dredging 42,000 cubic yards, more or less, of material from the harbor; also Grand River, Ohio.

Rate paid, 17½ cents per cubic yard, scow measurement.

Contract expires December 10, 1891.

Contract completed and closed.

COMMERCIAL STATISTICS.

The following statistics for the year 1891, relative to the Harbor, Ohio, were compiled from information furnished and others.

Articles.	Tons.	A.	B.
<i>Imports.</i>			
Ore	698, 831	Coal	
Flour and grain	53, 947	Sugar	
Stems	6, 700	Canned goods	
Lumber	4, 882	Miscellaneous	
Miscellaneous	1, 054		
Total	764, 514	Total	

Vessels arriving

Vessels departing

Total tonnage for 1891 (estimated), 1,300,000, varying little from last annual report.

The draft of the largest vessel entering the harbor was 16 feet. The depth of water in the harbor prevents the largest vessels from coming in.

The following lines of transportation were established:
The Mentor Steamship Company,
The Mitchell Steamship Company.

M M II.

IMPROVEMENT OF ASHTABULA HARBOR

The original project for the improvement of the harbor was adopted in 1826, at which time there was a depth of 16 feet at the bar. This project has been modified from time to time to meet the demands of commerce and increased dredging of the lake. As at present being carried out, running out into the lake to 16 feet depth and through bar and between piers to secure 17 feet depth.

The last project, adopted in July, 1890, provided for the construction of piers to 22 feet natural depth in lake and excavation of piers to 20 feet at mean low water.

Before operations were commenced rock was blasted to a depth of 16 feet below water surface, extending across the whole width of the wide reef, which required blasting and dredging in order to secure the present depth of 16 feet.

At the close of the last fiscal year, a contract was awarded to Messrs. Carkin, Stickney and Cram, of East Saginaw,

shale rock and loose material which form the bottom of the and extend into the lake.

authority of the Chief of Engineers, the contract was extended y 30 to August 30, and subsequently to September 30, 1891. is continued under the contract until September 26, when it pleted.

ount of rock and shale measured in scows and removed sub- to June 30, was 22,271 cubic yards, and the total under the was 28,956 cubic yards.

spring of 1892 it was ascertained that the channel through had become filled. The necessity for dredging the channel tely was urgently represented. The amount required was too attract any competition if advertised, and a dredge, scows, and therefore hired by the day to do the necessary work. The lge available at the harbor was owned by the Pennsylvania Company. It was hired with the scows at \$50 per day, in- crew, and the same parties furnished a tug at the rate of \$30

xather was exceedingly bad for work upon the bar. Dredging menced April 7, and on the 6th of June it was suspended. hat time the dredge could work on the bar but 45½ hours. amount of material removed was 1,895 cubic yards. As a result about 100 feet wide was dredged to a depth of 18 feet through

nual formation of a bar at the mouth of the river must be ex- t least until after the piers have been extended into deeper the lake. How far any such trouble may be experienced after one can be determined by experience only, but it is not prob- ; the formation will be as rapid as before, because the action ves on the bottom will be less, and the drift from the river will rger area of deep water into which it may fall.

oula is one of the largest shipping points for ore on Lake Erie, necessity for deeper water in the channel is urgent. The Lake d the Pennsylvania Railroad companies are expending large in improving their facilities for receiving and shipping iron the present channel is insufficient for the demands upon it.

la Harbor is in the collection district of Cuyahoga, Ohio. There is a fixed t of the fifth order, varied by flashes, on the west pier. ter, New York, is the nearest work of defense.

priations for improving this harbor from 1826 to 1890 have le, as follows:

26.....	\$12,000.00	June 23, 1874.....	\$35,000.00
38.....	2,403.50	March 3, 1875.....	25,000.00
829.....	6,940.25	August 14, 1876.....	5,000.00
831.....	7,015.00	June 18, 1878.....	12,000.00
2.....	3,800.00	March 3, 1879.....	9,000.00
833.....	3,400.00	June 14, 1880.....	20,000.00
834.....	5,000.00	March 3, 1881.....	20,000.00
335.....	7,591.00	August 2, 1882.....	20,000.00
337.....	8,000.00	July 5, 1884.....	22,500.00
8.....	8,000.00	August 5, 1886.....	30,000.00
344.....	5,000.00	August 11, 1888.....	25,000.00
1852.....	10,000.00	September 19, 1890.....	40,000.00
553.....	42.64		
366.....	24,708.82	Total.....	467,401.21
367.....	54,000.00	Total expended to June	
371.....	15,000.00	30, 1892, including lia-	
372.....	15,000.00	bilities.....	458,734.46
373.....	16,000.00		

REPORT OF THE CHIEF OF ENGINEERS, U. S. ARMY.

Money statement.

July 1, 1891, balance unexpended	\$33,00
June 30, 1892, amount expended during fiscal year	30, 20
July 1, 1892, balance unexpended	8, 88
July 1, 1892, outstanding liabilities	23
July 1, 1892, balance available	8, 66
Amount appropriated by act approved July 13, 1892	70, 00
Amount available for fiscal year ending June 30, 1893	78, 66
{ Amount (estimated) required for completion of existing project	244, 39
{ Amount that can be profitably expended in fiscal year ending June 30, 1894	150, 00
{ Submitted in compliance with requirements of sections 2 of river and harbor acts of 1866 and 1867.	

COMMERCIAL STATISTICS.

The following statistics for the year 1891, relative to the commerce of Ashtabula Harbor, Ohio, were compiled from information furnished by the collector of customs and others:

Imports:	
Ore	1, 536
Lumber	
Pig iron	
Ice	
Miscellaneous	
Total	1, 536
Exports: Coal	50

Shipping.	No.	Tonnage.
Vessels arrived	1, 046	Not known
Vessels departed	1, 008	Do.
Vessels built	1	

Total tonnage for 1891 (estimated), 2,500,000, varying little from amount given last annual report.

No new lines of transportation have been established during the year.

The draft of the largest vessel entering the harbor is 21 feet.

The depth of water in the harbor prevents the largest vessels from loading to depth.

M M 12.

REMOVING SUNKEN VESSELS OR CRAFT OBSTRUCTING OR ENDANGERING NAVIGATION.

In the spring of 1892 information was received that two small wrecks which had been abandoned in the harbor of Port Clinton, Ohio, were obstructions to navigation at that place. One of the wrecks was the hull of the sailboat *Rescue* and the other of the steam tug *Wilcox*.

An examination of the situation was made by Lieut. Judson, from his report the officer in charge of the improvement of that harbor considered that the obstructions were such as are covered by section 4 of the river and harbor act of June 14, 1880. An estimate of \$100,000 was submitted for removal of the wrecks. This was approved and the work was done by the Secretary of War May 17, 1892.

Although the obstructions are small it was not convenient to undertake their removal previous to June 30. The work will be done at the first convenient opportunity in the summer of 1892.

M M 13.

[Printed in House Ex. Doc. No. 61, Fifty-second Congress, first session.]

PRELIMINARY EXAMINATION OF GRAND RIVER, OHIO, BETWEEN RICHMOND AND THE MOUTH.

UNITED STATES ENGINEER OFFICE,
Cleveland, Ohio, November 20, 1890.

GENERAL: In compliance with circular letter from the office of the Chief of Engineers, U. S. Army, dated September 20, 1890, I have the honor to submit the following report of "preliminary examination" for the improvement of "Grand River, Ohio, between Richmond and the mouth," as provided for in section 17 of the river and harbor act approved September 19, 1890.

It is assumed that the object of the survey or examination required is in view to deepening the existing channel of the river so that large vessels can get to Richmond docks to discharge their cargo.

Grand River rises in the northeastern part of the State of Ohio, and flowing first north, then west, and then north, after a very circuitous course empties into Lake Erie at a point about midway between its eastern and western extremities. Fairport Harbor is at the mouth of Grand River. Richmond is located on the west bank of the stream about 1½ miles from the present mouth of the river, and before the improvement Fairport Harbor was the port of entry for the neighboring country. At Richmond, and from thence down to the piers, the river is about 320 feet wide and of varying depth. An irregular channel, of least width 100 feet and least depth of 9 feet, averaging about 12 feet, now exists, but with comparatively small amount of dredging can be made 160 feet wide and 18 feet deep.

The docks of the railroad company and ore storage companies extend along the right bank of the river from point opposite Richmond to United States piers at Fairport, about 1 mile in length.

At Richmond there has been built a dock 1,000 feet long, on which there is being erected a grain elevator capable of storing 1,000,000 bushels, also two immense storage warehouses for storing general merchandise.

Fairport Harbor is now the third harbor on the list of Lake Erie harbors in order of amount of ore received and coal shipped. During the season of navigation now drawing to a close (1890), Fairport Harbor has received over 1,000,000 tons of ore as against 829,000 tons in 1889, and against only 112,000 tons in 1885, and the amount of this year's commerce bids fair to increase still more as other railroad companies are preparing to build to said harbor. With present dock facilities 100,000 tons of ore can be handled daily, and there is storage capacity for 1,000,000. The use of steam shovels for loading from dock to cars is a good dispatch to the ore.

Several improvements in the way of new docks and new hoisting ma-

chinery, and other general improvements, are contemplated before next season.

Fairport Harbor, therefore, amply repays for all moneys expended for its improvement, and deserves liberal appropriations for its speedy improvement.

Owing to the increased size of vessels now in use on the lakes in transporting ore and coal, and on account of the very large increase, in recent years, in the commerce seeking Fairport Harbor, Ohio, it is necessary and proper that all projects for the improvement of the channel should contemplate not less than 18 feet from lake to inner end of railroad docks on Grand River.

On the above basis the following rough estimate of cost to improve Grand River, Ohio, from Richmond to the mouth, to give channel 18 feet deep, is submitted:

Channel 150 feet wide, 7,000 feet long, and excavated to 18 feet depth—average cut of 3 feet—gives 115,560 cubic yards, scow measure, at	125,000	
Contingent expenses, say		2,500
Total		25,000

I am of the opinion that Grand River is worthy of improvement, and that the demands of commerce and prospective, call for the improvement of the channel at the locality mentioned.

Fairport Harbor is in the collection district of Cuyahoga, Ohio. There is a fixed white light of the third order on the shore and a beacon on the east pier.

There were no revenue collected during the eleven months ending May 31, 1890. During same period the imports, consisting of iron ore and lumber, amounted in value to \$4,000,000; and the exports, consisting of coal, amounted in value to \$192,000.

Five hundred and seventy-two vessels entered (aggregate tonnage not stated), and 460 vessels cleared, whose aggregate tonnage was 96,000 tons.

The largest cargo of vessels entering or clearing was 2,476 tons, and the deepest draft was 16½ feet.

The above statistics, obtained from the deputy collector at Fairport Harbor, were all that could be obtained, the collector of customs at Cleveland being unable to furnish items and figures desired.

In further compliance with terms of circular letter from the office of the Chief of Engineers, U. S. Army, dated September 29, 1888, I have to respectfully submit estimate of cost of survey, as follows:

FIELD WORK.

Services of an assistant engineer, say twenty-five days, at \$160 per month	\$133.33
Services of leadsmen and rodmen, say twenty-five days, at \$90 per month	75.00
Services of two laborers, as axmen and boatmen, say twenty-five days, at \$2 per day each	100.00
Hire of two boats, say twenty days, at \$1 per day each	40.00
	348.33

OFFICE WORK.

Services of assistant engineer, making estimates, profiles, maps, plotting, etc., say thirty days, at \$160 per month	\$100.00
Total	508.33

survey should be made as soon as possible if it is to be made inter.

Very respectfully, your obedient servant,
L. COOPER OVERMAN,
Major of Engineers.

g. Gen. THOMAS L. CASEY,
Chief of Engineers, U. S. A.

rough Col. Henry L. Abbot, Corps of Engineers, Division Engi-
Northeast Division.)

[First indorsement.]

NORTHEAST DIVISION, ENGINEER OFFICE,
New York, November 22, 1890.

pectfully forwarded to the Chief of Engineers, U. S. Army.
ncur with Major Overman's opinion that, for the reason stated,
nd River, between Richmond and the mouth," is worthy of im-
nent.

HENRY L. ABBOT,
Colonel of Engineers, Bvt. Brig. Gen., U. S. A.,
Engineer Northeast Division.

RY OF GRAND RIVER, OHIO, BETWEEN RICHMOND AND THE MOUTH.

UNITED STATES ENGINEER OFFICE,
Cleveland, Ohio, November 10, 1891.

GENERAL: In compliance with letter from the office of the Chief of
eers, U. S. Army, dated November 25, 1890, I have the honor to
t the following report of a survey of "Grand River, Ohio, between
ond and the mouth," as provided for in section 17 of the river and
r act approved September 19, 1890.

was assumed that the object of the survey or examination required
ith view to deepening the existing channel of the river so that
sized vessels can get to Richmond docks to discharge their cargo.
nd River rises in the northeastern part of the State of Ohio, and,
g first north, then west, and then north, after a very circuitous
empties into Lake Erie at a point about midway between its
n and western extremities. Fairport Harbor is at the mouth of
l River. Richmond is located on the west bank of the stream,
1½ miles from the present mouth of the river, and, before the im-
ment of Fairport Harbor, was the port of entry for the neighboring
ry.

Richmond and from thence down to the piers the river is about
et wide and of varying depth. An irregular channel, of least
of 90 feet and least depth of 9 feet, averaging about 14 feet, now
, but with comparatively small amount of dredging can be made
et wide and 18 feet deep.

docks of the railroad company and ore-storage companies extend
right bank of the river from point opposite Richmond to United
piers at Fairport, about 1 mile in length.

Richmond there has been built a dock 1,035 feet long, on which
has been erected a grain elevator capable of storing 1,000,000
ls, also two immense storage warehouses for storing general mer-
ise.

port Harbor is now the third harbor on the list of Lake Erie har-

season.

Fairport Harbor, which includes Grand River as far therefore amply repays for all moneys expended for it and deserves liberal appropriations for its speedy improvement.

Owing to the increased size of vessels now in use transporting ore and coal, and on account of the very recent years in the commerce seeking Fairport Harbor it is necessary and proper that all projects for the improvement for harbors and rivers should contemplate not less than a lake to inner end of railroad docks on Grand River.

On the above basis the following approximate estimate is given for the improvement of Grand River, Ohio, from Richmond to the mouth to give channel 18 feet deep is submitted:

Channel 160 feet wide, 7,310 feet long, and excavated to 18 feet depth, giving a net cut of 4.016 feet, gives 160,000 cubic yards, place measure	160,000 cubic yards, scow measure, dredging at 18 cents per cubic yard
Contingent expenses, say

Total

I am of the opinion that Grand River, from Richmond to Fairport Harbor, is worthy of improvement, and that the demands of commerce and prospective, call for the improvement of the channel mentioned. A chart (tracing) of locality is transmitted in a separate package.*

Fairport Harbor is in the collection district of Cincinnati. There is a fixed white light of the third order on the shore on the east pier.

During the eleven months ending May 31, 1891, the revenue collected was \$1,500.

The imports, consisting of iron ore and lumber, etc., amounted in value to \$4,542,493, and the exports, consisting of iron ore, etc., amounted in value to \$125,350.

Five hundred and ninety-three vessels entered with

[First indorsement.]

NORTHEAST DIVISION, ENGINEER OFFICE,
New York, November 14, 1891.

Respectfully forwarded to the Chief of Engineers, U. S. Army.
There can be no doubt that Grand River below Richmond is worthy
improvement. Whether this shall be done by the United States or
the local interests is a matter for Congress to determine.

HENRY L. ABBOT,
Colonel of Engineers, *Bvt. Brig. Gen., U. S. A.,*
Engineer Northeast Division.

M M 14.

[Printed in House Ex. Doc. No. 42, Fifty-second Congress, first session.]

PRELIMINARY EXAMINATION OF CONNEAUT HARBOR, OHIO.

UNITED STATES ENGINEER OFFICE,
Cleveland, Ohio, November 3, 1890.

GENERAL: In compliance with circular letter from the office of the
Chief of Engineers, dated September 20, 1890, I have the honor to sub-
mit the following report of "preliminary examination" for survey of
Conneaut Harbor, Ohio, as provided in section 17 of the river and har-
bor act approved September 19, 1890.

Conneaut Harbor, Ohio, is situated at the mouth of Conneaut Creek,
about 30 miles west of Erie, Pa., and 13 miles east of Ashtabula, Ohio.
The creek empties into Lake Erie near the boundary line between the
States of Pennsylvania and Ohio, and although a narrow stream pre-
sents a depth of 15 feet after passing inside the piers. The attention of
the General Government was first called to this locality for a harbor in
1809, when there was a depth of only 2 feet at the entrance to the har-
bor.

Since that date the appropriations have amounted to \$112,629.39,
the last appropriation being that of June 14, 1880, for \$6,000. The work
for the improvement of this harbor has been continued during fifty-one
years, with many interruptions and suspensions for want of funds.

The best channel depth ever obtained at the entrance was only 11
feet; the more usual depth being from 8 to 9 feet. The project for
improvement was designed to give a depth of 12 feet, but the plan was
never carried to completion for want of sufficient appropriations. The
appropriation of 1880 of \$6,000 was expended in making the most press-
ing repairs, and it was estimated by the then engineer in charge that at
least \$35,000 should be expended in renewals and repairs, and an
annual expenditure of \$1,000 to maintain piers. No appropriation has
since been made since 1880, and no work done since 1881.

The commerce of the port has always been very small. We may
therefore conclude that the hopes entertained for this harbor when its
improvement was undertaken have never been realized.

Reports that surveys were being made for extending a line of railroad
to Conneaut Harbor for a lake terminus have been circulated a number
of times, but nothing definite has yet been done.

At present the harbor is virtually destroyed; the piers are nearly
destroyed, breaches having been made in both, so that the stream now
is an outlet through the east pier, causing a bar, dry at low water,
at the entrance at end of piers. Very extensive repairs and renewals
are necessary, together with a large expenditure for dredging.

A rough estimate as to what the proposed improvement would be as follows:

For a channel entrance 160 feet wide, 18 feet deep:

Dredging from 18 feet depth in lake to inside, say 150,000 cubic yards of material, at about 18 cents per cubic yard	
Repairs and renewals of piers and revetment, estimated in 1880 at \$35,000, which now would cost not less than	
Additional renewals, repairs, and enlargement of harbor necessitated by present demands of lake commerce, say	
Contingent expenses on above estimate, say 15 per cent	

Total

With a line of railroad from the coal regions of Pennsylvania to Conneaut Harbor the commerce would doubtless increase rapidly and a harbor be frequented by vessels of the larger class now navigating the lakes, and render the reconstruction of the harbor necessary. If a line of railroad constructed, I am of the opinion that the desirability of improvement to restore the harbor and to deepen and widen entrance is a worthy one, but without such a road built or assured outlet for the improvement of the harbor is injudicious.

Conneaut is in the collection district of Cuyahoga, Ohio. The fixed white light of the sixth order upon the bank of lake near the creek, the light beacon having been removed from end of pier, in view of the dilapidated condition of the piers. Fort Porter, 12 miles distant, is the nearest work of defense.

The amount of revenue collected during the eleven months ending May 31, 1885, was \$15,950. During the same period the value of imports was \$80, and of the exports \$125.

Ten vessels, with an aggregate tonnage of 360 tons, entered the harbor, and twelve vessels, with an aggregate tonnage of 395 tons, cleared.

The above is the latest report received. The office of deputy engineer for the port was abolished in 1885.

In further compliance with terms of circular letter from the Chief of Engineers, U. S. Army, dated September 29, 1885, I respectfully submit estimate of cost of survey, as follows:

FIELD WORK.

Services of an assistant engineer, say 25 days, at \$160 per month	
Services of leadsman and rodman, say 25 days, at \$90 per month	
Services of two laborers, as boatmen and axmen, say 25 days, at \$2 per day each	
Hire of two boats, at \$1 each per day, say 20 days	

OFFICE WORK.

Services of assistant engineer, 30 days, making estimates, profiles, maps, plotting, etc., at \$160 per month	
---	--

Total

This survey should be made as soon as possible if it is to be made in winter.

Very respectfully, your obedient servant,

L. COOPER OVERMAN
Major of Engineers

Brig. Gen. THOMAS L. CASEY,
Chief of Engineers, U. S. A.

(Through Col. Henry L. Abbot, Corps of Engineers, Division of Engineers, Northeast Division.)

[Fourth indorsement.]

U. S. ENGINEER OFFICE,
Cleveland, Ohio, November 24, 1890.

spectfully returned to the Chief of Engineers, U. S. Army, through
 H. L. Abbot, Division Engineer.

thin the past few days I have received a communication from Mr.
 Dick, president of the Pittsburgh, Butler and Shenango Railroad
 any, stating that the company which he represents expects to ex-
 its lines to Conneaut Harbor, Ohio, for a lake terminus; that they
 ying track at the rate of a mile per day; that they expect to make
 nection with the "Nickel Plate Railroad," at a point 10 miles
 Conneaut Harbor, by January 1, 1891; that they expect to com-
 to Conneaut Harbor not later than June 1, 1891; and intend work-
 ; their terminal at Conneaut Harbor this winter.

view of the above I am of the opinion that Conneaut Harbor, Ohio,
 rthy of further improvement, as the prospective demands of com-
) justify such expenditure.

L. COOPER OVERMAN,
Major of Engineers.

[Fifth indorsement.]

NORTHEAST DIVISION, ENGINEER OFFICE,
New York, November 26, 1890.

spectfully returned to the Chief of Engineers. The facts stated in
 urther indorsement show that "the construction of a railroad to com-
 ate with the iron and coal regions" may now be expected at an
 day, and I accordingly concur with Major Overman in thinking
 his harbor is "worthy of improvement."

HENRY L. ABBOT,
*Colonel of Engineers, Bvt. Brig. Gen., U. S. A.,
 Engineer Northeast Division.*

SURVEY OF CONNEAUT HARBOR, OHIO.

UNITED STATES ENGINEER OFFICE,
Cleveland, Ohio, November 10, 1891.

NERAL: I have the honor to transmit herewith, in separate pack-
 a chart* (tracing) of Conneaut Harbor, Ohio, and to submit the
 ring report of a survey of said harbor in accordance with letter
 . office of the Chief of Engineers, U. S. Army, November 29,

neaut Harbor, Ohio, is situated at the mouth of a creek of the
 name, about 30 miles west of Erie, Pa., and 13 miles east of Ash-
 a, Ohio, near the boundary line between the States of Pennsylvania
 Ohio.

neaut Creek, although a narrow stream, drains a large watershed
 represents a depth of 15 feet and over after coming inside the shore

Considerable current is usually found and the volume of dis-
 re is equal or greater than that of most streams of similar width
 lepth. The banks are firm and the valley near the harbor would
 l excellent room for docks for storage of ore and coal.

e attention of the General Government was first called to this
 ty for construction of a harbor in 1829, when there was a depth of
 2 feet across the bar at mouth of the stream. Since that date ap-

* Not printed.

appropriations amounting to a total of \$112,629.39 have been expended for the harbor, a large part being for repairs to maintain the structure during fifty years.

The last appropriation was that provided by the act of June 1881 and was \$6,000 "for repairs." The appropriations were irregular and intermittent, with many suspensions, so that the plans for the improvement of the harbor, as projected, were never carried to completion. The best channel depth ever obtained was 11 feet at entrance, the depth being 8 feet.

The commerce seeking the harbor has always been small, and it is concluded that the hopes entertained for this harbor when its improvement was undertaken were never realized, mainly for want of a communication direct from the harbor to the coal regions and furnaces south of the harbor.

The failure to attract an adequate commerce to the harbor has caused Congress to omit from the regular appropriation bills during the past 20 years any allotment for Conneaut Harbor, Ohio, and since 1881 no work has been done towards keeping the harbor in repair. It is now almost completely destroyed, and the channel entrance closed by a sand bar. The piers have rotted down and been destroyed by the storms so as to be practically useless. The stream, following its natural bent, has broken through the east pier, flowing to the eastward, instead of northward through the channel provided by the piers. A large deposit of sand, etc., from the lake storms has choked the channel between the piers.

At present there is neither town nor harbor, nor any commerce, except some small fishing vessels which frequent the old harbor to discharge small quantities of fish for local consumption. Very extensive repairs and renewals, together with a large expenditure for dredging, would be necessary to restore the harbor as it was and give the 12-foot depth heretofore sought.

From a consideration of the data obtained it is considered that it would be as economical to ignore the old piers and build new, as to rebuild the old piers on old lines; such a change would also enable us to make a better location of the piers, and this is very necessary if Conneaut Harbor is to be improved to accommodate the class of vessels which are employed in the commerce of the lakes.

The additional or extra dredging rendered necessary by relocating the piers would not be much, as the widening and deepening of the present channel would necessitate the removal of at least one pier entirely, and considerable of the structure of the other pier, to straighten the channel; and the portion of the pier which is left would need to be strengthened and sustained by piling to prevent undermining. The depth of channel is dredged to 18 feet, as the piers were for only 12 feet channel depth.

SCHEME A (SEE TRACING).

To widen and deepen the existing old channel at Conneaut Harbor to secure 17 feet depth and 160 feet width between piers, extend 17 feet natural depth in lake to inside the shore end of the old channel at present located, would incur the following expenditure, approximately estimated:

To repair and partly rebuild the present west pier (the best), say
To construct a new east pier to replace old pier, to be same length as existing west pier, say 1,200 linear feet, at \$70 per linear foot.....

construct the additional length of piers to 17 feet natural depth in lake, by an extension of 800 feet for each pier, or for both 1,600 linear feet, at \$75 per linear foot.....	\$120,000
The piers to be 200 feet apart, with full channel width of 160 feet, and 160 feet channel depth. Dredging required for 160 feet width from 17 feet natural depth in lake to inside, say 2,100 feet, will require the removal of—	
120,000 cubic yards of soft material, at 18 cents.....	18,000
50,000 cubic yards of stones and gravel, at 30 cents.....	10,500
30,000 cubic yards of bowlders, etc., at 50 cents.....	10,000
retment inside of shore line for 500 linear feet on the west side and 700 linear feet on the east side, a total of 1,200 linear feet, at \$10 per foot.....	12,000
	<hr/>
contingent expenses, at least 15 per cent.....	290,500
	43,575
Total.....	<hr/> 334,075

SCHEME B (SEE TRACING).

To relocate the channel and construct new piers will incur the following expenditure, approximately estimated:

construct two new piers of the same length as the old piers, say 1,200 feet each, or 2,400 linear feet of piers, at \$75 per linear foot.....	\$180,000
construct the additional length of piers to 17 feet natural depth in lake, by an extension of 800 feet for each pier, or for both 1,600 linear feet, at \$75 per linear foot.....	120,000
dredging new channel from 17 feet natural depth in lake to inside, for a depth of 160 feet, length say of 2,100 feet, will require the removal of—	
120,000 cubic yards of soft material, at 18 cents.....	21,600
50,000 cubic yards of stones and gravel, at 30 cents.....	15,000
30,000 cubic yards of bowlders, etc., at 50 cents.....	15,000
retment inside of shore line, 500 linear feet on the west side and 700 linear feet on the east side, a total of 1,200 linear feet, at \$10 per foot....	12,000
	<hr/>
contingent expenses, at least 15 per cent.....	363,600
	54,540
Total.....	<hr/> 418,140

It will be seen from the above that the difference in cost, per the approximate estimates, between the two plans for improvement is small when compared with the total cost of either plan, and too small not to make it advisable to adopt the plan which provides for the relocation of the channel and construction of two new piers throughout.

A line of railroad to Conneaut Harbor for a lake terminus is being constructed, and the grading, it is reported, will be finished by the end of this year to within 2 miles of the harbor.

With a line of railroad from the coal regions of Pennsylvania to Conneaut Harbor, the commerce of this harbor will rapidly increase and the harbor be frequented by vessels of the larger class by which lake commerce is now transported. This will render the reconstruction of this harbor necessary, and on plan in accordance with present requirements. With a line of railroad assured, I am of the opinion that the relocation of the harbor and the widening and deepening of the channel is a worthy improvement, and the prospective demands of commerce call for the improvement.

Conneaut is in the collection district of Cuyahoga, Ohio. There is a red white light of the sixth order upon the bank of lake near mouth of creek, the light-beacon having been removed from end of pier on account of the dilapidated condition of the piers.

Fort Porter, New York, 100 miles distant, is the nearest work of defense.

The amount of revenue collected during the eleven months ending May 31, 1885, was \$15.95. During the same period the value of the imports was \$80 and the exports \$125.

Ten vessels, with an aggregate tonnage of 360 tons, entered; and twelve vessels, with an aggregate tonnage of 395 tons, cleared.

The above is the latest report received. The office of deputy collector for the port was abolished in 1885.

Very respectfully, your obedient servant,

L. COOPER OVERMAN,
Major of Engineers.

Brig. Gen. THOMAS L. CASEY,
Chief of Engineers, U. S. A.

(Through Col. Henry L. Abbot, Corps of Engineers, Division Engineer, Northeast Division.)

[First indorsement.]

NORTHEAST DIVISION ENGINEER OFFICE,
New York, November 14, 1891.

Respectfully forwarded to the Chief of Engineers, U. S. Army.

In view of the favorable location of Conneaut Harbor for the iron and coal trade, and of the construction of a railroad, now well advanced, to develop its natural advantages, I consider the harbor to be worthy of improvement.

HENRY L. ABBOT,
*Colonel of Engineers, Bvt. Brig. Gen., U. S. A.,
Engineer Northeast Division.*

APPENDIX N N.

IMPROVEMENT OF ERIE HARBOR, PENNSYLVANIA, AND OF DUNKIRK, BUFFALO, WILSON, OLCOTT, AND OAK ORCHARD HARBORS, AND OF TONAWANDA HARBOR AND NIAGARA RIVER, NEW YORK.

REPORT OF MAJOR E. H. RUFFNER, CORPS OF ENGINEERS, OFFICER IN CHARGE, FOR THE FISCAL YEAR ENDING JUNE 30, 1892, WITH OTHER DOCUMENTS RELATING TO THE WORKS.

IMPROVEMENTS.

- | | |
|--|--|
| 1. Erie Harbor, Pennsylvania. | 5. Tonawanda Harbor and Niagara River, New York. |
| 2. Preservation and protection of Presque Isle Peninsula, Erie Harbor, Pennsylvania. | 6. Wilson Harbor, New York. |
| 3. Dunkirk Harbor, New York. | 7. Olcott Harbor, New York. |
| 4. Buffalo Harbor, New York. | 8. Oak Orchard Harbor, New York. |

EXAMINATION AND SURVEY.

9. Port Day, above Niagara Falls, New York.
-

UNITED STATES ENGINEER OFFICE,
Buffalo, N. Y., July 7, 1892.

GENERAL: There are forwarded herewith the annual reports for the year ending June 30, 1892, for the harbors of Erie, Pa.; Dunkirk, Buffalo, Wilson, Olcott, and Oak Orchard, N. Y.; and for Tonawanda Harbor and Niagara River, New York.

* * * * *

Very respectfully, your obedient servant,

E. H. RUFFNER,
Major of Engineers.

Brig. Gen. THOMAS L. CASEY,
Chief of Engineers, U. S. Army.

and only 6 feet in depth), provided for closing all of the harbor by means of a breakwater, in which should be 200 feet wide, and for extending to deep water parallel piers, one on each side of the opening. This is substantially in force at the present time, excepting that it is only 6 feet apart.

Present works.—The present works consist of (1) a breakwater north and south from the main shore to the south side of the harbor; (2) a pier on the south side of the breakwater nearly east and west in position; (3) a pier on the north side of the channel parallel with and 350 feet distant from the breakwater mentioned, and (4) a catch-sand jetty, built in 1883, about 100 feet from the north pier, for the purpose of arresting the mud into the channel. The construction of this jetty is such as to not serve the purpose for which it was built, and it requires frequent repairs since its construction. These works are known as (1) the breakwater, (2) the south pier, (3) the north pier, and (4) the catch-sand jetty.

The piers and breakwater consist of timber crib-work on stone and covered with pine-plank deck.

The catch-sand jetty consists of a single row of piles driven together and bound together with oak walings.

OPERATIONS DURING THE FISCAL YEAR 1891.

Repairs to piers and breakwater.—Between stations 1 and 2 on the north pier six courses of superstructure on channel side were removed and new deck built; 60 decayed deck planks replaced with new; 600 linear feet of waling repaired and oak fenders replaced; 150 feet of the pier to provide a landing for harbor tugs repaired; on the south pier minor repairs were made to the deck, and to the breakwater at the junction with the south breakwater. Repairs were completed July 13 and completed on August 10, 1891.

Dredging in channels.—No dredging was done in the channels during the fiscal year.

channels were made monthly and the water gauge tested frequently and maintained in good order.

Extension of the north pier.—The extension of this pier eastward into Lake Erie 450 feet, more or less, under contract of J. B. Donnelly, Oswego, N. Y., was begun in June, 1891. The contractor carried on his work energetically and completed his contract on October 2, 1891. The total length of the extension was 452.15 feet. The cost of the extension was \$32,025.77, or \$70.83 per lineal foot of completed pier.

Surveillance of the peninsula.—The watchman was on duty during the entire year. He enforced the rules in regard to the building of fires and cutting of trees or any growth on the peninsula, so far as possible over so large an area (about 4 square miles). No depredation was committed.

Fires occurred on the peninsula as follows: Two small fires on the bay shore on August 30, 1891, extinguished before any damage was done; a fire in marsh grass on the bay shore of the "neck," on October 3, 1891, burned over 20 acres of marsh, but was extinguished before any damage was done to large growth; a fire in large hemlock on the bay shore at "Big Bend," on April 24, 1892, burned trees over an area of 100 feet square, before the fire could be extinguished. In addition to his duties on the peninsula, the watchman maintained a careful watch over the public property stored in the boathouse and warehouse, and acted as engineer of the steam launch.

CONDITION OF THE WORKS.

South breakwater.—This structure is old and much decayed. Its total length is 2,024 feet; height above mean lake level, $1\frac{1}{2}$ feet; width, to 12 feet. No repairs were made during the year. At the close of the year the structure was intact and no repairs urgently needed.

South pier.—Length of this pier is 1,220 feet. The pier is in fair condition, but the timbers of the superstructure are becoming soft and show decay. At the junction of the pier and south breakwater some of the stone filling has been washed out and should be replaced. To do this will cost about \$50.

North pier.—This pier is 2,437 feet long, having been extended 452 feet during the year. The westerly 993 feet of the pier is old, but with repairs to the deck, costing about \$250, it can be put in fair condition. The remainder of the pier, 1,444 feet long, is in good condition, requiring only minor repairs, which will cost about \$200.

Catch-sand jetty.—About 450 feet of the jetty remain intact but it is not considered worth repairing.

The channel.—The total length of the channel, from the 18-foot contour at low water in the lake to the same contour in the harbor, about 1,100 feet, is made up as follows:

	Feet.
from 18-foot contour at low water in the lake to the east or outer end of the north pier, outer channel.....	1, 250
between piers.....	2, 450
from west or inner end of piers to 18-foot contour at low water in harbor, inner channel.....	3, 400

The width of the channel when completely clear is 300 feet. A length of channel of 2,000 feet, lying between the piers, is kept thoroughly scoured out by the strong currents which run in and out and thus maintain the good condition at all times. The outer and inner channels require repeated dredging in order to maintain them at the required depth. No dredging was done in the inner channel throughout the year. At the close of the year it was in good condition, the depth of

water being $18\frac{1}{2}$ to 20 feet at mean lake level, or not less than 17 feet at low water, for a width of 275 feet. A point of the bar above the north pier which encroaches upon the north half of the outer channel along the pier extension, built during the first half of the year, was removed by dredging in June, 1892. At the close of the year the outer channel was also in good condition, being $18\frac{1}{2}$ to 20 feet deep at mean lake level, or not less than 17 feet at low water, for its entire width of 300 feet.

DOCK LINES.

A brief description of the manner of locating docks at Erie Harbor is given in the Annual Report of the Chief of Engineers for 1891, page 2874. No new docks were built during the year, but the increasing demand for dock room will probably lead to dock extensions in the near future. This fact emphasizes the advisability of establishing a dock line along the city front.

PROPOSED OPERATIONS.

Repairs to piers and breakwaters.—No repairs are necessary on the south breakwater at the close of the year. The repairs required on the south pier, consisting of the replacement of about 20 cubic yards of stone filling and strengthening of the pier at the junction with the south breakwater, will be made during July, 1892. Repairs needed in the easterly half of the north pier, consisting of the closing of a hole in the crib-work with sheet piling, the replacement of about 25 cubic yards of stone filling, and the renewal of 50 decayed deck plank will be made during July, 1892. The superstructure of the westerly 993 feet of the north pier is fast becoming weak. Its entire renewal will be necessary in the near future. It can be placed in fair condition for a year or two by patching the deck and fender piles and walings. It is proposed to make these repairs during July, 1892. The total cost of the above repairs is estimated at \$500.

The cost of rebuilding the superstructure over the westerly 993 feet of the pier, renewing fender piling, oak walings, and mooring piles is estimated at \$20,000.

Dredging.—Sand bars are liable to form suddenly across the harbor entrance during storms from the northeast. Should there be any serious filling in of the outer channel due to these storms the dredging necessary to clear the channel will be done promptly.

Extension of the north pier.—The existing project provides for the extension of this pier to a depth of 16 feet in the lake. To complete the project would require a further extension of 300 feet, and in order to check the rapid encroachment of the bar above the pier upon the channel this extension should be made as soon as possible and as funds become available. The cost of the extension would be 300 feet of pier, at \$80 per foot, \$24,000.

REMARKS.

The bar formation above the north pier continues to grow and encroach upon the pier. At the close of the year the shore line of the bar was at station 12 + 10, 997 feet from the end of the pier, showing no advance lakeward; but the bar had risen above water, forming a continuous shore line northward to the easterly point of the peninsula and adding another permanently closed pond to the peninsula formation

Another bar formation, about 600 feet outward from the present shore is developing. In consequence, the gradual shoaling eastward toward the end of the pier will continue and eventually necessitate further extension of this pier. The extension of the south pier, as proposed for in the existing project, is not deemed necessary or advisable under existing conditions. To complete the project would require an extension of 1,000 feet, at a cost of \$65,000.

Proposed operations may be confined, therefore, to the extension of the north pier, repairs to piers and breakwater, and dredging in the channels.

The amounts needed for repairs and dredging can not be definitely stated, as damages by storms are liable to occur at any time. Provision should, however, be made for emergencies. I would therefore estimate the needed and possible expenditures for the coming year as follows:

Extension of north pier.....	\$24,000
Rebuilding of superstructures.....	20,000
Repairs to piers and breakwaters.....	5,000
Dredging in channels.....	5,000
Total.....	54,000

Money statement.

July 1, 1891, balance unexpended.....	\$70,106.13
Received from sales of material.....	4,716.89
	<hr/>
	74,823.02
June 30, 1892, amount expended during fiscal year.....	37,131.23
	<hr/>
July 1, 1892, balance unexpended.....	37,691.79
July 1, 1892, outstanding liabilities.....	504.00
	<hr/>
July 1, 1892, balance available (includes \$20,000 reserved for Presque Isle)	37,187.79
Amount appropriated by act approved July 13, 1892.....	40,000.00
	<hr/>
Amount available for fiscal year ending June 30, 1893.....	77,187.79
	<hr/>
{ Amount that can be profitably expended in fiscal year ending June 30, 1894	54,000.00
{ Submitted in compliance with requirements of sections 2 of river and	
{ harbor acts of 1866 and 1867.	

COMMERCIAL STATISTICS.

Arrivals and departures of vessels for the year ending December 31, 1891.

Vessels.	Arrivals from—				Departures to—			
	Home ports.		Foreign ports.		Home ports.		Foreign ports.	
	No.	Tons.	No.	Tons.	No.	Tons.	No.	Tons.
Steam	908	1,022,507	41	3,176	915	1,033,930	40	3,917
Sail.....	349	138,897	16	2,515	253	137,000	15	1,624

Greatest draft of vessels, 16½ feet.

Decrease of tonnage, 1891 under 1890, 74,108.

Amount of revenue collected, fiscal year ending June 30, 1891.....	\$21,250.22
Value of foreign imports.....	67,095.92
Value of foreign exports.....	2,239.50
	<hr/>
Enrolled tonnage, port of Erie, 1891.....	25,444.60
Enrolled tonnage, port of Erie, 1892.....	24,214.99
	<hr/>
Decrease, 1892.....	1,229.61

IMPORTS BY LAKE.

[Tons of 2,000 pounds.]

Articles.	Year ending December 31—					
	1886.	1887.	1888.	1889.	1890.	1891.
	<i>Tons.</i>	<i>Tons.</i>	<i>Tons.</i>	<i>Tons.</i>	<i>Tons.</i>	<i>Tons.</i>
Merchandise.....	17,770	13,488	15,782	36,657	42,837	21,802
Limestone.....	3,360			6,000	24,434	14,223
Stone.....				17,640		4,000
Plaster.....				2,433	9,295	15,710
Lath.....	428	338	460	540		901
Shingles.....			100		110	
Produce.....					5	1
Stave bolts.....					2,488	1,506
Ice.....						
Copper.....					3,523	
Phosphates.....					500	
Barley.....	1,689	1,970	2,383	2,138	2,644	1,203
Corn.....	41,670	10,000	46,083	98,230	200,721	58,729
Oats.....			1,090	8,823	80	1,200
Wheat.....	34,360	45,062	9,530	33,815	45,354	200,132
Rye.....				2,815	6,888	17,400
Flax seed.....	13,925	3,104	2,836	250	1,500	16,000
Flour.....	133,310	91,935	96,935	162,225	143,423	97,138
Lumber.....	14,466	6,900	16,809	22,500	20,379	14,300
Pig iron.....	1,734	8,521	12,477	2,464	11,567	3,170
Iron ore.....	102,208	235,658	269,250	417,270	554,403	441,600
Total.....	364,920	416,976	473,735	804,800	1,033,796	917,737

EXPORTS BY LAKE.

[Tons of 2,000 pounds.]

Articles.	Year ending December 31—					
	1886.	1887.	1888.	1889.	1890.	1891.
	<i>Tons.</i>	<i>Tons.</i>	<i>Tons.</i>	<i>Tons.</i>	<i>Tons.</i>	<i>Tons.</i>
Coal:						
Anthracite.....	107,406	157,083	279,400	258,534	332,666	470,716
Bituminous.....	127,849	73,762	125,848	151,869	234,266	176,704
Pig and manufacturing iron.....	4,900	397	1,262		4,017	
Merchandise.....	78,490	93,228	119,236	88,600	127,074	109,600
Tar and oil.....					1	2
Stone.....					1,836	
Timber.....					409	
Total.....	318,645	324,470	525,716	499,003	700,269	756,430

N N 2.

PRESERVATION AND PROTECTION OF PRESQUE ISLE PENINSULA, ERIE HARBOR, PENNSYLVANIA.

This peninsula forms the harbor of Erie, which is a land-locked bay about 5 miles long, having a maximum width of $1\frac{1}{2}$ miles. The peninsula is a low sand formation about 6 miles long, varying in width from 300 feet at the "neck," which is nearly 2 miles long, and joins the body of the peninsula to the mainland at its western end, to $1\frac{1}{2}$ miles at its widest part. The preservation of the peninsula is of vital importance to Erie Harbor, and it is for the purpose of preserving the harbor that the protection of the weak parts of the peninsula formation has been deemed necessary. The weak portion is the long narrow neck at the western end. The object for which all the works of protection have been constructed is the prevention of a breach through this narrow neck. This danger exists during severe storms from the westward.

At the present time no works exist excepting a few dilapidated pile

no longer of service, and the main line of piles, mattresses, and ballast on mattresses, of the shore protection partially coned in 1889. No severe storms occurred during the year excepting the winter, when the shores were well protected by ice. The " " remains intact, and at its normal width and height above water.

A slight accumulation of sand along the protection work coned in 1889 is noticeable.

ng the lake front, extending a mile or more west from the flash ouse the gradual wearing away of the bank continues, but as ninsula is here one-half mile or more wide, the safety of the har- not threatened at this point. There was no work done on any of otection works during the year. A report of the Board of Engi- on the construction of shore protection for the peninsula at Erie r, Pennsylvania, was published in the Annual Report of the Chief gineers, for 1890, page 2800. In accordance with the recommen- of the Board and the approval of the Chief of Engineers, no diture will be made at present to protect the "neck" of the penin- n its lake side, and \$20,000 of the funds available for the improve- of Erie Harbor are reserved to close any breach which may occur neck of the peninsula.

Commercial statistics.—The commercial statistics are the same as those tted for Erie Harbor.

N N 3.

IMPROVEMENT OF DUNKIRK HARBOR, NEW YORK.

et.—The object of this improvement is to form an artificial harbor indentation of the shore line of Lake Erie, in front of the city of rk, N. Y.

ect.—The original project was adopted in 1827, and with its sub- it modifications provided for the construction of a pier running m the west shore of the indentation, and a detached breakwater el with the pier and about 2,000 feet distant from the city front. ening between the two structures provided a harbor entrance, h which a channel leading to the docks was to be deepened to . By 1832, the sum of \$28,439.84 had been expended on the al plan, and a breakwater was then 2,564 feet long, and the pier feet long. Subsequently various improvements and repairs were

In 1848 the breakwater was demolished.

870 the question of the improvement of this harbor was referred oard of Engineers. The Board recommended a plan which pro- for a detached breakwater 2,860 feet long, one part of which, feet long, was to be nearly parallel with the shore, the other part nearly parallel with the axis of the channel entrance, 560 feet nd terminating at the position of the dumb beacon. This break- with the pier already built, was to form the harbor, and the old al was to be enlarged to 170 feet wide and 13 feet deep. This proj- in force at the present time.

ent works.—The present works consist of an unfinished detached ater 1,341 feet in length, a part of the 2,300-foot section provided ject; a pier 1,410 feet long, exclusive of the light-house crib, and nel 100 feet wide and 12 to 14 feet deep. The breakwater and nsist of timber work, the cribs being filled with stone and decked

ations during the fiscal year.—At the beginning of the year a ct was in force with Gustavus O. Grimard, Buffalo, N. Y., for

rebuilding 150 feet of superstructure at the west end of the breakwater, building and placing one crib and 170 feet of new superstructure at the east end of the west pier, westward from the beacon, and tearing down and rebuilding an additional 50 feet of superstructure where most needed.

The contractor began operations on July 27, 1891, and by the time his contract expired on December 1, 1891, had not fully completed the construction of the 170 feet of superstructure at the east end of the west pier. In order to complete this portion of the work, and render it secure for the winter, an extension of time of contract to January 1, 1892, was granted. Under this extension the contractor was able to complete the section by December 20. No work was done on the breakwater or other parts of the pier, and on January 1, 1892, the contract was abandoned. Owing to extreme low water prevalent during the season of 1891, there was but 10 feet of water on the inner and outer bars in the channel. Vessels experienced much trouble in getting around in consequence. In order to relieve navigation as much as possible the dredge *Hingston & Woods, No. 6*, was employed to dredge out the bars to a depth of 13 feet. Dredging operations were begun on October 2 and completed on October 10. The total quantity of material dredged was 2,900 cubic yards, at a cost of \$665, or about 23 cents per cubic yard, scow measurement.

CONDITION OF THE WORKS.

Breakwater.—The superstructure for a distance of 200 feet at the west end requires extensive repairs, 100 feet thereof must be entirely rebuilt. The remainder of the structure, 1,141 feet in length, remains intact, though much decayed, and requires minor repairs to the slope.

West pier.—The 420 linear feet at the west or shore end are buried in the sand and need no longer be kept in repair. The next, following, 724 feet of pier, is in a wrecked condition and requires new superstructure. The next, following, 90 feet, are nearly new, but the crib wall on the lake side for a length of 30 feet is gone and stone filling washed out to a depth of 5 feet under water. The remainder of the pier to the beacon, viz, 170 feet, is new and in good condition.

The channels.—The channel to be maintained is about 2,800 feet long, extending from the 14-foot curve in the lake to the city docks, 170 feet wide and 13 feet deep. At the close of the year the channel was 100 to 150 feet wide and not less than 12 feet deep. Filling in takes place rapidly on account of the littoral currents carrying the sand across the channel to the outer bar. Against this the existing works afford no protection.

PROPOSED OPERATIONS.

The rebuilding and repair of superstructure on the breakwater and pier should be made as rapidly as funds become available. The cost of this work is estimated as follows:

Rebuilding 100 feet of superstructure in breakwater	\$4,000
Rebuilding 724 feet of superstructure in piers	28,960
Repairs to breakwater	
Repairs to piers	
Total	\$3,960

The estimate for repairing existing structures is less than one-third of former estimates and is based on the opinion that the structures will endure with minor annual repairs, for some years in fair condition.

ork to be done to complete the existing project is estimated to cost \$170,000. The estimate is given in detail in the Annual Report of the Chief of Engineers for 1891, page 2880. It is important that existing structures should be put in at least a fair condition before the work to be done to complete the project is begun. Of the work to be done, the construction of the arm of the breakwater, 1,000 feet in length along the eastern edge of the channel would no doubt be of the first importance, as it would afford needed protection to the harbor.

The proposed application of the appropriation asked for, for the fiscal year ending June 30, 1894, is the continuation of needed renewal and repair of existing works, dredging in channel, and continuation of the present project.

Money statement.

1891, balance unexpended	\$20,926.33
1892, amount expended during fiscal year	6,861.93
1892, balance unexpended	14,064.40
1892, outstanding liabilities	7.00
1892, balance available	14,057.40
appropriated by act approved July 13, 1892	20,000.00
available for fiscal year ending June 30, 1893	34,057.40
amount (estimated) required for completion of existing project	128,116.11
amount that can be profitably expended in fiscal year ending June 30, 1891	34,000.00

Amount appropriated in compliance with requirements of sections 2 of river and harbor acts of 1866 and 1867.

COMMERCIAL STATISTICS.

Arrivals and departures of vessels, compiled by the Collector of Customs, Dunkirk, N. Y.

Class.	Arrivals from --				Departures to			
	Home ports.		Foreign ports.		Home ports.		Foreign ports.	
	No.	Tons.	No.	Tons.	No.	Tons.	No.	Tons.
Vessels	25	5,039	3	980	26	5,156	2	361
.....	17	5,233	3	1,024	18	5,398	2	654
1	42	10,272	6	2,021	44	10,551	4	1,615

Net tonnage over season of 1890, 1,969 tons.
 Total amount of revenue collected during the year ending December 31, 1891, \$1,683.87.
 Maximum draft of vessels, 12½ feet.
 Total value of imports same year, \$21,287.97.
 Number of lines of transportation established.

N N 4.

IMPROVEMENT OF BUFFALO HARBOR, NEW YORK.

Object.—The object of the improvements is to protect the entrance to Buffalo Harbor, protect the shore south of Buffalo Creek, and to form a harbor of refuge where vessels may lie in safety.

History.—From 1819 to 1874 various projects, with their modifications, were submitted, and such as bear the approval of the War Department.

provide for the following works, to wit: North pier, south pier, masonry sea wall, sand catch or pile pier, and breakwater and shore arm.

The north and south piers are at the entrance to Buffalo Creek. The sea wall beginning at the south pier extends southward along the lake shore. The sand catch extends from the shore into the lake at a point about 8,000 feet southward of the south pier. The breakwater, commencing about on the line of the south pier extended, lies on a line parallel with the shore about one-half of a mile from it, and when completed will extend from the line of the south pier southward for a distance of 7,000 feet. The shore arm of the breakwater is intended to cover the opening between the south end of the main breakwater and the shore, leaving a passageway for vessels.

All of the above-mentioned works have been constructed, except 800 feet of the main breakwater, and the shore arm, and 220 feet of the sand catch.

The breakwater, as originally contemplated, consisted of a substructure of separate timber crib structure of timber crib work with stone. The present project contemplates the reconstruction of the work of reconstruction substituting solid concrete in place of the timber crib work. In the present project a considerable plant has been accumulated consisting of steamboat, scows, steam derricks, engines, concrete etc.

For the accommodation and large quantities of material, authority has been granted for the building of a dock with storage ground on the lake front of the south pier.

Operations during the fiscal year. At the beginning of the year work was in progress, setting concrete blocks and getting machinery ready for mixing and carrying concrete. On July 4 a heavy gale washed 362 blocks, each weighing about 7 tons, from the breakwater. With the help of a diver the blocks were all recovered, the last having been raised August 23. The laying of the concrete began August 4, and was practically finished October 12. An experimental top parapet 300 feet long, 5 feet high, was begun October 17 and finished on the 19th. All top dressing was finished November 8. Prior reports describe machinery and method of construction, and it remains to note that a great volume of concrete was quickly and successfully laid, the cable road being a perfect success, and previous experience having led to skill and promptness in the details of construction. The length of the concrete superstructure built during the season is 1,967.9 feet. The volume of the concrete is 25,969.28 yards, the cost \$213,165.69. This makes the cost per running foot \$108.32, and per cubic yard \$8.21. In 1889 this cost was \$110.19 per foot, and \$9.19 per cubic yard.

Experience and improved arrangement of the machinery, and especially the moving of cars by cable instead of by hand, account for the less cost.

The work on the extension of the breakwater was well started at the end of last year; the first crib was sunk July 17; the last crib, September 23; and the superstructure finished November 9. The length of the extension is 452.6 feet; the total cost, including the inspector, is \$60,189.91, or \$132.99 per running foot.

CONDITION OF THE WORKS.

Breakwater.—The concrete superstructure is in good condition, showing no settling or disintegration. The wooder

th of about 2,900 feet is in good condition, and will hold its own several years to come with minor repairs each year. The total length of the breakwater is now 6,803.9.

South pier.—Has settled in some places, where too close dredging by city about 8 years ago had undermined it. These places were re-dredged in 1887, but have settled again.

North pier.—The Delaware, Lackawanna and Western Railroad Company occupies this pier yet, notwithstanding the fact that the license for such occupation was revoked by the Secretary of War on February 2, 1891. It is understood that the Department of Justice will take the necessary steps to protect the interests of the Government in this mat-

Coalhouse.—Is in fair condition.

No operations are proposed with funds on hand, they not allowing of a more efficiently advantageous application. The next appropriation will be for the pier in continuing the breakwater and in building more or less of the pier arm.

Money-statement.

1, 1891, balance unexpended.....	\$269,317.42
Amount received for cost of repair of damage to work.....	173.95
Expenses.....	16.70
Total.....	269,508.07
1, 1892, amount expended during fiscal year.....	230,604.08
1, 1892, balance unexpended.....	38,903.99
1, 1892, outstanding liabilities.....	15.00
1, 1892, balance available.....	38,888.99
Amount appropriated by act approved July 13, 1892.....	300,000.00
Amount available for fiscal year ending June 30, 1893.....	338,888.99
Amount (estimated) required for completion of existing project.....	146,223.17
Amount that can be profitably expended in fiscal year ending June 30, 1894.....	146,223.17
Amount committed in compliance with requirements of sections 2 of river and harbor acts of 1866 and 1867.....	

COMMERCIAL STATISTICS.

Arrivals and departures of vessels for the year ending December 31, 1891.

Vessels.	Arrivals from—				Departures to—			
	Home ports.		Foreign ports.		Home ports.		Foreign ports.	
	No.	Tons.	No.	Tons.	No.	Tons.	No.	Tons.
Steamers.....	2,968	3,604,326	361	81,538	2,982	3,588,326	350	67,788
Sailing vessels.....	1,387	691,808	558	76,571	1,380	663,635	561	76,973
Total.....	4,355	4,296,134	1,092	199,994	4,362	4,251,961	1,006	181,520

Amount of revenue collected during the year ending December 31, 1891, \$827,595.76.
 Value of imports same year, \$4,859,602.
 Value of exports same year, \$539,588.
 Increase of tonnage in 1891, tons, 11,566.95.
 Vessels enrolled at this port, 1891, 325.
 Greatest draft of vessels, 16 feet.
 Lines of transportation established, none.

12 REPORT OF THE CHIEF OF ENGINEERS, U. S. ARMY.

Receipts by lake for the years ending December 31, 1886, 1887, 1888, 1889, 1890, and 1891.

Articles.	1886.	1887.	1888.	1889.	1890.	1891.
<i>Grain.</i>						
Wheat..... bushels..	41,430,440	48,111,189	27,548,110	26,051,600	24,868,630	79,965
Corn.....do.....	29,155,370	30,199,490	36,422,279	47,127,450	44,136,066	29,691
Oats.....do.....	1,014,679	4,656,280	7,897,310	14,209,800	13,860,780	12,044
Barley.....do.....	787,730	1,459,429	842,090	1,474,579	5,165,790	4,371
Rye.....do.....	129,630	304,540	513,720	1,906,769	1,281,630	3,000
Flour (as grain) ..do....	22,919,950	20,000,800	26,234,650	27,403,550	35,408
Totals.....	95,425,790	104,737,710	99,448,150	118,273,430	164,130,816	164,130,816
Flour.....barrels.....	4,582,190	4,001,360	5,244,930	5,480,710	6,245,590	7,402
<i>Lumber.</i>						
Headings.....barrels..	75,000	548,000
Hoops.....number.....	424,000	47,900
Lumber.....feet.....	279,493,000	264,000	279,493,000	242,525,000	287,334,000	262,720
Lath.....pieces.....	12,166,500	12,000	9,688,000	10,663,500	34,250,000	5,520
Railroad ties.....number..	54,780	80	196,600	442,570	197,110	29
Staves.....do.....	1,289,500	1,980,350	945,000	200,000	970,000	42
Stave bolts.....corals..	9,710	6,450	9,100	3,120	6,560
Shingles.....number.....	58,582,000	36,705,000	58,582,000	36,331,500	73,500,000	44,800
Posts.....do.....	255,000	7,000	4,000	15,000	48,000
Logs.....feet.....	13,659,000	31,560,000	28,750,000	24,450,000	29,400,000	46,800
<i>Other articles.</i>						
Copper.....packages.....	835
Copper.....tons.....	24,968	87,611	28,164	28,853	49,436
Copper, cakes and bars..	35,663	6,000
Flaxseed.....bushels.....	3,056,010	2,342,669	753,800	4,429,670	2,681,880	6,100
Feed.....sacks.....	386,570	404,790	774,280	810,720	1,024,700	1,100
Iron ore.....tons.....	28,430	30,760	248,850	288,000	551,940
Iron, pig.....do.....	15,510	119,780	23,220	36,400	35,919
Lead.....pigs.....	106,770	105,590	116,820	265,130	434,550
Lard.....packages.....	103,810	103,750	132,040	217,520	283,770
Oil cake.....do.....	239,230	106,520	184,730	301,340	255,550
Pork.....barrels.....	31,550	26,960	30,860	41,180	55,900
Seed.....bags.....	71,690	22,700	19,300	72,680	50,250
Malt.....bushels.....	234,640	482,710
Wool.....bales.....	36,900

Principal exports by lake, 1886 to 1889.

Articles.	1886.	1887.	1888.	1889.
Coal.....tons.....	1,562,050	1,904,060	2,546,405	2,157,810
Cement and plaster.....barrels..	378,940	413,890	370,790	105,794
Salt.....do.....	126,040	109,120	143,460	21,115
Salt.....tons.....	2,635	8,942	4,115
Railroad iron.....do.....	45,894	40,528	14,914

Principal exports by lake, 1890 and 1891.

Articles.	1890.	1891.
Coal.....	<i>Tons.</i> 2,157,810	T 2
Cement and plaster.....	105,794
Salt.....	21,115
Salt.....	2,637
Railroad iron.....	34,643
Total.....	2,321,643	2

N N 5.

ST OF TONAWANDA HARBOR AND NIAGARA RIVER, NEW YORK.

o provide a navigable channel from the entrance to the er at Lake Erie to the north end of Tonawanda Harbor. To remove obstructions so as to make a channel 400 feet feet deep, which includes work at the following places: reef known as Horseshoe Reef at the entrance of Niagara

shoal place at the head of Strawberry Island. shoal places abreast of the lower end of Rattlesnake Island. l width of the river between Tonawanda Island or Whites he main land along the entire front of Tonawanda. ct was approved in 1888.

n to the above considerable work to be done in the channel 7 water works pier and a lately discovered shoal in the Reef Channel.

during the fiscal year.—On May 30, 1891, bids were opened g rock, sand, gravel, and clay near Strawberry Island. t was awarded to Hingston & Woods, at \$3.74 per cubic k excavation, measured in place, and 74 cents per cubic l, gravel, and clay dredging, measured in place. This con- een carried out, and the contractors removed 9,086 cubic k and 4,324 cubic yards of sand, gravel, and clay. The hannel through the shoal 200 feet wide and 16 feet deep at level. The total number of cubic yards removed near Island is 28,764.

shoe Reef shoal was removed by the hire of dredge and r day and 1,650 cubic yards were removed, and much ad- bed or rolled into deep water.

0, 1892, bids were opened for removing obstructions in the er and dredging Tonawanda Harbor. The contract was Hingston & Woods, dredging clay at 21 cents per cubic und, gravel, etc., 25 cents per cubic yard.

Money statement.

balance unexpended.....	\$69,535.25
amount expended during fiscal year	40,392.86
<hr/>	
balance unexpended	29,142.39
outstanding liabilities.....	\$1,910.00
amount covered by uncompleted contracts.....	25,000.00
<hr/>	
	26,910.00
<hr/>	
balance available	2,232.39
appropriated by act approved July 13, 1892	75,000.00
<hr/>	
available for fiscal year ending June 30, 1893	77,232.39
<hr/>	
estimated) required for completion of existing project	1,008,690.16
that can be profitably expended in fiscal year ending June	
in compliance with requirements of sections 2 of river and	200,000.00
acts of 1866 and 1867.	

OF THE CHIEF OF ENGINEERS, U. S. ARMY.

COMMERCIAL STATISTICS.

Arrivals and departures of vessels from Tonawanda Harbor, New York, season of 1891

Vessels.	Arrivals from—				Departures to—			
	Home ports.		Foreign ports.		Home ports.		Foreign ports.	
	No.	Tons.	No.	Tons.	No.	Tons.	No.	Tons.
Steamers	297	144,625	78	24,452	308	150,108	71	19,110
Sailing vessels.....	694	283,567	57	21,161	703	287,047	45	13,110
Total	991	428,192	135	45,613	1,011	437,155	116	32,220

The decrease of tonnage in 1890 was 176,227.
 Revenue collected December 30, 1891, was \$72,948.22.
 Value of imports, same year, \$300,182.
 Greatest draft of vessels, 13 feet.
 No new lines of transportation established.
 Principal imports and exports, lumber.

N N 6.

IMPROVEMENT OF WILSON HARBOR, NEW YORK.

Object.—To obtain a 12-foot channel from Twelve Mile Creek to Ontario.

Project.—The first project for this harbor was submitted in 1873. No action was taken on it. The second project, submitted in 1873, to extend piers to the 12-foot curve in Lake Ontario, to dredge a channel, 12 feet deep, between the piers, and 100 feet wide from the land end of the piers to the deep water in the creek.

Present works.—The present works are the east and the west piers and a shore protection. The piers are the prolongation of piers built by private parties in 1846, and extend to the 12-foot curve in Lake Ontario. The shore protection joining the shore end of the east pier was built in 1888, and is 360 feet long. The channel has an available depth of 9.5 feet at mean lake level for a width of 100 feet from the west pier. During the fiscal year ending June 30, 1892, the lake end of the west pier has been repaired at a cost of \$75.

CONDITION OF THE WORKS.

West Pier.—The pier needs repairs, as follows: Starting from the land end, the first 200 feet is entirely rotten, and should be entirely rebuilt from the cribs; the next 150 feet is in good condition; the next 150 feet is much decayed at many parts, and in two places is much sunk out of line, dipping both to the east and to the west.

This last length should be rebuilt, at least from low water, and may possibly be necessary to rebuild a part of the cribs, in order to level them off. The remainder of the pier is in good condition, except the extreme end, which was considerably damaged during the past few years. The end crib with superstructure should be rebuilt.

East Pier.—This pier requires about 75 new planks to replace those decayed in its flooring in various places throughout its length. A portion of the west side should be rebuilt from the water line upward for a length of 50 feet, the present timbers being much rotted. It would be well also to sheath the end of this pier with plate iron. Shore protection is in good condition.

The channel.—This is now in fairly good condition. It is available, mean lake level, for vessels drawing 8.5 feet. The amount of rock developed in the course of the dredging, as required to be removed in order to obtain a full depth of water, was much greater than originally proposed. The latest examinations give the quantity to be taken out, in order to obtain a depth of 12 feet, as 4,200 cubic yards in place. The amount of dredging yet to be done is about 700 cubic yards in place. This rock should be removed at any future time, it would be well to dump it along the lake sides of the piers.

Estimated cost of the work.—In order to complete the present project a following work is necessary—the removal of 4,900 cubic yards [in place] of material from the channel. This will cost—

Place:		
4,200 cubic yards rock excavation, at \$3 per cubic yard		\$12,600
700 cubic yards dredging, at 30 cents per cubic yard		210
		<hr/>
Total		12,810
Contingencies, 10 per cent		1,281
		<hr/>
Total		14,091

The necessary repairs to the piers are estimated to cost \$13,364.20. The total cost of the work that should be done to carry out the project is:

Cleaning the channel	\$14,091.00
Repairs of piers	13,364.20
	<hr/>
Total	27,455.20

This work can all be done in one season; if only one of the items be taken up, it should be the repairs of the piers. There being no funds on hand, nothing is proposed to be done.

Money statement.

July 1, 1891, balance unexpended	\$100.25
June 30, 1892, amount expended during fiscal year	78.76
	<hr/>
July 1, 1892, balance unexpended	21.49
	<hr/>
Amount (estimated) required for completion of existing project	27,455.20
Amount that can be profitably expended in fiscal year ending June 30, 1894	27,455.20
Amount submitted in compliance with requirements of sections 2 of river and harbor acts of 1866 and 1897.	

COMMERCIAL STATISTICS.

Arrivals and departures of vessels for the year ending December 31, 1891.

Vessels.	Arrivals from—				Departures to—			
	Home ports.		Foreign ports.		Home ports.		Foreign ports.	
	No.	Tons.	No.	Tons.	No.	Tons.	No.	Tons.
Steamers			39	9,282			39	9,282
Sailing vessels.....	1	280	3	213	2	319	5	300
Barges.....								
Total	1	280	42	9,495	2	319	44	9,582
Total, 1890	5	562	46	12,067	5	562	43	12,067

Amount of revenue collected during the year ending December 31, 1891, \$163.90.

Value of imports same year, \$2,419.

Value of exports same year, \$678.

Decrease of tonnage same year, 5,527.

Vessels enrolled at this port, none.

New lines of transportation established, none.

Principal imports, lumber, shingles, and posts; principal exports, fruit.

N N 7.

IMPROVEMENT OF ÖLCOTT HARBOR, NEW YORK.

Object.—To obtain a protected channel 13.5 feet deep at mean lake level from Lake Ontario to the bridge crossing Eighteen Mile Creek at Main street.

Project.—The first project for this work was submitted in 1846, but it was not executed. The present project was adopted in June 18, 1891, and is substantially the project of 1866, with some enlargements. It provides for two piers extending into the lake with a channel between them. The piers are nearly parallel to each other, about 200 feet apart. The east pier is 850 feet long, and the west pier 873 feet. Between the piers the channel is about 180 feet wide, the limiting lines being 10 feet from the piers, and from the shore ends of the piers it decreases to 48 feet in width at the Main Street Bridge. The channel depth is 13.5 feet, measured from mean lake level.

Present works.—East and west piers, and the channel between them.

Operations during the fiscal year.—In the river and harbor act of 1890 the amount of \$30,000 was appropriated for this harbor. Contracts were made with Ira Farnsworth for the repairing of the piers, and with Hingston and Woods for channel excavation. Both contracts have been carried out. The piers have been placed in a good condition at an expense of \$8,991.87, and the channel has been excavated to the required depth at an expense of \$15,213.48. Ten thousand two hundred and eighty-seven cubic yards of rock, at the rate of \$1.24 per cubic yard, and 10,110 cubic yards of sand, gravel, and clay, at the rate 24 cents, have been removed.

Condition of works.—Both piers are in good condition, and the channel has the full depth between the limits as given.

used operations.—The project for this harbor is completed, and raising funds will be used for maintenance.

Money statement.

1891, balance unexpended	\$30,296.94
1892, amount expended during fiscal year.....	24,800.71
1892, balance unexpended	5,496.23
1892, outstanding liabilities	17.00
1892, balance available	5,479.23

COMMERCIAL STATISTICS.

Arrivals and departures of vessels for the year ending December 31, 1891.

Vessels.	Arrivals from—				Departures to—			
	Home ports.		Foreign ports.		Home ports.		Foreign ports.	
	No.	Tons.	No.	Tons.	No.	Tons.	No.	Tons.
.....	2	52	4	651	3	51	4	651
vessels.....	1	76	18	764	1	76	16	622
1.....	1	108	23	1,415	4	127	20	1,273
.....			1	12			1	12

of revenue collected during the year ending December 31, 1891, \$371.21.

of imports same year, \$1,229.85.

of exports same year, \$1,820.15.

se of tonnage same year, 2,911.

s enrolled at this port, none.

st draft of vessels, 10 feet.

ness of transportation established, none.

al imports, lumber; and principal exports, fruit.

N N 8.

IMPROVEMENT OF OAK ORCHARD HARBOR, NEW YORK.

t.—To furnish a protected channel 200 feet wide and 12 feet deep in Lake Ontario to deep water in Oak Orchard

ct.—The original plan for this improvement, adopted in 1836, and in contracting the mouth of the creek to 200 feet by two break-running from the shore, one on each side, and a channel between parallel piers from the 12-foot curve in the creek to the same curve in the lake. This project, except as to the breakwaters, is still in force.

nt works.—The present works are the east and west piers and a protection. The piers extend to the 12-foot curve in the lake.

OF THE CHIEF OF ENGINEERS, U. S. ARMY.

ection, beginning at the shore end of the east pier, and is 91 feet long. The channel has a depth of 13.5 feet for the whole width between the piers, excepting a few feet along each pier.

Operations during the fiscal year.—In the river and harbor act for 1890 the amount of \$5,000 was appropriated for this harbor. Contracts were made with Hingston & Woods to widen the channel at the rate of \$1.40 per cubic yard for rock excavation and 34 cents per cubic yard for mud, sand, and gravel measured in place. Three thousand three hundred and eighty-eight cubic yards of rock and 350 cubic yards of mud, sand, and gravel have been removed, giving a channel of 13.5 feet depth below mean lake level.

Condition of the works.—The east pier and the shore protection are in good condition and need but slight repairs—a few planks for decking. The west pier has settled for a length of 130 feet about 2½ feet on the harbor side, where it intersects the shore line; also on the outer end toward the lake. Both settlements were caused by very rough weather in the fall of 1891 during the low stage of water, 2.2 feet below mean lake level, under the piers in those places. The channel is in good condition.

Proposed operations.—If the weather should on the one hand allow it, the settled portion of the pier will be repaired. The object of this harbor has been completed, but a certain amount of money should be appropriated to keep it in repair.

Money statement.

July 1, 1891, balance unexpended.....	\$5,485.
June 30, 1892, amount expended during fiscal year.....	4,963.
July 1, 1892, balance unexpended.....	522.
July 1, 1892, outstanding liabilities.....	8.
July 1, 1892, balance available.....	513.

COMMERCIAL STATISTICS.

Arrivals and departures of vessels for the year ending December 31, 1891.

Vessels.	Arrivals from—				Departures to—			
	Home ports.		Foreign ports.		Home ports.		Foreign ports.	
	No.	Tons.	No.	Tons.	No.	Tons.	No.	Tons.
Steamers.....	6	131	4	692	6	131	1
Sailing vessels.....	1	76	4	692	4	586	1
Total.....	7	207	4	692	7	717	1
In 1890.....	5	250	5

Amount of revenue collected during the year ending December 31, 1891, \$196.8

Value of imports, same year, \$2,282.

Increase of tonnage same year, 1,298.

Greatest draft of vessels, 10 feet.

New lines of transportation established, none.

Principal imports and exports, lumber, shingles, and posts.

N N g.

[Printed in House Ex. Doc. No. 67, Fifty-second Congress, first session.]

**PRELIMINARY EXAMINATION OF PORT DAY, ABOVE NIAGARA FALLS,
NEW YORK.**

**UNITED STATES ENGINEER OFFICE,
Buffalo, N. Y., February 28, 1891.**

GENERAL: In accordance with the provision of the river and harbor act of September 19, 1890, directing an examination of Port Day, above Niagara Falls, and Department letter of September 20, 1890, assigning the examination to my charge, I have the honor to submit the following report of preliminary examination:

Port Day is the name given to the head, or entrance from Niagara River, of what is known at Niagara Falls as the Hydraulic Canal, and is about 2,000 feet above the head of Goat Island.

In November, 1880, an examination of this locality was made under the direction of Maj. Walter McFarland, and his report may be found in the annual report of the Chief of Engineers, U. S. Army, for fiscal year ending June 30, 1881, page 2454. From this report it will be learned that at the time of the first examination the improvement contemplated was to provide a safe water communication between Niagara Falls and Tonawanda, on the Niagara River, 10 miles above, where connection could be had with the Erie Canal. The improvement suggested in the report was the making of a good and secure channel through which canal boats drawing 5 feet of water could pass to Port Day, by making a cut close to the American shore through the rocky ledge forming the Schlosser Rapids, the estimated cost of which was \$17,000.

After some correspondence with the authorities of Niagara Falls, my inquiries concerning the nature and extent of improvement contemplated and the demands of commerce for such improvement were referred to the Cataract Construction Company, who are engaged in the construction of the tunnel and works of the Niagara Falls Power Company. Upon meeting the resident engineer of the company, Mr. Albert H. Porter, at Niagara Falls, I was shown the plans of the company, and was informed that the improvement of river desired was to furnish a channel for the navigation of lake vessels to convey raw material to, and the manufactured products from, the extensive factories and works, in addition to those now existing, which it was expected would in the near future be established at Niagara Falls upon the completion of the development of the enormous water power now under way.

The plans upon which work is in progress are estimated to develop 10,000 horse power, which is said to be equal to the water power of

Lawrence, Holyoke, Turners Falls, Manchester, Bellows Falls, Lewiston, Cohoes, Oswego, Patterson, Augusta, Ga., Minneapolis, Rochester and Lockport combined. With this immense power developed, with uncertainty concerning its continuance, and in a central and otherwise favorable locality, it would seem that a very large commerce must soon be built up which would justify very considerable improvements in the way of providing for cheap water transportation. This prospective commerce, however, though it may be considered well assured, is in the future more or less distant.

At this present the commerce of Niagara Falls may be estimated the following statement of annual output of manufacturing establishments as furnished by the Business Men's Association of Niagara Falls:

Articles.	Quantity.	Value.
Flour..... barrels.....	942,000	\$4,000,000
Paper and pulp..... tons.....	9,156	1,000,000
Barrels.....	603,600	1,000,000
Blank books.....		
Beer..... barrels.....	40,000	1,000,000
Other industries.....		
Total.....		\$8,000,000

Twenty-seven thousand cars of mill freight are now handled each year.

From the above it will be seen that this locality is already a manufacturing center of no small importance, and with the greatly increased future importance which seems to be well assured to it, it appears to be a locality worthy of improvement with regard to its communications.

Work now in progress on the upper part of the Niagara River furnishes a channel 16 feet in depth as far as Tonawanda, and from Tonawanda to a point near the head of Conners Island, about 2½ miles. At Port Day, there is shown on the lake survey chart a channel with a least depth of 11 feet, and for most of the distance a least depth of 12 feet. This channel is on the east side of Grand Island. A similar channel exists on the west side of this island, but to reach Port Day from the latter channel would necessitate the crossing of the river at a point not more than 2½ miles above the falls, which is entirely too arduous for the ordinary and constant use of a commercial highway.

An examination of the accompanying map, showing the soundings recently made by the Cataract Construction Company from Port Day up to a point above Conners Island, indicates that a navigable channel could be made on a line just inside of Grass and Conners islands at a comparatively small cost. Unfortunately, in the company's soundings were not made far enough out in the stream to show the nature of the bottom in the deeper water, but such soundings as were made indicate that there is considerable gravel, and the bottom contours just inside of the two islands would seem to show that the gravel is of sufficient depth that a good channel could be made without much rock excavation, except on the crossing of Schlosser Rapids. The current above and below these rapids is not of very great velocity, the water surface having a gentle slope, while in crossing the rapids it falls about 10 feet.

An embankment extending from Port Day to the foot of Grass Island and from the head of the island to the head of the rapid, about 100 feet altogether, would give an additional depth of 1 foot below the

make a safe entrance to Port Day. In my opinion it would be
e to make a survey at this locality to ascertain more definitely
r surfaces, and the nature of the bottom that would have to be
d to provide a channel with a depth, say, of 13 or 14 feet. The
ould cover the distance from Port Day to deep water just
wners Island, and a few points on the river above. The esti-
st of such a survey is \$500. I am indebted to the courtesy of
art Porter, engineer of the construction company, for a map*
ings in the vicinity of Port Day, a copy of which is appended,
with a map* of the Niagara River from Tonawanda to Niagara
larged from the lake survey chart.
ry respectfully, your obedient servant,

AMOS STICKNEY,
Major of Engineers.

Gen. THOMAS L. CASEY,
Chief of Engineers, U. S. A.

gh Col. Henry L. Abbot, Corps of Engineers, Division Engi-
rtheast Division.)

[Third indorsement.]

NORTHEAST DIVISION, ENGINEER OFFICE,
New York, March 10, 1891.

stfully returned to the Chief of Engineers, U. S. Army. It is
on that Port Day above Niagara Falls is worthy of improve-

HENRY L. ABBOT,
Colonel of Engineers, Brevet Brigadier-General, U. S. Army,
Engineer Northeast Division.

VEY OF PORT DAY, ABOVE NIAGARA FALLS, NEW YORK.

UNITED STATES ENGINEER OFFICE,
Buffalo, N. Y., December 31, 1891.

AL: I have the honor to submit the following report on the
f Port Day above Niagara Falls, N. Y., and upon plans with
s of cost for navigable channels to that point.

report of February 28, 1891, upon the preliminary examina-
his locality, it was stated that the improvement desired was to
navigable channel for lake vessels. In order to obtain the
y data for planning such a channel, and for making estimates
t was necessary to make a survey that would not only cover
ediate vicinity of Port Day, but several points on the river
e lake survey charts showed shoal places as far up as Tona-
Authority having been received for such a survey, a party was
the field under the charge of Mr. Ernest Siegesmund, assist-
neer. The principal part of the survey was made in July and
1891, and was supplemented by a few days' work in December,
supply some information which the plotting of the notes showed
eded. The results of the survey are presented in the charts

* Omitted.

...nying this report. The soundings in the charts are possible to a stage of river corresponding to a stage 10 feet below the mean level at Buffalo. This stage very near corresponds to the lowest stage of the river at Port Day during the past five years. Any lower stage would be of exceedingly rare occurrence. In the adopted project for the improvement of the Niagara River from Lake Erie to Tonawanda the channel is 400 feet wide and 18 feet deep at mean lake level, which is intended to accommodate the largest class of lake vessels.

To continue the channel for vessels of this class from Tonawanda to Port Day, maintaining the 18-foot depth, but with decreased widths, would cost an exceedingly large sum of money, far beyond any amount the expenditure of which would be justified by the possible demands of commerce in the near future. To make such a channel at the least cost, work would be required over a great stretch of river and beyond the limits of this survey, as it would be to reduce the excavation to a long embankment, extending to the westward of the mouth of the river. This is made evident by the estimate of the amount of excavation necessary for a channel from Tonawanda to Conners Island, and in open water, which shows that 1,966,499 cubic yards of solid rock would be excavated, which, estimated at \$4 per yard, would cost \$7,865,996. An inquiry then presents itself as to whether any channel of sufficient size to accommodate the largest class of lake vessels, of 12 feet at mean lake level, and 18 feet at low-water stage, a large amount of rock excavation would be necessary between Tonawanda and the head of Conners Island, in addition to the excavation that will be necessary for any channel below Conners Island. For that depth (12 feet at mean lake level) there is a natural channel, of ample width, for the whole distance from Tonawanda to near Conners Island, except the crossing of a reef about 9,000 feet above Cayuga Island, where the channel would have to be excavated for a distance of about 1,500 feet. As a channel of this depth would permit the passage of a smaller class of lake vessels, an estimate of the cost is presented, which is \$1,341,029.80. This is still a very large sum, but a channel with less depth than 12 feet at mean lake level for lake vessels is hardly worth considering.

The proximity of the Erie Canal, connection with which can be made from the Niagara River at Tonawanda, makes it proper, however, to consider smaller channels such as would serve for the passage of canal boats. Estimates are therefore added for a channel of 7 feet depth at low water, which is the depth on the lock sills of the Erie Canal, and for 6 feet depth at low water, which is 8 feet at mean lake level, and which would have a depth of 7 feet for the greater part of the season of navigation. An examination of the charts shows comparatively deep water in the river within about 1,000 feet of Port Day, but it can not be utilized for a channel for navigation, as it is within 1 mile of the falls; and even if it were possible to make a safe channel by inclosing it with a dike, there would still be required a large amount of excavation to make harbor room inside of Grass Island. I therefore deem imperative to locate the channel inside of the line connecting Conners and Grass islands, where, inside of a dike, vessels would be safe, and the approach to which from the river above would be safe. In all of the plans presented the width of the channel provides as follows: In open river, 300 feet; from Conners Island to

200 feet; from head of Grass Island to Port Day, a gradual ascent from 200 feet to 400 feet, to allow for harbor room and space. In all of the plans provision is made for a loose stone embankment extending from Conners Island to Port Day. This embankment would be made entirely from the rock excavated from the channel, suitable portions of which could be selected and laid so as to face on the slopes. The embankment would serve three purposes: First, it would be a guard to prevent vessels being drawn to the falls; second, it would raise the water surface in the channel, more than pay for itself in reducing the amount of excavation; third, it would be a guide for vessels navigating the channel. At the time of the survey the fall of water surface from the head of Conners Island to Port Day was 1.795 feet. It is estimated that with the embankment this fall would not exceed two-tenths of a foot, as the expense of water would be only that which leaked through the stone embankment and the amount used by mills. The variation in the water surface of the river depends, of course, upon the variation of the level of Erie at the head of the river, except in the case of minor oscillations due to wind, which are of short duration. An examination of the records kept in this office of the readings of the Erie water gauge shows the following stages of the water during the periods of navigation from May 1 to December 1 for five years, from 1891, inclusive:

Year.	1 to 2 feet above zero.	Zero to 1 foot above.	Zero to 1 foot below.	1 to 2 feet below zero.
	<i>Days.</i>	<i>Days.</i>	<i>Days.</i>	<i>Days.</i>
.....	19	148	44	3
.....	0	71	135	8
.....	1	33	123	57
.....	14	114	86	0
.....	1	2	144	67
.....	35	368	532	135

zero of the gauge is at the mean lake level. The level of the lake is below a stage of minus 2 feet so seldom as not worth considering. From the above table it will be seen that the one thousand and seventy days of navigation of the past five years the lake surface was more than 1 foot below zero 12.62 per cent total number of days; below zero, 62.34 per cent; at or above 7.66 per cent; more than 1 foot above zero, 3.27 per cent. In a channel 12 feet deep at mean lake level, or 10 feet at a stage of minus 2 feet there would be 10 feet least depth practically at all times. Eleven feet depth or more for 87.38 per cent of the time; 12 feet depth or more for 66 per cent; 13 feet depth or more for 3.27 per cent. The excursions of the lake surface at Buffalo during the past five years were .99 feet, being from 3.2 feet below to 7.79 feet above zero. The excursions of the Niagara River surface at Port Day during the past five years, was 6 feet, being from 2 feet below mean surface to 4 feet above mean. The highest and lowest stages were caused by storms. The following is a summary of the estimates of cost of navigable channels of various depths from Tonawanda to Port Day:

12 feet depth at mean lake level (if excavated in open river above Conners Island)	\$8,694,824.60
10 feet depth at mean lake level	1,341,029.80
8 feet depth at mean lake level	380,377.80
6 feet depth at mean lake level.....	257,829.00

With regard to the commercial need for a navigable channel to Day, I have to add to what was stated in my preliminary report the commerce of Niagara Falls has increased about 20 per cent in the year. I am informed that it is the intention of the company controlling the Hydraulic Canal to double the capacity of the canal, which furnishes the power for most of the manufacturing establishments at Niagara Falls. The Cataract Construction Company, engaged in the development of an enormous water power, have vigorously pushed work on one of their tunnels during the past year, and it is now nearly completed. This one tunnel, it is estimated, will furnish one hundred thousand horse power, and negotiations are now pending for the use of sixty thousand horse power by various manufacturing interests. Work on another tunnel of equal or greater capacity will, it is understood, be commenced in a short time.

The completion of the development of this enormous power, under the control of a company whose stockholders represent an immense amount of capital, can not fail to build up a manufacturing center which will consume large amounts of raw material, such as grain, lumber, or iron, that will be largely obtained from the Northwest, whence transportation by vessels on the lakes will be the cheapest method of getting these articles to the mills, and after being manufactured into the products of the mills their return to the westward by the lake transportation eastward by the Erie Canal will be the cheapest method of getting them to their markets.

As stated in my preliminary report, this very large prospective commerce, though seemingly well assured, is in the future; but it is already a considerable commerce, and the assurance of great increase in an early day is very strong.

The only navigation of the river below Tonawanda at present is by a steamboat of light draft (5 feet) running between Buffalo and Niagara at the foot of Sugar street, Niagara Falls, opposite Conners Island. The principal business of the steamer is to carry excursionists.

If it should be decided to enter upon the work of improving this part of the river I would recommend that a beginning should be made by providing for a navigable channel of the lesser depth which could be increased as the increase of commerce should demand. A channel 8 feet deep at mean stage is estimated to cost \$1,000,000. According to the experience of the past five years, such a channel would provide a least depth of 6 feet at all times during the season of navigation, a least depth of 7 feet for 87.38 per cent of the time, and a least depth of 8 feet 37.66 per cent of the time. No work was required above Conners Island. A water route would be opened between Tonawanda and Buffalo, and by connection with the Erie Canal to western points, which would be navigable for the largest class of vessels that can use the canal. A channel with a least depth of 12 feet at mean stage, extending as far down the river as Conners Island, could be made at an estimated cost of \$149,749.60. Attention is invited to the accompanying letters of Messrs. Albert H. Porter, John J. Macdonald, and Lautz & Long.

I am indebted to Mr. Albert H. Porter, resident engineer, and George B. Burbank, consulting engineer of the Cataract Construction Company, for information and courtesies extended.

The report of Mr. Ernest Siegesmund, assistant engineer, is appended. Accompanying this report are the charts* (five sheets) showing the results of the survey, with channels laid out upon them; also a

* Omitted.

of the town of Niagara, N. Y., showing the hydraulic canal, of tunnel, and proposed canal of the Niagara Falls Power Company, the shore line of the river up to and including Cayuga Island, with oad connections.

Very respectfully, your obedient servant,

AMOS STICKNEY,
Major of Engineers.

Fig. Gen. THOMAS L. CASEY,
Chief of Engineers, U. S. A.

REPORT OF MR. ERNEST SIEGESMUND, ASSISTANT ENGINEER.

UNITED STATES ENGINEER OFFICE,
Buffalo, N. Y., December 19, 1891.

MAJOR: According to your order dated July 23, 1891, to survey the Niagara River above Port Day to determine a plan for making a safe navigable channel for large boats from Tonawanda to Port Day, I made preparations at once and started from Port Day July 24, in the morning, with a steam launch, a rowboat, one recorder, and two laborers. Neither the captain of the steam launch nor I had ever been at Port Day, but as we had to go there with the launch, I laid out a course northerly of Cayuga Island, near the mouth of Chippewa Creek to Port Day, according to the lake survey of 1875. This course proved the correctness of the map in this vicinity very roughly. We arrived safely, to the great astonishment of many persons watching our crossing of the river.

Port Day is the name of the inlet of the hydraulic canal, half a mile above the falls, in the village of Niagara Falls, furnishing water power for several flour and barrel factories, paper mills, etc. This canal was planned and commenced by James Porter in 1847. But this was not the first attempt to utilize the water power of the Niagara River. It was made in 1767 by a British officer, De Peyster, who erected a sawmill near the foot of the present mill stream. In 1800, Mr. Stedman erected mills a little below this point.

In 1886, the Cataract Construction Company was incorporated and commenced its operations October, 1890, to develop 120,000-horse power for manufacturing purposes. This company must be called the instigator of the present movement to improve the Niagara River above Port Day. It is not only the owner of the power, but also of many acres of land along this power, and it has the intention to erect factories as well as sell privileges. It is desirable for these factories to have the material of the West brought directly to their doors. To do this the Niagara River needs deepening.

A report of the preliminary examination was made February 28, 1891. It recommended a survey of the river as "worthy of improvement." This recommendation was approved; \$500 allowed, and the survey ordered March 11, 1891. The survey commenced July 24, and was finished August 15, 1891. After plotting the field notes of this survey, it was found necessary to cover about 4,000 feet westerly from sheet 1, near Gratwick's dock, with soundings in order to get the extent of the ledge extending across the river at this point. These soundings were taken December 10 to 1891, and at once plotted. I started at Port Day and surveyed the shore line to Conners Island, a distance of about 2½ miles. Triangulations were made to Cayuga Island, Buckhorn Island, and Grand Island, but I found that the limits of your survey were somewhat too far extended to be covered with the amount of money on hand, and with your consent I narrowed it down; that is, I covered all the essential points on the American side and left out the Canada side, Navy Island, Buckhorn Island, and Grand Island.

The survey has been divided in three parts:


- .1 from Port Day to above Conners Island.
- .2 above, in front, and below Cayuga Island.
- .3 below Gratwick's dock.

Three points were selected, as an examination of the lake survey map of 1875 showed shoal places in these localities.

The running of the shore line with the transit, as shown on Sheet No. 2, was commenced.

Soundings were taken in the following manner: All the lines from the shore

* Omitted.



observations had been taken by the Cataract Construction Com these gauges were read mornings and evenings during the sun since 1886 show that the mean stage of the water at Port Day this gauge of 575.246. The lowest water during this period oc gauge reading 573.2. All soundings are reduced to this gauge 1 low mean water level. The highest water occurred January 10 ing 579.2.

The average gauge reading during the survey at Port Day w mean stage; at Cayuga Island, -0.2 foot below mean stage; at G foot below mean stage; difference between highest and lowest period 0.4 foot. Two extra gauges were set above and below S a line of levels run from the Port Day gauge to these gauges, to ing opposite Conners Island, at the foot of Sugar street, and to Island, giving a rise of water surface of 1.795 feet between Port of Conners Island.

Borings were made around the islands and along the shore, v tion of mud and sand had taken place.

On all important points current measurements were made w kindly loaned by Albert H. Porter, resident engineer of the Ca Company, at Niagara Falls.

The bottom of the first section is all bare rock; only around th the main shore is a small accumulation of mud, sand, etc. 1 steamboat landing at the foot of Sugar street went through 4 fe

The second section at Cayuga Island is also rock bottom, he thin covering of sand or mud in the indentations of the rock su mulation along the island. Piles driven for a dock went throug

The third section at Gratwick's dock is rock in the center of t main shore and Grand Island is some sand and mud. After plot you determined to submit four projects for the improvement by

1. A channel, with embankment for vessels of all sizes now nav feet deep at low water. The estimate is based on rock ex river for the whole length.
2. A channel with embankment 10 feet deep at low-water mark.
3. A channel with embankment 7 feet deep at low-water mark.
4. A channel with embankment 6 feet deep at low-water mark.

This low-water mark is 2 feet below mean stage of the rive respectively call for 18 feet, 12 feet, 9 feet, and 8 feet at the mea The first project would give a continuous channel of 18 feet dep Port Day, as the project for improving the Niagara River from wanda, giving an 18-foot channel, is already approved and under

low water. The embankment will be 6 feet wide on top with slopes 1:1, above mean, the highest water known according to gauge readings at 1886. The center line of this embankment is intended to be 50 feet from the center line of the channel, and is 9,000 feet long. The embankment will inclose the area from Conners Island to Grass Island and to Port Day about 1.59 feet, allowing 0.2 fall for the embankment. No excavation is needed near Cayuga Island for a 6-foot, 7-foot, or 10-foot channel; the 16-foot channel requires excavation for a length of about 10,000 feet; for a 6-foot, 7-foot, or 10-foot channel a length of about 10,000 feet must be excavated for a 16-foot channel. No excavation necessary for a 6-foot or 7-foot channel; for a 10-foot channel a cutting through a ledge across the river for a length of about 1,500 feet; for 16-foot channel a length of about 10,000 feet must be excavated.

ESTIMATED COST.

Project No. 1.

Excavation, 1,966,499 cubic yards in place, at \$4 per cubic yard	\$7,865,996.00
Embankment: Placing 83,300 cubic yards of loose rock in line of channel, at 25 cents per cubic yard	20,825.00
Embankment with selected stones laid by hand, 23,420 square yards, at 75 cents per square yard	17,565.00
	7,904,386.00
Contingencies, 10 per cent.	790,438.60
Total	8,694,824.60

Project No. 2.

Excavation, 295,182 cubic yards in place, at \$4 per cubic yard	\$1,180,728.00
Embankment: Placing 83,300 cubic yards of loose rock in line of channel, at 25 cents per cubic yard	20,825.00
Embankment with selected stones laid by hand, 23,420 square yards, at 75 cents per square yard	17,565.00
	1,219,118.00
Contingencies, 10 per cent.	121,911.80
Total	1,341,029.80

Project No. 3.

Excavation, 76,852 cubic yards in place, at \$4 per cubic yard	\$307,408.00
Embankment: Placing 83,300 cubic yards of loose rock in line of channel, at 25 cents per cubic yard	20,825.00
Embankment with selected stones laid by hand, 23,420 square yards, at 75 cents per square yard	17,565.00
	345,798.00
Contingencies, 10 per cent.	34,579.80
Total	380,377.80

Project No. 4.

For a channel 6 feet deep at low water or 8 feet at mean water level, 200 feet with embankment from Conners Island to Port Day:

Rock excavation, 49,000 cubic yards in place, at \$4 per cubic yard....	\$196
Embankment: Placing 83,300 cubic yards of loose rock in line of embankment, at 25 cents per cubic yard	20
Facing with selected stones laid by hand, 23,420 square yards, at 75 cents per square yard	17
	234
Contingencies, 10 per cent.	23
Total	257

Finally, please allow me two remarks: Would it not be most economical and advantageous to the Government, if any of the projects for improvement of the Niagara River above Port Day is approved and adopted, to let the whole work in one tract and not divide it into small contracts according to the appropriations made? It requires a special plant adapted for this locality. None of the drill boats, dredges or scows at the present time on the lakes could do the work on account of lack of water.

If any one of the projects is adopted, would it not be well to connect the Bull gauge with the Port Day gauge by a line of levels, establish bench marks, and water gauges on all points where improvements are contemplated, in order to have more definite data than at present on hand? The mean lake level is known; the mean stage of the water at Port Day is also known. What is needed is to establish the mean stage of the water on all necessary points along the river, a distance of about 23 miles.

Annexed please find a letter from Mr. MacIntire, vice-president of the Business Men's Association at Niagara Falls, giving statistics of the place from January 1, 1891.

A letter from A. H. Porter, resident engineer of the Cataract Construction Company, giving details of the work done during the same period.

A letter from Messrs. F. C. M. Lantz and Long, proprietors of Cayuga Island, setting forth their intentions.

Very respectfully, your obedient servant,

ERNEST SIEGESMUND,
Assistant Engineer

Maj. AMOS STICKNEY,
Corps of Engineers, U. S. A.

LETTER OF MR. JOHN J. MACINTIRE.

NIAGARA FALLS, N. Y., November 4, 1891

MY DEAR SIR: In reply to your inquiry of this date regarding the amount of business done by the different industries at this place, I beg to hand you the enclosed statement. The figures given are as near the actual figures as can be given without going over the records of the different firms. I take pleasure in calling your attention to the rapid increase in the volume and value of the output of our mills and factories during the past year.

The best indication of the rapid growth here is the increase in the freight traffic by the railroads here, said increase being 20 per cent during the past year.

I trust the information given will be such as you require.

Very truly yours,

JOHN J. MACINTIRE

E. SIEGESMUND, Esq.

and value of output of the mills and factories at Niagara Falls, N. Y., January 1 to November 1, 1891.

Industries.	Output.	Value.	No. of employés.	Wages.
.....	954,000 barrels ..	\$5,247,000	120	\$54,000
lts.....	12,000 tons.....	875,000	148	52,500
s.....	5,400 tons.....	129,600	42	17,260
arks.....	568,000 barrels ..	189,450	136	51,400
company.....	36,000 barrels ..	252,000	43	22,650
ts and chain works.....	209,000	228	56,200
sk manufactory.....	543,000	176	41,750
irms.....	293,000	178	112,160
power company.....	20,000	8	5,000
works.....	38,000	28	17,320
s.....	178,000 boxes ..	11,000	36	3,900
l.....	7,407,050	1,143	434,170

Eight business now averages per day, 70 car loads in; 45 car loads out. has been signed and recorded at the county clerk's office between the Soo Company of Chicago and the Niagara Falls Power Company for 3,000 per for forty-five years at \$24,000 a year. The first quarterly payment of rental rent has to be made December 1, 1892. The Soo Paper Company's mills so it is said, the largest establishment, both wood pulp and paper, in the States, perhaps the world. They will be built principally on made land, Tenth, Buffalo, the southern extension of Bademan street and the Niagara rd will cover from 11 to 12 acres. Their output is expected to be more than doubled output of the present four paper and three pulp mills.

OF MR. ALBERT H. PORTER, ENGINEER, CATARACT CONSTRUCTION COMPANY.

NIAGARA FALLS, N. Y., November 14, 1891.

SIR: Your letter of October 31 in reference to notes of progress on the tunnel the report of your office is received.

Present contract includes the construction of a tunnel and open cut 6,700 feet, soffit of which lies at an average depth of 170 feet beneath the surface, the lining seven-tenths per cent.

Cross section is to be 335.5 square feet, and of uniform area; excavation to when necessary with permanent timbering in five block timber arches with and tunnel to be lined throughout with four rows of brick.

Work was commenced on October 4, 1890. On November 1, 1891, 3,437 feet of had been driven, and 2,938 feet of bench had been taken out, 3,093 feet of remaining yet to be excavated.

3,437 feet of heading driven, 2,877 feet was permanently timbered, 394 feet or masonry section.

Headings and first bench are to be taken out until headings meet, when the bench is to be excavated to the required depth, and the masonry put in as as the preparation of the excavation admits.

Quantities of brick and other material are already on the ground.

Greater part of the tunnel thus far constructed lies in gray slate and shale, string temporary timber support in headings for the safety of the men, and that the permanent timbering be kept up within a short distance of the

780 feet from the east bank of the lower river a stratum of Queenston limestone encountered which forms a substantial roof, no permanent timber being used. At this point rapid progress was made, 78 feet of heading being driven one week of thirteen shifts.

Company is at present excavating the first section of their canal, and filling embankments on the lands under water, forming the same. The earth is loosened by being thrown into scale boxes, which are raised from the pits by travellers mounted to run along the banks. These boxes are placed on trucks from the derricks and rapidly hauled to the point of fill. Excellent rock side walls is being taken from the excavation, steam drills being used on the Both centrifugal and cylinder pumps are used in keeping excavation clear of

Yours very truly,

ERNEST SIGESMUND,
Assistant Engineer.

ALBERT H. PORTER,
Engineer.

LETTER OF MESSRS. FRED. C. M. LAUTZ AND BENJAMIN G. LONG.

BUFFALO, N. Y., July 31, 1891

DEAR SIR: The undersigned, owners of Cayuga Island, desire to call your attention to the fact that we have purchased and commenced to improve the aforesaid island to the end that it may be utilized for commercial purposes. Cayuga Island containing 140 acres, has a river frontage of 6,800 feet which can be utilized for dockage if a navigable channel connecting with Lake Erie is brought contiguous to it.

You are undoubtedly aware of the fact that the river front in and about Tonawanda has already been taken up with docks and mill sites, and that a move farth down the river for commercial purposes, especially in view of the tunnel developments at Niagara Falls, is inevitable.

A careful consideration will convince any one that Cayuga Island is the next available river frontage down the river. With the foregoing in view, a terminal railroad company has been organized, who have laid out and secured the right way for a railroad upon and around the island to connect with the New York Central and Erie railroads upon the main land, so that so soon as a proper channel shall be made to or near the island mills may be erected and lumber yards established which will be in reach of the cheap power from the falls tunnel, and be contiguous to the river navigation, and also have railroad facilities unsurpassed elsewhere in the country.

Our object in securing this property is to so improve it that the foregoing commercial undertakings may be properly equipped in every way for economical business purposes.

Respectfully submitted,

FRED. C. M. LAUTZ
BENJAMIN G. LONG

MR. ERNEST SIEGESMUND,
U. S. Assistant Engineer.

APPENDIX O O.

IMPROVEMENT OF HARBORS ON LAKE ONTARIO EAST OF OAK ORCHARD,
NEW YORK.

REPORT OF CAPTAIN DAN C. KINGMAN, CORPS OF ENGINEERS, OFFICER IN CHARGE, FOR THE FISCAL YEAR ENDING JUNE 30, 1892, WITH OTHER DOCUMENTS RELATING TO THE WORKS.

IMPROVEMENTS.

- | | |
|---|--|
| 1. Harbor at Charlotte, New York. | 4. Harbor at Little Sodus Bay, New York. |
| 2. Harbor at Pultneyville, New York. | 5. Harbor at Oswego, New York. |
| 3. Harbor at Great Sodus Bay, New York. | 6. Harbor at Sacketts Harbor, New York. |

UNITED STATES ENGINEER OFFICE,
Oswego, N. Y., July 9, 1892.

GENERAL: I have the honor to transmit herewith annual reports for the fiscal year ending June 30, 1892, for the following works under my charge.

I have the honor to be, very respectfully, your obedient servant,
DAN C. KINGMAN,
Captain of Engineers.

Brig. Gen. THOMAS L. CASEY,
Chief of Engineers, U. S. A.

O O I.

IMPROVEMENT OF HARBOR AT CHARLOTTE, NEW YORK.

OBJECT.

To secure a navigable channel at the mouth of the Genesee River on Lake Ontario.

Charlotte Harbor is the port of the city of Rochester, which is situated 2 miles above the head of the navigable part of the river forming harbor.

PROJECT.

1829.—To obtain a channel 480 feet wide and 12 feet deep, formed and protected by parallel piers extending to deep water of the lake; executed in 1834.

1882.—To secure and maintain by pier extension and dredging a channel of navigable width and 15 feet depth at extreme low water. No dredging had heretofore been done, the channel having been formed and kept open with 12 feet depth by the current of the Genesee River.

PRESENT WORKS.

The piers which define and maintain the channel are formed of cribs of timber, 20 feet wide and 30 feet long, sunk, end to end, in as close contact as practicable, by filling with loose stone. The cribs forming the original work were built of round logs hewed flat, and were allowed to settle into the natural sand bottom upon which they were placed, each crib having two to four oak guide piles driven on each side of it to keep the crib in line while settling into the sand. A continuous timber superstructure was then built upon the cribs for the full width of 20 feet, and for 3 to 5 feet in height above the mean lake level. The whole was then filled with loose stone and decked with 3-inch pine plank. The cribs being below water, and not subject to decay, were built of hemlock timber, fastened together with tie-heads and drift-bolts, while the continuous superstructure was built of white pine timber and plank similarly fastened. The latter has a life of about 15 years.

(1) *West pier.*—This has a total length of 3,257 feet, in addition to a shore-return of 137 feet.

The width was originally 20 feet throughout, but the accretion against the west side of the west pier has advanced the lake shore line 1,200 feet, and the inner 900 feet was last renewed 10 feet wide.

Section A, from the shoulder-angle of the return, at the shore line of 1829, for 554 feet northward toward the lake. The cribs forming its superstructure were built in 1829-1834 of flatted logs, and are still sound. The superstructure was first rebuilt in 1853, was again rebuilt in 1864-1868, and was last rebuilt in 1869, with a width of 10 feet. It is now decayed to mean water level (2.4 feet above extreme low water) and its renewal has not since been necessary.

Section B, 564 feet northward from last section. The cribs were built in 1829-1834, and are still sound. Superstructure was first rebuilt in 1853, was again rebuilt in 1864-1868, and was last rebuilt with oak timber in 1885, part 10 feet and part 20 feet wide, by the Ontario Beach Improvement Company, forming, with Section A, the water front of the accretion since 1829, which is now owned by the New York Central and Hudson River Railroad Company, and is leased and occupied by the Ontario Beach Improvement Company as a summer resort.

The following sections extend out into the lake to 14 feet depth at extreme low water:

Section C, 1,402 feet long, 20 feet wide. The cribs were built in 1829-1834, and are still sound. The superstructure first rebuilt in 1853, was again rebuilt in 1864-1868, and was last rebuilt in 1887-'88, 20 feet wide and 4 feet above mean water.

Section D, 235 feet long, 20 feet wide. The cribs were built in 1829-1834, and are still sound. The superstructure was first rebuilt in 1853; was again rebuilt in 1864-1868, and was last rebuilt in 1891.

on E, 303 feet in length; cribs 30 feet long, 20 feet wide, built of 10-inch by 12-inch hemlock timber, with tight floors placed upon the natural sand bottom in about 12 feet of water. Superstructure built of 12-inch white pine, 20 feet wide, was bonded back 11 feet from the preceding section; cribs and superstructure were built in 1883.

on F, 201 feet in length (including timber horns projecting at each end), formed of cribs 50 feet long, 20 feet wide, built of sawed 12-inch hemlock timber, the floor being built in center only, supported for width of 4 feet along each side. Sunk in 13 feet depth of riprap foundation, about 4 feet in thickness and 30 feet wide, resting upon the natural sand bottom. These cribs differed from the usually built cribs at this harbor in being 50 feet long instead of 30; in having a riprap foundation; in having a partly open bottom, which feature has given unsatisfactory results as compared with the usually built tight bottoms. They also differ in having, besides the drift-bolt fastenings, six vertical pine posts in each crib extending from the grillage bottom to the top of the superstructure, and screw-bolts to each timber; and they have a tight deck laid longitudinally upon the top set of cross-ties. Cribs and superstructure were built in 1885.

East pier.—This has a total length of 2,896 feet, in addition to an uncompleted shore connection 402 feet in length, upon which the superstructure has not been renewed since its original construction in 1829—and which is now destroyed to nearly the level of the surrounding water, only its guide piles showing above low water. The width was originally 20 feet throughout, the substructure being of separate cribs and the superstructure of continuous timber work, similar in construction to the west pier structures before described.

Section A, 325 feet in length, measuring toward the lake from a large oak pile at the south end of the work now visible; cribs 30 feet long, 20 feet wide, built 1829–1833, and still sound below mean lake level. Superstructure, 20 feet wide, decayed to mean lake level, having been first renewed in 1864–1867 and not since then. The stone filling of the superstructure was taken in 1887 to refill exposed portions of the pier; this portion being within the shore line.

Section B, 100 feet in length; cribs are the same as last section, and posts sunk upon them in 1868–1870. Superstructure was first rebuilt in 1868–1870, received general repairs in 1880, and was last renewed for 4 feet above mean lake level in 1887 by the United States Army and Navy Service of whose station it forms the water front. The following are in the open lake:

Section C, 225 feet in length; cribs are the same as last section, and posts sunk upon them in 1868–1870; superstructure was first rebuilt in 1868–1870, received general repairs in 1880, and was last renewed in 1887.

Section D, 797 feet in length; cribs are the same as last section and posts sunk upon them in 1868–1870; superstructure was first rebuilt in 1868–1870; received general repairs in 1880 and was last rebuilt in 1887.

Section E, 995½ feet in length; cribs are the same as last section; rebuilt in 1833–1834 and had tops sunk upon them in 1868–1870. The superstructure was first rebuilt in 1868–1870; 750 feet of it was again rebuilt in 1881–1882, and the whole was last rebuilt in 1890.

Section F, 303 feet in length; cribs 30 feet long 20 feet wide, built of 10-inch by 12-inch hemlock timber with tight floors placed upon the natural sand bottom in about 12 feet of water. Superstructure of

Work of renewing the superstructure by hired labor, on section D of the west jetty was begun on August 11, 1891, and completed on September 19, 1891. In addition to this work, the same party repaired small breaks in the jetties, and also repaired injuries to them from their being run into by the barges *Wheeler* and *Hiauwatha*. They also finished up certain work on section E of the east jetty which could not be completed last year. In rebuilding Section D, they were obliged to take down and put up again a corresponding elevated walk leading to the light-house. The amount of funds for these several objects was as follows:

For small breaks—	
.....	\$40.92
.....	13.87
.....	54.79
Damage to piers by vessels	\$144.90
Amount of sum was collected from the parties in fault by the United States Treasury for the northern district of New York, without a suit, and then taken up and accounted for.	
Section E, east jetty—	
.....	\$217.38
.....	18.91
.....	236.29
On account of elevated walk	\$170.47
Amount of sum expended for renewal of superstructure on Section D, west jetty, there was for labor and new material, \$2,179.43, divided as follows:	
.....	\$1,182.37
Wages	804.43
.....	122.12
Materials	70.51
.....	2,179.43

It was also expended for reading water gauge during the portion of the year when there were no employes available for this work,

and the amount of money of the Chief of Engineers \$8,000 was expended from the appropriation towards the purchase of a dredging plant. The amount expended for all purposes, including engineering and office and inspection to the work during the year was \$23,291.35.

REMARKS.

It was made this season after the spring freshets show that no bar exists beyond the end of the jetties. My assistant engineer who has made surveys suggests "that this bar may have been formed in part from material washed from the channel between the jetties, and in part from this supply of material has been annually renewed by the material which has passed through the jetties (which are not sand-tight) and by the material which has been driven into the channel from the adjacent beaches. The enormous amount of material on the shore-line (some 1,200 feet since the jetties were built) shows that there has been plenty of material in motion to keep up the channel. The Ontario Beach Improvement Company has claimed this sand bar by covering it with soil and sod out to the line; so that this material is now held in place. The recent work has increased the channel capacity between the jetties some 10 per cent, so that the current can now have but little tendency

to scour it. If this is so, then there is reason to hope that the bar will not form again. This theory is ingenious, but it can only be demonstrated by a series of sediment observations taken during high water in the river, at the upper and lower ends of the jettied channel, and by comparative surveys of the channel during the same period. I am of the opinion that the river itself brings down a good deal of silt, and that a portion of it may now be deposited in the channel, reducing its depth so that it may ultimately require redredging.

As stated elsewhere, the channel between the jetties has a general depth of 15 feet at extreme low water; but the jetties terminate near the 13-foot contour, and therefore the full depth of the improved channel is not available. I doubt if a dredged channel would maintain itself beyond the end of the jetties. I can find no record that it has ever been tried. But I have so little confidence in it that I would recommend that the approved project of extending the jetties to the 15-foot contour be carried out as rapidly as the means provided will allow.

Name of harbor, Charlotte, N. Y.; collection district, Genesee, N. Y. (at Charlotte). A fixed red light of the fourth order on crib 300 feet inside of outer end of west pier. A range light of 3 fixed lights, 2 red and 1 white, 60 feet high, at inner end of west pier. Forts Niagara and Ontario, N. Y., are the nearest works of defense.

Money statement.

July 1, 1891, balance unexpended.....	\$34,620.86
February 26, 1892, received from vessels for damages to piers.....	144.90
Total to be accounted for	34,844.76
June 30, 1892, amount expended during fiscal year.....	23,291.35
July 1, 1892, balance unexpended	11,553.40
Amount appropriated by act approved July 13, 1892	25,000.00
Amount available for fiscal year ending June 30, 1893.....	36,553.40
{ Amount (estimated) required for completion of existing project.....	109,650.00
{ Amount that can be profitably expended in fiscal year ending June 30, 1894	75,000.00
{ Submitted in compliance with requirements of sections 2 of river and harbor acts of 1866 and 1867.	

COMMERCIAL STATISTICS.

Arrivals and departures of vessels at Charlotte Harbor, New York, during fiscal year ending June 30, 1892.

ARRIVALS.

Trade engaged in.	Steamers.		Sailing vessels.		Barges.	
	Num. ber.	Registered tonnage.	Num. ber.	Registered tonnage.	Num. ber.	Registered tonnage.
Home, on lake	371	48,784	65	8,698	211	71,531
Foreign, on lake	242	113,521	295	55,239	753	58,856
Total	613	162,305	360	63,937	364	130,387
American owned	377	54,906	46	4,869	285	98,161
Canadian owned	236	107,399	314	59,068	79	31,960

Sum total, 1,337 arrivals; 356,353 registered tons.

Arrivals and departures of vessels at Charlotte Harbor, etc.—Continued.

DEPARTURES.

Trade engaged in.	Steamers.		Sailing vessels.		Barges.	
	Num-ber.	Registered tonnage.	Num-ber.	Registered tonnage.	Num-ber.	Registered tonnage.
Steamers, on lake	376	54,975	40	4,644	204	67,720
Sailing vessels, on lake	236	106,969	317	59,229	160	62,401
Total	612	161,944	357	63,873	364	130,121

Grand total, 1,333 departures; 355,938 registered tons.

Maximum draft of vessels, 14 feet.

Maximum tonnage of vessels, 1,583 registered tons.

Maximum load of vessel, 2,130 net tons of coal.

Receipts and shipments by lake at Charlotte Harbor, New York, during fiscal year ending June 30, 1892.

RECEIPTS.

Trade.	Passen- gers.	Grain.	Merchan- dise.	Live animals.	Poles and ties.	Post and wood.	Lumber, etc.
	<i>No.</i>	<i>Net tons.</i>	<i>Net tons.</i>	<i>Tons.</i>	<i>Tons.</i>	<i>Tons.</i>	<i>Tons.</i>
Steamers, on lake	22,108		44				
Sailing vessels, on lake	9,222	6,385	529	683	2,282	3,668	4,425
Total	31,330	6,385	373	683	2,282	3,668	4,425

SHIPMENTS.

Trade.	Passen- gers.	Merchan- dise.	Oil.	Coal.
	<i>No.</i>	<i>Net tons.</i>	<i>Tons.</i>	<i>Net tons.</i>
Steamers, on lake	22,359	59		186,162
Sailing vessels, on lake	8,540	375	73	275,922
Total	30,899	434	73	462,084

Navigation closed December 20, 1891.

Navigation opened March 29, 1892.

Total revenue collected at port during the fiscal year, \$98,858.50.

For the purpose of reduction to tons weight, the following is assumed: Barrels oil, equal 1 ton; M lumber, 1 equal 1½ tons; crates fruit, 20 equal 1 ton; railroad ties and poles, 12 equal 1 ton; cords wood and posts, 1 equal 1½ tons; live animals, 4 equal 1 ton.

O O 2.

IMPROVEMENT OF HARBOR AT PULTNEYVILLE, NEW YORK.

OBJECT.

To furnish a protected channel of navigable width and not less than 10 feet depth at the mouth of Salmon Creek; Wayne County, N. Y., which is situated 21 miles east of the Genesee River.

PROJECT.

The present project of 1872 provides for a breakwater running 330 feet eastward from the west side of the creek, a west pier or jetty thence northward into the lake, and an east pier or jetty about 200 feet eastward from the west pier and parallel to it.

Also a dredged channel between the piers and behind the breakwater, and extending about 400 feet up the creek.

After this work was done, at a total expenditure of \$71,000, it was found that the waves from the lake when driven by a gale from the northwest would strike the east pier in such a way as to be reflected upon the beach within the harbor, and that in receding they would sweep the gravel and sand from the beach into the excavated channel along the harbor side of the breakwater, where it would lodge across the mouth of the creek.

Accordingly it was further proposed in 1884, in order to make dredging of permanent value and effect, to build a sand-tight structure 500 feet in total length, parallel to the breakwater and about 100 feet from it, between it and the harbor beach, so as to arrest this movement of material.

A part of this structure has been built during this fiscal year.

PRESENT WORKS.

(1) *West breakwater*.—This has a total length of 330 feet, and is formed as follows:

Section A, from the west shore line eastward, 150 feet in length and 15 feet in width, formed by cribs of flatted round logs built at local expense before the General Government undertook the improvement of the harbor in 1867, \$30,000 having been thus expended before that year.

The superstructure, 15 feet wide and 4 feet above mean lake level, was repaired by the United States in 1871, was rebuilt in 1877, and is still in place in a half-ruined condition. The gravel from the lake beach has washed over it, and has aided in closing the entrance to the creek.

Section B, from section A to the west pier which it joins, is 180 feet in length, 20 feet in width, formed of tight-bottomed cribs, each 30 feet long, 20 feet wide, and 9½ feet deep. These cribs were built by the United States in 1871 of sawed 12 by 12 inch hemlock timber and were sunk, in close contact, in a trench dredged to the depth of the proposed channel about 8 feet below extreme low-water level. The cribs were filled with loose stone gathered from the neighboring shore of the lake. The continuous superstructure which was built by the United States upon these cribs in 1871 was formed of sawed pine timber framed together, bolted with drift bolts, and ballasted with loose stone from the lake shore.

It has not since been renewed, and is decayed, though still in position. The substructure of this and the preceding section is sound up to mean level, 2.4 feet above extreme low water.

(2) *West pier*.—This has a total length of 558½ feet and is 20 feet wide. Its substructure is formed of separate cribs sunk in close contact and its superstructure is continuous, except on section D, which is separate.

Section A is 222 feet long, from the shoulder angle northward. It is formed of eight separate cribs, one irregular angle crib, measuring 12 feet on its face, and seven regular cribs, each 30 feet long and 20 feet wide, and 7 feet deep.

The cribs were built in 1874–1875 of sawed hemlock timber 12 inches square, had tight floors upon screw-bolted grillages, and were sunk, in 1874–1875, in a foundation trench dredged to 8 feet depth at extreme low water where the natural depth was 4 feet. The cribs are still sound.

The superstructure was built upon them in 1874–1875 of 12-inch square hemlock timber, 4 feet high, filled with loose stone and decked with 3-inch plank laid crosswise, with 2-inch spaces. This has not since been renewed, and is decayed, but still in position.

Section B is 93½ feet long, from last section northward. It is formed of 3 separate cribs, each 30 feet long, 20 feet wide, and about 9 feet deep. They were built in 1880 like those of the last section, with similar superstructure, and sunk in a trench dredged to 8 feet depth.

The cribs are still sound, but the superstructure needs repair.

Section C is 183 feet long, from last section northward. It is formed of six separate cribs, each 30 feet long, 20 feet wide, and about 9 feet deep. They were built in 1881 like those of the last section, except that the foundation trench was not dredged for all the cribs, its completion being prevented by encountering a mass of large boulders embedded in clay, which the dredge could not remove.

One of the cribs rested partly upon these boulders and partly upon the riprap, and others rested partly upon the sand among the boulders, and about 100 feet afterwards settled irregularly.

The superstructure was built upon the cribs in the same season, 1881, and has not since been repaired, except that in 1884 it was leveled for 35 linear feet by raising the east wall 2 to 3 feet and relaying the deck. The rest of this again needs similar leveling, refilling, and redecking.

Section D is 60 feet long from the last section northward, and forms the outer end of the pier.

It is formed of two cribs similar to those built in 1874–1875 and since, each 30 feet long and 20 feet wide and 12 feet deep. The cribs were built in 1883, of sawed hemlock timber, 10-inches by 12-inches square (the two upper courses being of white pine), with tight floors upon screw-bolted grillages, and they were sunk upon a bed of riprap 3 feet deep and 40 feet wide, where the natural depth was about 10 feet at extreme low water.

The superstructure was built upon them 4 feet high in 1883. The cribs have not settled at all, and now stand level, 2 feet above the adjoining irregular work.

The outer end of the pier was breached at some time prior to 1890, the stone filling was washed out to 9 feet depth, from the outer half of the outer crib, and the entire end wall is now gone, the deck and superstructure remaining.

It is fully exposed, and will be destroyed unless the end is rebuilt.

(3) *East pier.*—This has a total length of 571½ feet, with a varying width of 15 to 20 feet as described below. It was all built between 1872 and 1880, and was formed of cribs, each 30 feet in length, 15 to 20 feet in width, and generally 6 to 8 feet in depth.

The cribs were built of sawed hemlock timber, 12 inches square, had tight floors upon screw-bolted grillages, and were sunk on the natural bottom in depths of 6 to 8 feet at extreme low water. The superstructure

was built upon the cribs during the same year, of 12-inch square pine timber, 4 feet high, filled, as were the cribs, with loose stone, and decked with 3-inch pine plank laid crosswise with 2-inch spaces.

The different sections were built, first at one end of the jetty and then at the other end, as seemed most useful.

Section A, beginning 140 feet from the present shore line (at the end of a private wharf, since destroyed), extending 85½ feet northward into the lake, 16 feet wide. It was formed of three cribs, two 30 feet long and one 25½ feet long. They were sunk upon the remains of a wrecked private wharf, leveled to receive them. Its superstructure, also 16 feet wide, was 4 feet high. Both were built in 1880 and are still in fair condition.

Section B, 62 feet in length, 15 feet in width, built in 1877, and since renewed.

Section C, 30 feet in length, 20 feet in width, built in 1877, and since renewed.

Section D, 92 feet in length, 20 feet in width, built in 1873, and since renewed.

Section E, 180 feet in length, 120 feet in width, built in 1872, and since renewed.

Section F, 122 feet in length, 20 feet in width, built in 1873, and since renewed.

In all these sections the cribs are sound, but the superstructure needs renewal, without which the whole will be wrecked.

(4) *Sheet-piling.*—Of the construction to arrest sand drift from harbor beach to the channel into the creek, which was proposed in 1884, 200 linear feet, from the east side of the creek outlet eastward toward the land end of the east jetty have been built during the last year. The work was done by hired labor, at a total cost of \$1,550, equal to \$7.75 per linear foot complete.

The structure is sand-tight, being formed of the Wakefield patent sheet-piling. Its oak wale-pieces are below the line of no decay, they are tied back by iron rods to logs buried in the bottom, to which the accretion will probably form, so that the structure may be expected to be a permanent one. It has already produced an effect causing a scour of the shoal between it and the breakwater, but dredging will be needed to make a navigable channel.

CHANNEL.

The channel between the jetties, behind the breakwater and 400 feet up the creek, had been dredged in former years to 8 to 9 feet depth at extreme low water; but it has since shoaled to 7 feet between the jetties and 2 feet at the creek outlet. The dredging which has been done is as follows:

In 1874, 14,189 cubic yards of sand and gravel were removed at 10 cents per cubic yard, making an effective depth of 8 feet at extreme low water, 25 to 40 feet wide and 600 feet long between the jetties, 10 feet depth for 50 feet width, and 400 feet length behind the breakwater to the creek outlet, and 40 feet width for 400 feet up the creek.

This dredging made known the existence, in the space between the jetties, of a deposit of bowlders embedded in stiff clay and cemented gravel, which was most difficult to remove, none of which was t

until the next year (1875) when a special contract was made for its oval. Under this contract, in 1875, 7,700 cubic yards of hard material was excavated between the jetties at 70 cents per cubic yard, and 3 yards of sand and mud was excavated from the creek channel at 10 cents per cubic yard. This gave an entrance 130 feet wide and 9 feet deep at extreme low water.

In October, 1879, the space between the jetties (then 222 feet long) was cut to 7 and 8 feet depth at extreme low water, but the cut behind the breakwater and up to the creek outlet had filled with sand from the shore beach, so that there was only 2½ to 4 feet depth at extreme low water for 30 feet width. In 1880 the entrance was dredged to 8 feet depth at extreme low water for 150 feet eastward from the west jetty, and for 250 feet northward from its junction with the west breakwater. Behind the breakwater the same depth was made for a width from the breakwater of 70 feet at the shoulder angle, narrowing to 50 feet at the creek outlet, and thence for 50 feet width up the creek. In this year 20,450 cubic yards of sand, gravel, and mud at 18 cents per cubic yard, and 806 cubic yards of bowlders and hard material at \$1 per cubic yard.

Since 1880 no more dredging has been done. Meantime the west jetty has been extended 336 feet further into the lake, so that the entrance is better sheltered from sand drift, and 7 to 8 feet depth continues in it.

The space behind the breakwater has again filled with sand from the shore beach, but this movement will be arrested by the sand-tight jetty just completed.

The removal of 10,000 cubic yards of sand from behind the breakwater and from the creek outlet will give an effective depth of 7 to 8 feet at extreme low water, and will make the harbor available for the small vessels suited to the coasting trade.

OPERATIONS.

An examination by borings was made along the line of the proposed sand-tight jetty in the fall of 1891, to see if it would be possible to drive sand there. The results indicated that work of this kind would be highly practicable. Accordingly, material was purchased and a party was sent to Pultneyville to construct about 200 feet of this work. Operations were begun November 19 and continued until January 9, 1892, when it was completed. The Wakefield patent sheet piling was used, and the owner was paid a royalty of 20 cents a running foot of completed structure for the use of this patent. The piles as used here were built up of three oak planks, 12 inches wide and 2 inches thick, laid so as to form a tongued-and-grooved pile of proper length. They were driven in close contact, and form a structure that is nearly water-tight.

This particular form of patent sheet piling has been elsewhere used on the public works, and has been described in the Reports of the Board of Engineers. The piling is strengthened by oak wale pieces, which are below the line of no decay, and is tied back by iron rods 12 feet long to logs in the bottom, the object of this being to prevent the piling from being pushed over towards the channel by sand which will probably gather behind it.

The cost was \$1,555.85, or \$7.78 per running foot complete.

REPORT OF THE CHIEF OF ENGINEERS, U. S. ARMY.

The items are as follows:

Labor	\$73
Lumber	63
Stone (and protecting the outer end)	21
Other materials and tools	99
Use of patent right	3
Total	1,55

In addition to this, \$62.63 was expended during the year for engineering and office expenses.

REMARKS.

A good deal of dredging has been done in the past before the construction of the bulkhead, but the moving sand has reduced the available depth between the jetties to about 7 feet; and along the breakwater and at the mouth of the creek the channel is only three or four feet deep.

This renders it impossible for vessels to approach the wharves and warehouses which are within the mouth of the creek, and therefore the harbor is well-nigh useless. The few vessels that do come here are forced to lie at the outer end of the jetty channel in an exposed position and transfer their cargoes to small boats. As a result commerce has almost disappeared.

The superstructure of the breakwater and a greater part of the jetties is now very much decayed. It ought to be renewed at once. If this was done, and the sand-tight bulkhead completed and the channel thoroughly dredged out, it would complete the proposed improvement and make valuable the work already done. It would afford a safe and good harbor for small vessels (the class that most frequent it) between Great Sodus Bay and Charlotte, a stretch of exposed coast some 35 miles long, which now offers no shelter whatever. There is no other place in my district where a small amount of money would produce such marked benefit.

The amount now available, being less than \$400, is too small to enable me to undertake any of this work.

Name of harbor, Pultneyville, N. Y.; collection district, Genesee, N. Y.; nearest light-house, Great Sodus, New York; nearest work of defense, Fort Ontario, N. Y.

Money statement.

July 1, 1891, balance unexpended	\$1,98
June 30, 1892, amount expended during fiscal year	1,61
July 1, 1892, balance unexpended	38
Amount appropriated by act approved July 13, 1892	1,00
Amount available for fiscal year ending June 30, 1893	1,38
{ Amount (estimated) required for completion of existing project	9,00
{ Amount that can be profitably expended in fiscal year ending June 30, 1894	9,00
{ Submitted in compliance with requirements of sections 2 of river and harbor acts of 1866 and 1867.	

COMMERCIAL STATISTICS.

Arrivals and departures of vessels at Pultneyville Harbor, New York, during fiscal year ending June 30, 1892.

ARRIVALS.

Trade engaged in.	Steamers.		Sailing vessels.	
	Number.	Registered tonnage.	Number.	Registered tonnage.
Home, on lake	4	300		
Foreign, on lake			4	208
Total	4	300	4	208
American owned	4	300	1	76
Canadian owned			3	132

Sum total, 8 arrivals; 508 registered tonnage.

DEPARTURES.

Trade engaged in.	Steamers.		Sailing vessels.	
	Number.	Registered tonnage.	Number.	Registered tonnage.
Home, on lake	4	300	1	76
Foreign, on lake			3	132
Total	4	300	4	208

Sum total, 8 departures; 508 registered tonnage.

Greatest draft of vessels, 7 feet.
 Greatest tonnage, 76 registered tons.
 Greatest load, 92 net tons.

Receipts and shipments by lake at Pultneyville Harbor, New York, during fiscal year ending June 30, 1892.

Trade.	Receipts.			Shipments.	
	Passen- gers.	Merchan- dise.	Lum- ber, etc.	Passen- gers.	Fruit.
	No.	Net tons.	Tons.	No.	Tons.
Home on lake	1,000			1,000	
Foreign on lake		6	103		56
Total	1,000	6	103	1,000	56

For the purpose of reduction to tons weight the following is assumed: Barrels oil, 7 equal 1 ton; M lumber, 1 equals 1½ tons; crates fruit, 20 equal 1 ton; railroad ties and sleepers, 12 equal 1 ton; cord wood and posts, 1 equals 1½ tons; live animals, 4 equal 1 ton.

003.

IMPROVEMENT OF HARBOR AT GREAT SODUS BAY, NEW YORK.

OBJECT.

To secure a navigable channel from Lake Ontario to Great Sodus Bay with a depth of 15 feet.

Great Sodus Bay is 5 miles long, 2 to 3 miles wide, is deep and land-locked, and is situated midway between Oswego and the Genesee River. It is the coal-shipping port on Lake Ontario for the Pennsylvania Railroad.

PROJECT.

1828.—To contract the entrance to Great Sodus Bay to 470 feet by breakwaters extending from east and west shores and to define and project a channel 470 feet wide by piers extending to deep water in Lake Ontario.

1882.—To extend the piers to the 15-foot curve in the lake and to dredge the channel between them to 15 feet depth at extreme low water.

PRESENT WORKS.

The project has been completed, except that the east jetty lacks 800 feet of reaching the curve of 15 feet at extreme low water and that the channel has not been dredged to 15 feet for its full width.

The dredging has been confined to the western half of the space between the jetties, and the channel thus made has not been permanent.

Excavation of the full width (except 50 feet next the jetties) to 15 feet at extreme low water is now in progress.

The piers and jetties which contract the entrance and define and shelter the channel were originally built in 1829-1834. They were started from the west and the east sides of the entrance, naturally about 4,000 feet wide, and were extended toward the center, where they were continued northward by parallel jetties, 470 feet apart across the 8-foot bar, from deep water in the bay to the deep water in the lake.

These piers and jetties defining and sheltering the channel were built in 1829-1834, and were composed of cribs of timber, each 18 feet wide and 30 feet long, sunk, end to end, upon the natural sand and gravel bottom in as close contact as practicable by filling them with loose stone.

The cribs were formed of round logs, hewed flat, framed together and bolted with iron drift bolts and wooden treenails. They appear to have had floors of slabs, and were allowed to settle into the natural sandy bottom for part of the season before building upon them the continuous superstructure of hewed timber. These old cribs, in part or whole, are still in place, and are sound below mean water level.

The different portions of the works may be described as follows:

(1) *West breakwater.*—This has a total length of 2,200 feet. It was built 18 feet wide, with a superstructure of same width, in 1829-1834.

The cribs were each 18 feet wide and 30 feet long, and starting at the beach line gradually increased in depth to 8 feet where they joined the jetty.

Section A, 1,603 feet long from west shore eastward. The

re built in 1829-1834, and are still sound. The superstructure was built at the same time, was never renewed, and has long since decayed. The whole structure is buried and covered by the accretion of the beach and outside it, and its maintenance has been unnecessary.

Section B, 241 feet long in continuation of the last section. The cribs re-built in 1829-1834, and are still sound. The superstructure was renewed, for the same reasons given above, until 1888, when the pier of the adjoining tract of land formed by accretion renewed it 14 feet wide and 4 feet high.

Section C, 266 feet long in continuation of the last section to the west jetty. The cribs were built in 1829-1834, and are still sound. The superstructure was renewed by the United States in 1877-1878 207 feet, 14 feet wide and 59 feet next the angle, 18 feet wide. Upon the latter stands the inner range beacon and the United States water gauge.

(2) *West pier or jetty.*—This is 1,580 feet long, 18 to 20 feet wide, as follows:

Section A, from shoulder-angle at junction with breakwater, 975 feet northward, 18 feet wide. The crib structures built in 1829-1834 were wrecked to an average depth of $4\frac{1}{2}$ feet below mean water level between 1857-1866. Had new cribs sunk on top of the wrecked ones 1866-1868. Are now sound, but in bad condition, allowing free passage of sand into the channel.

The superstructure was built of timber filled with loose stone at the same date as the cribs and was continuous over them. It received several repairs in 1845 and again in 1852, but was all decayed and destroyed prior to 1866, when its renewal was begun, and was completed 1869.

In 1877, 150 feet from the angle northward was again renewed, and in 1880 the remaining 825 feet was also renewed.

Section B from the last section northward is 285 feet long and 18 feet wide. The cribs were built in 1829-1834, and otherwise have the same story as the last section, and were rebuilt as they were in 1866-1868. The superstructure was built upon them in 1866-1868 and was first rebuilt in 1880.

Section C from the last section northward is 134 feet long and 18 feet wide, the outer 40 feet being a pier head 40 feet square. This section was built in 1869, and was formed of three cribs, each 30 feet long and 18 feet wide, and a pier head at the outer end 40 feet square, having an open central space 18 feet square for a light-house foundation. The cribs were built of sawed hemlock timber 12 inches square and had tight floors laid upon screw-bolted grillages. The foundation was prepared for these cribs by dredging a trench to about 12 feet depth, in which they were sunk. The superstructure, five courses high of 12 by 12 inch square timber, was built upon them in 1869, but had to be leveled and re-laid in 1871, the structure having settled. The superstructure was next renewed in 1880, when the pier-head crib was cut down to line of decay and decked at that level with solid timber.

Section D extends from the last section 185 feet northward to the 15-foot curve of extreme low water. This section is 20 feet wide and was rebuilt complete in 1883. The cribs were each 30 feet long and 20 feet wide and 13 feet deep, and were formed of sawed 10-inch by 12-inch timber (the lower 11 feet of hemlock and the upper 2 feet of pine) and had partly open floors made of 3-inch plank laid 18 inches apart upon screw-bolted grillages. The cribs were placed upon a riprap foundation 40 feet wide and 4 to 5 feet thick, deposited in a trench dredged

for the purpose, costing 50 cents per cubic yard for the excavation \$1.30 per cubic yard for the riprap in place.

The superstructure was built upon the cribs the same season consisted of four courses of 10-inch by 12-inch pine timber (with a deck of 3-inch pine plank) filled with loose stone.

In 1891 the outer end was cut by a barge down to 6 feet below requiring the renewal of the end of the crib and superstructure.

3. *East breakwater*.—This has a total length of 1,651 feet Charles Point on the east side of the bay westward to joint the jetty. The original structure, built in 1829–1843, was not repaired until 1874. The superstructure and cribs were then graded about the level of the surface of the bar in which the cribs were bedded, which was at an average depth of 1 foot below extreme water level.

Section A, the shore arm, from Charles Point to the lake arm, 100 feet long and 14 feet wide, with 5 feet total height of cribs and structure. It was built in 1884 and sunk upon the natural bottom about 2 feet depth of water. It is in good order.

Section B is 511 feet long and 14 feet wide; is part of the structure built in 1829–1843.

It was first renewed in 1874, when four courses of continuous structure was framed together of 12-inch by 12-inch pine timber, lower courses being securely bolted to the remains of the old crib, the top being at 4 feet above extreme low water. In 1877–1878, the plank and joints being decayed and broken but the side timber sound, the top was renewed by adding one course of 10-inch by 12-inch pine timber and laying a new deck.

Section C is 415 feet long, 14 feet wide, and has a history similar to the last described section. Its first renewal was made in 1870–1871, its next and last renewal in 1887–1888.

Section D is 172 feet long, 14 feet wide, was built in 1829–1834, was first renewed in 1870, but not since then. The superstructure is much decayed.

Section E is 264 feet long, 14 feet wide, was built in 1829–1834, was first renewed in 1876. The superstructure has not since been renewed until 1891, when the western 126 feet was renewed in the same form, the half toward the harbor being $4\frac{1}{2}$ feet above extreme low water, and the lake half $7\frac{1}{2}$ feet above the same plane.

Section F is 74 feet long to the east jetty, and is 18 feet wide. It was built in 1829–1834; was first rebuilt in 1876, and was next renewed in 1891, when it was built in parapet form as described in the last section.

(4) *East pier or jetty*.—This is 1,294 feet long from its junction with the east breakwater northward into the lake to the 10 $\frac{1}{2}$ -foot extreme low water. It was originally built 18 feet wide in 1829–1834, in the same style of crib work as formed the west jetty, and was extended to 20 feet wide in 1883–1884. Its details are as follows:

Section A from angle with east breakwater 440 feet northward, was built in 1829–34. It was first repaired in 1853 by renewal of the superstructure, and this was again done in 1875–1876.

In 1891 the superstructure was again renewed, this time in the same form with the half next the channel $4\frac{1}{2}$ feet above extreme low water and the other half 3 feet higher.

Section B is 500 feet long and 18 feet wide, and was original in 1834. The superstructure was first repaired in 1853, but destroyed between that date and 1869, when it was necessary to rebuild the cribs and superstructure from an average depth of $4\frac{1}{2}$ feet below

level to about 4 feet above the same plane. The superstructure is rebuilt in 1890, not in parapet form.

Section C is 154 feet long and 20 feet wide, and was built complete; the details of its construction being the same as those of Section of the west jetty, which has already been described.

Section D is 200 feet long and 20 feet wide, and was built complete. It was formed of cribs each 50 feet long, 20 feet wide, and 12 feet high, built of 10 by 12 inch hemlock timber with two upper courses of timber.

Floors were built in the center only, and were omitted for a width of 10 feet along each side. The cribs were sunk upon a riprap foundation 10 feet wide, deposited in a trench dredged for the purpose in the bottom.

A continuous superstructure of four courses of pine timber was upon the crib in 1885. The cribs forming this work differ from the usually built cribs at this harbor in being 50 feet long each, instead of 25 feet; in having a riprap foundation; in having partly open floors;

and in having, besides the usual drift-bolt fastenings, 6 vertical pine bolts in each crib, extending from the grillage bottom to the top of the superstructure and screw-bolted to each timber; and they have a deck, laid longitudinally, directly upon the top set of cross-ties. This completes the description of the structures of this harbor, except the sheet-piling, which has been placed along the west jetty to arrest the drift of beach sand through it into the channel. Of this sheet-piling the first section was driven in 1877 along the west face of the west jetty beginning 150 feet north from the shoulder angle and extending 150 feet northward. In 1887 the second section was built, beginning at the north end of the first and extending 245 feet northward along the west face. At this point the substructure of cribs projected outside the face of the superstructure about 3 feet, so that it was necessary to drive the third section of sheet-piling on the east face, where it was driven 367 feet northward along the channel face of the jetty during which a tight cross bulkhead being driven between two cribs to unite the two sections.

The sheet-piling consists of 4-inch by 12-inch oak plank, each 20 feet long, driven to a depth of 14 to 15 feet below extreme low-water level, and secured to the superstructure of the jetty by 1-inch lag screws in each pile and also by 6-inch by 12-inch continuous oak wale screw-bolted through the piles to the timbers of the crib work. To arrest sand movement into the channel there was also built in 1880 upon the west sand beach, where it had formed against the west jetty, a system of overlapping sand-catch fences of a total length of 760 feet. These fences have been effective, but they can not be maintained beyond high-water line, and there is generally a wide beach of sand between them and the water, from which the prevailing winds blow the sand across the jetty. This is the source from which comes a considerable portion of the sand which refills the channel.

CHANNEL.

—The space 470 feet wide between the jetties had a natural depth of 8 feet in October, 1829. This was the actual depth at that time when the water level must have been at least 1 foot, and was only 2 feet, above extreme low water, this being the average stage level at this time of year. No plane of reference for lake water

level was established until 1837, when the present accepted extreme low-water level was determined at Oswego and fixed as the zero of the gauge, whose readings have since been daily observed in feet and hundredths above this zero.

The records of channel depths in the various reports are much confused during recent years, as well as formerly, by variously stating the depth as at ordinary low-water level, or at ordinary water level, or the actual depth at a given date. In this report all such references will be reduced to the plane of extreme low water without further comment.

1836.—The first dredging was done at Great Sodus Harbor in 1836 and was continued during 1837 and 1838, \$45,390 being thus expended.

1838.—The result was to form a channel 100 feet wide, 1,500 feet long with $11\frac{1}{2}$ feet depth, by removing about 30,000 cubic yards of gravel and sand.

1844.—In August, 1844, this channel was found to have shoaled to 10 feet depth.

1856.—The next dredging was done in 1856, when 6,233 cubic yards of sand was removed at a cost not stated.

1867.—Nothing more was done until 1867, when renewal of the jetty was begun and dredging was done under its shelter; 9,200 cubic yards were then dredged at 24 cents per yard.

1868-'69.—The dredging was continued in 1868 and 1869, completing a channel next the west jetty, 200 feet wide, 1,700 feet long, and 10 feet deep, by removing about 40,000 cubic yards additional.

1870.—In 1870 the sand had again drifted into this 200-foot channel to such an extent as to reduce it to 8 feet depth for its west half, and to 4 to 5 feet depth for its east half, the undredged eastern 270 feet of the space between the jetties varying from 2 feet at the inner end to 4 feet at the outer end.

1873.—In 1873, 19,000 cubic yards of sand were removed at 35 cents per cubic yard measured in position, again making the channel 9 feet deep for 150 feet next the west jetty, and this was continued in

1874 by removing 44,977 cubic yards of sand at $22\frac{1}{2}$ cents, scow measurement, and again making the 200-foot channel 9 to 11 feet deep.

1881.—No further dredging was done until 1881, when the channel had again shoaled to a governing depth of 8 feet and dredging resumed.

1882.—During 1881 and 1882, 42,050 cubic yards were removed at 12 cents per cubic yard in scows, making a channel 2,300 feet long and 11 feet wide next the west jetty, and $11\frac{1}{2}$ feet deep.

1886.—In 1886 dredging was again needed and 13,224 cubic yards were removed at 16 cents per yard, making a channel 50 feet wide and 12 feet deep.

1887.—In 1887 the depth was again reduced to 9 feet and a centrifugal pump was employed to take the sand from the channel and deposit it on the other side of the west jetty. Thirteen thousand cubic yards of sand were thus removed, at $12\frac{1}{2}$ cents per cubic yard, making 10 feet deep for 50 feet width.

1889-'90.—In 1889 and 1890 a dipper dredge was again employed. 40,942 cubic yards of sand, gravel, and cobblestones were removed at 18 cents per cubic yard, scow measurement, and a navigable channel was again made 100 feet in width, 2,000 feet long, and 15 feet deep, located 30 feet from the west jetty.

1892.—In 1892 this was found to have again filled so that there was but $8\frac{3}{4}$ feet depth, and the new United States dredge *Frontenac* was put to work on May 30 to improve the channel.

Following summary shows the total dredging which has been done and the sums expended therefor. For the first item, the number of cubic yards is computed from the dimensions of the channel, and the cost given is the sum of the appropriations for 1836, 1837, and 1838, less \$1,000 used for repairs, the reports stating that the sums for those years were otherwise confined to dredging:

Date.	Cubic yards.	Rate per yard.	Cost.
1836.....	33,000	\$1.37	\$45,390
.....	6,233
.....	9,200	.24	2,208
.....	40,000	.24	9,600
.....	19,000	.85	6,250
.....	44,977	.22½	10,120
.....	42,000	.23	9,660
.....	13,224	.16	2,116
.....	13,000	.12½	1,625
.....	40,942	.18	7,370
.....	261,576	94,340

Comparison of a detailed survey made in 1870 with another survey in 1892 by the same assistant engineer serves to give a definite idea of the effect of the dredging done during the interval.

It has been repeatedly stated in the annual reports that the refilling of the dredged part of the channel was caused by the movement of the sand from the cut from the undredged part.

Comparison of the surveys show that between 1870 and 1892 the dredged part has deepened for an average of 3 feet over an area of 100 feet long and 270 feet wide. This indicates a movement of 30,000 cubic yards of material into the cuts made at different times. The total, 142,000 cubic yards, has come through and over the west end of the jetty. Much of this movement through the jetty was made possible by the usual manner in which the renewal of the wrecked cribs below the jetty was done in 1867-'68. The original cribs had then been deepened to depths varying from 2 to 10 feet below water (averaging 4½ feet), where they were sunk, on top of the old substructure, cribs whose ends were extended across from side to side, placed 3 inches apart and close together. These spaces between the floor plank formed, above the level of the sandy bottom, clear passages for sand to pass under the jetty, and through these spaces the sand has flowed whenever there were waves to move it.

OPERATIONS.

Work of renewing the superstructure in the parapet form, according to the most recent and approved plans, on section A of the east jetty 10 feet long; section F of the east breakwater, 74 feet long, and on section E of section E of same, in all 640 feet, was begun in 1891, and finished November 6 of the same year. The lumber required for the work was purchased by public notice and proposals; the other material and labor required was secured in the market.

In addition to the foregoing the same party repaired the old light-house on the west face of the west jetty, and also placed some 80 tons of stone in the portion of the east jetty that was rebuilt the previous year and which was not wholly filled with stone at that time. They also put up a tool house for use in connection with the work

at the shore end of the west jetty. Old material taken from the which was being repaired was used in its construction.

The total expenditure for the foregoing work was \$6,481.78, divided follows:

Labor	\$3.
Lumber	2.
Iron	
Other material and tools	
Total	6.

There was also expended for engineering and office expenses during the fiscal year, \$1,243.62. Under the authority of the Chief of Engineers \$4,000 was expended from the appropriation for this work to the purchase of a dredging plant for use in the harbors in this district.

On the 29th of May, 1892, the United States dredging plant was taken to Great Sodus Harbor to deepen the channel between the jetties. It continued to work until the end of June. It was much delayed on account of breaking its spuds, which had been badly worn on other occasions and also by breaking a casting which could not be quickly repaired. Notwithstanding this, however, it removed, up to June 30, 11,219 cubic yards of material. The exact cost of the work can not be given at present time. The total sum paid during the fiscal year in connection with this work was \$500.66, and there was outstanding at the close of the year \$1,235. This amount is estimated, as there are some bills for supplies which have not yet been received and examined. This would make the total cost per cubic yard a little less than 15½ cents, which compares favorably with the cost of previous work done under contract. The price, 15½ cents, is really too large, because it includes the cost of rope and other similar material which can be used for several seasons.

The dredge will be forced to stop work at this place early in July on account of lack of funds.

REMARKS.

It appears from the records that a large amount of dredging has been done in this channel in the past, and it also appears that this dredging has always been done near the west jetty, the side from which the winds and waves bring in the sand. This has put the channel in a place where it could be most quickly filled.

The sand, whether blown over the jetty by the wind or washed through it by the waves, would most of it come to rest within 200 feet of the west jetty, or, in other words, in the west half of the channel. The surveys confirm this and show that the eastern half of the channel where no dredging has ever been done is deeper now than it was 40 years ago. In view of this, it seems best to me to begin dredging on the east side of the channel and to gradually widen toward the west. In the meantime every effort should be made to keep the channel from getting into the channel, both by inducing a growth of vegetation above the high-water line along the shore of the west of the channel and by making the jetty perfectly sand-tight below water.

I think the pier should be extended to the 15-foot curve as rapidly as the means provided will allow and that the channel should be deepened to its full width in the manner before stated.

The work will cost \$58,000. In addition to this there should be available every year the sum of \$5,000 for the maintenance of existing

renewing from time to time, as required, the perishable portions of structures.

name of harbor, Great Sodus Bay, New York; collection district, Oswego, New York; nearest light-house, Big Sodus, New York, a fixed white light of the fourth order, varied by a white flash every 2 minutes, on a bluff three-fifths mile west of entrance to the bay; a fixed white light of the sixth order 180 feet inside of the west end of the west pier, and a fixed red light of the sixth order at elbow of west pier. Nearest work of defense, Fort Ontario, New York.

Money statement.

July 1, 1891, balance unexpended	\$13,763.80
June 30, 1892, amount expended during fiscal year.....	12,226.06
July 1, 1892, balance unexpended	1,537.74
July 1, 1892, outstanding liabilities	1,235.00
July 1, 1892, balance available.....	302.74
Amount appropriated by act approved July 13, 1892.....	15,000.00
Amount available for fiscal year ending June 30, 1893.....	15,302.74
Amount (estimated) required for completion of existing project.....	43,000.00
Amount that can be profitably expended in fiscal year ending June 30, 1894.....	40,000.00
Submitted in compliance with requirements of sections 2 of river and harbor acts of 1866 and 1867.	

COMMERCIAL STATISTICS.

Arrivals and departures of vessels at Great Sodus Harbor, New York, during fiscal year ending June 30, 1892.

ARRIVALS.

Trade engaged in.	Steamers.		Sailing vessels.		Barges.	
	No.	Registered tonnage.	No.	Registered tonnage.	No.	Registered tonnage.
American, on lake	20	4,605	17	2,947	19	7,114
Foreign, on lake	1	277	61	9,787	2	643
Total	30	4,882	78	12,734	21	7,757
American owned	29	4,605	4	854	7	1,817
Canadian owned.....	1	277	74	11,880	14	5,970

Sum total, 129 arrivals; 25,403 registered tonnage.

DEPARTURES.

Trade engaged in.	Steamers.		Sailing vessels.		Barges.	
	No.	Registered tonnage.	No.	Registered tonnage.	No.	Registered tonnage.
American, on lake	28	4,636	11	1,935	5	1,355
Foreign, on lake	4	391	66	10,600	16	6,432
Total.....	32	5,027	77	12,535	21	7,787

Sum total, 130 departures; 25,340 registered tonnage.

Greatest draft of vessels, 10½ feet.
 Greatest tonnage, 541 registered tons.
 Greatest load, 993 net tons of coal.

Navigation closed November 26, 1891; navigation opened April 1, 1892.
For the purpose of reduction to tons weight the following is used: 7 thousand feet lumber, 1 equals 1 ton; railroad ties and poles, 12 equal 1 ton; cordwood and posts, 4 equal 1 ton.

O O 4.

IMPROVEMENT OF HARBOR AT LITTLE SODUS B.

OBJECT.

To secure a channel from Lake Ontario into Little Sodus Bay of navigable width and of depth not less than 15 feet at extreme low water.

Little Sodus Bay is 2 miles long, three-fourths of a mile wide, and land-locked, and is situated midway between Little Sodus, or 13 miles west of Oswego. It is the coal landing for Lake Ontario for the Lehigh Valley Railroad.

PROJECT.

To construct by breakwaters the entrance to the harbor 250 feet wide and maintain a channel 200 feet wide and deep at extreme low water by parallel piers.

PRESENT WORKS.

The project has been completed, except that the

steristic of this and of all the bays on the south shore of Lake

breakwaters were built to the sides of the natural cut through (which was 1½ feet deep in 1853 and 5½ feet deep in 1866), and once extended north to deep water in the lake by parallel jet-
feet apart.

structures were formed of cribs each 20 feet wide and 30 feet
nk end to end upon the natural bottom in as close contact as
ble by filling them with loose stone gathered from the shores
ay and lake, and by dredging gravel and dumping it into them.
ribs were formed of sawed hemlock timber 12 inches square,
y 12 inches, which was framed together and bolted with iron
The cribs had tight plank floors and the earlier ones were al-
o settle into the natural gravel and sand bottom for part of the
efore building the continuous superstructure of 12 by 12 inch
iber, also filled with loose stone. For the later cribs, a founda-
s prepared by dredging a trench or by depositing riprap.

ch side of the bay the junction of the breakwaters with the
s effected by stake and fascine structures, intended to aid the
n of drift.

ifferent portions of the work are described as follows: In every
ere depth of water is mentioned in this report, the depth at
low-water level, or below the zero of the Oswego gauge is

West breakwater.—The original structure was built in 1868, and
eculiar one, specially designed for this place. It was of trian-
oss section, with 13 feet base and 13 feet sides, built in separate
, 25 feet long, of 6 by 6 inch hemlock frames, planked inside
uch hemlock plank, the whole filled with loose stone and sunk
e crest of the bar at depth of half a foot to 2 feet.

tal length from the west shore to the west jetty was 650 feet.
akwater was built in 1868, and was destroyed the same year.

70-71 the present rectangular crib-work structure, 469 feet long
et wide, was built 30 feet outside of and parallel with the origi-

It is still in place, though much decayed; all but 90 feet ad-
the west jetty is buried beneath the accretion of the west beach,
90 feet was repaired in 1887. In 1887, there was built in front

reakwater and adjoining the jetty 200 feet of stake and fascine
This has gathered the gravel, and it is probable that minor re-

50 feet of the breakwater is all that will be here needed in

West pier or jetty.—This is 1,960 feet long and 20 feet wide, ex-
across the bar north to deep water in the lake.

n A is 186 feet long, 20 feet wide, and was built with its super-
re in 1854.

ribs forming the substructure were each 20 feet wide and 30
g, sunk end to end upon the natural gravel bottom in as close
as practicable by filling them with loose stone. The cribs were
of flatted logs framed together and bolted with iron bolts and
tree nails.

appear to have had floors of slabs, and to have been allowed to
to the natural gravel bottom for part of the season before
; upon them the continuous superstructure of hewed timber.

cribs are still in place and are sound below mean water level.
perstructure was never renewed and has disappeared. The

breakwater and the beach line being outside of this portion of the jetty it is no longer necessary to keep it in repair.

Section B is 50 feet long and 20 feet wide, extending north in prolongation of section A. It was originally built in 1854 when the section was built, but instead of being abandoned as that has been the superstructure was renewed in 1867, and has since been maintained and forms the inner end of the west jetty. The west breakwater is 40 feet north of its south end.

Section C is 860 feet long in prolongation of the last section and is 20 feet wide, and was built with superstructure in 1867-'68. The cribs forming the substructure were each 20 feet wide and 30 feet long with tight floors, and were sunk upon the natural bottom, in depth increasing from 6 to 11 feet at extreme low-water level, by partly filling with loose stone. After building the continuous superstructure of five courses of 12 by 12 inch pine upon the cribs, the filling was completed by dredging the channel to 12 feet depth close beside the cribs, and dumping the sand and gravel directly into the crib from the dipper. The cribs settled into the cut so that the west side was under water and the structure nearly rolled over.

In 1870-'71 it was necessary to remove the deck, build up the west side vertically for 3 feet, and relay the deck. No future dredging can safely be done nearer to this section than 30 feet.

In 1889 the superstructure was first renewed, built this time with 12 inch pine timber, and with longitudinal tight deck.

Section D is 150 feet long and 20 and 30 feet wide. It was built in 1870-'71 in prolongation of the preceding sections. The substructure formed of four cribs, each 20 feet wide and 30 feet long, and one 10 feet square for a pier head. They were built of sawed 12 by 12 inch hemlock timber, with tight floors, and were sunk upon the natural bottom at depths of 10 to 11 feet by filling with loose stone. The continuous superstructure of four courses of 12 by 12 inch pine timber was built upon the cribs the same season.

The superstructure was first renewed upon it in 1889-'90, at the time when renewal was made upon sections B and C.

Section E is 242 feet long in prolongation of last section; is 20 feet wide, and was built complete in 1881.

The cribs forming the substructure are each 30 feet long, 20 feet wide and were built of 10 by 12 inch sawed hemlock timber, framed together and bolted with $1\frac{1}{2}$ inch iron drift bolts. The floors are of 3-inch hemlock plank, laid 18 inches above the bottom, and with 3-inch spaces, and screw-bolted grillage.

The cribs were filled with loose stone and sunk end to end in contact upon the natural bottom, having an average depth of 12 feet. There was no dredging or riprap for their foundation. The continuous superstructure of four courses of 10 by 12 inch pine timber was built upon the cribs the same season and filled with loose stone. The deck was of 3-inch pine plank, laid crosswise and close together.

Section F is 272 feet long in prolongation of the last sections, is 20 feet wide, and was built complete in 1883. The cribs were similar to those of the last section, except that their floors were laid 30 inches above the bottom and with 18-inch spaces. These cribs were also sunk upon the natural bottom, having an average depth of 12 feet without dredging or riprap for foundation. The bottom was leveled for the first time only by throwing in stone.

The continuous superstructure of four courses of 10 by 12 inch pine

t the same season, and was of 3-inch pine plank, laid crosswise close together.

Section G is 200 feet long in prolongation of the last section, is 20 feet wide and was built complete in 1885. The cribs differ materially from the former ones. Foundation was prepared for them in 1884, by digging upon the natural bottom a mass of random stone 3 feet in thickness and 30 feet in width. Upon this, in 1885, were sunk the cribs, which were each 50 feet long and 20 feet wide and 14 feet deep and were built of 10 by 12 inch sawed hemlock timber, except the two upper courses, which were of pine. In addition to the usual drift-bolt fastening each crib also had six vertical pine posts extending from the grillage bottom to the top of the superstructure and screw bolted to each timber. The cribs were of 3-inch hemlock plank, 36 inches above the bottom, but not clear across, an open space of 4 feet width being left along each side to permit the stone filling to settle. This last feature has proved to be a radical defect, resulting only in a waste of stone filling and requiring (in 1888) the repair of the outer end, from which all the stone had washed away. It was necessary to build and sink within the crib four interior cribs to close these openings, and then to refill the outer end of the crib with stone.

The superstructure of four courses of 10 by 12 inch pine, which was built within the crib the same season, 1885, has a longitudinal tight deck of 3-inch pine, laid directly upon the top set of cross-ties without the usual deck joints. This section, upon which the light-house stands, completes the west jetty.

East breakwater.—This is 1,850 feet long, consisting of 1,680 feet of shallow crib work, is 20 feet wide, placed upon the crown of the bar depths varying from half a foot above extreme low-water to 2½ feet above the same plane. It extends from near the east side of the bay to the east jetty, its junction with the east shore being effected by 170 feet of stake and fascine work.

Its details are as follows:

Section A is 170 feet long and 8 feet wide and was built in 1885, from the east shore to the crib-work breakwater, which it joins 50 feet west of its east end.

It consists of stakes and wire-bound fascines, made from the trunks and branches of hard-wood brush, the whole paved with stone gathered from the lake shore. It has stood without attention and is covered by an accretion of sand and gravel.

Section B is 150 feet long and 20 feet wide and was built in 1878, forming the east end of the breakwater.

It is formed of 5 cribs, each 30 feet long, 20 feet wide, and 3 feet deep, resting upon the natural bottom. The cribs were built of 10 by 12 inch hemlock timber, with a close floor of hemlock plank one foot above the bottom.

A continuous superstructure of two courses of 10 by 12 inch pine over was built over the cribs the same year and the whole filled with stone and decked with 3-inch pine plank laid cross-wise, with 3-inch spaces. The deck was 4½ feet above extreme low water. It has since been renewed, except its western 50 feet, which was included in the portion rebuilt in parapet form in 1891.

Section C is 750 feet long and 20 feet wide. It was originally built in 1875, its cribs and superstructure being of the same description as those of the last section. Its superstructure was not renewed until 1891, when it was rebuilt in parapet form. The parapet on the lake

front is 9 feet wide and 3 feet higher than the harbor front, which was rebuilt at 4½ feet above extreme low-water level for 11 feet width.

The whole was filled with loose stone and decked with 3-inch longitudinal pine plank, laid tight.

Section D is 540 feet long, 20 feet wide, formed of cribs and superstructure like those of the last section, and was built in 1874. The superstructure was renewed in the parapet form just described during 1889-'90.

Section E is 240 feet long, 20 feet wide, formed of cribs and superstructure like those of the last sections, and was built in 1873. The superstructure was renewed in the parapet form just described during 1889-'90 in connection with the last section. This joins the east jetty with which its superstructure is bonded.

(4) *East pier or jetty.*—This is 1,510 feet long from the west end to the east breakwater across the bar to 11½ feet depth in the lake. It is located parallel with the west jetty and 250 feet from it. The superstructure cribs of the inner 512 feet were placed in a trench dredged 12 feet wide in a gravel bar to a depth of 8 feet below extreme low water; the next 757 feet were placed in a trench dredged to 10 feet below extreme low water, and the remaining 241 feet were placed upon the natural bottom at depths of 10 to 11 feet. The details are as follows, beginning at the inner end:

Section A is 512 feet long, 20 feet wide, and was built in 1872-'73. The cribs forming its substructure were built of 12 by 12 inch saw hemlock timber framed and drift-bolted together. They had close floors of hemlock plank laid 18 inches above the bottom upon the timbers of a screw-bolted grillage.

Each crib was 30 feet long, 20 feet wide, and 9 feet deep. The site was prepared for the cribs by dredging across the gravel bar a trench 30 feet wide to 8 feet below extreme low water, where the natural depth was 2 feet at the inner end, increasing gradually to 6 feet at the outer end. The cribs were sunk in this trench by filling them with loose stone.

The continuous superstructure of 4 courses of 12 by 12 inch pine timber was afterward built upon them and was also filled with loose stone and was decked with 3-inch pine plank laid cross-wise, with 3-inch spaces. The superstructure was not repaired until 1887, when it was renewed by building 4 courses of 10 by 12 inch pine timber, decked with 3-inch longitudinal plank laid tight.

Section B is 757 feet long in prolongation of the last section, 20 feet wide, and was built in 1881 and 1882. Each crib was 30 feet long, 20 feet wide, and 12 feet deep, the lower 10 courses of hemlock and the upper 2 courses pine. The cribs had close floors of 3-inch hemlock plank laid 18 inches above the bottom upon the timbers of a screw-bolted grillage.

The site was prepared for the cribs by dredging a trench to 10 feet below extreme low water, where the natural depth was from 6 to 8 feet.

The continuous superstructure of 4 courses of 10 by 12 inch pine timber was built the same season, filled with loose stone, and decked with 3-inch pine plank laid cross-wise, but tight together. It has since been renewed.

Section C is 241 feet long in prolongation of the last section, 20 feet wide, and was built complete in 1883. Each crib was 30 feet long, 20 feet wide and 12 feet deep, the same as those of the last section. The foundation was not prepared for them by dredging or riprapping, but that the bottom was leveled for the first crib by throwing in

e natural depth when the cribs were placed was 10 to 11 feet at me low water. The superstructure was built the same year, similar at of the last section.

is completes the present east jetty, which is, however, about 500 shorter than its proposed length.

CHANNEL.

ior to beginning the construction of jetties, there was no permanent channel across the bar which closed the entrance to the

e general width of this bar from deep water within the bay to deep r in the lake outside was from 600 to 900 feet between 6-foot curves, t 2,000 feet between 12-foot curves and 2,600 feet between 15-foot as. These and all depths stated in this report are at extreme low- r level, which coincides with the plane of zero of the Oswego gauge h was established at extreme low-water level in 1837.

e natural depths upon the crest of the bar and in the channel is it varied from time to time.

1828 and in 1845 the crest of the bar was above water for most of istance across the bay, the channel (at the same place as the pres- ne) being stated in the annual report for 1847 "to have but 2 feet iter." If this meant the actual depth, which seems probable, its ation to extreme low-water level would give but half a foot.

1853, just before the first jetty construction, the depth was 1½ feet v extreme lowwater, which was in 1854 increased by dredging, r the shelter of the west jetty to 6 feet. The amount of material removed to make the channel, which appears to have been about eet long and 80 feet wide, must have been about 4,000 cubic yards. 1866, when the next work of improvement was begun, a detailed ey showed this cut to be 5½ feet deep.

the west jetty was extended during 1866, 1867, and 1868, the chan- as enlarged by dredging close beside the jetty and in August, 1868, s reported to be 10 feet deep, 80 feet wide, and 800 feet long. The int dredged is not reported, but it appears to have been about 0 cubic yards, at a cost of 24 cents per cubic yard in scows.

1869, the width had been increased to 200 feet, but in 1871 22,625 : yards of gravel, sand, and mud were excavated, at 17 cents per : yard, to make the depth 10 feet over this width for 1,200 feet h from bay to lake.

1872 and 1873 small amounts of dredging were done at \$1 per : yard, but it was limited to the excavation of a trench for crib lations and no dredging was then done in the channel.

1875, a shoal about 100 feet wide had formed across the channel ind-drift around the outer end of the west jetty, which then ex- d only to the 9-foot curve. This shoal had 8 feet depth and cubic yards were dredged from it at 25 cents per cubic yard, to : a passage 80 feet wide next to the west jetty.

exceptionally high stage of water prevailed in the lake in 1876, o further dredging was done until 1879, when the deepening of hannel to 15 feet depth at extreme low water was begun. During and 1880 and 1881 there were removed 25,931 cubic yards of gravel, , and mud, at 18 cents per cubic yard, scow measurement, and the was continued in 1881 at 17¾ cents per yard. At the latter rate, 2 cubic yards, scow measure, were removed up to December 1, 1881, ng 15 feet depth at extreme low water for 200 feet width and 1,700 ngth.

1891, continuing until December 1, when weather per this time 7,257 cubic yards of sand and mud and 54 hardpan were removed. The work done showed that too tough to be dredged economically and that blasting necessary for its removal.

OPERATIONS.

The work of removing the superstructure of 800 Breakwater, being all of Section C, and the western 50 ft was begun August 10, 1891, and continued until December it was practically completed. There remains only a smoothing up and finishing now to be done. This was necessary to the strength and durability of the work, give it a neat and workmanlike appearance.

The superstructure was built in the parapet form, the most recent and approved design. The lumber and iron work were purchased by public notice and sealed proposals and material and labor required were obtained in open market. It is an excellent piece of work, and is well and economically done. The cost was \$7,594.85, divided as follows:

Labor
Lumber
Iron
Stone
Other material and tools
Total

Or at the rate of \$9.49½ per foot. This sum includes unusually large amount of stone, which it was necessary in order to fill the superstructure on account of its increase in weight.

Under authority of the Chief of Engineers \$5,000 was appropriated for this work towards the purchase

not employed at that time. The dredge continued to work when the condition of the lake permitted until the end of November, when it returned to Oswego and was laid up for the winter.

The total material dredged and removed from the channel was as follows:

	Cubic yards.
Hardpan	541
Sand	7, 257

The amount expended for this purpose was \$2,622.29, the cost being reasonable considering the character of the material. In addition to the foregoing sums, \$1,001.52 has been expended during the fiscal year for engineering and office expenses.

REMARKS.

The reports show a small increase in the number of vessels arriving at this harbor over that of last year; but no material increase can be looked for till the effective depth of the channel is increased.

The completion of the project requires about 665 feet of pier extension, and the dredging of 20,000 cubic yards of hard material from the channel. This will cost in all about \$58,500, and in addition to this sum there should be available \$6,000 every year for the repair and maintenance of the existing works. The material to be dredged through from the outer end of the jetties to the 15-foot curve in the lake appears to be all hardpan, and it is reasonable to expect that a cut dredged through it would remain open, unless filled up by sand moved out beyond the ends of the jetties from the beach.

There is no satisfactory evidence that the beach sand is moved out as far as this. Even if the sand does move out to the location of the cut in moderate quantities it might be cheaper to maintain the channel by dredging than it would be to keep the increased length of jetties in repair.

I think the experiment worth trying at this place, and I would, therefore, recommend that the jetties be not extended any farther until after the proposed dredging has been completed and the effect observed for one or two seasons.

The small amount of money now available will be applied during the present season to dredging and blasting in the channel according to the project.

Name of harbor, Little Sodus Bay, New York. Collection district, Oswego, N. Y. Nearest light-house, Fair Haven, N. Y.; a fixed white light of the fourth order near the head of the west pier, a fog bell attached; a mast range light at inner end of west pier. Nearest work of defense, Fort Ontario, N. Y.

Money statement.

July 1, 1891, balance unexpended.....	\$17, 960.52
June 30, 1892, amount expended during fiscal year.....	16, 218.66
July 1, 1892, balance unexpended.....	1, 741.86
Amount appropriated by act approved July 13, 1892.....	6, 000.00
Amount available for fiscal year ending June 30, 1893.....	7, 741.86
Amount required for completion of existing project.....	52, 500.00
Amount that can be profitably expended in fiscal year ending June 30, 1894.....	40, 000.00
Balance with requirements of sections 2 of river and harbor act of 1867.	

2580 REPORT OF THE CHIEF OF ENGINEERS, U. S. ARMY.

COMMERCIAL STATISTICS.

Arrivals and departures of vessels at Little Sodus Harbor, New York, during fiscal year ending June 30, 1892.

ARRIVALS.

Trade engaged in.	Steamers.		Sailing vessels.		Barges.	
	No.	Registered tonnage.	No.	Registered tonnage.	No.	Registered tonnage.
Home, on lake	57	2,673	51	8,017	46	11,168
Foreign, on lake	27	3,832	158	33,470	62	22,590
Total	84	6,505	209	41,487	108	33,758
American owned	58	2,728	28	1,537	36	8,506
Canadian owned	26	3,777	181	39,950	72	25,162

Sum total, 401 arrivals; registered tonnage, 81,750.

DEPARTURES.

Trade engaged in.	Steamers.		Sailing vessels.		Barges.	
	No.	Registered tonnage.	No.	Registered tonnage.	No.	Registered tonnage.
Home, on lake	59	2,707	26	2,565	30	8,034
Foreign, on lake	25	3,798	182	38,770	78	25,734
Total	84	6,505	208	41,335	108	33,768

Sum total, 400 departures; registered tonnage, 81,598.

Greatest draft of vessels, 13 feet.

Greatest tonnage, 585 registered tons.

Greatest load, 1,193 net tons of coal.

Receipts and shipments by lake at Little Sodus Harbor, New York, during the fiscal year ending June 30, 1892.

Trade.	Receipts.						Shipments.		
	Passengers.	Grain.	Fish,	Railroad ties.	Posts.	Lumber, etc.	Passengers.	Merchandise.	Coal.
	No.	Net tons.	Net tons.	Tons.	Tons.	Tons.	No.	Net tons.	Net tons.
Home, on lake	512						492	8	7,250
Foreign, on lake		235	12	3,746	567	279			122,144
Total	512	235	12	3,746	567	279	492	8	129,394

Navigation closed December 14, 1891.

Navigation opened April 8, 1892.

For the purpose of reduction to tons weight the following is assumed: Barrels oil, 7 equal 1 ton; M lumber, 1 equal 1½ tons; crates fruit, 20 equal 1 ton; railroad ties, 12 equal 1 ton; cords wood and posts, 1 equal 1½ tons; live animals, 4 equal 1 ton.

O O 5.

IMPROVEMENT OF HARBOR AT OSWEGO, NEW YORK.

OBJECT.

To provide a basin sufficient for the needs of commerce at the mouth of the Oswego River, New York, on Lake Ontario, and to secure and maintain a navigable channel into said basin and river.

PROJECT.

1827.—To inclose by breakwaters a western basin of 11 acres area outside the bulkhead line, and an eastern basin of 9 acres outside the bulkhead line, and to protect the entrance channel by piers. (Completed 1829; pier extended 1869).

1870.—To inclose by a breakwater of crib work filled with stone a western basin of 100 acres area outside the 9-foot curve and the existing western breakwater. (Completed in 1882.)

1882.—To build an eastern breakwater on the alignment of the western, sheltering an eastern basin. (Superseded by project of 1888.)

1883.—To build spurs projecting from lake face of western breakwater for the purpose of breaking the accumulated and reflex waves occasioned by that breakwater, which have rendered the entrance exceptionally difficult and dangerous during storm. (Completed in 1889.)

1888.—To remove the east breakwater. (Removed in 1889.)

1891.—To make permanent and to shelter the western entrance to the outer harbor.

PRESENT WORKS.

(1) The present works consist of the inner west breakwater, which lies wholly within the outer breakwater, by which it has been superseded.

(2) The inner east breakwater, which has been private property since 1852.

(3) The Fort Wharf, which constitutes a storage place for harbor material.

(4) The outer west breakwater, whose maintenance constitutes the principal work.

(5) The detached spurs outside the outer breakwater, two of which have been built, and of which two more are proposed to shelter the present western opening.

(6) The deepening of the entrance and river channel in the inner harbor. These works are described in detail as follows:

(1) *The inner west breakwater.*—This is 1,993 feet long, of masonry and crib work of varying widths, and was built between 1827 and 1837. The original structure was composed of cribs of timber, each 30 feet wide and 30 feet long, sunk end to end upon the natural lake bottom in as close contact as was practicable by filling with loose stone.

The cribs were formed of round logs, hewn flat, framed together and bolted with iron drift bolts and wooden treenails. They had floors of slabs, and were allowed to settle upon the natural bottom for a part of a season before building upon them the continuous superstructure of timber.

As the works in which the cribs were placed increased gradu-

ally from nothing, at the origin of the shore arm on the west side of the mouth of the river, to about 21 feet (at extreme low-water level) at the pier head next the river channel. The construction of the timber breakwater was completed in 1829.

In 1837 the construction of a masonry superstructure of cut limestone was begun at the shore end. This masonry superstructure was built upon the shore arm, and extended for 90 feet along the lake arm during 1837, and 1838, and 1839. This portion still stands, the shore arm being in a fair state of preservation. The masonry was extended 155 feet farther along the cribs of the lake arm in 1845.

This portion was not completed and has gradually been destroyed, the great cost and the unsatisfactory character of foundation afforded by the crib work apparently accounting for its not being completed and extended. This is the only masonry superstructure which has ever been constructed at Oswego Harbor.

Between 1830 and 1837 large sums were expended in attempting to form, in front of the lake arm of the breakwater, a mole of pierre perdue. A mass of loose stone from 100 to 500 pounds each was deposited in front of the crib work, extending 6 feet above water, and along the whole lake front, about 1,100 feet.

In 1834 it was given a regular profile and was paved with massive blocks of Chaumont limestone, 2 feet thick, weighing 3 tons each.

This pavement was 30 feet wide from the face of the breakwater and the foot of the slope was formed of larger blocks, each 10 feet long, 3 feet wide, and 2 feet thick. The waves moved these stones so freely and destroyed the mole so thoroughly that its maintenance was abandoned in 1837.

During low water in 1891, about 150 of these blocks were recovered and piled on the fort wharf for use in repairs of the outer breakwater.

Section A: The shore arm is 250 feet long, 30 feet wide, and has a crib-work base with cut limestone superstructure in parapet form. Built in 1828 with superstructure reconstructed of masonry in 1837-1838. In fair preservation.

Section B, from the angle eastward, 110 feet. The same description as the last except that its condition is ruinous.

Section C, located back of section B, overlapping it and extending past it a distance of 138.8 feet, including a 10-foot passage for boats cut through its east end in 1884.

This section is of crib work, 22 feet wide, built back of the ruined masonry structure in 1854. Its superstructure was rebuilt on its south or harbor face in 1887 and on its lake face in 1891.

It serves to shelter the United States reservation slip, where the United States boats, scows, and dredge are moored, and to provide a storage place for timber needed for repairs.

Section D: This is 838, 2 feet long, in prolongation of the line of the masonry portion of the lake arm eastward, and extends to the light-house pier head. It was built in 1828-29 and its superstructure was rebuilt and repaired in 1844, 1854, 1860, 1867, 1874, and was burned in the great fire of 1881.

It was transferred to the Light-House Department in 1886 and was rebuilt by the Light-House Department in 1889, in parapet form. The superstructure is built of 12 by 12 inch pine timber, filled with loose stone, and decked with 3-inch pine plank. It is 27 feet wide, founded upon the original crib work 30 feet wide.

At the west end of this section, a small crib-work jetty has just been built in 1892, 12 feet wide and 48 feet long, to shelter the passage way

or boats from the outer to the inner harbor. The jetty is formed of a piece of parapet and of old timbers from the 1884 breach in the outer breakwater, and its superstructure is of blocks of stone taken from the adjoining lake bottom.

A bridge crosses the passage way high enough to permit the passage of the Life-Saving Service boat at extreme high water.

Section E is the light-house pier head of irregular form, extending 2 feet on prolongation of the line of the breakwater and 132 feet at a right angle to it northward. Upon a mass of loose stone, inclosed by the cribs forming the pier-head substructure, stands the masonry light-house, which was built in 1836.

The cribs were built in 1835 and its superstructure was repaired and rebuilt repeatedly until the transfer to the Light-House Department in 1886, by which department it was last repaired in 1889.

Section F is 432 feet long, in prolongation northward into the lake of the pier head, and is 30 feet wide. It was not included in the transfer to the Light-House Department. It was built in 1869 of cribs 30 feet long and 30 feet wide, which varied in depth from 15 to 26 feet. These were built of 12 by 12 inch sawed hemlock timber, with floors of 3-inch hemlock plank laid, with 7-inch spaces, upon the screw-bolted members of the grillage which formed the bottom of each crib, the floor-plank being 18 inches above the bottom.

The cribs were sunk upon the natural bottom, without dredging or apron, in depths varying from 11 feet at the pier head to 15 feet at the outer end.

The material forming the bottom was sand and the cribs settled very irregularly into it 4 to 10 feet, making a crooked line and requiring much leveling.

The superstructure of 6 courses of 12 by 12 inch pine, with 3-inch pine deck-plank laid cross-wise 3 inches apart, was completed in 1870 and has not since been renewed. It is thoroughly decayed and its renewal, in parapet form, is the next work proposed. Another section, which was a part of the inner west breakwater, but has disappeared, was designed in 1869 and then made a part of the project, but was not built until September and October, 1871. It was a crib 100 feet long, 10 feet wide, and 7 to 11 feet high, placed across the right angle at the junction of the light-house pier head with the breakwater, the inclosed triangular space being filled with about 1,000 cubic yards of loose stone.

Its purpose was to relieve the pier head and the light-house from the shock of the waves caused by west and northwest winds. It was sunk upon the existing bottom, where the depths at extreme low water were from $2\frac{1}{2}$ to $6\frac{1}{2}$ feet. The material which formed the bottom was the remains of the mole (which had long before disappeared) and the accretion of sand which had formed against the breakwater where the original depth was about 20 feet. The crib and its superstructure were completed, with its deck at $4\frac{1}{2}$ feet above extreme low water, and the space behind it was filled with stone to the same level in October, 1871. During the first northwest gale, in November, it rolled over toward the waves (apparently by the material in its front being cut out and by the reflux of waves which rolled over it into the inclosed space) and was entirely destroyed during the same winter. It now has no place in the project.

The inner east breakwater.—This is 750 feet long, 30 feet wide, and extends on the prolongation eastward of the line of the inner west breakwater. From its pier head it is distant 357 feet.

It flows between them into the lake.

From this point the breakwater extends to the east shore, beyond Fort Ontario. It was built in 1828-29 in the same manner as the west breakwater. It was repaired in 1844, and in 1852 was transferred to Gerrit Smith, esq., whose estate since 1852 has owned and maintained it, building under its shelter wharf property which for many years was very valuable, but which is now not so much used as formerly. In consequence the breakwater has not been properly maintained some years past and it is now in a damaged state.

The northwest angle was destroyed in 1891 and has not been repaired and the shore arm is in a ruinous state.

(3) *The fort wharf.*—This is the landing in the inner harbor for Fort Ontario, and forms a part of the United States Reservation pertaining to that fortification. It has been of value in connection with the improvement as a storage place for timber and stone and for fire and launching cribs.

It has a water front of about 250 feet with an area of about 100 by 200 feet. The crib work forming its water front was built in 1828 when the fort was built; was repaired in 1863, when the fort was reconstructed, and has recently been repaired above line of no decay on the harbor work.

It is now in good order.

(4) *Outer west breakwater.*—This is 6,033 feet long, enclosing 1,000 acres (outside the 9-foot curve) of what was formerly the open bay in front of the city and of the inner harbor. It starts from the west shore, nearly a mile westward from the river outlet, and extends eastward upon a line generally parallel to the lake front and 1,200 feet to a point opposite the entrance to the inner harbor.

The breakwater is formed of timber crib work filled with loose stone and is 35 feet wide throughout.

Its substructure is formed of cribs each 35 feet long and 35 feet wide, a few of those last built being of double this length. Its superstructure is also timber and is now in parapet form. Its height on the shore is 9½ feet above extreme low-water level; on the lake arm, 13½ feet.

The original survey for the location of this breakwater was made in September, 1869, the plan was approved by the Board of Engineers on March 30, 1870, and the work of construction was begun on July 1, 1871. It was completed in July, 1882. Its details are as follows: The cribs being built upon essentially the same plan as those of the shore arm next described.

The shore arm is 916½ feet long, from a ledge of rock forming the shore, northeastward into the lake to 18 feet depth at extreme water.

Section A, from the shore 846½ feet, was built in 1871-72 on a natural bottom, the cribs being built to fit the irregular ledges of rock on which most of them were placed. The cribs were formed of 12 by 12 inch hemlock timber, all the sticks being of the same length and 35 feet.

The timber forming the grillage bottoms were screw-bolted together. Those forming the sides were driftbolted, course by course, as by the sides being held together against the outward thrust of the stone filling by the dovetailed heads of the cross-ties and by inclined halvings.

The floors were of 3-inch hemlock plank, laid 3 inches apart, 18 inches above the bottom, upon the timbers of the grillage, a central 3 feet square being left without plank.

The continuous superstructure built the same season upon these cribs was formed of 12 by 12 inch pine timber, 7 courses in height, with a horizontal deck 8 feet above extreme low water, of 3-inch pine plank laid crosswise, 3 inches apart, upon 6 by 12 inch pine joists, loose of random sizes being filled in to the top of the joists. The planks were fastened by 9 by $\frac{1}{2}$ inch spikes, and by six longitudinal bands of 2 by $\frac{1}{2}$ inch flat iron.

This deck was maintained by annual repairs of varying extent every year from the first until the rebuilding of the superstructure in 1887. It was then built in low-parapet form of the 1887 model, which differed radically from the 1884 model hereafter described.

The 1887 model is a parapet 9 $\frac{1}{2}$ feet high above extreme low water one-third the width, with the inner two-thirds 5 $\frac{1}{2}$ feet high. Its timbers are held together by screw bolts and vertical oak posts instead of by tie leads. Deck joists are omitted, and the 4-inch pine deck plank laid longitudinally and tight together upon the top set of cross-ties. It cost no more than the work which it replaced. It has neither needed nor received any repairs whatever since its completion in December, 1887.

Section B is 70 feet long, in prolongation of the last section, to the angle of the lake arm. It was built in 1871-'72, at the same time and in the same manner as the last section. Its superstructure had the same frequent repairs until 1884, when, with the following section, the superstructure was renewed in parapet form of the 1884 model. It has received some repairs during the year 1892, and the superstructure on the exposed face needs repair below water.

Section C of the lake arm is 2,910 feet long from the angle eastward. Points upon the lake arm are described by naming their distance from the angle, measuring along the axis.

It was built in 1872-'77 of cribs, each 35 feet long and wide, of depths varying from 18 to 30 feet. The cribs were sunk upon the natural bottom of the lake, and about 30 of the first ones settled more or less irregularly into the sand. The bottom upon which the rest stood was of stones and gravel, and very little settlement took place, generally about one to two feet on the lake face.

During the winter of 1872-'73 700 linear feet of cribs, from 80 feet from the angle eastward, were left without superstructure to settle until next season. During the winter the three last cribs, from station 671 to 776, each 28 feet deep, were moved off the line by the waves, the last one being thrown ashore. All the cribs left open were damaged and had to have shallow cribs sunk upon the damaged substructures. This was done in 1873, and the superstructure was built over them 8 feet high above extreme low water with a flat deck. This was annually repaired until 1883, when a more serious damage occurred, by which the superstructure was wrecked and the crib tops carried away to 12 feet depth for 105 feet.

In 1884 these crib tops were rebuilt by sinking 12-foot cribs on top of the original bases, and then the entire superstructure for 2,910 feet, including this piece, was renewed. The renewal was in parapet form upon what is described as the 1884 model. The parapet is 13 feet high next the lake for a width of 12 feet. It offered more resistance to waves, which had formerly rolled over the lower superstructure. It was replaced. In October, 1884, immediately after the completion of the superstructure was shifted inward from station 475 to 615. The crib tops were not broken apart by this movement,

feet above the still-water level within the harbor. The breakers made the break were 19 feet above still-water level, at 10 miles per hour.

The opening thus made through the breakwater is an improvement in the sanitary condition of the harbor, and was so convenient an entrance for the harbor, and was so convenient an entrance for the harbor, that it has not been rebuilt and will be made a permanent.

Since the building of the 1884 parapet upon this section it has received damage at many points and has had costed though much less than required by the original flat section.

The part of the breakwater which is in a condition of deterioration, and which may need radical repairs, is the section of the break. The actual breach was limited, as above stated, to 19 feet, and it has not since extended. But the section strained and its joints opened for 100 feet farther on the breakwater has only been maintained by closely watching it and using screw bolts and steel plates to check indicated yielding.

The exposed face of all the cribs of the substructure from the angle eastward for about 700 feet (including the breach) are more or less shaken, and show signs of deterioration. For instance, on January 8, 1890, five courses went out of the crib next the angle, 5 to 10 feet below water level. This is at 24 to 47 feet eastward from the angle.

Through this submerged opening all the stone fell out down to 10 feet below water, and a large breach followed.

It was repaired by building within the parapet into the breakwater, filling the spaces between the ties and filling these crevices with concrete above water with concrete and lowering them down to 10 feet below water. In this way concrete was formed.

In this way there were formed 3 monoliths, each 8 by 20 feet high, which put no strain upon the shaken breakwater. The method of construction had been proven necessary by the condition of the breakwater in the preceding season of similar interior cribs filled with concrete.

t, were made of creosoted Georgia pine, and these are sound. The other timbers already show some decay.

Section D is 570 feet long in prolongation of the last section eastward. It was built in 1877-1879 with its cribs placed upon the natural bottom, some being double the usual length, and with a continuous superstructure, which was 8 feet high with a flat deck. The superstructure needed repairs at various times, and in 1890-'91 it was renewed in its present form, in connection with the following section.

The details of this parapet differed entirely from that of 1884, which was renewed, and is also different from the 1887 shore-arm model. Like the latter it is tied together by the tensile strength of screw bolts and oak timbers, and its deck is of 4-inch pine plank laid tight. But none of its timbers are dovetailed or cut into, all being left square and full size throughout. It is the only work ever built at Oswego in which no timbers were displaced by gales occurring during its incomplete state.

When 600 feet had been finished the incomplete end of it was left without dock or stone filling through the winter of 1890-'91. There was an absence this winter of the usual ice covering to protect it, and the gales were as severe as ordinary. But construction was resumed in the spring of 1891, and the work was completed without having to replace a single stick. Its evident stability promises a freedom from repairs equal to that of the shore-arm work.

Section E, is 900 feet long in continuation of the lake arm. Its cribs were built in 1880 and differ from all the preceding ones in that they rest upon a foundation prepared by dredging a trench in the natural sand bottom 3 to 6 feet deep and 55 feet wide, and then filling it with random stone. Its superstructure was renewed with that of the last section in 1891. A part by contract and a part by hired labor.

The comparison of cost by the two systems is made in an appended report by United States Assistant Engineer Judson, who designed this class of structure under the direction of Captain Palfrey, Corps of Engineers.

Section F 490 feet long in continuation of the lake arm to the eastern end. The cribs were built in 1880-1882, and founded upon random stone placed in a trench 3 to 10 feet deep and 55 feet wide. The cribs were about 18 feet deep, and the original superstructure was built in two courses high with a flat deck. In 1884 the parapet superstructure was built upon the other, all of white pine, without the creosote in the timbers and supports which was used in section C.

It was damaged by a schooner which was wrecked against the breakwater in 1885, but the shaken timbers held together until 1891, when the injury was repaired in an effective manner by screw-bolting a large iron plate to the face, below water, and filling back of it with grout of cement.

This carries the description to the east end of the lake arm, where the channel arm extends southward, inclosing the beacon crib.

Section G, the channel arm, is 246 feet long from the south side of the lake arm, with which it makes about a right angle, and extends eastward. It was built in 1882, its cribs being placed upon a foundation of random stone, filling a trench dredged 4 to 10 feet in the natural sand bottom, where the depth of water was 20 feet and the sand 15 or more feet deeper.

The superstructure upon the first 62 feet next the lake arm had originally a flat deck, 6 feet above extreme low water, and the parapet was renewed in 1882—like that on the preceding section. The inner 184 feet

has still the flat deck, but its change to parapet form is part project.

Its inner angle was cut into 12 feet below water by a barge requiring extensive repairs.

The detached spurs.—Under the project of 1883 two or more detached spurs outside of the outer breakwater were provided for. Of two have been built; one, situated 250 feet west of the eastern end of the breakwater, was built in 1885. The other, situated midway of length of breakwater, was built in 1889. The first one consisted of a single crib 100 feet long, 40 feet wide, and 25 feet deep, with 4-inch floors, 4 feet above the bottom, and with vertical sides to mean level. It is placed with its long axis at a right angle to the face of the breakwater and is separated from the latter by a space of 10 feet. The natural bottom at the place was of sand 15 or more feet deep. A trench was dug for each side and each end of the crib, 10 feet deep, 15 feet wide at bottom, and this trench was filled with random riprap. The riprap base thus formed extended 10 feet outside of the crib line all around. It was leveled at 21 feet below extreme low water and the crib sunk upon it.

The superstructure was built upon the crib 14 feet high, sloping the sides to a central top width of 12 feet. The angle at the top of the slope is covered and joined to the substructure by plates of 1/2-inch tank iron. The crib is formed of one thickness of 12-inch hemlock timber, with three top courses of oak. The superstructure is of one thickness of 12 by 12 inch pine timber; the whole is bolted, and strengthened with screw-bolted posts. The deck is 6 inches thick, of 6 by 12 inch pine timber laid lengthwise of the crib and bolted tight together. The whole is filled with loose stone. The structure has not yet yielded at any point, but its foundation proved to be deficient. It was undermined by the wave action along the west face. In 1888 the west side had settled to 7 or 8 feet lower than the eastern side. It now stands in this position. Large amounts of dredge material, sand, stone, gravel, etc., have been deposited along the west side, filling up to 10 feet depth, and the structure appears secure from further settlement.

The 10-foot space between the crib and the breakwater, through which violent wave action occurs, has not shown any of the scour which was looked for, and which was to be expected from its extremely exposed location.

The second detached spur, built in 1889, was located 2,600 feet westward of the eastern end of the breakwater, about midway of its length.

It also is placed with its long axis at a right angle to the breakwater face, from which a 10-foot space separates it.

Its length is 150 feet, with a bottom width of 40 feet, a surface width of 26½ feet, and a top width of 14 feet. The sides slope regular 5 feet above the bottom to the top of the superstructure. The deck is of 6-inch hemlock, laid tight, 24 inches above the bottom. Non-parallel timbers are framed or dovetailed, but the structure is tied together by screw bolts through vertical oak posts, and through the sides and ends.

The crib and its superstructure were completed within the time allotted, except laying the 6-inch tight deck. Its lower twelve courses were caulked, and the crib was ballasted with 1,200 tons of stone fill, which was towed to position, when it was sunk by admitting water through the valves.

The valves were at once closed again, so that a gale which occurred immediately after sinking it, and before it could be filled with

to displace the crib, the waves keeping the superstructure filled to top, 12-feet above water level. The natural bottom at the location of stone and bowlders, so that it was only necessary to level the with 2 feet of riprap. The crib has not settled and is in good con-

The *outer east breakwater* was located upon the prolongation east-of the line of the outer west breakwater and 351.75 feet from it, entrance to the harbor being between them. It was built in 1882 the project of that date as the first section of an outer east break- intended to be 2,700 feet or more in length.

It consisted of a lake arm 213 feet long, with a channel arm 35 feet Its cribs were like those of the west breakwater, and its super- structure was built in parapet form the same season as the cribs. removal was projected in 1888 and was done in 1889.

HARBOR DREDGING AND RIVER CHANNEL.

17.—The first dredging of which there is record was done in 1847, a gravel bar which had formed across the river channel, appar- about 1,000 feet inside the breakwater, was removed at private

18.—The next was in 1855, when the United States dredge, under tion of the engineer in charge, was engaged in deepening the en- to the river near the head of the upper island, at the expense of

19.—This dredge was sold in 1861 for lack of funds to maintain any other dredging is reported up to this date and it is not likely any had been done, as the harbor and its entrance had a general in excess of the draft of the lake vessels.

20.—In 1866 the inner harbor adjoining the west breakwater too shoal to accommodate the increased draft of vessels to 12 feet, its dredging was commenced. The quantity and price of excava- is not stated, but \$29,451 was thus expended to June 30, 1867, in about \$12,000 was estimated as the cost of completing it. The it was continued and finished in 1868 and 1869, 12 feet depth at ex- low-water level being reported in the west inner harbor.

21.—No further dredging was done until June, 1881, when work for contract was begun for deepening the river channel from the rance near the light-house to the head of the island at the foot of ayler street for a length of 1,426 feet and a width of 125 feet. This ened to 15 feet below extreme low-water level the west half of the channel, outward from the south line of Schuyler street, excepting all area next the upper island wharf, where bed rock prevented ing below 12 feet. Twenty-three thousand one hundred and ay-two cubic yards, at 70 cents per yard in scows, were removed in The material taken out was gravel and stones, with many large of bed rock, brought down by ice from the river bed in the rapids re the city, the thickness of layer worked upon by the dredge vary- from 1 to 6 feet. Before beginning work many borings were made he area to be worked upon to determine whether the channel could ade. Bed rock, with 12 feet depth, was found beneath the loose bit, extending out 50 feet into the channel from the east and west ad wharves, just north of Schuyler street. No attempt was made remove this.

22.—In September, 1883, dredging was resumed to deepen the alf of the channel. This was done at 30 cents per cubic yard, in

increased to 14 feet, and the inner harbor, above the
ing of 1884, was too shallow, except at a high stage.

1890.—During 1890 and 1891 rock excavation was
done at private cost to extend the channel above the
Marine Elevator (the most northern one on the east
New York, Ontario and Western Railroad coal trans-
east island wharf. This at once increased the available
harbor to 16 feet, and large steamers came to use
was fully complete.

1891.—In 1891 the clearing off of loose material
was continued by the United States up to the north
street, a farther distance of 900 feet for 240 feet
dredged 100 feet next the wharves on each side. The
effective depth was increased 2 to 3 feet, the rock slope
at the north end of the work to 7½ feet at the south
end. Eight hundred and forty-five cubic yards were
scows. In addition there were many pieces of material
which the dredge could not raise, which were taken
States hired labor with a derrick boat. The aggregate
cubic yards, costing an average of \$7.58 per yard.
this the ridges of gravel and stones were leveled off
of railroad iron over the bottom.

1892.—In 1892 the new United States dredge *Fron*
to remove a ridge of sand 20 to 50 feet wide, which
outer harbor along the harbor face of the outer break-
terial washed over and through it. This work began
and extended thence westward to Station 1400. It
duced the original depth of about 17 feet to 10 to
moval required the dredging of 10,010 cubic yards,
yards of stone crib filling, which was saved. The dredge
in 1892, the boat passage, which was originally drawn
inner old breakwater to the outer harbor in 1884.

work done by contract, has been prepared by Assistant Engineer Judson, who was in local charge of the work, and it is transmitted herewith. The report indicates that if the whole work had been done by unskilled labor a very considerable saving might have been made. The cost of this work, including the cost of bonding into former work, equivalent to 15 feet additional length, was \$7,065.35.

Dredging near the mouth of the river by E. J. Hingston, under formal written contract, was begun July 20, 1891, and was continued until August 31, when, finding that he could not complete his contract by September 20, the date upon which it expired, he asked for and was granted an extension of time till November 1, 1891. By authority of the Chief of Engineers he was not required to pay the cost of inspection during this period. A second extension was afterwards granted November 15, with the usual requirements that the contractor should pay the increased cost to the United States. The work was finally completed November 13, 1891. The object of this work was to clear off the gravel and loose rock that covered the ledge that forms the bed of the river, over an area of about 24,000 square yards, from the north side of Cayuga street northward to connect with dredging that had previously been done.

The actual amount of material removed by the dredge to accomplish its purpose was 16,845½ cubic yards, at a cost, including the inspection, \$4,291.97. The contract price was 23½ cents a cubic yard. In connection with this work a small party of laborers with a derrick scow and other suitable appliances removed from this area some 127 pieces of stone that were too large for the dredge to handle. The cost of this work was \$458.04; \$98 was also expended in drilling and blasting certain very large bowlders that could not be removed in any other way. This makes the total cost of the work done at the mouth of the river \$4,848.01. The estimate was \$6,000. The effect of this work is to increase the depth of this much used portion of the harbor nearly 2 feet. It is an important, valuable, and, I think, a permanent improvement. The lake face of the old breakwater near the stone pier was repaired by hired labor for a length of 136 feet, the cost being \$706.82.

Advantage was taken of the low stage of the water to recover some large blocks of stone from, 2 to 4 tons in weight, which had been placed many years ago in the lake in front of the old west breakwater. They were not much needed now, on account of the shelter afforded by the new breakwater. Ninety-five of them were taken out and placed on the Government wharf. They are very useful in repairing breaks in the cribs below the water line.

The cost of this work was \$209.25. A bad place in the outer breakwater east of the first spur crib was repaired in December. This weakness was developed by a gale that occurred November 25, but was the result of injuries done to the pier in 1885, when a schooner collided with it and was wrecked against it. The outer half of the timber wall, a width of about 4 feet and a length of about 15 feet had been split and carried away, and the remaining half of the timber had been greatly weakened. The injury was partly above and partly below water. It was repaired by studding the back of the break with large-headed spikes, which were allowed to project 3 or 4 inches. A steel plate three-eighths inch thick and large enough to cover the entire break was then bolted over it, and the cavity was thoroughly filled with very rich concrete of cement and sand. The cost was \$269.32. The approach to the boat slip through the old west pier was deepened by the United States dredge. A new and higher bridge was built over

the slip, and two small cribs, made of old material, were sunk to the cut from being filled with gravel by the waves moved by the east gales, to which it is somewhat exposed. The aggregate length of the two cribs is 48 feet. The superstructure is built of large stone. The cost of this work, not including the dredging, was \$679.04.

The United States dredging plant, consisting of one Osgood dredge and two dump scows of 160 cubic yards capacity each, is now in process of construction at Oswego, N. Y., at the date of my last report. It was completed in October, 1891, and sent to Little Bay to be used in deepening the jetties channel. This plant by the order of the Chief of Engineers was paid for from the appropriation for improving the harbor at Oswego, N. Y., being \$3,000. The plant is cared for at Oswego when not in use, and is accounted for in the Oswego returns. The plant returned from Little Sodus Bay in the end of November and was laid up for the winter. In the spring of 1892, after some minor repairs and repainting, the dredge began the removing a ridge of sand that had formed against the new breakwater on the harbor side, reducing the available depth from 18 or 20 feet to 12 feet, and preventing deep-draft vessels from tying up to the breakwater, as it is intended they should. This sand had been carried to the breakwater by waves from the lake, before it was built up in the parapet form. The total amount of sand removed was about 10,000 cubic yards. The dredge also took up and placed upon the breakwater 385 cubic yards of stone which has been used in repair work. The dredge assisted in breaking up and destroying a wrecked and sunken schooner which was an obstruction in the harbor, and it deepened the approach to the boat slip as before described. The total expended for operating the dredge was \$1,660.29. In addition to this, \$719.60 worth of stone was purchased for filling a portion of the parapet of the outer breakwater.

Many minor repairs not mentioned above have also been made on the piers and breakwater from time to time as they were required. The aggregate cost of them all was about \$700.

The cost of watching and care of all plants and material during the fiscal year was \$1,226.67. The engineering and office expenses amount to \$2,084.70.

The repair of plant and the purchase of tools and non-expendable material was \$931.55.

The sum total of all these amounts somewhat exceeds the expenditure for the fiscal year. This is due to the difference in the value of material on hand at the beginning and end of the fiscal year.

REMARKS.

The superstructure of the light-house pier extension, which was built in 1869, is very much decayed. It should be renewed as soon as possible and in the parapet form. The parapet should also be extended to the extremity of the channel arm, which extends inward to the eastern end of the lake arm of the breakwater. The space (300 feet) between the ends of these two sections forms the entrance to the harbor, and as the superstructure of them both is very low, it is difficult to see the ends of them at night, and vessels sometimes get into them in entering and leaving the harbor. If these sections were built in the parapet form, which is 12 feet high above low water, and the ends toward the channel were painted some color that would

at night, the entrance would be much better and safer. This will cost about \$25,000.

removal of what had been built of the east breakwater, thus leaving that portion of the project which called for an eastern breakwater, leaves the whole in a rather unsatisfactory condition. The entrance to the western basin, which would have been sheltered by the east breakwater, is now exposed to the full force of the northeast gales, and at the same time it is almost impossible for a sailing vessel unaided to enter the west basin at all when driven by a strong wind from the northwest. I am of the opinion that the east breakwater will not be built either as originally proposed or else in some modified form before the harbor will be entirely safe and satisfactory.

The present project permits the break, 140 feet long in the outer breakwater near the shore arm, to be left open, and proposes the construction of two properly located spur cribs to protect the entrance. The cribs adjacent to the break are considerably shaken and need extensive repairs, and the spur cribs should be built as soon as possible. The work will cost about \$75,000. In addition to the sums above mentioned there should be available every year about \$25,000 to be used for repair and maintenance of the existing works.

While all of the harbors on Lake Ontario under my charge show a general increase in the number of vessels arriving and departing, no special increase need be looked for under the existing conditions.

The present fleet on the lakes seems ample for the local business, and existing tolls on the Welland Canal, discriminating as they do against foreign ports and harbors of the United States, will be sufficient in the future to prevent any considerable growth of commerce between our western ports and those of the upper lakes. The continuation of the canal on this canal, combined with the removal of the tolls from the New York State canals, has had the effect of building up the Lake Ontario ports at the expense of those of Lake Ontario, and has virtually isolated Ontario from the chain of lakes. The year that sees a free communication opened between the lakes, either by removal of tolls from the Welland canal or by the construction of a canal by the United States, will show an increase of business at the Lake Ontario ports that will surprise those who are unacquainted with the real value of this route unhampered by unnatural and most adverse restrictions.

Location of harbor, Oswego, N. Y.; collection district, Oswego, N. Y.; nearest light, Oswego, N. Y. A fixed white light of the third order at the eastern end of the east breakwater; a fixed red light of the fourth order on eastern end of the west breakwater, a fog bell attached. Nearest works of defense, Fort Ontario, N. Y.

Money statement.

, 1891, balance unexpended	\$29,170.51
do, 1892, amount expended during fiscal year	23,381.95
	5,788.56
, 1892, balance unexpended	5,788.56
, 1892, outstanding liabilities	10.00
	5,778.56
, 1892, balance available	5,778.56
Amount appropriated by act approved July 13, 1892	40,000.00
	45,778.56
Amount available for fiscal year ending June 30, 1893	45,778.56
Amount (estimated) required for completion of existing project	85,000.00
Amount that can be profitably expended in fiscal year ending June 30, 1894	85,000.00
Amount appropriated in compliance with requirements of sections 2 of river and harbor acts of 1866 and 1867.	

2594 REPORT OF THE CHIEF OF ENGINEERS, U. S. ARMY.

REPORT OF MR. WILLIAM PIERSON JUDSON, ASSISTANT ENGINEER.

UNITED STATES ENGINEER OFFICE,
Oswego, N. Y., June 1, 1891

SIR: I have the honor to submit herewith a comparative statement of the cost, by contract and by hired labor, of work done upon the Oswego outer breakwater during the year 1891:

The works were precisely alike, and therefore offer an opportunity for interesting comparison of actual cost by the different methods.

In each case the work consisted in removing the original timber superstructure of rectangular cross section 5.7 feet high above mean water level and 35 feet wide which was built in 1879-'80, and replacing it by timber superstructure in parallel form, 3.4 feet high on the harbor side and 11.4 feet high on the lake side, built on the 1890 model, which was designed specially for this work.

This renewal was required upon 1,470 feet of breakwater. Of this, 1,300 feet were done by contract and 170 feet by hired labor. The latter proved to be the cheaper method, costing \$146 per 100 feet less, although its smaller extent put on it an undue premium for the cost of overseeing and derrick engine.

To make a more just statement of the cost by hired labor, if 1,300 feet had thus been built, there is taken the actual cost for material, labor, tools, tug hire, etc., per 100 feet, and to this is added the cost for the overseer, the timekeeper, and the runner which were employed on the smaller work, and also the cost of two stowmen who would have been needed in addition for the larger work. All being calculated per 100 feet for the four months needed for building 1,300 feet.

This shows that for 1,300 feet the actual cost by hired labor for all material, labor, overseeing, and tug hire, etc., would be per 100 feet, \$3,614. The direct estimate, dated July 25, 1890, of probable cost by hired labor, per 100 feet, was \$3,937. The actual gross cost by contract for 1,300 feet, as stated in last Annual Report, averaged per 100 feet \$3,937. This included cost of inspector and of certain work of bonding into the adjoining structure at the beginning.

There was, however, at the last end, to permit bonding into the future work, an amount omitted, so that the actual total was less than 1,300 feet of complete work and this reduction was not allowed for in computing the above average.

The following itemized statements show the average cost per 100 feet, based on the accurate average amounts of each class of the various materials actually used, per 100 feet.

Office and engineering expenses are not included in the comparison, because they would be the same in either case.

Actual cost of parapet superstructure per 100 feet, by contract.

Designation.	Amount.	Rate in place.
Pine for leveling feet, B. M.	4,105	\$33.00
Pine timber..... do.....	58,220	29.00
Pine plank..... do.....	13,626	28.00
Oak timber..... do.....	20,094	38.00
Driftbolts, for leveling..... pounds..	315	.05
Driftbolts..... do.....	2,524	.024
Screw bolts..... do.....	11,897	.034
Wood screws..... do.....	200	.04
Spike..... do.....	1,176	.03
Mooring cleat.....		8.00
Pitch and tallow around head of vertical bolt.....		
Removing and replacing stone ballast..... yards..	714	60 cents.
Inspector, ($\frac{1}{2}$ month at \$100 per month).....		
Total per 100 feet.....		

Actual cost of parapet superstructure per 100 feet by hired labor.

Designation.	Amount.	Rate of purchase.	Itemized cost.
Iron for leveling.....feet, B. M.	4, 105	\$18.90	\$77.58
Iron timber.....do.....	58, 220	18.90	1, 109.36
Iron plank.....do.....	13, 626	18.90	257.53
Black timber.....do.....	20, 094	27.65	555.63
			1, 991.10
Drift bolts for leveling.....pounds.	315	.022	6.93
Drift bolts.....do.....	2, 524	.022	55.53
Raw bolts.....do.....	11, 897	.023	273.63
Wood screws.....do.....	290	.031	6.20
Wedges.....do.....	1, 176	.028	28.81
			371.10
Booring cleat.....			5.00
Wax and tallow around heads of vertical bolts.....			2.80
Wool and repair of tools.....			31.36
			39.16
Total.....			2, 401.30
Wages of mechanics and laborers, construction and ballasting.....			803.24
Foreman, subforeman, time-keeper, and engine runner.....			104.11
Hoisting and boring timber, and transporting to and from hired machine, including four handlings (could have been cheaper done by hand on the work).....			220.20
Light hire, towing derrick scows.....			85.07
Total.....			1, 212.71
Cost per 100 feet.....			3, 614.07

Comparison of these results shows that if the 1,300 feet built by contract had been done by hired labor there would have been a saving to the Government of \$466.

Very respectfully, your obedient servant,

WM. PIERSON JUDSON,
Assistant Engineer.

Capt. DAN C. KINGMAN,
Corps of Engineers, U. S. A.

COMMERCIAL STATISTICS.

Arrivals and departures of vessels at Oswego Harbor, New York, for fiscal year ending June 30, 1892.

ARRIVALS.

Trade engaged in.	Steamers.		Sailing vessels.		Barges.			
	No.	Registered tonnage.	No.	Registered tonnage.	On lake.		On river.	
					No.	Registered tonnage.	No.	Registered tonnage.
Time, on lake.....	463	104, 914	199	26, 236	106	26, 169
Time, on river.....	74	6, 240	583	79, 299
Time, on lake.....	345	60, 374	715	121, 608	316	92, 120
Totals.....	872	171, 528	914	147, 844	422	118, 289	583	79, 299
American owned.....	527	111, 154	202	27, 569	147	33, 436	583	79, 299
Canadian owned.....	345	60, 374	714	120, 275	275	84, 853

Total, 2,791 arrivals; 518,960 registered tonnage.

2596 REPORT OF THE CHIEF OF ENGINEERS, U. S. ARMY.

Arrivals and departures of vessels at Oswego Harbor, New York, etc.—Continued

DEPARTURES.

Trade engaged in.	Steamers.		Sailing vessels.		Barges.			
	No.	Registered tonnage.	No.	Registered tonnage.	On lake.		On river.	
					No.	Registered tonnage.	No.	Registered tonnage.
Home, on lake	460	104,870	203	27,752	120	29,847
Home, on river	64	6,240	589
Foreign, on lake	344	60,196	702	118,334	291	87,272
Totals	870	171,306	905	146,086	411	117,119	589

Sum total, 2,775 departures; 514,420 registered tonnage.

Greatest draft of vessels, 16½ feet.
 Greatest tonnage of vessel, 1,552 registered tons.
 Greatest load of vessel, 1,624 net tons coal.

Receipts and shipments by lake and river at Oswego, New York, for the fiscal year June 30, 1892.

RECEIPTS.

Trade.	Passengers.	Grain.	Coal.	Merchandise.	Fish.	Oil.
	No.	Net tons.	Net tons.	Net tons.	Net tons.	Tons.
Home on lake	3,921	19,444	1,474	1,931	41
Home on river	1,603	4,952	9,763	18
Foreign on lake	247	48,425	1,722	13
Total	4,168	69,472	6,426	13,416	54	18

SHIPMENTS.

Trade.	Passengers.	Grain.	Coal.	Merchandise.	Oil.
	No.	Net tons.	Net tons.	Net tons.	Tons.
Home on lake	7,453	130	135,471	1,754	40
Home on river	34,296	3,288	8,605
Foreign on lake	161	300,211	1,763	18
Total	7,614	34,426	438,970	12,122	58

Navigation closed December 28, 1891; navigation opened April 5, 1892; to enue collected at port during fiscal year, \$267,109.16.

For the purpose of reduction to tons weight the following is assumed: Bar 7 equal 1 ton; M lumber, 1 equals 1½ tons; crates fruit, 20 equal 1 ton; ties and poles, 12 equal 1 ton; cords wood and posts, 1 equals 1½ tons; live; 4 equal 1 ton.

O O 6.

IMPROVEMENT OF HARBOR AT SACKETTS HARBOR, NEW YORK.

OBJECT.

To deepen the natural harbor formed by Ship House Point over an area of about 15 acres to 12 feet at extreme low water. This project was adopted in 1881. Previous to this, in 1826-'28, \$6,000 had been expended for the same purpose.

PRESENT PROJECT.

To limit the excavation to an area of about 6 acres, and to define the entrance and provide a mooring place by building a crib 18 feet square on the point of the shoal extending into the harbor from the end of Ship House Point.

Also to check shore drift by extending a jetty across the end of Ship House Point from the crib above described to the bay outside the point.

PRESENT WORKS.

A crib 18 feet square has been built, as proposed in the project; a jetty of stake and fascine work, loaded with stone, has been completed for a distance of 164 feet, and about 24,000 cubic yards of material have been dredged from the sheltered area, giving a depth of 12 feet at extreme low water.

OPERATIONS.

None, other than the collection and reporting of commercial statistics

REMARKS.

This harbor is formed by a natural spur of loose rock and gravel about 800 feet in length, extending in an easterly direction from the shore, so as to form a small sheltered bay, and the value of the harbor depends upon the preservation of this natural spur or breakwater.

During the war of 1812 the United States began the construction of a large naval vessel upon this spur. The vessel was never launched, work on it having been stopped by the treaty of peace; but the vessel was cared for for a great many years, and incidentally the site on which it stood was prevented from washing away, due to wave action. A few years ago the vessel was sold and broken up, and since then this spur has not been cared for, and consequently is slowly being destroyed by the seas.

It is not, however, exposed to very heavy waves, and can be protected by a rough wall of blocks of stone of one or more tons in weight built parallel to it and from 50 to 100 feet in advance of it, where the water is about 6 feet deep. This stone can be advantageously obtained from quarries in the neighborhood and it is estimated that it can be put in for about \$3 a cubic yard.

Five thousand dollars could advantageously be applied to this purpose and would be sufficient to complete the proposed wall.

No more dredging seems to be necessary at present.

Sacketts Harbor, N. Y.; collection district, Cape Vincent, N. Y.; Sacketts Harbor, N. Y.; a fixed white light of the fifth order, west of town; nearest works of defense, Fort Ontario, N. Y.

Money statement.

July 1, 1891, balance unexpended.....	\$503.35
June 30, 1892, amount expended during fiscal year	54.69
July 1, 1892, balance unexpended.....	448.26
{ Amount (estimated) required for completion of existing project	5,000.00
{ Amount that can be profitably expended in fiscal year ending June 30, 1894	5,000.00
Submitted in compliance with requirements of sections 2 of river and harbor acts of 1866 and 1867.	

COMMERCIAL STATISTICS.

Arrivals and departures of vessels at Sacketts Harbor, New York, during fiscal year ending June 30, 1892.

ARRIVALS.

Trade engaged in.	Steamers.		Sailing vessels.		Barges.	
	No.	Registered tonnage.	No.	Registered tonnage.	No.	Registered tonnage.
Home, on lake.....	84	4,617	73	3,327	7	1,284
Foreign, on lake.....			14	656		
Total.....	84	4,617	87	3,983	7	1,284
American owned.....	84	4,617	82	3,750	7	1,284
Canadian owned.....			5	233		

Sum total, 178 arrivals; registered tonnage, 9,884.

DEPARTURES.

Trade engaged in.	Steamers.		Sailing vessels.		Barges.	
	No.	Registered tonnage.	No.	Registered tonnage.	No.	Registered tonnage.
Home, on lake.....	80	4,317	76	3,428	6	1,124
Foreign, on lake.....	4	300	10	497		
Total.....	84	4,617	86	3,925	6	1,124

Sum total, 176 departures; registered tonnage, 9,666.

Greatest draft of vessels, 11 feet.

Greatest tonnage, 918 reg. tons.

Greatest load, 600 tons net.

Receipts and shipments by lake at Sacketts Harbor, New York, during fiscal year ending June 30, 1892.

Trade.	Receipts.						Shipments.			
	Passengers.	Grain.	Merchandise.	Hay.	Coal.	Lumber.	Passengers.	Merchandise.	Coal.	Lumber.
	No.	Net tons.	Net tons.	Net tons.	Net tons.	Tons.	No.	Net tons.	Net tons.	Tons.
Home on lake.....	2,057		1,853	613	4,060	168	2,326	361	40	16
Foreign on lake.....		13	2			930		77		
Total.....	2,057	13	1,855	613	4,060	1,098	2,326	438	40	16

Navigation closed December 7, 1891. Navigation opened April 12, 1892.

For the purpose of reduction to tons weight the following is assumed: Barrels oil, 7 equal 1 ton; M lumber, 1 equal 1½ tons; crates fruit, 20 equal 1 ton; railroad ties and poles, 12 equal 1 ton; cords, wood and posts, 1 equal 1½ tons; live animals, 4 equal 1 ton.

APPENDIX P P.

IMPROVEMENT OF SHOALS IN ST. LAWRENCE RIVER, AND OGDENSBURG HARBOR, NEW YORK, AND OF RIVERS AND HARBORS ON LAKE CHAMPLAIN, NEW YORK AND VERMONT.

REPORT OF MAJOR M. B. ADAMS, CORPS OF ENGINEERS, OFFICER IN CHARGE, FOR THE FISCAL YEAR ENDING JUNE 30, 1892, WITH OTHER DOCUMENTS RELATING TO THE WORKS.

IMPROVEMENTS.

- | | |
|--|--|
| Shoals between Sister Islands and Crossover Light, St. Lawrence River, New York. | 6. Plattsburg Harbor, New York. |
| Ogdensburg Harbor, New York. | 7. Burlington Harbor, Vermont. |
| Breakwater at Rouse Point, Lake Champlain, New York. | 8. Otter Creek, Vermont. |
| Great Chazey River, New York. | 9. Ticonderoga River, New York. |
| Breakwater at Gordon Landing, Lake Champlain, Vermont. | 10. Narrows of Lake Champlain, New York and Vermont. |

UNITED STATES ENGINEER OFFICE,
Burlington, Vt., July 9, 1892.

GENERAL: I have the honor to transmit herewith annual reports for the river and harbor works under my charge during the fiscal year ending June 30, 1892.

Very respectfully, your obedient servant,

M. B. ADAMS,
Major of Engineers.

Brig. Gen. THOMAS L. CASEY,
Chief of Engineers, U. S. A.

P P 1.

IMPROVEMENT OF SHOALS BETWEEN SISTER ISLANDS AND CROSSOVER LIGHT, ST. LAWRENCE RIVER, NEW YORK.

The project for this improvement resulted from the survey and report made in compliance with a provision in the river and harbor act of August 11, 1888, directing a survey of the shoals between Sister Islands and Crossover Light to be made; it was submitted July 2, 1889, and is in Report of the Chief of Engineers, 1889, pages 2463 and

Aggregate

The act of September 19, 1890, appropriated \$5,000
ment. Bids were invited November 8, 1890, and open
1890, but, owing to informalities in two of the three
work was readvertised, and bids were again opened
the contract being then awarded to William James
removal of the lower and upper shoals at \$16 per cubic
in place.

Work was to have been done during the winter of
ing to ice not forming of sufficient thickness to bear
machinery, etc., to be used in connection with the o
in the date for the completion of the contract was
granted so as to include the winter season of 1891-2.

It is recommended that this improvement be defini
rence River above Ogdensburg in making further app
as bad shoals exist without the limits named in the
appropriations heretofore.

It is expected that work will soon be commenced a
completed on time, August 31, 1892.

Money statement.

July 1, 1891, balance unexpended.....

July 1, 1892, balance unexpended

July 1, 1892, amount covered by uncompleted contracts

July 1, 1892, balance available.....

Amount appropriated by act approved July 13, 1892

Amount available for fiscal year ending June 30, 1893

{ Amount (estimated) required for completion of existing proj
Amount that can be profitably expended in fiscal year ending Jun
Sumbitted in compliance with requirements of sections 2 of
harbor acts of 1866 and 1867.

Principal items of merchandise carried by this tonnage were:

	1890.	1891.
	<i>Tons.</i>	<i>Tons.</i>
Wool, and corn	21,750	44,600
.....	202,384	22,562
.....	112,182	135,000
Freight	35,000	53,000
.....		7,000
Total	371,316	260,162

P P 2.

IMPROVEMENT OF OGDENSBURG HARBOR, NEW YORK.

When operations were commenced at this harbor the channel afforded a depth of 5 to 12 feet only, and now there are three channels from deep in the St. Lawrence River to the nearest docks or wharves, in which a depth of water from 15 to 16 feet deep is afforded, and a channel 12 to 15 feet deep has been made along the city front.

The original project formed for the improvement by a board of engineers in 1868 provided for dredging the channel of the Oswegatchie below the bridge, deepening the channel along the city front on the St. Lawrence River and across the bar northeast of the light-house, and the construction of a pile pier to prevent the water of the Oswegatchie spreading over the bar or shoal between these channels. The pier was only recommended in the event of the water of the Oswegatchie not following the line of the deepened channels after dredging was completed. The dredging provided in the project was completed in 1866, and the piling proved unnecessary; consequently operations were confined to dredging the channels, which were left in good con-

dition. The harbor was surveyed in 1880 and showed considerable shoaling of the channels during the four years of inactivity between 1876 and 1880, and that it was due to sawdust and other waste products of saw-mills which had been thrown into the Oswegatchie River in violation of the local regulations forbidding it; that the shoaling amounted to some 100,000 yards and would cost \$12,000 for removal. The practice of throwing these waste products into the river has now ceased.

The original project was estimated to cost \$175,000; \$107,000 had been expended in 1880, leaving \$68,000 still due for the improvement, the cost of the piling not being required.

In 1882 the harbor was recommended to be placed in condition to receive the largest vessels that would be able to pass through the entrance of the Welland Canal, at an estimated cost of \$76,000, as follows:

Bar, 80,000 yards, at 30 cents per yard	\$24,000
Watertown and Ogdensburg Railroad Wharves, 40,000 yards, at 40 cents per yard	16,000
Oswegatchie mouth, 20,000 yards, at 20 cents per yard	4,000
Front channel, 160,000 yards, at 20 cents per yard	32,000
Total, 300,000 yards	76,000

The above estimate was intended to provide for a depth of 15 feet over the bar and 16 feet over the bar. With the completion of the

last contract 237,677 cubic yards had been dredged, at a cost of \$51,000, leaving 62,323 cubic yards to be dredged, and \$25,000 due the improvement under the project of 1882.

In 1889 a new project was formed and approved for the improvement of this harbor, it being found that the scheme of 1882 would be insufficient for the wants of the harbor when fully carried out.

The last project provides depths of 16.5 feet in all the channels, based on 17-foot depths of dredging, at an estimated cost of \$158,950, as follows:

290,000 yards, exclusive of hardpan and solid rock, at 25 cents per yard	\$72,500
72,000 yards of hardpan and solid rock, mostly hardpan, at \$1 per yard	72,000
	144,500
Add 10 per cent for contingencies	14,450
Total	158,950

The act of September 19, 1890, appropriated \$42,000 for this improvement. Bids were duly invited and a contract has been made for the removal of 325,000 cubic yards of sand, mud, gravel, etc., at the very favorable price of 11 cents per cubic yard, which has since undergone modification by allowing 22 cents per yard for 50,000 yards of hardpan without changing the total amount to be paid under the contract.

The survey made at the harbor in November, 1890, shows a more favorable condition of things at the mouth of the Oswegatchie River than any other survey has shown. It is found that there will be no solid rock encountered within the limits of the channel, but that all the material to be removed from that locality will be hardpan, with more or less of bowlders embedded therein.

Operations under the last contract have been commenced with two dredges. One hundred and eight thousand four hundred and fifty-five yards of ordinary material and 9,286 yards of hardpan have been removed to date, and it is expected that the contract will be completed on time, November 30, 1892.

It is expected to apply additional funds as appropriated in the completion of the project of 1889, under the contract system.

The accompanying commercial statistics were obtained through the kindness of the collector of customs.

Money statement.

July 1, 1891, balance unexpended	\$41,130.80
June 30, 1892, amount expended during fiscal year	13,574.57
July 1, 1892, balance unexpended	27,556.23
July 1, 1892, outstanding liabilities	\$2,569.68
July 1, 1892, amount covered by uncompleted contracts	21,777.03
	24,346.71
July 1, 1892, balance available	3,209.52
Amount appropriated by act approved July 13, 1892	40,000.00
Amount available for fiscal year ending June 30, 1893	43,209.52
{ Amount (estimated) required for completion of existing project	76,950.00
{ Amount that can be profitably expended in fiscal year ending June 30, 1894	50,000.00
{ Submitted in compliance with requirements of sections 2 of river and harbor acts of 1866 and 1867	

COMMERCIAL STATISTICS.

harbor, Ogdensburg, N. Y.; collection district, Oswegatchie; nearest
 , Ogdensburg, N. Y.

e of vessels entered and cleared, foreign ports and coastwise, 1888 to 1891.

Year.	Foreign ports.				Coastwise.	
	American vessels.		Foreign vessels.		Entered.	Cleared.
	Entered.	Cleared.	Entered.	Cleared.		
	<i>Tons.</i>	<i>Tons.</i>	<i>Tons.</i>	<i>Tons.</i>	<i>Tons.</i>	<i>Tons.</i>
.....	12,068	10,503	66,769	62,918	180,214	191,790
.....	18,689	8,472	85,064	80,972	227,819	230,317
.....	12,888	10,868	76,517	66,699	276,888	287,634
.....	23,475	17,411	48,955	45,833	824,010	825,494

Merchandise received and shipped, foreign ports.

Articles.	1888.	1889.	1890.	1891.
	<i>Tons.</i>	<i>Tons.</i>	<i>Tons.</i>	<i>Tons.</i>
.....	76,566	75,815	117,114	127,152
.....	13,092	3,676	5,278	17,521
.....	676	625	164	9
.....	15,000	25,000	15,000	11,051
.....	32,053	65,356	100,192	49,962
.....	2,685	4,750	2,725	1,224
.....	4,000	3,440	2,200	1,000
.....	29,000	59,000	31,109	1,000
.....	96,506	161,847	156,668	208,918

Merchandise received and shipped, coastwise.

Articles.	1888.	1889.	1890.	1891.
	<i>Tons.</i>	<i>Tons.</i>	<i>Tons.</i>	<i>Tons.</i>
.....	25,812	47,454	38,142	58,084
.....	145,345	181,101	227,409	221,678
.....	22,359	12,436	16,791	14,152
.....	76,991	66,230	112,181	138,638
.....	25,000	49,000	31,119	1,878
.....	2,756	2,310	700	500
.....	34,807	47,875	47,271	56,000
.....	332,870	406,406	473,613	490,930

	1888.	1889.	1890.	1891.
collected.....	\$225,570	\$219,481	\$224,078	\$223,454
lected.....	2,946	4,490	3,895	2,772
orts.....	1,957,440	1,628,674	1,618,616	1,729,389
orts.....	1,689,675	1,970,974	2,105,214	1,764,831

	1890.	1891.
chandise entered for warehouse and transhipped in bond ..	\$174,558.00	\$72,251.00
chandise entered for transshipment and exports.....	38,357.00	417,813.00
nties on the above.....	74,641.12	176,376.30

Merchandise received under consular seal from foreign ports.

Articles.	1890.		1891.	
	Packages.	Value.	Packages.	Value.
Tea.....	129,669	\$1,358,244	54,550	\$587,750
Curios.....	203	9,288	758	1,000
Raw silk.....	6,957	3,941,752	5,089	1,100,000
Silk goods.....	81	46,646	108	1,000
Straw braid.....	1,119	25,165	524	1,000
Chinese groceries.....	3,027	27,202	2,028	1,000
Firecrackers.....	8,659	34,426	127	1,000
Copper matte..... pounds..	2,419,112	173,428	15,287,364	1,000
Skins.....			24	1,000
Total		5,616,151		4,688,750

The greatest draft of vessels at Ogdensburg Harbor is 16 feet.

P P 3.

BREAKWATER AT ROUSE POINT, LAKE CHAMPLAIN, NEW YORK

The project for this improvement was adopted in 1885 and contemplates the construction of a straight breakwater of rubble and stones, extending from Stony Point in the general direction of the southern point of the 6-foot curve south of Windmill Point until the 18-foot curve is reached, a total distance of about 2,000 feet.

Operations have been carried on in accordance with the original plan and under contracts dated August 22, 1885, October 28, November 2, 1888, and December 6, 1890. Under the first contract 550 feet of breakwater adjoining the shore were built, which was later extended 140 feet, respectively, under the second and third contracts, and was further extended 210 feet under the fourth and fifth contracts. It is found that another extension 125 feet will carry the structure into 18 feet of water, and it is therefore proposed to terminate the breakwater with another extension of 135 feet, or a total length of 1,835 feet, but with a total length of 1,835 feet instead of 2,000 feet.

The estimated cost of the breakwater was originally placed at \$110,000. There have been four appropriations to date, amounting to \$83,500, the last being that of September 19, 1890; and as it is now estimated that \$15,000 more will be sufficient to carry the structure out to the 18-foot curve and to close in the outer end, the total cost will be \$98,500. The improvement is completed and closed at 1,835 feet in length. The good effects of the improvement are apparent along the town front by the comparative calmness of the water at the docks there during southeasterly winds which formerly caused considerable commotion.

Money statement.

July 1, 1891, balance unexpended.....	\$7
June 30, 1892, amount expended during fiscal year.....	7
July 1, 1892, balance unexpended.....	15
Amount appropriated by act approved July 13, 1892.....	15
Amount available for fiscal year ending June 30, 1893.....	15

COMMERCIAL STATISTICS.

of harbor, Rouse Point, N. Y.; collection district, Champlain; nearest light-Vindmill Point.

Vessels entered and cleared and duties collected.

Year.	Entered.		Cleared.		Duties collected.
	No.	Tons.	No.	Tons.	
.....	1,082	108,517	1,136	100,816	\$318,606.77
.....	1,129	112,635	1,170	113,843	340,670.40
.....	963	83,015	940	92,490	361,987.78
.....	977	92,996	863	81,586	323,462.00

	1890.	1891.
lected		
essed on merchandise entered for warehouse and transporta- other ports	\$361,987.78	\$323,462.00
tax collected	80,815.25	151,225.00
ected	2,137.20	2,142.00
eous collections	8,011.95	4,803.00
	17.84	4,170.00
al collected and assessed	461,970.02	485,802.00

Number of tons shipped and arrived, Rouse Point, New York.

Articles.	1890.		1891.	
	Shipped.	Arrived.	Shipped.	Arrived.
	Tons.	Tons.	Tons.	Tons.
als, plank, etc	30,000	56,000	218,750
iber	4,000	8,000	136,000
eous articles	52,000	280,000
	3,800	44,338	11,500	60,000

est draft of vessels, 10 feet. The accompanying commercial statistics were
d through the kindness of the collector of customs.

P P 4.

IMPROVEMENT OF GREAT CHAZY RIVER, NEW YORK.

project for this improvement was adopted in 1889, after the sur-
s made in accordance with the river and harbor act of August
88.

first estimate for improvement provided for a channel with a least
of 50 feet and a least depth of 6 feet; but, owing to the cost un-
is scheme, amounting to \$34,000, a second plan was considered,
provides for a least depth of 5 feet and a least width of 40 feet;
a finding that the cost under this plan would be about one-half
mer estimate, work was recommended to be done in accordance
ith, and the project was adopted.

appropriation of \$10,000 by act of September 19, 1890, has been
and (under contract dated February 16, 1891) in dredging

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26,337 $\frac{9}{11}$ yards of bowlders, sand, mud, and clay, in a single cut, the mouth of the river to Champlain village.

Operations were commenced June 22, 1891, and were completed October 5, 1891, consuming the available funds.

The channel produced by the work as above is only a little more than half that provided under the project for the improvement, and may, of course, be very generally widened in order to complete it.

Money statement.

July 1, 1891, balance unexpended	\$3,000
June 30, 1892, amount expended during fiscal year	9,000
Amount appropriated by act approved July 13, 1892	5,000
{ Amount (estimated) required for completion of existing project	3,000
{ Amount that can be profitably expended in fiscal year ending June 30, 1894	3,000
{ Submitted in compliance with requirements sections 2 of river and harbor acts of 1866 and 1867.	

COMMERCIAL STATISTICS.

Name, Great Chazy River, New York; collection district, Champlain; nearest house, Isle La Motte; greatest draft, 6 feet.

Vessels arrived, departed, etc., 1889, 1890, and 1891.

	1889.		1890.		1891.
	No.	Tons.	No.	Tons.	
Steam vessels arrived.....	38	1,520	36	1,210	26
Canal boats arrived.....	56	5,432	44	4,562	10
Steam vessels departed.....	38	1,520	36	1,210	26
Canal boats departed.....	53	5,136	44	4,562	17
Cargoes:					
Value of arrivals.....		\$154,500		\$155,000	
Value of departures.....		83,500		81,000	

Number of tons arrived and shipped, 1890 and 1891.

Articles.	1890.		1891.
	Shipped.	Arrived.	Shipped.
Potatoes.....	Tons. 1,000	Tons. 1,000	Tons. 1,000
Hay.....	6,000		800
Iron.....	100	300	
General merchandise.....	611	1,652	500
Coal.....		5,000	

BREAKWATER AT GORDON LANDING, LAKE CHAMPLAIN, VERMONT.

The project for this improvement was adopted in 1887 and has for its object the construction of a breakwater, composed of rubble and

nes, extending in a straight line from a point 250 feet south of the dock or landing, where the water is about 3 feet deep at lowest stage, to a point on the 16-foot curve and a little north of the line drawn from the dock to Cumberland Head.

At inception the object of the undertaking seems to have been the ordering of increased shelter on the west shore of Grand Isle, Lake Champlain, which incidentally has involved the protection of the dock landing which gives the improvement its name.

An appropriation of \$18,750 was made for this work in the act of August 5, 1886; \$10,000 was appropriated by act of August 11, 1888, and \$6,000 by act of September 19, 1890, for completion.

The original project was modified so as to make the total length of the breakwater 675 instead of 800 feet.

Operations were carried on by contract. The first contract, dated August 1, 1887, comprised the construction of 500 linear feet of breakwater next the shore. Work was commenced August 11, 1887, and was completed August 31, 1889. The second contract was dated December 1888, and comprised an extension to the above shore section, and when completed, September 30, 1889, left 675 linear feet of breakwater completed, with the exception of the large facing stones and core of rubble on 135 feet at its outer end. For completion of the improvement according to the modified project \$6,000 was asked for and appropriated by act of September 19, 1890, which has been consumed under contract dated December 26, 1890, with Edwin H. French, of Fulton, N. Y., for the extension and completion of the breakwater at Gordon Landing, Vermont.

Operations under the last contract were commenced July 6, 1891, and closed September 19, 1891, completing the improvement.

Money statement.

July 1, 1891, balance unexpended.....	\$5,956.05
June 30, 1892, amount expended during fiscal year	5,956.05

COMMERCIAL STATISTICS.

Name, Gordon Landing; collection district, Vermont; nearest light-house, Cumberland Head. The proprietor of the landing states that three-fourths of the business of Grand Isle, Vt., is done there, and that 1,500 tons of freight are probably loaded each season; also, that large quantities of produce and fruit, as well as stock of the market, pass over the dock at the landing. Greatest draft of vessels, 8.5 feet.

P P 6.

IMPROVEMENT OF PLATTSBURG HARBOR, NEW YORK.

The original project for the improvement of this harbor was adopted probably in 1836, the date of the first appropriation, and proposed the construction of a breakwater about 1,000 feet east of the steamboat

There were 1,250 linear feet of breakwater constructed between the years 1836 and 1875, which is now being extended 300 feet farther northward.

The modification of 1870 provided for an extension of the former structure southeastward, the dredging of some shoal areas within the breakwater, and the protection of a portion of the adjacent beach by revetment.

Operations were confined to necessary repairs and the dredging of limited areas near the steamboat docks after the completion of the project of 1870, until the act of September 19, 1890, made provision for northward extension to the breakwater, 300 feet, and for very general repairs to the superstructure.

The appropriation of September, 1890, amounted to \$32,500, which has been pledged under one contract, covering the renewal of the timber superstructure by one composed of large facing stones, with rubblestone core, and an extension of 300 feet to the breakwater finished on top in the same manner as has just been described. The cross-section of this superstructure is much like that shown in Fig. of the sheet of cross-sections herewith, but the inner and outer slopes were made equal and the timber of the old substructure was cut down only to low-water level, since the line of no decay is a foot or more above low water.

Work is progressing fairly well under the above contract, the extension has been completed, and the work of rebuilding the old superstructure is about half done. The contract does not require completion until November, 1892, and it is confidently expected will be finished on time. The report of a survey at the mouth of Saranac River, Plattsburg, N. Y., that appeared as House Ex. Doc. No. 72, Forty-eighth Congress, second session, comprehending the dredging of 110,000 yards there, would amount to a considerable extension of the limited amount of dredging operations, and therefore may be said to constitute a further modification in the project for this improvement. It is thought that only small amounts of dredging will be required at this harbor for several years, unless the plan for improving the mouth of Saranac River should be undertaken. A more recent report as to this improvement has appeared as House Ex. Doc. No. 22, Fifty-first Congress, second session.

Money statement.

July 1, 1891, balance unexpended.....	\$31.12
June 30, 1892, amount expended during fiscal year	15.84
July 1, 1892, balance unexpended.....	15.27
July 1, 1892, outstanding liabilities.....	\$4,052.68
July 1, 1892, amount covered by uncompleted contracts.....	8,991.32
	13.04
July 1, 1892, balance available.....	2.23

COMMERCIAL STATISTICS.

Name of harbor, Plattsburg, N. Y.; collection district, Champlain; nearest light house, Cumberland Head; two beacon lights on the Plattsburg breakwater.

Arrivals of vessels.

	1888.		1889.		1890.		1891.	
	No.	Tons.	No.	Tons.	No.	Tons.	No.	Tons.
.....	838	386,867	861	389,150	1,004	559,531	998	558,427
.....	44	3,715	41	3,530	46	4,278	49	4,427
.....	800	71,938	820	73,829	908	77,900	936	80,835
Total	1,682	462,520	1,722	466,509	1,956	641,708	1,983	643,689

Departures of vessels.

	1888.		1889.		1890.		1891.	
	No.	Tons.	No.	Tons.	No.	Tons.	No.	Tons.
.....	838	386,867	861	389,150	1,004	559,531	998	558,427
.....	44	3,715	43	3,654	47	4,376	49	4,427
.....	800	71,938	820	73,829	908	78,180	936	80,835
Total	1,682	462,520	1,724	466,633	1,956	642,086	1,983	643,689

Value of goods shipped and arrived.

	1888.	1889.	1890.	1891.
.....	\$296,905	\$435,000	\$441,200	\$437,316
.....	60,586	109,000	113,460	117,381

Vessels enrolled at Plattsburg, N. Y., December 31, 1890 and 1891.

1890.		1891.	
.....	587	609
.....	56,250	58,550
.....	53,404	55,610


Value of tons arrived and shipped, Plattsburg Harbor, New York, 1890 and 1891.

Articles.	1890.		1891.	
	Shipped.	Arrived.	Shipped.	Arrived.
	Tons.	Tons.	Tons.	Tons.
.....	13,500	17,350
.....	1,200	1,200
.....	600	16,000	17,000
.....	600	1,275
Merchandise	400	257	2,000	1,500
.....	2,500

..... draft of vessels, 10 feet.
 above commercial statistics were kindly furnished by the collector of customs.

IMPROVEMENT OF BURLINGTON HARBOR, VERMONT.

The first project for the improvement of this harbor was probably completed in 1836. Modifications of the original project have been made
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want of rectilinearity from end to end, is found in its development by successive extensions under different administrations in distant periods of time, when the immediate and present conditions of the harbor necessarily presented very different phases. It is proposed to gradually withdraw the ends of the breakwater, from water 36 and 39 feet deep into water about 40 feet deep, then to keep at a distance of about 1,000 feet from the existing structure, so as to reduce the cost.

The establishment of a harbor line, to limit the extension of wharves out into the harbor, may become necessary in view of the injurious restrictions of the harbor capacity, especially if the extension of the breakwater being only 1,000 feet distant. The 360 feet northward extension to the breakwater, with a timber structure, was the last new work at this harbor. The present structure meets all expectations thus far, and appears to be a strong and durable breakwater; the cost is practically the same as for a timber water with timber superstructure, and since no timber is used in the water, the life of this new structure, as far as one can judge, is said to be illimitable.

In further explanation of the above form of construction, the accompanying sheet of cross sections of breakwater, Champlain, New York and Vermont, is appended.

Regarding the respective merits of these several forms of breakwater depends on the circumstances of construction, the cost of the breakwater, the locality, depth of water, force of the wave action, and the nature of the water; nevertheless, the following remarks are perhaps the experience had in this vicinity.

When the water is 16 feet deep or less, and if the breakwater is strong, the form as in Fig. 4 is probably as good as any other. When the water is deeper this form is too expensive, owing to its great amount of rubble stone foundation required. The

t no great increase of cost. It will be observed that the lower f stone are held in place by the top timbers of the cribs, and stone in the outer exposed face must be lifted bodily as well out from among the adjoining stones in order that the waves e an impression on this structure; and since none of the stones n thus displaced, though subjected to repeated storms, and known cost of construction, it is reasonable to say that it is 7 durable and cheap.

ropriation of September 19, 1890, \$20,000, being for repairs, be expended as repairs become absolutely necessary, and will, , probably be sufficient for several years. Some 1,625 cubic stone for filling and riprap to the breakwater, and 55,331 feet, timber, were purchased under contract, and repairs were made inear feet of the superstructure by hired labor during the year. work is regarded as necessary at present, and no additional asked for this harbor.

ompanying commercial statistics were supplied by the collector as in response to the usual request for them.

Money statement.

1, balance unexpended.....	\$19,531.62
92, amount expended during fiscal year.....	3,771.73
2, balance unexpended.....	15,759.89
(estimated) required for completion of existing project.....	129,000.00
nd in compliance with requirements of sections 2 of river and acts of 1866 and 1867.	

COMMERCIAL STATISTICS.

harbor, Burlington, Vt.; nearest light-house, Juniper Island; collection rmont.

Arrivals and departures of vessels in foreign trade.

	1888.		1889.		1890.		1891.	
	No.	Tons.	No.	Tons.	No.	Tons.	No.	Tons.
.....	219	28,462	196	25,524	180	23,304	116	15,568
.....	219	28,462	196	25,524	180	33,304	106	14,324

Arrivals and departures of vessels in coastwise trade.

	1890.		1891.	
	No.	Tons.	No.	Tons.
.....	1,449	589,505	2,057	581,843
.....	1,440	587,705	2,072	580,826

Merchandise shipped.

Articles.	1890.		1891.	
	Tons.	Tons.	Tons.	Tons.
.....	4,253	\$66,040	5,228	\$81,180
handise.....	33,000	116,000	38,000	140,000
.....	37,253	182,040	43,228	221,180

Merchandise received.

Articles.	1890.		1891.	
	Tons.		Tons.	
Lumber	33,376	\$409,505	28,658	\$22
Lath.....	1,258	6,411	1,258	
Shingles.....	620	5,662	582	
Railroad ties.....			555	
Coal.....	95,000	380,000	109,800	
General merchandise.....	75,000	600,000	78,460	
Total.....	205,254	1,401,578	219,121	1.3

Greatest draft of vessels, 10 feet.

P P 8.

IMPROVEMENT OF OTTER CREEK, VERMONT.

The project for this improvement was adopted in 1872, and, as filed in 1882 and 1884, proposes the formation of a channel from Genesee, Vt., to Lake Champlain, of a navigable width and a least draft of 8 feet. (See Reports of Chief of Engineers, 1872, page 273; page 712, and 1884, page 2159.)

The appropriation of September 19, 1890, has been consumed, a contract dated January 12, 1891, with Willard Johnson, of Fultz, Y., in dredging 20,995 cubic yards from such shoals as formed the obstructions. Operations were commenced October 14, and were completed December 4, 1891.

The improvement was originally estimated to cost \$58,146. There have been \$41,500 appropriated to date, leaving \$16,646 still due for improvement under the existing project.

It is believed that \$10,000 (to be applied to rock excavation) would be sufficient to carry out the original scheme of improvement; but it is feared that the improvement is not going to prove very permanent when fully completed. It has been found necessary to repeat operations at Bull Brook Bend, after leaving that part of the stream in excellent condition, and there is nothing to give assurance that accretions may not occur on the shoal there again or elsewhere during the course of freshets.

The accompanying commercial statistics were kindly furnished by the collector of customs, district of Vermont.

Money statement.

July 1, 1891, balance unexpended	\$1.
June 30, 1892, amount expended during fiscal year	1.
Amount appropriated by act approved July 13, 1892.....	10,

COMMERCIAL STATISTICS.

Name, Otter Creek, Vermont; nearest light-house, Split Rock; collection district, Vermont; greatest draft of vessels, 7.5 feet.

Arrivals and departures, 1887, 1890, and 1891.

Kind of craft.	1887.		1890.		1891.	
	Arrived.	Cleared.	Arrived.	Cleared.	Arrived.	Cleared.
.....	112	112	33	33	40	40
.....	168	168	30	30	4	4
.....	294	294	185	185	213	207

tons of merchandise, in 1879, arrived, 6,350; cleared, 500; 1890, arrived, 3,361; cleared, 370; 1891, arrived, 4,449; cleared, 1,812.

P P 9.

IMPROVEMENT OF TICONDEROGA RIVER, NEW YORK.

project for this improvement was adopted in 1881, its object being the formation of a channel of navigable width and a least depth of 12 feet at low water between the falls of Ticonderoga and Lake Champlain, a distance of about 2 miles. (See Report of Chief of Engineers, 1881, p. 26.)

The improvement was estimated to cost \$42,516, of which amount \$10,000 have been appropriated and \$26,016 are still due the improvement.

The appropriation of September 19, 1890, has been expended, under contract dated January 12, 1891, with Willard Johnson, of Fulton, N. Y., for dredging 8,132 cubic yards from shoal places near the mouth of the river. Operations commenced May 7 and were completed May 21,

in order to carry out the scheme of improvement the existing channel requires very general widening and deepening, and when this has been accomplished the permanency of the improvement is regarded as entirely questionable. At the rate at which work has been carried on, it is more than keeping pace with the annual accretions has been accomplished; we may gain a little on the channel in one part only to lose it in another part, and thus alternating and remitting of efforts, with the aid of the small appropriations, no permanent gain has been effected. In the prosecution of this work, if it is deemed best to prosecute it, larger appropriations seem to be absolutely necessary for the purpose of generally benefiting the channel.

Money statement.

1891, balance unexpended	\$1,977.75
1892, amount expended during fiscal year.....	1,977.75
Amount (estimated) required for completion of existing project.....	26,016.00
Amount appropriated in compliance with requirements of sections 2 of river and harbor acts of 1866 and 1867.	

COMMERCIAL STATISTICS.

Commercial statistics for 1890 were obtainable, and those for 1881, 1886, and 1891 are also obtainable. The amount received: 1881, 38,000 tons; 1886, 55,000 tons; 1891, 63,000 tons, and 34,000 tons in 1892.



APPENDIX Q Q

IMPROVEMENT OF OAKLAND HARBOR, CALIFORNIA.

REPORT OF COLONEL G. H. MENDELL, CORPS OF ENGINEERS, OFFICER
IN CHARGE, FOR THE FISCAL YEAR ENDING JUNE 30, 1892.

UNITED STATES ENGINEER OFFICE,
San Francisco, Cal., July 6, 1892.

GENERAL: I have the honor to transmit herewith annual report
upon the work of improving harbor at Oakland, Cal., etc., for the year
ending June 30, 1892.

Very respectfully, your obedient servant,

G. H. MENDELL,
Colonel, Corps of Engineers.

Brig. Gen. THOMAS L. CASEY,
Chief of Engineers, U. S. A.

IMPROVEMENT OF OAKLAND HARBOR, CALIFORNIA.

Operations of the year.—At the beginning of the year five contracts
were in force, namely: 1. For delivery and placing stone upon the
north jetty. 2. For construction of piers of a highway bridge to span
the tidal canal at Park street, Alameda. 3. For a steel highway bridge
and draw span at Park street, Alameda. 4. For dredging tidal basin.
5. For dredging the eastern end of the tidal canal.

The first of these contracts, involving 13,367 tons of stone delivered
and 61,299 square feet of dry masonry, was fulfilled on October 12, 1891,
5,000 tons of stone having been delivered and 31,528 square feet of ma-
sonry having been laid during the year.

The contract for bridge piers was fulfilled on August 20, 1891. The
quantity of work done during the year was 600 cubic yards of excava-
tion and 2,351.8 cubic yards of concrete.

The contract for the highway bridge was practically completed on December 7, 1891, several small items being placed a few days later. The amount of metal in the bridge is 195.2 tons.

Work was begun on the contract to dredge the tidal basin July 1, 1891, and the contract was fulfilled on January 13, 1892. The quantity dredged and securely placed ashore behind levees is 632,905 cubic yards. The distance over which the spoil was transported in pipes varied between 3,000 and 6,000 feet.

At the beginning of the year work had been done under the contract for excavation of the eastern end of the tidal canal to the extent of 250,820 cubic yards dredged and securely placed ashore. No work was done in continuation of this contract until January 26, 1892. The work done during the year, as measured, is 142,088 cubic yards. The agreed date for fulfillment of the contract is June 30, 1892, at which date there remained to be dredged an estimated quantity of 107,000 cubic yards.

Contracts made during the year.—A contract was made on September 28, with the San Francisco Bridge Company, for enlarging the portion of the channel in front of the city of Oakland to a width of 325 feet and depth of 18 feet or less, at low water. The work is to be begun on July 18, 1892. The estimate of work to be done is 125,000 cubic yards. The date set for completion is May 18, 1893.

Present condition of the work.—A survey just made shows that the depths in the channel have been fully maintained. They are practically identical with those found at the close of the last fiscal year. The channel from the Bay of San Francisco to the wharves of Oakland is for almost all the distance 300 feet in width, with a low-water depth of 14 feet, to which the tide adds an average additional of 4 feet, making 18 or more feet available at high tide.

In front of the wharves of Oakland the width of the channel is reduced to 225 feet.

The north jetty is practically completed, with length of 9,203 feet. The terminal mound is as yet, however, in the rough, and is to be laid up in dry masonry.

The length of the south jetty as it stands, is 11,868 feet, of which 2,465 is in an unfinished condition, and the contemplated extension is about 700 feet, making the projected length 12,680 feet.

The excavation of the tidal basin is completed. The amount of excavation made in the tidal canal is 1,002,445 cubic yards. The quantity estimated to complete the canal is 1,503,832 cubic yards.

The work yet required to carry out the project is mainly embraced in the following items, namely: Completion of the canal; bridge at High street, Alameda; extension and completion of the south jetty; dam at mouth of San Leandro Estuary, and dredging interior channels.

Future operations.—It is expected to apply the next appropriation to further excavation of the tidal canal and protection of its banks: to completion of the south jetty, and to dredging channels, both in the lower and upper parts of the harbor.

Appropriations for improving harbor at Oakland, Cal.

June 23, 1874	\$100,000	August 2, 1882	\$200,000
March 3, 1875	100,000	July 5, 1884	139,600
August 4, 1876	75,000	August 5, 1885	60,000
June 18, 1878	80,000	August 11, 1888	350,000
March 3, 1879	60,000	September 19, 1890	250,000
June 14, 1880	60,000		
March 3, 1881	60,000	Total	1,534,600

COMMERCIAL STATISTICS.

Following tables, compiled by Mr. Le Conte, show the general traffic that passed through the jetties each year since the beginning of the present century:

Year.	Traffic by steam ferries.				Traffic by vessels.		
	No.	Trips.	Passengers.	Freight.	No.	Register.	Freight.
1861	1	600	None	60,000	1,415	70,750	94,300
1862	3	5,400	210,240	129,000	1,085	109,125	211,627
1863	3	8,600	858,352	1,051,788	1,129	129,714	178,448
1864	3	9,400	892,210	1,150,379	1,004	144,004	257,614
1865	3	8,000	974,901	1,142,918	1,031	143,886	215,829
1866	3	8,000	1,553,769	1,202,230	1,156	163,553	255,738
1867	3	6,000	444,142	1,439,134	1,326	200,226	305,437
1868	3	6,000	318,402	1,487,924	1,673	188,974	284,050
1869	3	8,520	210,423	1,654,451	1,224	130,913	231,660
1870	3	8,520	200,000	1,876,635	1,384	162,957	295,932
1871	3	8,600	100,000	1,755,247	1,745	185,358	338,123
1872	3	8,600	100,000	1,925,957	2,153	213,971	393,478
1873	3	8,600	100,000	2,259,086	2,310	232,706	437,923

The report shows a growth in traffic in twelve months of 16 1/2 per cent, equal to 378,573 tons.

The freight carried by ferry steamers, 2,259,086 tons, represents miscellaneous merchandise, and includes most of the rail traffic of San Francisco.

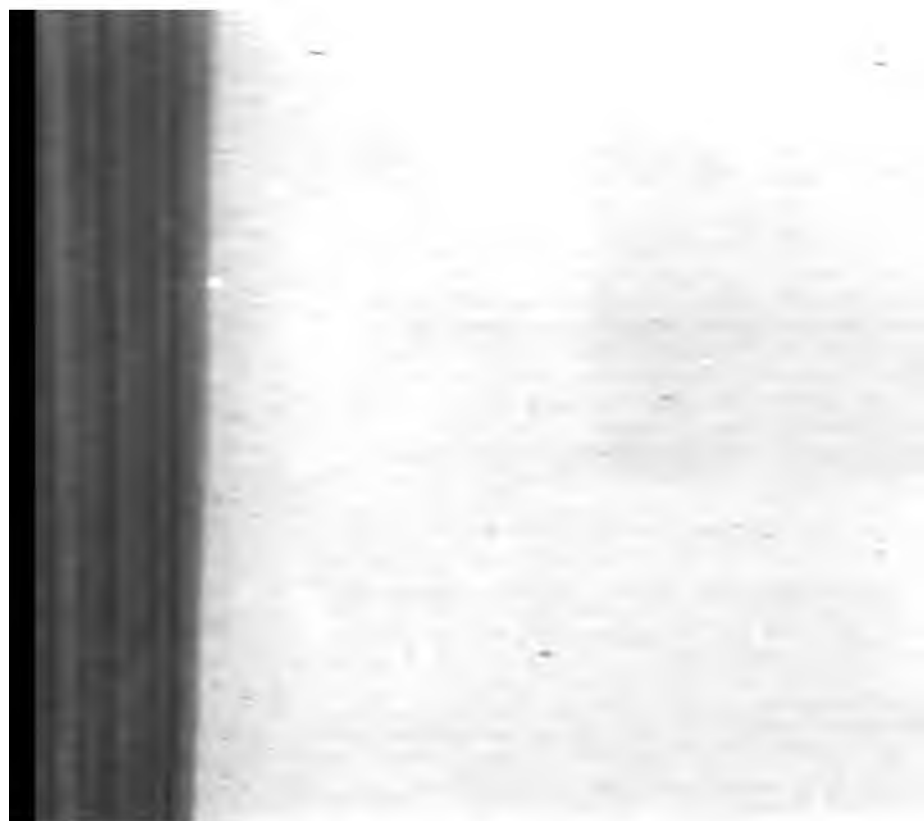
The remaining traffic is carried on by vessels with registered tonnage between 200 and 1,265 tons, drawing 14 to 18 feet, and by fleet, variable in number, drawing 10 feet or less.

The classification of freight carried otherwise than on ferry steamers is as follows:

.....	Tons.
.....	207,588
.....	130,677
.....	16,614
.....	41,200
.....	41,904

Money statement.

1, balance unexpended	\$247,266.79
92, amount expended during fiscal year	164,847.49
2, balance unexpended	82,419.30
2, amount covered by uncompleted contracts	63,676.37
2, balance available	18,742.93
appropriated by act approved July 13, 1892	150,000.00
available for fiscal year ending June 30, 1893	168,742.93
(estimated) required for completion of existing project	841,000.00
that can be profitably expended in fiscal year ending June 30, 1894	300,000.00
and in compliance with requirements of sections 2 of river and harbor acts of 1866 and 1867	



APPENDIX R R.

PROVEMENT OF NAPA RIVER, REDWOOD CREEK, AND SAN LUIS OBISPO, WILMINGTON, AND SAN DIEGO HARBORS, CALIFORNIA.

REPORT OF LIEUTENANT-COLONEL W. H. H. BENYAURD, CORPS OF ENGINEERS, OFFICER IN CHARGE, FOR THE FISCAL YEAR ENDING JUNE 30, 1892, WITH OTHER DOCUMENTS RELATING TO THE WORKS.

IMPROVEMENTS.

Napa River, California.	4. Wilmington Harbor, California.
Redwood Creek, California.	5. San Diego Harbor, California.
San Luis Obispo Harbor, California.	

EXAMINATION.

Deep-water harbor on Pacific Coast between Points Dume and Capistrano, California.

HARBOR LINES.

Establishment of harbor lines at San Pedro, Wilmington Harbor, California.	8. Establishment of harbor lines in San Diego Harbor and adjacent waters, California.
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UNITED STATES ENGINEER OFFICE,
San Francisco, Cal., July 6, 1892.

GENERAL: I have the honor to transmit herewith reports upon the works of river and harbor improvement under my charge for the fiscal year ending June 30, 1892.

Very respectfully, your obedient servant,

W. H. H. BENYAURD,
Lieut. Col., Corps of Engineers.

Brig. Gen. THOMAS L. CASEY,
Chief of Engineers, U. S. A.

REPORT OF THE CHIEF OF ENGINEERS, U. S. ARMY.

COMMERICAL STATISTICS.

(Compiled by Messrs. & Co.)

Articles.	1887.	88.
Imports.		
Wharfage	500	513.00
Terminal charges	1,200,000	1,200,000
Stevedoring	1,500	40
Wharf rent, 1887, 1888, etc.	4,500	0
	4,000	1.00
Exports.		
Wharfage	5,000,000	1,000,000
Terminal charges	25,000,000	17,000,000
Stevedoring	425,000	5,000
Wharf rent	2,000	1.00
	2,500	
	7,000	1.20
	8,000	1.70
	2,000	1.00
Total amount of freight on Redwood Creek, July 1, 1886, to July 1, 1887.....		90,000
Total amount of freight on Redwood Creek, January 1, 1888, to December 31, 1888.....		64,000
Total.....		4,000

RR 3.

IMPROVEMENT OF SAN LUIS OBISPO HARBOR, CALIFORNIA.

The project for this improvement is to construct a breakwater on Wharfe Reef at Fort Seward, extending from Point San Luis to Wharfe Island and thence to a point where the outer reef rises above high water. The structure is intended to protect the anchorage and landing at Fort Seward. Its total length when completed, including the island, will be about 2,300 feet.

That part of the breakwater between Point San Luis and the island, in length about 300 feet, was completed two years ago, and a commencement was made upon the extension beyond the island by depositing rock in the deeper portions of the reef. This latter work has been continued during the past year under a contract with the San Francisco Reefs Company. It was intended to commence as near the island as possible, working thence outward and filling up the low places in the reef to the water of mean low water as the work progressed. Owing to the almost constant recurrence of a heavy swell over the reef the contractor experienced great difficulty in mooring his vessels in position to deposit the rock properly in place. He had recourse, therefore, to a temporary structure extending from the island. By this means the breakwater was advanced about 900 feet beyond the island and built above high water throughout that length. To the close of the fiscal year there had been delivered and placed in the above length of structure rock in the depressions beyond a total of 7,870.5 tons of rock.

This contract is still in force, and work under it will be completed in

gust. The time of completion has been extended three times, the delay in the work being due partly to causes beyond the control of the contractor, but in a great measure to the imperfect plant with which it was attempted to carry on the work. The latter defect was remedied by a complete change in the outfit for transporting the rock from Morro to the site of the work.

Under permission granted by the Light-House Board the material required for the present contract is obtained at Morro Rock, a light-house locality 20 miles to the northward of Port Harford. This locality is the only one within any reasonable distance where suitable rock for breakwater can be obtained, and it is considered that we must depend upon that locality entirely for all future supplies of material for breakwater.

The difficulties attending the passage of loaded barges over Morro Rock and the distance which the material must be transported under conditions of heavy seas and winds necessarily add greatly to the cost of construction. Any small amount appropriated would only deter the contractors from seeking the work, except at prices greatly in excess of what it should cost. It is, therefore, desirable in the interests of economy that appropriation be made large enough to justify anyone undertaking the work to provide a suitable plant for the operations.

The amount of \$130,000 could be profitably expended in continuing operations during the next fiscal year.

The total amount appropriated for this improvement is \$65,000, as follows:

of August 11, 1888.....	\$25,000
of September 19, 1890	40,000

And the total amount expended, including outstanding liabilities, is \$2,053.07.

Money statement.

July 1, 1891, balance unexpended	\$39,082.56
June 30, 1892, amount expended during fiscal year.....	19,435.79
July 1, 1892, balance unexpended	19,646.77
July 1, 1892, outstanding liabilities	\$6,699.84
July 1, 1892, amount covered by uncompleted contracts.....	11,952.04
	18,651.88
July 1, 1892, balance available	991.89
Amount appropriated by act approved July 13, 1892.....	30,000.00
Amount available for fiscal year ending June 30, 1893.....	30,994.89
Amount (estimated) required for completion of existing project.....	189,900.00
Amount that can be profitably expended in fiscal year ending June 30, 1894	130,000.00
Amount submitted in compliance with requirements of sections 2 of river and harbor acts of 1866 and 1867,	

COMMERCIAL STATISTICS.

	Year the improvement began (1888).		Year ending December 31, 1891.	
	Incoming.	Outgoing.	Incoming.	Outgoing.
Vessels:				
Steam..... number.....	603	603	506	504
Sailing..... do.....	34	34	13	11
Total.....	637	637	519	515
Tonnage.....	452, 149.34	452, 149.34	335, 970	335, 000
Draft, greatest..... feet.....	18	18	18	18
Merchandise, general..... tons.....	8, 802	5, 669	7, 418	1, 331.7
Coal..... do.....	2, 985		1, 211.8	
Lumber..... do.....	8, 834, 400		2, 175.9	
Grain..... tons.....		23, 305	20.5	21, 734.4
Bituminous rock..... do.....		15, 342		19, 275
Butter..... do.....				1, 542.3
Cheese..... do.....			1.2	16.7
Live stock..... do.....			85.5	2, 067.4
Hides..... do.....			5.3	
Asphaltum..... do.....				141.1
Chrome ore..... do.....				31.4
Onyx..... do.....				42.1

Statistics for both years furnished by the Pacific Coast Railway.

Total amount of freight entered and cleared in 1888.....	68, 425
Total amount of freight entered and cleared in 1891.....	48, 771.8

No new lines of transportation have been established during the year.

R R 4.

IMPROVEMENT OF WILMINGTON HARBOR, CALIFORNIA.

Before the commencement of this improvement in 1871 there was a depth of less than 2 feet at the entrance to the harbor at mean low tide. The extension of jetties from Rattlesnake Island and Timms Point on the easterly and westerly sides of the harbor respectively, and the dredging of a portion of the interior space, have resulted in giving a depth in the interior channel of not less than 16 feet, and at the entrance of fully 14 feet at the same stage of water. It is expected that future operations will increase this latter depth to 16 feet.

Operations during the past year have consisted in making needed repairs to the east jetty, in raising and extending the west jetty, and in dredging and removing, by blasting, portions of hard material from the inner channel in the vicinity of Deadmans Island.

At the commencement of the fiscal year two contracts for the delivery of stone for the jetties were in force. The total amount of rock delivered during the season of active operations was 21,309 tons. Of this amount 17,763 tons were deposited upon the line of the west jetty, extending that structure a further distance of 800 feet. The remainder, 3,546 tons, was used in strengthening the line of single work of the east jetty at the south end of Rattlesnake Island, where the encroachments of the sea threatened to result in serious damage to the work. The west jetty has now a length of 3,450 feet, and the east jetty, extending to Deadmans Island, has a length of 6,600 feet.

At the close of the dredging operations last year it was found that certain shoal places existed in the channel in the vicinity of Deadmans Island, consisting of material too hard to be removed by the dredging machine. Experiments were therefore made with dynamite, with a view of disintegrating the material and permitting the strong current of the ebb tide to complete the work of removal.

These operations, conducted at intervals when weather and sea would permit, were entirely successful, and a channel depth of not less than 16 feet at mean low tide can now be carried inside Deadmans Island.

Surveys were made from time to time during the year for a study of the changes taking place in the interior and exterior channels as a result of the operations in progress. The survey made at the close of the fiscal year shows a deepening of the entire channel, giving as a final result a depth at the entrance of fully 14 feet at mean low tide.

The various operations at the locality were in charge of Lieut. James J. Meyler, Corps of Engineers.

The Board of engineer officers appointed to consider the subject of harbor lines, rendered a report under date of July 18, 1891, recommending for adoption certain bulkhead and pier head lines on the easterly and westerly sides of the harbor. The lines as laid out received the approval of the Secretary of War, July 28, 1891.

Any funds made available for the coming fiscal year will be expended in extending the east jetty beyond Deadmans Island, and in keeping in repair the works already constructed. Should the full amount of \$51,000, required to complete the present project, be appropriated, no further project will be presented until the results obtained by contemplated jetty extension are fully determined.

The total amount appropriated for this work is \$904,000, as follows:

Act of—		Act of—	
March 3, 1871	\$200, 000	March 3, 1881	\$33, 000
June 10, 1872	75, 000	August 2, 1882	100, 000
March 3, 1873	150, 000	July 5, 1884	50, 000
March 3, 1875	30, 000	August 5, 1886	75, 000
June 18, 1878	20, 000	August 11, 1888	90, 000
March 3, 1879	12, 000	September 19, 1890	34, 000
June 14, 1880	35, 000		

The total amount expended to close of the fiscal year, including outstanding liabilities, is \$894,450.64.

This work is in the collection district of Los Angeles.

Money statement.

July 1, 1891, balance unexpended	\$85, 568. 18
June 30, 1892, amount expended during fiscal year	75, 893. 60
July 1, 1892, balance unexpended	9, 674. 58
July 1, 1892, outstanding liabilities	125. 22
July 1, 1892, balance available	9, 549. 36
Amount appropriated by act approved July 13, 1892	51, 000. 00
Amount available for fiscal year ending June 30, 1893	60, 549. 36

COMMERCIAL STATISTICS.

Commercial statistics for the year the improvement began (1871).

	Incoming.	Outgoing.
Vessels:		
Steam	160	160
Sailing	65	65
Total	225	225
Freight	25, 313	9, 675
Lumber	10, 938, 336	
	tons.	feet.

Statistics of commerce for the year ending December 31, 1891.

[Furnished by the collector of customs at Wilmington, Cal.]

	Foreign commerce (outer harbor).		Domestic commerce.	
	Incoming.	Outgoing.	Incoming.	Outgoing.
Vessels:				
Steam.....			392	392
Sailing.....	41	35	156	156
Total.....	41	35	548	548
Tonnage.....	62,231	53,455	261,455
Draft, greatest.....feet..	28.5		18.5
Merchandise, general.....tons..	2,648	1,626	5,250	10,922
Coal.....do.....	95,239			
Lumber.....feet.....			67,151,268	

The total amount of revenue collected at the port during the year ending December 31, 1890, was \$93,279.77.

The rates upon freight have been reduced since the first expenditure for the improvement of this harbor from \$7.50 to 62½ cents per 1,000 feet on lumber and from \$5.00 to 75 cents per ton on general merchandise from the outer anchorage.

Total amount of freight entered and cleared in 1871.....	Tons. 50,039
Total amount of freight entered and cleared in 1891.....	208,831
Increase.....	158,792

One new line of transportation has been established during the year, viz, the Los Angeles Terminal Railway.

R R 5.

IMPROVEMENT OF SAN DIEGO HARBOR, CALIFORNIA.

The present approved project for the improvement of San Diego Harbor contemplates the construction of a jetty on Zuinga Shoals at the entrance of the harbor, keeping in repair the dike built across the mouth of the San Diego River to divert the waters of that stream into False Bay, and maintaining a channel of 24 feet at mean low water through the middle ground.

Jetty.—It is intended to construct a jetty about 7,500 feet in length on Zuinga shoals, extending out from Coronado North Island, with a view of gaining a depth of 26 feet at mean low tide where there is now at same stage scant 21 feet. No work has yet been done. Proceedings were instituted in the United States circuit court for the southern district of California for the purpose of condemning a strip of land, 18.85 acres in extent, on Coronado North Island, needed for jetty purposes. The case was brought to trial about the middle of June, and an award of \$13,942.46 was made by the jury for the land in question. Upon possession of the same being obtained by the Government the construction of the jetty will be commenced with the balance of the appropriation of September 19, 1890, combined with any funds appropriated by the bill now being considered in Congress. The work will project from the shore on Coronado North Island about 1,600 feet east of the entrance to the harbor. It will be built of stone deposited upon a brush foundation in the shape of mattresses, both materials being deposited in place from a tramway built from the shore. The structure will first be carried to half tide, with a view of ultimately raising it high water.

In view of the fact that the auxiliary works, such as the wharf and amway, needed in connection with the construction are only of a temporary character and will be exposed to the destructive action of the sea and teredo and will therefore need frequent repair and renewal, it is desirable that the work be carried to completion as early as possible. The recommendation made in the report of last year for sufficient funds for carrying on the work is therefore repeated.

Dike.—Previous to the adoption of the present project for the improvement of the entrance to the harbor the only work undertaken by the Government had for its object the prevention of injury to the inner harbor by material brought down the San Diego River in flood stages and deposited in the bay in the vicinity of Old Town.

The object was accomplished by building a strong earthen levee, closing up the mouth of San Diego River, and excavating a new channel leading to False Bay. This work was completed in 1876, and operations since have been simply with a view of keeping the levee in repair.

The work was examined several times during the year and was found to be in good condition and needing no repairs.

Middle Ground.—At the commencement of the year dredging at the head of the middle ground was in progress with an allotment made from the appropriation of September 19, 1890. A channel 250 feet wide and 22 feet deep was completed, enabling the deeper-draft vessels to take a straight channel into the harbor instead of turning Ballast Point. The channel, however, did not maintain its full depth, and it partially shoaled up again. It is not intended to attempt the further improvement of this channel until the jetty construction shall have been extended out sufficiently to cut off the movement of sand over adjoining Shoals.

A survey of the entire bay, from National City to Ballast Point, in connection with the establishment of harbor lines, was completed October 7, 1891. The report of the Board of Engineer Officers, recommending for adoption certain pier head and bulkhead lines, was forwarded to the Chief of Engineers June 13, 1892. The lines as laid out received the approval of the Secretary of War June 21, 1892.

An appropriation of \$200,000 could be profitably expended in jetty construction during the fiscal year ending June 30, 1893.

The total amount appropriated for San Diego Harbor since 1875 is 142,500, as follows:

Total of—	
March 3, 1875.....	\$80,000
March 3, 1879.....	1,000
August 11, 1888.....	1,000
September 19, 1890.....	60,500

and the total amount expended is \$91,740.19.

Money statement.

July 1, 1891, balance unexpended	\$58,512.16
June 30, 1892, amount expended during fiscal year.....	7,752.35
July 1, 1892, balance unexpended	50,759.81
Amount appropriated by act approved July 13, 1892	50,000.00
Amount available for fiscal year ending June 30, 1893.....	100,759.81
Amount (estimated) required for completion of existing project.....	284,000.00
Amount that can be profitably expended in fiscal year ending June 30, 1894	200,000.00
Submitted in compliance with requirements of sections 2 of river and harbor acts of 1866 and 1867.	

2630 REPORT OF THE CHIEF OF ENGINEERS, U. S. ARMY.

COMMERCIAL STATISTICS.

Commercial statistics for the year the improvement began.

	Vessels.	Freight.
Entered	No. 201	Tons. 24,58
Cleared	195	11,647

The vessels (including 109 steamers) had an aggregate tonnage of 122,311 tons.

Commercial statistics for the year ending December 31, 1891.

[Furnished by Mr. A. E. Higgins, special deputy collector.]

	Incoming.	Outgoing.
Vessels:		
Steam.....No.	296	265
Sailing.....do.	141	138
Total.....	437	403
Tonnage.....	223,869	231,638
Draft, greatest.....feet	244	
Merchandise, general.....tons	85,963	
Coal.....do.	73,265	
Lumber.....feet	37,044,000	
Value of imports.....	\$579,473.82	
Value of exports.....		\$552,483.00

Amount of revenue collected at the port during the year ending December 31, 1891, \$94,644.61.

Total amount of freight entered and cleared in 1876..... tons.. 36,456
 Total amount of freight entered and cleared in 1891..... do.. 210,606

Increase..... 174,150

One new line of transportation has been established during the year, viz: The Pacific Mail Steamship Company, San Francisco to Panama and intermediate ports.

R R 6.

EXAMINATION FOR DEEP-WATER HARBOR ON THE PACIFIC COAST BETWEEN POINTS DUME AND CAPISTRANO, CALIFORNIA.

[Printed in House Ex. Doc. No. 39, Fifty-second Congress, first session.]

OFFICE OF THE CHIEF OF ENGINEERS,
 UNITED STATES ARMY,
 Washington, D. C., December 18, 1891.

SIR: I have the honor to submit herewith a copy of report, dated December 8, 1891, of the Board of Engineer Officers constituted under the terms of the river and harbor act approved September 19, 1890, to examine the Pacific coast between Points Dume and Capistrano with a view to determining the best location for a deep-water harbor, together with project and estimates for the work.

The Board after full examination concludes that the selection of a site for a deep-water harbor within the limits designated by the act is

stricted to the harbors in Santa Monica Bay and San Pedro Bay, and of the opinion that San Pedro is the better of these, and submits alternative estimates of the cost of the necessary breakwaters as follows:

constructed of rubble and concrete.....	\$4,594,494
constructed entirely of rubble.....	*4,126,106

After a careful consideration of the facts in the case as presented by the Board, its views as to the location and general estimates of construction are concurred in by me. The difference in cost of the two breakwaters, for the same arcs of protection, is over \$700,000 in favor of San Pedro, and when the other advantages of San Pedro, as alluded by the Board, are taken into consideration, it would seem that selection has been properly made.

Very respectfully, your obedient servant,

THOS. LINCOLN CASEY,
Brig. Gen., Chief of Engineers.

ION. L. A. GRANT,
Acting Secretary of War.

REPORT OF BOARD OF ENGINEERS ON DEEP HARBOR ON PACIFIC COAST BETWEEN POINTS DUME AND CAPISTRANO, CALIFORNIA.

SAN FRANCISCO, CAL., *December 8, 1891.*

GENERAL: The Board of Engineer Officers appointed under the provisions of the river and harbor act of September 19, 1890, to examine the Pacific coast between Points Dume and Capistrano, with a view to determining the best location for a deep-water harbor, has the honor to submit the following report.†

The section of the act under which the Board was appointed is as follows:

That the Secretary of War is authorized and directed to appoint a board of three senior officers of the United States Army, whose duty it shall be to examine the Pacific Coast between Points Dume and Capistrano, with a view to determining the best location for a deep-water harbor. The said board shall report to the Secretary of War a project for said harbor, with the estimated cost of the same, who shall lay the report before Congress at its next session, with the views of the commission and the Chief of Engineers of the United States Army thereon; and the sum of five hundred dollars, or so much thereof as may be necessary, is hereby appropriated for that purpose.

TOPOGRAPHY.

Points Dume and San Juan Capistrano differ in latitude thirty-five minutes, and in longitude a little more than a degree, and in direct distance they are about 75 miles apart. The general trend of the coast between them is southeast, broken by two bays, Santa Monica Bay, lying between Point Dume and Point Vicente, and San Pedro Bay, lying to the eastward of Point Firmen. The shore between Points Vicente and Firmen is high and rocky, forming the seaward boundary of San Pedro Hill, which rises to the height of 1,475 feet. Passing from Point Dume towards Santa Monica, the foreshore for a distance of 15 miles is abrupt and rocky, being the foothills of the adjacent Cahuenga mountains.

Proceeding to the eastward, there is a stretch of about 16 miles of beach ending at Malaga Cove, which is in greater or less degree navigable in a commercial way.

† A correction of this amount since the above letter was written makes this estimate \$4,137,561.

‡ Same accompanying this report not reprinted; printed in House Ex. Doc. No. 39, Congress, first session.

At and above the village of Santa Monica, the shore is a plateau about 80 feet above the sea, nearly vertical to the sea, covered in front by a sand beach. A depression in the plateau enables a railway to reach the level of a wharf built in front of the town.

About 2 miles to the eastward, low land encircling a lagoon called La Ballona extends for 2 or 3 miles, separated from the ocean by low sand dunes and a fine beach. Thence to Redondo, the foreshore consists of sand dunes, the plateau being retired. Railroads reach the shore both at Ballona and Redondo.

At Malaga Cove a rocky formation having San Pedro Hill close in the background abuts on the ocean, and including Rocky Point, Points Vincente and Firmen, and the intervening ground, extends to the northern end of San Pedro Bay, a distance of 12 miles, quite impracticable for commercial purposes.

Between Point Firmen and Point Lasuen, 16 miles to the eastward, lies San Pedro Bay. In this interval are included Wilmington Harbor, Long Beach, and Anaheim Landing.

The foreshore from Point Firmen to Point Lasuen is practicable throughout its length for commercial uses both upon the plateaus and the lower lands.

Within the limits of San Pedro Bay two lines of railway reach the shore at Wilmington and Long Beach. A short distance beyond Point Lasuen lies Newport Bay, a lagoon which receives the waters of the Santa Ana River. This point has connection by rail with Los Angeles.

To the eastward of Newport Bay the shore again becomes impracticable, and maintains this character to Point San Juan Capistrano, a distance of 15 miles.

It appears from this description that possible commercial sites are topographically limited in Santa Monica Bay to a shore-line length of 16 miles, and in San Pedro Bay and Newport Bay to the same distance of 16 miles. Of the shore line between Points Dume and Capistrano the three impracticable portions, namely, east of Point Dume, about Point Vincente, and between Newport Bay and Capistrano, include 42 miles.

Los Angeles is the metropolis of all adjacent territory, and is the center of all the railroads herein mentioned. It is distant from—

	Miles.		Miles.
Santa Monica	18	San Pedro	22
Ballona	17	Long Beach	22
Redondo	22	Newport	40

A notable characteristic of the shore line of San Pedro Bay and in less degree of Santa Monica Bay is the occurrence of shallow lagoons close to the ocean and separated from it by thin lines of sand beach.

The most important of these lagoons is close under Point Firmen, known now as Wilmington Harbor, it having been improved by the Government. In its natural state it had at the entrance a depth not exceeding 2 feet at low water. It has now 14 feet at that stage. It is hoped, by operations now in progress, to increase the depth by 2 or 3 feet. Vessels drawing 18.6 feet have entered at high water.

The other lagoons in San Pedro Bay are Alamitos and Anaheim. Ballona Lagoon is situated on the shore of Santa Monica Bay.

Reference to these lagoons is made principally in order to state the opinion of the Board to the effect that no one of them is capable of the degree of improvement necessary to make a deep-sea harbor as contemplated by the law.

HYDROGRAPHY OF SANTA MONICA AND SAN PEDRO BAYS.

Santa Monica Bay.—The hydrography between the village of Santa Monica and Redondo is quite uniform. Just to the westward of Redondo the 10-fathom curve of depth lies about seven-eighths of a mile from the beach, and the interval gradually increases towards Santa Monica, where it becomes $1\frac{1}{2}$ miles. Over all this frontage changes of depth are very gradual. A breakwater of given length placed along the assumed contour of the bed of the bay must on this account shelter a much larger area at Santa Monica than at any other point in Santa Monica Bay. At Redondo a submarine valley heads close to shore, the 10-fathom curve being only about 500 feet distant. Seaward of this point the valley widens and deepens rapidly, reaching a depth of 100 fathoms in $1\frac{1}{2}$ miles. Excessive depth of water makes a breakwater at this point impracticable on account of cost. The practicable locations for a harbor in this bay, topographically possible for a stretch of 16 miles, are limited by the last-named consideration to a line 14 miles in length.

The bay is free from hidden dangers. The bottom is sandy. There is a small area of rocky bottom indicated on the surface by the presence of kelp.

The approach to Santa Monica Bay is free from danger.

San Pedro Bay.—This bay lies shoreward of a line drawn from Point Men to Point Lasuen, the direct distance being about 14 miles. The average water in the bay is about 15 fathoms. The slope of the bottom in the bay seaward is fairly uniform.

Near the mouth of Wilmington Harbor, there is a rock with 30 fathoms over it, having deeper water around it. The presence of kelp indicates rocky bottom about Point Firman, and in the present arrangement of San Pedro there are also areas of rocky bottom. The east-end of San Pedro Bay appears to have a sandy bottom.

WINDS, WAVES, AND EXPOSURE.

The prevailing wind on the California coast is from the northwest, nearly parallel to the coast line north of Point Conception, which is in latitude $34^{\circ}27'$ N. At this point the trend of the coast changes from the northwest to west. This fact in connection with the bold topography of the shore causes the prevailing winds along the southerly coast of California to be westerly. This wind never becomes more than a moderate gale. It never produces the heaviest waves. The disturbance of the water due to it is, however, always an inconvenience to vessels lying at a wharf exposed to its action, and when the disturbance is greatest there is danger to vessels. This wind prevails on the southerly coast during the greater part of the year with intermission of winds in the autumn and winter. In the last-named season occur the southerly offshore winds, which produce the heaviest waves to which the coast line is exposed.

A northeasterly land wind, known as the "Santa Ana" occasionally blows from the dry hot plains lying to the eastward. Its duration is short, and it is severe, but having no fetch over the sea it raises no waves near shore. It occurs both on Santa Monica and San Pedro Bays.

The southeaster comes in the winter and spring, and brings rain. The storm first manifests itself by a wind from the southeast, which continues for a few hours, shifting then to the south and southwest.

The storm clears up when the wind gets to the northwest. In these storms a heavy sea is developed, which breaks upon the coast line in waves of great magnitude. These waves come from the south and southwest. The waves produced by the southeast wind are short, designated by sailors as choppy. The south and southwest seas, on the other hand, are long and heavy. A vessel at anchor under this exposure must, under these circumstances, get to sea with the possibility of otherwise going ashore. It is the heave of the sea rather than the wind, although the latter alone is sufficiently dangerous, that makes the strongest ground tackle, at times, of no avail.

Although southerly winds prevail during the winter seasons, and bring rain, yet their occurrence in violent form is not frequent, and a season has been known to pass without a severe storm. Nor is the duration of a storm rarely extended over two or three days.

In this respect the conditions of the southern coast of California are much less severe than in high latitudes. This consideration is of great importance, for the reason that owing to it a lighter profile may be adopted for a breakwater than would be permissible much farther north.

It also happens that a heavy westerly sea sets in unaccompanied by wind due to a cyclonic disturbance in midocean. The propagation shoreward of waves generated at long distance seaward is favored by the unusual depth of the Pacific Ocean.

It appears, then, that Santa Monica Bay is entirely open to the moderate down-coast or west winds, which prevail during the greater part of the year, and that it also is exposed to the dangerous winds and seas which occur during the winter months, coming from the south and southwest. The degree of exposure is, however, not absolutely equal in all parts of the bay. The easterly end, near Malaga Cove, is afforded protection from the winds and seas from the south by the high land to the southward, and which also affords partial protection from the southwest seas. Catalina Island also aids in some degree to shelter this portion of the bay from southerly seas.

On the other hand, Santa Monica Bay is entirely sheltered from the southeast winds by the high lands of San Pedro Hill.

San Pedro Bay is protected by the same high land from the prevailing down-coast wind. In ordinary weather the Bay of San Pedro is quiet and vessels lie safely at anchor, and for the most part discharge cargo with lighters while the wind prevails. It was doubtless this circumstance which made this point the embarcadero of this part of the coast for the Mexican trade before California was acquired by the Americans. In more recent times the greater part of the commerce of this part of the country has also been transacted here. Formerly all the deep-draft vessels from Australia and Puget Sound discharged cargoes in this bay; recently one of these ships discharged at the wharf at Redondo.

San Pedro Bay is also protected to a great extent from the southwest sea and wind by the island of Santa Catalina, which lies about 18 miles offshore to the windward. This island is $17\frac{1}{2}$ miles in length, and its height of 1,500 to 2,000 feet makes its shelter, as far as it extends, complete. It covers 48 degrees of the total arc of exposure from southwest seas, but leaves uncovered the angle between the westerly end of the island and Point Firmen, through which interval the direct southwest swells reach the San Pedro anchorage.

San Pedro Bay is also directly exposed to the southeast seas which approach through the interval between Point San Juan and the east

and of Catalina Island. While the winds and seas from the south are not regarded as formidable, those from points farther around the south that enter through the open space last referred to are considered to be heavier and more violent than those that approach the anchorage ground from the westward of Catalina.

The record of vessels wrecked at San Pedro shows, with one exception that the disasters occurred during the southerly storms, the heavy losses coming to the eastward of Catalina Island. The vessels were driven ashore on the west line of the bay. Among those lost were the *Las Biddle*, *Callao*, *Adelaide Cooper*, *San Luis*, *American*, *R. P.*; and the *Kennebec*. The exception noted was that of the *Amy*, which was driven ashore at Point Firmen during a northeast storm from Santa Ana wind gap.

The arc of exposure of Santa Monica, extending from Point Dume on the west to Point Vincente on the east, is 101 degrees, at Ballona 104 degrees, and at Redondo 90 degrees. Leaving out of consideration the unnamed point, regarded as impracticable on account of depth, we call the arc of exposure of Santa Monica Bay 102 degrees. From Point Firmen as a center the arc of exposure of San Pedro Bay around the west end of Catalina Island is 60 degrees. The arc protected by Catalina Island is 48 degrees, and the arc included between the easterly end of the island and Point Loma is 42 degrees, making the total exposure in San Pedro Bay to southeast and southwest winds and seas of 102 degrees. The aggregate angle of exposure of the two bays is therefore 102 degrees.

In order to secure a deep-water harbor in either bay a breakwater should be constructed in depths of 8 to 10 fathoms, and so located as to enclose the interior space over the arcs of exposure. These considerations make the undertaking one of great expense.

San Pedro Bay the best location for the proposed harbor is at the present anchorage ground on the westerly side of the bay under Point Firmen. The projection of the westerly shore, by which protection is afforded from westerly winds, and from which a breakwater could be constructed, affords advantages over any other section of the bay to the eastward for securing a protected anchorage.

The anchorage ground exists at the anchorage. Protection from storms from the open arcs of exposure to the southwest and southeast could be secured by the construction of a breakwater having two arms. Catalina Island affords protection from southwest seas, as before stated, over an arc of 48 degrees. As indicated on the chart the westerly arm could be started from a point on the shore under Point Firmen, and be extended in a direction south 41 degrees east (magnetic), for a distance of about 2,400 feet, which would carry it beyond a line projected from the center of the present anchorage ground to the westerly end of Catalina Island. The end of this arm is in 6 fathoms depth. Then leaving a gap of 1,500 feet the easterly arm could be given a direction north 10 degrees east along the 9½-fathom curve, and be extended about 1,500 feet, which would afford protection from the southeast seas. This could be extended easterly, as increased commerce would require, to enclose the interior space.

There will be some disturbance in the anchorage ground owing to the narrowing of the entrance, but not to any serious extent, as the westerly arm and Catalina Island cover the dangerous arc of exposure to the southwest, the projection of the easterly arm beyond the line of the westerly arm prolonged cuts off the heavier seas from the southeast.

When entering or leaving the anchorage ground sail vessels can take

the entrance or pass around the end of the easterly arm accord-
the most favorable conditions of the weather.

In Santa Monica Bay the practical location of a breakwater was
in front of the town of Santa Monica. A structure, located as shown
on the chart, and in length about 8,250 feet, would cover an anchorage
ground over the angle of exposure between Point Dume and Point
Point. The westerly 2,000 feet is in water of $7\frac{1}{2}$ to 9 fathoms, the
remainder of the structure being in 8 and 9 fathoms.

As at San Pedro, there must be some disturbance in the anchorage
ground, due to the approach of waves through the open spaces between
the ends of the breakwater and the shore.

In view of the fact that a lighter profile can be adopted at this
city than would suffice for similar structures on the upper coast,
breakwaters are designed, the profiles of which are shown on the
drawing herewith. One is built entirely of rubble deposited in place,
the other has a rubble base and a concrete superstructure. The rubble
is 20 feet thick on top, 10 feet above high water, with slope in front
on $2\frac{1}{2}$ to a depth of 12 feet below low water. The slope below
depth and in rear is 1 on 1. In the other the rubble rises to a
depth of 12 feet below low water, at which point it is 20 feet thick with
slope of 1 on 4 for a distance of 21 feet in front, and in rear of 12 feet
with lower slopes being 1 on 1. The concrete superstructure is 20 feet
thick and rises to a height of 10 feet above high water.

The estimated cost of these breakwaters is as follows:

	<i>Rubble and concrete.</i>
Santa Monica:	
232,222 cubic yards concrete at \$15	\$3,483,330
991,383 cubic yards rubble at \$1.50	1,487,074.50

Contingencies, 15 per cent.....

San Pedro:	
157,630 cubic yards concrete at \$15	\$2,364,450
988,082 cubic yards rubble at \$1.50	1,482,123.00
74,320 cubic yards rubble at \$2	148,640.00

Contingencies, 15 per cent.....

The westerly arm being built entirely of rubble.

	<i>All-rubble breakwaters.</i>
Santa Monica:	
282,250 cubic yards at \$2	\$564,500
2,431,458 cubic yards at \$1.50	3,647,187.00

Contingencies, 15 per cent.....

San Pedro:	
Westerly arm:	
74,320 cubic yards rubble at \$2	\$148,640.00
194,355 cubic yards rubble at \$1.50	291,532.50

Pedro—Continued.

Westerly arm:	
\$4,257 cubic yards rubble at \$2	\$388, 534
\$46,133 cubic yards rubble at \$1.50	2, 769, 200
	3, 597, 906
Contingencies, 15 per cent	539, 685
	4, 137, 591

Substituting a comparison of the two sites selected, it will be seen that the aggregate total arc of exposure is about the same in each, amounting 102°, though the distribution is different in the two bays. The total at Santa Monica, 77° included between lines drawn to the westerly end of Catalina Island and Point Dume, are fully exposed to the direct approach of the winds and seas from the west and southwest. The site receives but little protection on the southeast from Catalina Island, distant 36 miles, while the open area between this island and Point Vincente permits the approach of southerly seas that come around the easterly end of the island. To the moderate southwest swell, which is known to prevail the greater part of the year, the site is fully exposed.

San Pedro Bay is sheltered from the westerly winds by Point Firmen. It is open to the winds and seas from the southwest and to the prevailing southwest swell above noted, over an angle of 60° to the westward of Catalina Island. The other arc of exposure of 42° to the eastward of Catalina permits the approach of seas from the southeast, and also seas from the south, that double the easterly end of the island.

In its natural condition San Pedro Bay is better protected from the ferocious winds and seas than Santa Monica Bay.

To insure complete protection requires at the former place the construction of two detached breakwaters covering the exposed arcs, the combined length of these structures being about 8,000 feet.

At Santa Monica, a breakwater about 8,250 feet would be required over the anchorage ground over the arc between Point Dume and Point Vincente.

As shown by the foregoing estimates, the cost of the breakwater, and the adoption of either type, will be less at San Pedro than at Santa Monica.

The cost of construction for equal lengths of breakwater is in favor of San Pedro on account of the fact that at Santa Monica the breakwater must be located in depths of 7 to 9 fathoms. At San Pedro the westerly arm will be built of rubble in either case, and starting from the shore is extended only to the 6-fathom curve, the easterly arm alone being entirely in the greater depth of 9½ fathoms.

With the commencement of the construction of the westerly arm at San Pedro some protection from westerly swells will immediately be afforded.

San Pedro has further advantage in being supplemented by an inner harbor, which is expected, when completed, to afford at mean low water a depth of 16 feet at the entrance. This can accommodate shipping of 10 feet draft, and will relieve the anchorage ground to that extent. The inner harbor will also be a place of security for the plant during the period of construction.

The material for the breakwater at San Pedro must be brought from Catalina. At Santa Monica it may prove to be possible to obtain sufficient rock from the hills to the northward, but there is at present no satisfactory evidence that such will be the case. If stone shall be

derived from the shore, it must be deposited from a trestle of 100 fathoms depth, and exposed to heavy seas. If Catalina had depended upon for the supply, the cost of the work will be increased somewhat, as the length of carriage will be double that to San Pedro. The risk attending transportation is also increased.

In view of the fact that San Pedro Bay in its natural condition affords better protection both from prevailing winds and from disastrous storms than Santa Monica Bay; that protection can be secured at less cost for equal development of breakwater at the former than at the latter; that a larger area of protected anchorage from the prevailing westerly swells can be secured, the severe storms from the southeast being infrequent, and that there is already an interior harbor to be a valuable addition to the outer harbor, the Board considered San Pedro Bay as the better location for the deep-water harbor proposed by the act.

A breakwater with either of the profiles designed will fulfill the conditions of affording a protected anchorage. The cost is in favor of the one built of rubble. As the westerly arm in either case will be of the same type and should be built first, the data obtained during its construction will determine the question as to the profile to be adopted for the easterly arm.

Respectfully submitted.

G. H. MENDELL,
Colonel, Corps of Engineers

G. L. GILLESPIE,
Lieut. Col., Corps of Engineers

W. H. H. BENYAU,
Lieut. Col., Corps of Engineers

Brig. Gen. THOMAS L. CASEY,
Chief of Engineers, U. S. A.

R R 7.

ESTABLISHMENT OF HARBOR LINES AT SAN PEDRO, WILMINGTON, CALIFORNIA.

UNITED STATES ENGINEER OFFICE
San Francisco, Cal., July

GENERAL: The Board of Engineer Officers constituted by Order No. 20, from Headquarters Corps of Engineers, Washington, D. C., March 25, 1891, to consider the subject of harbor lines at San Pedro Harbor, California, rendered a report, under date of August 18, 1891,* recommending for adoption certain pier head and lines along the easterly and westerly sides of the harbor. The report was laid out and described in the report, received the approval of the Secretary of War.

On the westerly side of the harbor the lines covered the area practicable for commercial purposes. On the easterly side the lines were laid out to a limited extent only, covering the front to be used by the wharves of the Los Angeles Terminal Railway Company, as stated in the report, any future extension of these lines to be governed by the commercial necessities of the harbor.

*See Annual Report, Chief of Engineers, 1891, page 2976.

The Los Angeles Terminal Railway Company desire to have these lines extended northward in contemplation of the establishment of lumber yards on the southerly end of Rattlesnake Island. The Board therefore recommends for adoption the following described pier-head and bulkhead lines on the easterly side of the harbor, forming an extension of those heretofore approved by the Department:

Pier-head line.—Starting from the point last mentioned in the description of the pier-head line in previous report of the Board, which point is on range through northerly end of single work of east jetty and distant therefrom 320 feet; thence in a straight line to a point 805 feet easterly of the northeasterly corner of the San Pedro Lumber Company's wharf, as at present constructed, said distance being measured on a line at right angles to the direction of the main track of the Southern Pacific Railroad on west side of harbor; thence to a point on range through initial point "A" of survey, on Smiths Island, and distant therefrom 1,400 feet.

Bulkhead line.—Starting from the terminal point of the bulkhead line described in previous report of Board, which point is on range through the northerly end of the single work of the east jetty and distant therefrom 30 feet; thence to a point on same range and distant 230 feet from single work; thence parallel to pier-head line to a point on range through the northeasterly corner of the San Pedro Company's wharf; thence to a point on the range through initial point "A" of survey and distant 200 feet from the pier-head line.

As the original map, with the harbor lines previously adopted delineated upon it, is on file in the Department, the lines here recommended and described have been laid out upon a blue print of that map, which is transmitted herewith.*

Respectfully submitted.

G. H. MENDELL,
Colonel, Senior Member.

W. H. H. BENYAURD,
Lieut. Col., Corps of Engineers.

W. H. HEUER,
Major, Corps of Engineers.

Brig. Gen. THOMAS L. CASEY,
Chief of Engineers, U. S. A.

[First indorsement.]

OFFICE CHIEF OF ENGINEERS,
U. S. ARMY,
August 9, 1892.

Respectfully submitted to the Secretary of War.

The Board of Engineers constituted by Special Orders No. 20, headquarters Corps of Engineers, March 25, 1891, recommend for adoption harbor lines at San Pedro, Wilmington Harbor, California, additional to those established at this locality by the Secretary of War, July 28, 1891 (see Annual Report Chief of Engineers, 1891, page 2976).

It is recommended that the additional lines now proposed be approved, and that the Secretary place his approval both upon the report and the tracing* submitted.

H. M. ADAMS,
Major, Corps of Engineers, in charge.

WAR DEPARTMENT, August 10, 1892.

Approved.

L. A. GRANT,
Acting Secretary of War.

*Not printed.

ommending for adoption bulkhead and pier-head lines the fronts of San Diego and Coronado.* The lines received the approval of the Secretary of War. A coilines, as contemplated by the order, was not established insufficiency of funds for a survey of the entire bay.

Funds having afterwards been provided and the made, the Board has the honor to recommend for adopting described lines for San Diego Harbor and adjacent on the accompanying chart.† The lines formerly laid form a part of the system here described.

NATIONAL CITY.

Bulkhead line—Beginning at a point on the prolongation of tl Thirty-first street 1,350 feet southeasterly from the westerly line thence in a straight line to a point on the prolongation of th Twenty-fourth street 2,100 feet from the westerly side of Eighth a point on prolongation of the north side of Nineteenth street westerly side of Eighth avenue; thence to a point on the prolongerly side of Eleventh street 400 feet from the west line of Eighth point on the prolongation of the southerly side of Eighth stre westerly side of Seventh avenue; thence to a point on the prolenerly line of Fifth street 400 feet from the westerly side of Eigh to a point on the prolongation of the boundary line between Na Diego, and distant 400 feet from the westerly side of P avenue iured at right angles to the direction of said P avenue.

SAN DIEGO, ROSEVILLE, AND LA PLAYA.

Beginning at the point last above mentioned, thence in a strai P avenue to a line midway between Thor and Una streets prol point on the prolongation of the southerly side of South Thirty- from the westerly side of M avenue; thence to a point on the easterly side of Twenty-eighth street 440 feet from the southerly sid thence to a point on the prolongation of the southeasterly side eighth street 550 feet from the easterly side of Pierce avenue; t the prolongation of South Twenty-seventh street 810 feet from

northerly side of H street; thence parallel to Atlantic street to the prolongation of the northerly side of Cedar street; thence to a point on the prolongation of the northerly side of Hawthorne street 400 feet from the westerly side of California street; thence to a point on the prolongation of the southerly side of Palm street, 300 feet from the westerly side of California street; thence parallel to California street to the prolongation of the southerly side of Thorn street; thence to a point on the prolongation of the southeasterly side of Vine street 425 feet from the southwesterly side of California street; thence to a point on the prolongation of the southeasterly side of Wright street 530 feet from the southwesterly side of Atlantic street; thence to a point on a line 50 feet south of and parallel to the southeasterly side of Grand street prolonged 2,000 feet from the southwesterly side of Atlantic street; thence to a point on the line 50 feet south of and parallel to the southeasterly side of Sutherland street prolonged, 4,400 feet from the southwesterly side of Atlantic street; thence to a point on the prolongation of the southeasterly side of Noell street, 500 feet from the southwesterly side of Atlantic street; thence to a point on the prolongation of the northwesterly side of Bandini street, 5,400 feet from the southwesterly side of Atlantic street; thence to a point on a line 300 feet north of and parallel to the northwesterly side of Coats street prolonged, 5,200 feet from the southwesterly side of Atlantic street; thence to a point on the prolongation of the southeasterly side of Wetherby street, 3,600 feet from the southwesterly side of Atlantic street; thence to a point on the prolongation of the southwesterly side of Thirty-second street in Roseville, 2,900 feet from the southeasterly side of Main street, Roseville; thence to a point on the prolongation of the southwesterly side of Thirty-second street, 1,800 feet from the southeasterly side of Main street; thence to a point on the prolongation of the southwesterly side of Thirtieth street, 1,700 feet from the southeasterly side of Main street; thence to a point on the prolongation of the southerly side of Twenty-seventh street, 1,200 feet from the southeasterly side of Main street; thence to a point on the prolongation of the northeasterly side of Twenty-third street, 400 feet from the southeasterly side of Main street; thence to a point on the prolongation of the southwesterly side of Nineteenth street, 200 feet from the southeasterly side of Main street; thence to a point on the prolongation of the southwesterly side of Fourteenth street, 1,200 feet from the southeasterly side of Main street; thence parallel to Main street to the prolongation of the northeasterly side of Twelfth street; thence to a point on the prolongation of the southwesterly side of Eighth street, 500 feet from the southeasterly side of Main street; thence parallel to Main street to the prolongation of the northeasterly side of Sixth street; thence to a point on the prolongation of the southwesterly side of Second street, 500 feet from the southeasterly side of Main street; thence parallel to Main street to the prolongation of the southwesterly side of New Main street in New Roseville; thence to a point on the prolongation of the southeasterly side of Water street, 1,200 feet from the southwesterly side of New Main street, New Roseville; thence to a point on a line parallel to New Main street prolonged, and 1,900 feet westerly of its northerly side, 220 feet from the southeasterly line of Main street prolonged; thence to a point on a line parallel to New Main street prolonged, and 2,550 feet westerly of its northerly side, 380 feet southerly from the southerly side of Main street prolonged; thence to a point on a line parallel to New Main street prolonged, and 2,700 feet westerly from its westerly side, 920 feet from the southeasterly side of Main street prolonged; thence to a point on a line parallel to New Main street prolonged, and 1,930 feet westerly from its westerly side, 1,050 feet from the southeasterly side of Main street prolonged; thence to a point 110 feet due east, true meridian, from the corner stone of the United States military reservation at La Playa.

CORONADO.

Starting from a point on the prolongation of the easterly side of Prospect street, a distance of 1,030 feet from the northerly side of Second street; thence to a point on the prolongation of the easterly side of Orange avenue, and 450 feet from the northerly side of First street; thence to a point on the prolongation of the westerly side of K street, 250 feet from the northerly line of First street.

SPANISH BIGHT.

In the purpose of describing the bulkhead lines in Spanish Bight a true meridian was assumed, passing through the U. S. Coast and Geodetic Survey triangulation station on the northeast extremity of Coronado North Island, called "Channel Point," July 1887; points being designated by their distances east or west of this line north or south of the parallel of latitude passing through the triangulation station "Channel Point." The position of this station as determined by the U. S. Coast and Geodetic Survey in 1887 is latitude $32^{\circ} 42' 38.61''$, longitude $117^{\circ} 11' 35.55''$.

Starting from a point on the prolongation of K street, Coronado, 250 feet from northerly line of First street, thence to a point 3,050 feet south and 1,800 feet west of said Channel Point; thence to a point 3,850 feet south, 1,620 feet west of said Channel Point; thence to a point 4,000 feet south, 125 feet west of said Channel Point; thence to a point 4,450 feet south, 430 feet west of said Channel Point; thence to a point 5,330 feet due south of said Channel Point; thence to a point 1,000 feet south, 1,000 feet west of said Channel Point; thence to a point 8,320 feet south, 1,250 feet west of said Channel Point; thence to a point 8,150 feet south, 275 feet west of said Channel Point; thence to a point 6,150 feet south, 1,620 feet west of said Channel Point; thence to a point 6,000 feet south, 2,275 feet west of said Channel Point; thence to a point 4,400 feet south, 1,300 feet west of said Channel Point; thence to a point 3,300 feet south, 570 feet west of said Channel Point; thence to a point 120 feet due east of said Channel Point; thence in a quadrant of a circle of 120 feet radius, with the center at said Channel Point, to a point 120 feet due south of said Channel Point.

NORTHWESTERLY FRONT OF CORONADO NORTH ISLAND.

All points of the bulkhead and pier-head lines along this front are referred to straight lines connecting the triangulation stations on Coronado North Island known as "Channel Point," "Middle," and "Mud," located by the U. S. and Geodetic Survey in 1887. The connecting lines run from Channel Point to Middle and from Middle to Mud. For convenience of reference the points on these lines from which points on the bulkhead and pier-head lines are measured are designated by letters of the alphabet, the respective distances apart being as follows: From Channel Point to A, 500 feet; A to B, 500 feet; B to C, 500 feet; C to D, 500 feet; D to E, 1,000 feet; E to F, 1,000 feet; F to G, 1,000 feet; G to H, 1,000 feet; H to I, 1,000 feet; and along the second line from Middle to J, 1,000 feet; J to K, 1,000 feet; K to L, 1,000 feet; L to M, 1,000 feet; M to N, 1,000 feet; N to O, 1,000 feet; O to P, 500 feet; P to Q, 500 feet; Q to R, 300 feet; R to S, 100 feet.

The distances of all points of change of direction of the bulkhead and pier-head lines are measured on lines at right angles to said connecting station lines to the respective points. The bulkhead line may therefore be described as follows:

Beginning at a point 120 feet due north of the triangulation station Channel Point, thence to a point 800 feet from A, thence to a point 1,150 feet from B, thence to a point 1,300 feet from C, thence to a point 1,300 feet from D, thence to a point 1,000 feet from E, thence to a point 730 feet from F, thence to a point 200 feet from Middle, thence to a point 200 feet from Middle, with reference to line from Middle through Mud, thence to a point 450 feet from J, thence to a point 600 feet from K, thence to a point 600 feet from L, thence to a point 400 feet from N, thence to a point 500 feet from O, thence to a point 800 feet from P, thence to a point 100 feet from Q, thence to a point on R on the connecting line Middle through Mud.

PIER-HEAD LINES.

The pier-head lines are laid out in straight stretches, following essentially the foot contour, except south of the National City Wharf. It is not possible to follow this contour exactly. To avoid abrupt changes, and to interfere as little as possible with tidal currents, some departure from that contour has been made at a few points. This has had the effect of giving slightly increased or decreased depths at these points. The ruling depth, however, as adopted by the Board for the pier head is 24 feet, except south of the National City Wharf, where the ruling depth is 18 feet.

In front of National City.—The approximate distance of the National City pier-head line from certain fixed points is as follows:

On the prolongation of southerly side of Thirty-first street, 4,600 feet from westerly side of Eighth avenue; on the prolongation of southerly side of First street, 4,600 feet from the westerly side of Eighth avenue; on the prolongation of the southerly side of Nineteenth street, 3,550 feet from the westerly side of Eighth avenue; at the southwesterly corner of present National City Wharf, 3,600 feet from the prolongation of the southerly side of Ninth street, 3,600 feet from westerly side of Seventh avenue; on the prolongation of the boundary line between San Diego and National City, 3,350 feet from its intersection with the easterly line of Q avenue.

In front of San Diego.—Approximate distances from certain fixed points are as follows:

On prolongation of northerly side of Rigel street, 2,400 feet from northeasterly side of Q avenue; on prolongation of southeasterly side of South Twenty-eighth street, 1,390 feet from northeasterly side of Pierce avenue; on prolongation of nor-

f South Twenty-first street, 2,090 feet from northeasterly side of Pierce
 a prolongation of easterly side of Fifth street, 4,180 feet from southerly
 street; on prolongation of westerly side of Atlantic street, 1,070 feet from
 side of H street; on prolongation of southerly side of C street, 3,740 feet
 rly side of Atlantic street; on prolongation of northerly side of Ivy street,
 from westerly side of California street; on prolongation of southeasterly
 raszthy street, 8,000 feet from southwesterly side of Atlantic street; on
 on of southeasterly side of Sutherland street, 8,300 feet from southwest-
 f Atlantic street; on prolongation of southeasterly side of Wright street,
 from southwesterly side of Atlantic street; on prolongation of southeast-
 f Cotts street, 9,420 feet from the southeasterly side of Atlantic street.
 of *Roseville*.—Approximate distances from certain fixed points are as fol-

rolongation of the southwesterly side of Fourteenth street, 5,500 from the
 rly side of Main street; on the prolongation of the southwesterly side of
 st, 4,525 feet from the southeasterly side of Main street; on the prolongation
 h westerly side of Fourth street, 3,950 feet from the southeasterly side of
 t; on the prolongation of the southwesterly side of New Main street, New
 3,650 feet from the southwesterly side of Main street.

of *New Roseville and La Playa to the United States military reservation*.—Ap-
 distances from certain fixed points are as follows:

parallel to New Main street prolonged and 1,200 feet distant westerly
 outhwesterly side, 3,000 feet from the southeasterly side of Water street;
 ; on a line parallel to New Main street prolonged and 2,000 feet westerly
 uthwesterly side, 3,200 feet from the southeasterly side of Main street pro-
 a line parallel to New Main street prolonged and 3,000 feet westerly from
 urtherly side, 3,050 feet from the southeasterly side of Main street prolonged;
 arallel to New Main street and 4,000 feet westerly from its southwesterly
 feet from the southeasterly side of Main street prolonged.

of *Coronado*.—Approximate distances from certain fixed points are as fol-

rolongation of the southeasterly side of Prospect place, 1,175 feet from the
 rly side of Second street; on the prolongation of the southeasterly side
 avenue, 650 feet from the northwesterly side of First street; on the pro-
 of the northwesterly side of K street, 1,300 feet from the northeasterly side
 eet; a point 350 feet due north of triangulation point, Channel Point, on
 North Island.

of *Coronado North Island*.—Approximate distances from the points desig-
 scribing the bulkhead line at this place are as follows:

350 feet due north from Channel Point; 1,050 feet from A; 1,500 feet from
 et from C; 2,150 feet from D; 2,550 feet from E; 2,900 feet from F; 3,100 feet
 200 feet from H; 3,150 feet from I; 2,950 feet from Middle; 2,800 feet from
 h reference to line running through Middle and Mud; 2,920 feet from J;
 rom K; 3,100 feet from L; 3,180 feet from M; 2,650 feet from N; 1,800 feet
 0 feet from Q; thence to S on the line Middle through Mud.

fully submitted.

G. H. MENDELL,
Col., Corps of Engineers.
 W. H. H. BENYAURD,
Lieut. Col., Corps of Engineers.
 W. H. HEUER,
Major, Corps of Engineers.

ten. THOMAS L. CASEY,
Chief of Engineers, U. S. A.

[First indorsement.]

OFFICE CHIEF OF ENGINEERS,
 U. S. ARMY,
June 21, 1892.

fully submitted to the Secretary of War.
 rbor line board for San Diego Harbor and adjacent waters,
 a, constituted by paragraph 3, Special Orders No. 51, Head-
 Corps of Engineers, October 11, 1888, recommends for the
 of the Secretary of War harbor lines at National City, San

Diego, Roseville, La Playa, Coronado, Spanish Bight, and Coronado North Island, described in the within report and delineated upon the accompanying chart.

It is recommended that the lines selected be approved and that the Secretary place his approval both upon the report and the tracing submitted.

These lines include the harbor lines established at San Diego and Coronado by the Secretary of War April 2, 1890.

THOS. LINCOLN CASEY,
Brig. Gen., Chief of Engineers

WAR DEPARTMENT, *June 21, 1890*

The harbor lines described in the within report and delineated on the accompanying tracing are approved.

L. A. GRANT,
Acting Secretary of War

APPENDIX S S.

IMPROVEMENT OF SAN JOAQUIN, MOKELUMNE, SACRAMENTO, AND FEATHER RIVERS, PETALUMA CREEK, AND HUMBOLDT HARBOR AND BAY, CALIFORNIA.

REPORT OF MAJOR W. H. HEUER, CORPS OF ENGINEERS, OFFICER IN CHARGE, FOR THE FISCAL YEAR ENDING JUNE 30, 1892, WITH OTHER DOCUMENTS RELATING TO THE WORKS.

IMPROVEMENTS.

- | | |
|--|---|
| San Joaquin River, California. | 4. Petaluma Creek, California. |
| Mokelumne River, California. | 5. Humboldt Harbor and Bay, California. |
| Sacramento and Feather rivers, California. | |

UNITED STATES ENGINEER OFFICE,
San Francisco, Cal., July 1, 1892.

GENERAL: I have the honor to transmit herewith annual reports of operations on the public works under my charge for the fiscal year ending June 30, 1892. * * *

Very respectfully, your obedient servant,

W. H. HEUER,
Major, Corps of Engineers.

Sig. Gen. THOMAS L. CASEY,
Chief of Engineers, U. S. A.

S S I.

IMPROVEMENT OF SAN JOAQUIN RIVER, CALIFORNIA.

The project for the improvement of this river was adopted in 1877 slightly modified in 1888. Its object was to secure and maintain by digging a channel 9 feet deep at low water and 100 feet wide through tidal portions of the river and Stockton Slough to Stockton; a channel 12 feet deep at low water and 100 feet wide to Miller's warehouse in Mountain Slough; the temporary improvement of the low-water channel by dredging, scraping, small wing dams, the making of cut-offs to lighten the river, and the closure or partial closure of Paradise Cut and Laird Slough.

Up to June 30, 1892, the following appropriations have been made for this river:

August 14, 1876	\$20,000	August 5, 1886	\$18,750
June 14, 1880	20,000	August 11, 1888	25,000
March 3, 1881	40,000	September 19, 1890	75,000
August 2, 1882	40,000		
July 5, 1884	20,000	Total	258,750

Of this amount up to the present time there has been spent \$257,886.69, of which \$62,033.08 was spent in the last fiscal year.

Before the improvement of this river was commenced the low-water channel to Stockton was only about 6 feet in depth, while the upper river, above Stockton, was navigable to Hills Ferry for only a few months in the year during the high-water stage, and then only for boats drawing 2 feet or less of water. To-day the work of improvement has given a low-water channel of 9 feet to Stockton and rendered steamboating comparatively easy and safe. It has not, however, lengthened the time of navigation in the upper river.

The commerce of the river is carried on 10 steamboats of from 98 to 880 tons, averaging 462 tons, and drawing from 1½ to 6 feet of water, and on 14 barges of from 160 to 660 tons, averaging 300 tons.

Work on the river and sloughs has been continuous whenever funds permitted.

Up to 1888 all the dredging, as well as other work, had been done by contract, and the cost of dredging varied from 46 to 15 cents per cubic yard. In 1888 the Government built a dredge and did the work by hired labor and has done so ever since. This has brought the cost of dredging down to less than 10 cents per cubic yard, and this year it has been less than 7 cents.

At the end of the last fiscal year the dredge was laid up in Stockton Slough.

The river having fallen considerably, the United States dredge was put in order and work resumed in Stockton Slough on October 27, 1891. This was continued until March 9, 1892.

To maintain the required channel to a depth of 9 feet at low water, cuts were made 50 feet wide of an aggregate length of 16,166 feet, and 153,530 cubic yards of material removed. To do this cost \$10,526.16, or 6.8 cents per cubic yard.

On March 9, 1892, the river had risen so as to render further dredging unnecessary, and the dredge was shortly afterwards transferred to Lieut. Col. Benyaud for work in Redwood Creek, California.

On May 15, 1891, a contract had been made with Charles L. Bigelow to build a dam across Laird Slough, in the San Joaquin River, but it was not until August that the river had fallen sufficiently to enable work to be commenced.

The slough is situate about 2 miles above the village of Grayson, quite an important grain-shipping point on the river. The junction of the slough and the river is at a very sharp bend in the latter, and at this locality the slough is fully as wide as the river, about 200 feet, and has a cross-sectional low-water area of about 1,000 square feet. That of the river above the slough is probably 25 per cent greater, while immediately below the upper end of the slough the low-water cross-sectional area was only 320 square feet. It was estimated in August, 1891, just before the dam was commenced, that fully 75 per cent of the river flowed through Laird Slough and reentered the river by two branches of the slough at 1 and 2 miles, respectively, below the village of Grayson. The banks in this vicinity are apparently of hard pan.

ey are quite flat and generally about 10 feet above low-water level. e rise in flood stages is from 10 to 12 feet, both banks being overwed.

The necessity of closing Laird Slough was first urged in 1888 by the vigation companies, who claimed that unless the flow of water through was checked the crevasse would become the channel of the river and as cut off the town of Grayson as a shipping point altogether.

The contract with Charles L. Bigelow was made on the following basis r unit of material: Piles, at 26 cents per linear foot; driving piles, at .20 each; brush, at 70 cents per cubic yard; sand bags in place, at cents each; sand boxes in place, at \$3.60 each.

Work was commenced on August 19, 1891, by driving two parallel ws of piles across the slough; the rows were 10 feet apart; the piles re 6 feet apart between centers, and each pile was driven as far as acticable into the bottom, generally penetrating about 26 feet in depth. rush was cut, tied up into bundles from 12 to 20 feet in length, each ndle being from 12 to 15 inches in diameter. These bundles were then eed across the current between the rows of piling to a height of about feet. This formed a sort of brush mattress, which was sunk to the botm of the stream by means of loaded sand bags. While sinking the lower atress a considerable scour occurred in the bottom and one flank of e dam. This was checked by throwing in additional brush and bags f sand. When the bottom had thus been protected with brush between the piles similar mattresses were sunk in contact with the first a both the up and down stream sides of the dam, the banks in the imediate vicinity of the dam were revetted with brush, held down by and in bags, and the work was thus carried on in tiers of brush mats until the desired height was obtained. Waling pieces and stringers ere then bolted and spiked to the piles and the crest of the dam was overed with boxes, each 10 feet long by 20 inches wide by 15 inches eep, each divided into 5 separate compartments and filled with sand. he tops were then nailed on and each box was fastened to its neighors by nailed battens. In this manner 9,737 cubic yards of brush, 4,000 sacks of sand, 186 boxes of sand, and 54 piles aggregating 2,259 aining feet entered into the construction of the dam. At its crest he dam is 10 feet deep and 310 feet in length; at its base the thickess is about 60 feet. The crest is 10 feet above low-water stage in he middle and raises gradually 2 feet higher at the bank. The upream slope of the dam is 1 on 1; the downstream slope is about 1 on . The water on the upstream side of the dam is 3 feet higher than n the lower side. The leakage through the brush is very slight. The ork was completed on September 29, 1891, in forty-one working days, nd cost \$11,994.38, of which amount \$312.74 is chargeable to engineerig expenses, superintendence, advertising, and contingencies, or about 6 per cent.

The dam was examined on May 29, 1892, when the San Joaquin iver was in flood, about 10 feet above its low-water mark. The river as about bank full and level with the tops of the piles in the dam, id although the water was pouring over the dam it was perfectly inct and every pile in place. On June 3 or 4, 1892, a breach occurred e east bank of the slough, undermining about 50 linear feet of ore-protection work at the eastern extremity of the dam. This each should be repaired, but as the extent of the damage can not t be determined it is impossible to estimate the cost.

At the end of the last fiscal year contract had been made with John . Ferris for building a dam across the crevasse known as Paradise

Cut, but work had not been commenced. The contract was made at the following prices per unit of material and based on the following assumed quantities:

7,900 feet of piles, at 21 cents per foot.....	\$1,555.00
Driving 155 piles and fastening wales to same, at \$4.75.....	736.25
6,000 cubic yards brush in place, at 67½ cents per cubic yard.....	4,050.00
10,000 bags of sand in place, at 15 cents each.....	1,500.00
350 boxes of sand in place, at \$3.50 each.....	1,225.00
1,000 tons of rock in place, at \$3.30 per ton.....	3,300.00
1,000 running feet wale pieces, at 12½ cents per running foot.....	125.00

12,511.25

Actual construction was commenced early in August and the dam was finally completed and accepted on November 13, 1891.

The dam is situated within and across Paradise Cut, about 100 feet distant from the left bank of the San Joaquin River, and consists of piles of brush and rock ballast. The piles for the main weir are driven in four parallel rows across the stream; the rows are 10 feet apart, the piles in each row being 6 feet between centers; each pile is driven into the bottom of the cut an average distance of 26 feet. The crest of the weir is 8 feet above low-water level of the San Joaquin River; its length along the crest is 220 feet. On the prolongation of the crest of the weir on both sides of the river are flanks whose crests are 8 feet higher than that of the weir, and whose lengths are 50 and 72 feet, respectively.

These flanks consist of two rows of piles driven into the sloping banks of the cut and on the shore. The rows are 10 feet apart and the piles of each row 6 feet between centers.

Piles were also driven across the bottom and sides of the cut for a distance of 20 feet in front of the dam and 40 feet behind the dam to confine brush mats to the approach and overfall sides of the structure, as well as to protect the banks from washing. When the 322 piles which compose the skeleton of the dam were all driven, a temporary dam 108 feet long of sheet piling was made between the weir and the San Joaquin River and the great mass of water in Paradise Cut was pumped out so as to expose the bottom at the weir site and enable it to be prepared in a suitable manner to receive the brush. Brush (green willow in bundles) was then placed all along the bottom between all piles of the weir, dam, approaches, and overfall, the direction of the axes of the bundles of the brush being always parallel to the crest of the weir. Each bundle, after being placed in position, was cut and packed down as snugly as was practicable, and thus tier after tier of brush was built up until the mattresses of the approach and overfall were 4 feet in vertical height; that between the first and second rows of piles in the weir proper was, at reference, 8 feet; that between the second and third rows was, at reference, 6 feet, and that between the third and fourth rows, at reference, 4 feet above mean low-water level in San Joaquin River. The flanks were filled with brush to reference 16 feet. When the brush was thus placed throughout the dam all piles had wale pieces and caps bolted to them so as to form a series of cribs, and on top of this brush rock, in pieces varying in weight from 300 pounds to 1 ton, was placed until all the rock, aggregating 1,166 gross tons, was in position. This rock compressed the brush very materially, and in places the column of rock was nearly 10 feet in vertical height. No rock was placed in the approaches or overfall apron, the brush therein being held down by waling pieces bolted to the piles themselves.

short earthen levees run along the banks of Paradise Cut, one flank of the dam connects with one levee, and a crib 30 feet long, with earth, connects the dam with the other levee.

	-Feet.
Length of weir is	320
of prolongation of north flank.....	50
of prolongation of south flank.....	72
of crib on south bank.....	30
<hr/>	
Total length.....	372
mat on approach side.....	20
width of mate on south side of bank.....	30
of face of south wing.....	60
from dam to extreme end of north wing.....	60
mat at overfall.....	40
covered with brush on both sides at overfall for a distance of 50 feet.	

is in the work the following amount of material: 9,026.46 cubic feet of brush; 1,166 gross tons of rock; 322 piles, round and square. Payment was made for square piles except for driving as they came from the old dam; 7,055 linear feet of round piling; 90 posts for security in shore protection; 1 crib at end of dam.

Amount of money paid to contractor for work done was \$11,079.55. Plans are sent herewith with photographs showing both the upper and lower view of the dam.

Due to compression of the brush and some settlement, it will be necessary to again raise the crest of the dam to its original height by sand ballast. Estimated to cost \$2,000.

July 6, 1891, contract was made with John W. Ferris to dredge a cut at Head Reach, a bend in the river about 26 miles below Stockton. 11½ cents per cubic yard, measured in place in the cut. Work commenced immediately and continued until October 6, when it stopped until the banks, which were very soft, had time to dry out sufficiently to bear the weight of additional material. By this time the cut had been carried completely through, a distance of 3,084 feet, with a width of 150 feet and depth of 7 feet at low water. To make the cut 210,132 cubic yards of material were excavated and placed on the banks.

In December, 1891, the banks having dried and hardened, work was resumed, and on December 29, 1891, the contract was completed, having excavated an additional 16,800 cubic yards of material having been dredged and placed on the shore. The cut, as completed, is 3,084 feet in length, with a width of 150 feet between banks. The approaches to the cut have a width of 234 and 215 feet, respectively. The depth is 7 feet at low water with an additional cut through the center, 70 feet wide, and 9 feet deep at low water, making a practical channel 9 feet deep at low water. The cut-off is a complete success, and is used by all craft going up and down the river. The total cost of it was \$27,234.41. It is proposed when more money is appropriated, to widen and deepen this cut. Estimated cost, \$37,750.

Previous Annual Reports have recommended a cut-off at Twenty-one miles from the dam at a cost of \$65,000, but no appropriation has been made for that purpose. A double cut-off in the narrows in the river just below the junction of Stockton Slough and the San Joaquin River would be highly beneficial, as since the improvement of the river larger barges have been built, and these with great difficulty make sharp bends. In case this cut-off should be made it is recommended that it should have a cross-sectional area about equal to that of the river in the immediate vicinity. This would give it a width of about 150 feet and a uniform depth of 9 feet at low water. This would involve the removal of approximately 11,700 cubic yards of ma-

terial at an estimated cost of \$20,000. This is exclusive of the purchase of the right of way, and as this cut-off, if made, will be of great benefit to the people living in Stockton, by facilitating the tidal movements of the water in seasons of flood, it is thought that the right of way for the proposed cut should be deeded to the Government free of expense.

Wherever cut-offs have been made they have been self-maintaining.

Annual dredging will be necessary to keep Stockton Slough in a navigable condition; in fact, without it, in a short time, steamboats would be unable to reach Stockton.

While it can not be asserted that any reduction in the rates of freight or insurance have been made in consequence of the improvements, it is probable that if they were discontinued a considerable increase would soon follow. It is certain that the work done has been of the greatest benefit to the navigation of the river generally, and particularly to the city of Stockton.

With any future appropriations, depending on the amounts appropriated and the requirements of commerce, it is proposed to continue work in the following order, unless Congress should otherwise direct:

1. Dredging to maintain 9 feet depth to Stockton.....	\$25, 00
2. Repairing dam at Paradise Cut.....	2, 00
3. Making a cut-off at Twenty-one Mile Slough.....	65, 00
4. Making a double cut-off below the mouth of Stockton Slough.....	20, 00
5. Increasing cut at Head Reach, in depth, width, or both.....	37, 75
6. Survey of upper river to Firebaugh's Ferry.....	5, 00
7. Snagging and wing dams.....	10, 00
Total.....	164, 750

The above amounts could be advantageously expended in one fiscal year.

Money statement.

July 1, 1891, balance unexpended.....	\$63, 002.53
June 30, 1892, amount expended during fiscal year.....	62, 033.08
July 1, 1892, balance unexpended.....	969.45
Amount appropriated by act approved July 13, 1892.....	65, 000.00
Amount available for fiscal year ending June 30, 1893.....	65, 969.45

{ Amount (estimated) required for completion of existing project *.....
 { Amount that can be profitably expended in fiscal year ending June 30, 1894 164, 750.00
 { Submitted in compliance with requirements of sections 2 of river and harbor acts of 1866 and 1867.

COMMERCIAL STATISTICS.

The following statistics of the trade of the San Joaquin River were kindly supplied by Capt. H. J. Corcoran, manager of the California Navigation and Improvement Company, who own most of the steamboats and barges running on the river. A large amount of freight is carried between San Francisco and Stockton in small schooners, and of this it is impossible to get a correct statement:

Wheat.....	Tons 70, 000
Lumber.....	86, 000
Coal.....	45, 000
M.H. staves.....	63, 000
Miscellaneous freight and manufactured products.....	50, 000
Produce.....	30, 000
Pulp, hides, and other raw materials for manufacturing purposes.....	20, 000
Total.....	370, 000
Passengers.....	56, 000

* Indeterminate.

S S 2.

IMPROVEMENT OF MOKELUMNE RIVER, CALIFORNIA.

Before improvement the navigation of this river was difficult and dangerous on account of numerous snags and overhanging trees, and in 1884 a project for its improvement was made, having for its object the removal of the snags and trees obstructing navigation. Both forks of the river were cleaned out in 1884, 1885, 1886, 1887, and 1888, rendering travel to Bensons Ferry, the head of navigation, easy and safe. The project is completed the project as outlined, and since then no work of improvement has been done on the river.

The following are the appropriations made for this river:

84, July 5.....	\$8,500
86, August 5.....	2,500
88, August 11.....	2,000
Total.....	13,000

Of this amount \$12,457.62 has been expended up to the present time. Nothing was spent in the last fiscal year.

Since 1888 snags have reformed, which require removal, and overhanging trees have regrown. These should be again cleared out. Private parties have also cut a drainage canal, opening into the river near New Hope Landing. This has caused a serious bar to form, which makes the landing of boats difficult. The remedy is to compel the closure of the ditch and remove the bar by dredging.

Another obstruction is a point of land jutting out into the channel on the right bank near New Hope Landing. This landing, one of the most important shipping points on the river, is on the left bank of the river proper, at the junction of the two forks. From this point a ponoon bridge extends across the south fork of the river, which fork for a mile and a half below this point is not navigable. The river proper and the north fork at New Hope Landing make a junction which is nearly a right angle. The point of land referred to, being a tongue between the river and the north fork, is gradually but constantly extending into the fork, increasing the curvature of the bend, and, in consequence of the current, making it very difficult for a boat to make the landing at New Hope. To make the landing easy and safe and give a more direct entrance for the river into the north fork, this point, which contains about two-thirds of an acre, should be removed to a depth of 6 feet below low water.

The estimated cost of the improvements referred to is as follows:

Removing snags and overhanging trees.....	\$2,000
Dike and brush dam to close drainage canal.....	1,000
Removing sand bar caused by canal, 50,000 cubic yards, at 10 cents.....	5,000
Cutting off point near New Hope Landing, including grubbing and purchase of land.....	1,100
Total.....	9,100

All this work could be advantageously done in one fiscal year.

The statistics of trade, furnished by the California Transportation Company, for the Mokelumne River for the year from June 1, 1891, to June 1, 1892, are as follows:

Freight.....	tons..	44,241
Passengers.....		3,669

This commerce is carried on one stern-wheel steamer, the *Constance* (385 tons; draft, light, 2½ feet; loaded, 6 feet), which makes three round trips per week and carries all the freight offered. The effect of the improvements on the rates of freight and insurance is uncertain, but as the rate of freight by river is \$1 per ton less than by railway from competing points, a yearly saving is effected on the above statement of freight of \$44,241.

Money statement.

July 1, 1891, balance unexpended.....	\$542.38
July 1, 1892, balance unexpended.....	542.38
Amount appropriated by act approved July 13, 1892	2,500.00
Amount available for fiscal year ending June 30, 1893.....	3,042.76
{ Amount (estimated) required for completion of existing project.....	6,600.00
{ Amount that can be profitably expended in fiscal year ending June 30, 1894	6,600.00
{ Submitted in compliance with requirements of sections 2 of river and harbor acts of 1866 and 1867.	

SS 3.

IMPROVEMENT OF SACRAMENTO AND FEATHER RIVERS, CALIFORNIA.

The Sacramento River is a navigable stream from McIntosh Landing to the mouth of the river, a distance of 230 miles. Formerly it was navigable 45 miles farther upstream, to Red Bluff, but since the railroads have been in operation the boats have not found it commercially profitable to go above McIntosh.

The Feather River is a tributary of the Sacramento, entering that river about 16 miles above the city of Sacramento. It is a navigable stream up to its junction with the Yuba River, at the town of Marysville, a distance of 30 miles.

Before improvement navigation in these rivers was dangerous, on account of numerous bad snags, shallow bars, and rapids, and in 1874 a project for improvement was made and approved. It had for its object the temporary improvement of the low-water channel, by removing snags, building wing dams, and scraping bars. Work on this project has been continuous ever since whenever funds were available.

The following appropriations have been made for these rivers:

1875, March 3.....	\$15,000	1884, July 5.....	\$40,000
1878, June 18.....	15,000	1888, August 11.....	20,000
1879, March 3.....	20,000	1890, September 19.....	30,000
1880, June 14.....	45,000		
1881, March 3.....	60,000		
1882, August 2.....	250,000	Total.....	435,000

There has been expended up to the present time \$481,680.99, of which \$12,846.44 was expended during the last fiscal year.

At the commencement of the present fiscal year the snag boat *Seizer* was at work in the Upper Sacramento River and continued until September 11, 1891, when she was again laid up at Sacramento. During the season's work, which lasted ninety-one days, she removed and destroyed 974 snags, built 5 wing dams in the shallows below Placer City, aggregating 536 feet in length, and ran 914 miles. She consumed during the work 89½ tons of coal.

The cost of the work was \$10,719.54.

This left the river in good condition to the head of navigation, and steamboating was uninterrupted.

In June 1892, the river having again fallen, numerous snags brought up by the winter floods began to show themselves. The snag boat was accordingly again put in commission and sent to the upper river, where she is now at work.

The snagging during the present fiscal year has been under the charge of Assistant Engineer H. L. Demeritt, C. E., and the work has been satisfactorily and well done.

The commerce of the Sacramento River is carried on 8 steamers, 14 in barges, and 7 brick barges belonging to the Sacramento Transportation Company; 5 steamers and 7 barges belonging to the California Transportation Company; and 2 steamers belonging to the Southern Pacific Company. The steamers vary in size from 183 to 619 tons, averaging 303 tons, and the barges from 115 to 800 tons, averaging 581 tons. The larger of the steamers only go a short distance up the river, the commerce of the Upper Sacramento being carried on barges towed by small, light-draft steamboats.

On the Feather River a small steamer, 247 tons gross tonnage, belonging to the Southern Pacific Company, makes one round trip per week, with a barge, and carries all the freight that offers.

The snagging heretofore done had been of great benefit to navigation; without it, in fact, steamboating in the upper Sacramento River would be impossible. Freight and insurance rates have been greatly increased in consequence, and navigation is now safe.

At present all that has been attempted in these rivers is the temporary improvement of the low-water channels, and Congress by its appropriations has contemplated nothing more. To even continue this, regular snagging must be carried on or traffic in the upper Sacramento will be stopped.

The river and harbor act of September, 1890, provided for a Board of Engineers to examine and report on the Sacramento and Feather rivers, with a view to their future improvement. Their report has been printed as House Ex. Doc. No. 246, Fifty-first Congress, second session,* and contains information as to the condition and requirements of these rivers, to which attention is respectfully invited.

The following appropriations are recommended therein:

- 1) A specific yearly appropriation of \$25,000 for snagging, building up dams, etc., by means of the snag boat and crew, above the city of Sacramento.
- 2) A specific appropriation of \$275,000 for removal of obstructions in the lower Sacramento River, and \$25,000 for the closure of Jacobs Slough, on the east bank of the river, above the city of Sacramento.
- 3) A specific appropriation of \$300,000 for treatment of the Yuba River, near and above Marysville.
- 4) A specific annual appropriation of \$20,000 for improving the navigable channel of the Feather River.

If these amounts the whole of items Nos. 1, 2, and 4, and \$100,000 of item 3, making a total of \$420,000, can be advantageously expended during the fiscal year ending June 30, 1894.

* Printed also in Annual Report, Chief of Engineers, 1891, page 2990.

Money statement.

July 1, 1891, balance unexpended.....	826,163.01
June 30, 1892, amount expended during fiscal year.....	12,846.01
July 1, 1892, balance unexpended.....	13,319.01
July 1, 1892, outstanding liabilities.....	350.00
July 1, 1892, balance available.....	12,969.01
Amount appropriated by act approved July 13, 1892.....	150,000.00
Amount available for fiscal year ending June 30, 1893.....	162,969.01
(Amount (estimated) required for completion of existing project *..... Amount that can be profitably expended in fiscal year ending June 30, 1894 420,000.00 Submitted in compliance with requirements of sections 2 of river and harbor acts of 1866 and 1867.	

COMMERCIAL STATISTICS.

The following is a return of the freight carried on the Sacramento River by the California Transportation Company during the year from June 1, 1891, to June 1, 1892, kindly furnished by the company. Their boats only run as far as Clarksburg, about 35 miles above the mouth of the river:

Merchandise, fruit, produce, etc.....	Tons. 86,347
Lumber.....	19,000
Total.....	105,347
Passengers.....	14,453

The following is the amount of freight carried by the steamers of the Southern Pacific Company between San Francisco and Sacramento, including all way-freight, kindly furnished by Mr. E. C. Wright, general auditor of the company:

Freight taken north.....	Tons. 60,424
Freight taken south.....	61,343
Total.....	121,767

The following is the amount of freight carried by the boats and barges of the Sacramento Transportation Company, kindly furnished by the president of the company:

On the lower Sacramento River:

Wheat and other grain brought from the upper river for Port Costa and San Francisco.....	Tons. 102,040
Wood brought from upper river for San Francisco.....	11,250
Merchandise brought from San Francisco to be taken to upper river.....	11,440
Lumber brought from San Francisco to be taken to upper river.....	4,000
	128,730
Merchandise between San Francisco and Sacramento.....	Tons. 20,000
Coal, San Francisco to Sacramento.....	12,000
Lumber, San Francisco to Sacramento.....	13,000
Brick, Sacramento to San Francisco.....	50,000
	95,000
Total carried on lower river.....	223,730

* Indeterminate.

Upper Sacramento River:	
at, down river.....	102, 040
d, down river.....	11, 250
handise, up river.....	11, 440
ber, up river.....	4, 000
<hr/>	
tal carried on upper river.....	128, 730
nnage of Lower Sacramento River for year.....	450, 840
nnage of Upper Sacramento for year.....	128, 730
Following is the freight carried on the Feather River during the year, kindly d by Mr. E. C. Wright, General Auditor of the Southern Pacific Company, e boat the freight was carried:	
taken north.....	Tons. 3, 911
taken south.....	13, 030
<hr/>	
tal.....	16, 941
the total tonnage of the Feather River for the year.	

S S 4.

IMPROVEMENT OF PETULUMA CREEK, CALIFORNIA.

project for the improvement of this creek was made in 1880. Its was to straighten the channel by cut-offs, and to obtain, by ing, a channel 50 feet wide and 3 feet deep at low water, up to n of Petaluma. Up to the present time \$36,000 have been ap- ted for the improvement of this creek, of which amount \$35,892.95 n spent in completing the project, leaving a balance yet on hand .05.

following appropriations have been made for this work:

ne 14.....	\$8, 000
reh 3.....	8, 000
gust 3.....	14, 000
gust 11.....	2, 000
tember 19.....	4, 000
<hr/>	
tal.....	36, 000

re improvement the channel was very crooked, and in places bare water. Dredging was done by contract in 1880 and 1881, at 29 nd 45 cents per cubic yard; 21,135 cubic yards of material were d from cut-offs and from channel. In 1882 and 1883 25,290 cubic f material were dredged at 25½ cents, and in 1883 and 1884 43,267 ards were dredged, completing the project. The estimate for ting the project was \$25,868; the actual cost of completing the was \$27,656.91.

88 the channel refilled and was again redredged at a cost of 25 er cubic yard, or \$2,116.39. This gave a channel about 40 feet id 1 foot deep at low water and partially relieved the immediate of navigation. The channel again deteriorated and in 1891, urther funds became available, a contract was made for redredg- hannel at 15 cents per cubic yard.

his last contract work was commenced in August, 1891; the re- as a completed channel 6,800 feet long and 40 to 45 feet in

Of this length 4,200 feet was excavated to a depth of 3 feet the lowest low water, while the other 2,600 feet was dug to a f 28 to 30 inches below the same plane. Much of the dredging hard-pan.

The appropriation was insufficient to complete the whole work to the proper water depth. The excavated channel extends from the railroad wharf below the town up to the steamboat wharf in the town. Since the channel was completed there has been no detention therein of boats on account of lack of water. The cost of this last piece of work was \$3,846.36, and 37,025 cubic yards of material were removed from the channel. Experience has proved that this channel is not self-maintaining, and navigation can only be kept up by occasional dredging. Nearly all the drainage of the creek flows over highly cultivated agricultural land, in consequence of which deposits are large. The creek carries a very large commerce, as can be seen by the statistics herewith, and is sufficiently large to warrant the securing of a channel 15 feet deep at low water, to obtain which it is estimated will cost about \$30,000. As this channel will not be self-maintaining it will have to be redredged about once in every five years at an estimated cost of \$5,000.

The commerce of the creek is carried on one stern-wheel steamer, *Gold*, of about 294 tons, which makes six round trips per week, and many small schooners which carry the heavier freight.

Money statement.

July 1, 1891, balance unexpended.....	53,300
June 30, 1892, amount expended during fiscal year.....	5,800
July 1, 1892, balance unexpended.....	47,500
Amount appropriated by act approved July 13, 1892.....	10,000
Amount available for fiscal year ending June 30, 1893.....	10,000

{ Amount (estimated) required for completion of existing project*..... 30.
 { Amount that can be profitably expended in fiscal year ending June 30, 1894..... 30.
 { Submitted in compliance with requirements of sections 2 of river and harbor acts of 1866 and 1867.

COMMERCIAL STATISTICS.

The following statistics of the trade of Petaluma Creek are taken from a committee appointed by the board of trustees of the city of Petaluma for the purpose of collecting such statistics, and kindly furnished by them:

	Tons.	
Grain.....	24,000	Lumber.....
Coal.....	2,272	Mill stuff.....
Merchandise.....	25,845	Live stock.....
Produce.....	28,606	
Cannery products.....	6,348	Total.....
Iron.....	100	
Passengers, 6,460.		

IMPROVEMENT OF HUMBOLDT HARBOR AND BAY, CALIFORNIA

The project for this improvement was adopted in 1881, and has for its object the securing of a channel 13 feet deep and 200 feet wide at the head of Eureka wharves, and one 10 feet deep and 100 feet wide at Arcata and Hookton. This portion of the project was complete

* Indeterminate.

at a cost of \$96,061.55. These channels have since deteriorated what, but not sufficiently to impede navigation.

In 1882 an additional project was made, contemplating the increase of the path over the bar at the entrance to the harbor by means of a stone built up to low water and extending seaward from the south spit a distance of 6,000 feet. The estimated cost of this was \$600,000. In October, 1890, a Board of Engineer Officers, appointed to consider this, advised the modification and increase of this project to embrace the north and brush jetties starting from the shores of the north and south spits and extending seaward to the 18-foot curve. The estimated cost of the two jetties was \$1,957,615. This project was adopted. Appropriations for improving the entrance aggregating \$342,000 were made in the years 1884, 1886, 1888, and 1890; but as the amount for any one year was too small to warrant commencement of work, and, moreover, by a clause in the act money was not available until title to the land on the south spit was acquired free of expense to the Government, no work was done until 1888, when the land required was obtained and the tract let to the American Bridge and Building Company for work on the south jetty. Under this contract, which as extended expired on December 31, 1890, 1,605 feet of shore track, 1,152 linear feet of protection work, and 2,767.6 linear feet of pier over water were built; from 14,669 cubic yards of brush mattress, weighted with 44,943.5 tons of stone, were placed, building the jetty out 2,767.6 feet and raised to the level of high water for 900 feet of that distance. In May, 1891, operations were commenced under a contract made with Messrs. Brown & Brown in the preceding February, and work was vigorously prosecuted until the expiration of the contract in December, 1891. On June 30, 1891, the contractors had delivered in the south jetty 10,000 tons of stone, which were used in raising that jetty to a higher plane out from 900 feet reference to 1,800 feet. Early in May, 1891, an erosion of the north spit caused the widening of the entrance from 5,600 feet to 5,000 feet, the forming of a shoal in the north channel, and became so serious that it was determined to transfer operations to that side, and in June the contractor prepared to commence work by building a wharf and approaches, laying shore track, and driving piles needed for the construction of the jetty. All this was ready by the commencement of the present fiscal year.

North Spit.—In July it was found that a succession of high tides and heavy seas caused an erosion of the bank in the vicinity of the shore track making its protection necessary. This was done by building 300 linear feet of protection work requiring 252.24 cubic yards of brush mattress and 150 tons of stone. Work was begun on the proper and continued until September, when a spur raised to higher level had been run out 1,480.5 feet in a southerly direction; this consisted of a trestle and track, from which brush mats weighted with stone were sunk. This was found to be sufficient to arrest the erosion and to raise the North Spit to reform, as the sand on the seaward side deposited to the top of the stone in one tide after dumping. In the construction of this jetty it was found practicable, owing to the decided action of the waves, to use two brush mattresses, one sunk on the other, and thereby lessen the amount of stone. This effected a saving of 6 tons of stone per running foot.

South Spit.—The operations here were confined to raising the jetty already built to 6 feet above low water, and in building an extension 600 feet. Progress was slow until after the north jetty was completed for the season, about the 15th of September, when operations

ANNUAL REPORT OF THE CHIEF OF ENGINEERS, U. S. ARMY.

were vigorously pushed to the end. This jetty is now 3,699.2 feet in length, of which 2,200 feet is completed up to high water level. The outer portion slopes from high water level for a length of 499.2 feet to depth of 7 feet below low water. During the construction of the extension a scour commenced off the end of the trestle, and at one time the depth was 18 to 20 feet, necessitating the use of piles 60 feet in length. Fears were entertained that this scour might continue and make the further prosecution of the work very expensive, but this proved to be unfounded, as the conditions shortly afterwards changed, and the water again shoaled as the work proceeded.

The following is the amount of the different items delivered into the jetty during the fiscal year, at a cost of \$156,413.68:

		North jetty.	South jetty.
Crushed rock in place	feet	1,487	
Crushed rock over water	do	1,430	486.1
Crushed shells	cubic yards	8,777.14	4,738.45
Wages	hours	15,930	26,774

See Appendix, which is North Jetty.

The above work being finished by October 31, and being the amount contemplated in the contract, operations were stopped and will not be resumed until a fresh appropriation is available, as the amount of funds on hand, \$28,122.77, are not sufficient to warrant entering into a new contract.

As the south jetty is not yet extended to half the estimated distance, and the north one only commenced, it is hardly to be expected that very material results as to increased depth over the bar would be produced, yet as an indication in that direction the fact is noted that for the past year there has been a channel of 18 to 20 feet depth maintained, a condition never before known for so long a period in the history of this work.

Three tracings are sent herewith showing the progress of the work.

With the funds now on hand and any further appropriations that may be made it is proposed to continue the construction of both jetties, extending them out to the 18 foot-curve, a distance of 7,800 feet and 6,750 feet for the south and north respectively. The total cost of this work, ~~the amount already appropriated,~~ is estimated to be \$1,715,115, of which amount \$700,000 can be used to advantage in the next fiscal year, it being cheaper to prosecute the work with all possible dispatch. Owing to the ravages of the teredo the life of the piles supporting the trestle can not be expected to exceed three or four years, and further, any contractor bidding on this work calculates so as to reimburse himself for the outlay in his plant, costing \$40,000 to \$50,000 in every contract, it is therefore manifestly cheaper to the Government to complete the work as soon as possible.

The following appropriations have been made for the improvement of Humboldt Harbor and Bay, California:

March 3, 1881.....	\$40,000
August 2, 1882.....	40,000
Sixty estimates for training wall were made:	
July 5, 1884.....	62,500
August 5, 1886.....	75,000
August 11, 1888.....	125,000
September 19, 1890.....	80,000
Total.....	422,500

Money statement.

June 1, 1891, balance expended	\$184,525.45
June 30, 1892, amount unexpended during fiscal year	156,412.68
June 1, 1892, balance unexpended	28,112.77
Amount appropriated by act approved July 13, 1892.....	150,000.00
Amount available for fiscal year ending June 30, 1893.....	178,112.77
Amount (estimated) required for completion of existing project.....	1,565,115.00
Amount that can be profitably expended in fiscal year ending June 30, 1894.....	522,000.00
Amount committed in compliance with requirements of sections 2 of river and harbor acts of 1866 and 1867.	

COMMERCIAL STATISTICS.

The following statistics of trade of Humboldt Bay, from June 1, 1891, to June 1, 1892, are approximate calculations taken from information kindly furnished by the registry of the Humboldt board of trade at Eureka, Cal.:

Number of departures, steam vessels	291
Number of departures, sailing vessels	442
Total	733
Exports:	Tons.
Number	304,329
Produce	17,508
Merchandise	648
Dry-goods stock	408
Total	322,893

The above number 17,132 tons, valued at \$155,287.10, were shipped direct to foreign ports, and 7,424 tons, valued at \$89,546, were sent to San Francisco to be reloaded from there to foreign ports on vessels of too deep a draft to enter Humboldt Bay.

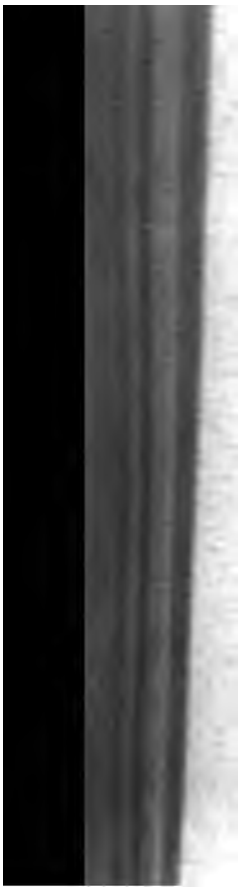
Foreign imports, direct:

Coal	tons.. 2,155
Passenger traffic:	
Arrivals	6,875
Departures	5,821
Total	12,696

The following is a statement of the freight carried by the steamer *North Fork*, on regular trips between San Francisco and Humboldt Bay, during the year ending June 1, 1891, to June 1, 1892, kindly supplied by Capt. Charles Nelson, the master of the steamer:

San Francisco to Humboldt Bay:	Tons.
General merchandise	5,127
Humboldt Bay to San Francisco:	
General merchandise	382
Number	12,760
Bales	3,720
Staves	2,610
Total	24,599
Passengers:	
San Francisco to Eureka	524
Eureka to San Francisco	707
Total	1,231

The following is a statement of the freight, passengers, and treasure carried by the *Coast Steamship Company*, making one round trip per week:



APPENDIX T T.

IMPROVEMENT OF CERTAIN RIVERS AND HARBORS IN OREGON AND WASHINGTON.

REPORT OF CAPTAIN THOMAS W. SYMONS, CORPS OF ENGINEERS, OFFICER IN CHARGE, FOR THE FISCAL YEAR ENDING JUNE 30, 1892, WITH OTHER DOCUMENTS RELATING TO THE WORKS.

IMPROVEMENTS.

- | | |
|--|--|
| <ol style="list-style-type: none">1. Coquille River, Oregon.2. Entrance to Coos Bay, Oregon.3. Umpqua River, Oregon.4. Mouth of Siuslaw River, Oregon.5. Entrance to Yaquina Bay, Oregon.6. Tillamook Bay and Bar, Oregon.7. Entrance to harbor at Nehalem Bay, Oregon.8. Upper Columbia and Snake rivers, Oregon and Washington. | <ol style="list-style-type: none">9. Columbia River between head of Rock Island Rapids and foot of Priest Rapids, Washington.10. Chehalis River, Washington.11. Skagit, Stillaguamish, Nooksack, Snohomish, and Snoqualmie rivers, Washington. |
|--|--|

EXAMINATIONS AND SURVEYS.

- | | |
|--|---|
| <ol style="list-style-type: none">12. Olympia Harbor, Washington.13. Tillamook Bay and Bar, Oregon.14. Swinomish Slough, Washington. | <ol style="list-style-type: none">15. Ship canal to connect Lakes Union, Washington, and Samamish with Puget Sound, Washington. |
|--|---|

HARBOR LINES.

16. Establishment of harbor lines in Olympia Harbor, Vancouver Harbor, and Bellingham Bay, Washington.

UNITED STATES ENGINEER OFFICE,
Portland, Oregon, July 12, 1892.

GENERAL: I have the honor to forward herewith annual reports * * * for the fiscal year ending June 30, 1892, for the following works of improvement of rivers and harbors under my charge.

* * * * *

Very respectfully, your obedient servant,

THOMAS W. SYMONS,
Captain, Corps of Engineers.

Brig. Gen. THOMAS L. CASEY,
Chief of Engineers, U. S. A.

IMPROVEMENT OF COQUILLE RIVER, OREGON.

Description of original condition.—At the time the work of improvement was begun the entrance to the Coquille River was considered very dangerous. It was by a long, tortuous, and narrow channel, skirting the south headland, studded with rocks from beyond the bar to the outside to a distance of one-half mile inside. The depth at low water was only about 3 feet, and the position of the bar channel was constantly shifting. The channel sometimes, at long intervals apart broke through the north spit and ran directly out to sea just south of Rockliffe Bank, but did not remain long in this position. The entrance at such times was comparatively safe and the channel was at its very best. The mean rise of tide was 4.1 feet.

Plan of improvement.—The plan of the improvement is to construct two parallel high-tide stone jetties, 600 feet apart, running out to sea a sufficient distance to open and maintain a channel over the bar with a least depth of 8 feet at low water, the north jetty starting from Rockliffe Bank and the south jetty from a point on the left bank inside the entrance. The cost of this work, as estimated in 1878 by Major J. L. Wilson, is \$264,200. The law of August 11, 1888, authorized in addition \$5,000 to be expended for snagging between Coquille City and Myrtle Point.

The law of September 23, 1890, authorized an amount not to exceed \$5,000 to be used in snagging.

Under date of May 8, 1891, the plan of improvement was changed to provide for an entrance width of 600 feet instead of 800 feet.

A careful estimate of the cost of completing the works at the entrance to the Coquille in accordance with the plan of improvement given above has been made, and it is found to be \$180,000. The correspondence on this subject is given under the head of "Recommendations and remarks."

The approved estimate of the cost of completing the work is \$180,000.

APPROPRIATIONS.

Act of—	
July 14, 1880.....	\$10.00
August 2, 1882.....	10.00
July 5, 1884.....	10.00
August 5, 1886.....	20.00
August 11, 1888.....	25.00
September 23, 1890.....	30.00
Total.....	105.00

Amount expended to June 30, 1891.—The amount which had been expended on the project for the improvement of the mouth of the Coquille to June 30, 1891, was \$77,012.61, and the amount expended for snagging was \$6,883.90, a total of \$83,896.51.

Results obtained to June 30, 1891.—The entrance has for considerable periods been straight and with a depth of from 8 to 10 feet at low water, but winds and currents have at other times heaped the movable sands into the channel, causing the waters to spread out over a wide angle, or the channel to break away to the northward, shallow in depth and bad in direction. A study of the various changes at the mouth of the Coquille led to the belief that the best results could not be attained unless the channel was kept from breaking away to the north and the

n sands were kept from encroaching on the channel. It was decided to use the present appropriation in building the north provided in the project.

ing this it is hoped and expected that the periods when the e is good can be materially prolonged.

vey of the entrance to the Coquille was made in September, y Mr. John R. Savage, assistant engineer.

survey shows a governing depth on the bar of 4 feet at low water. erving depth is constantly varying from this minimum to a f 10 feet at low water. The function of the jetties is to prolong iods of greater depth as much as possible.

nt expended during fiscal year ending June 30, 1892.—The amount ed during the fiscal year ending June 30, 1892, was \$18,403.47. 'ts obtained to June 30, 1892.—The results obtained at the river e over the condition a year ago have not been of a marked na. The construction of the north jetty has helped to concentrate rents upon the bar, and at times a good depth of 8 to 10 feet at ter has been prevalent. Ocean swells, however, soon occur and out the sands and bring the return of the ordinary bar depth 7 feet.

rt of operations for year ending June 30, 1892.—The snagging one during the previous fiscal year on the upper river between e City and Myrtle Point has been productive of very good re- much better than was anticipated. The removal of the snags bled the currents to scour away the accumulated débris at the shoals, and this has enabled boats to run on the river continu- uring the autumn, winter, and spring just passed.

perience gained leads to the hope that if the river channel be oughly cleared out, a good permanent navigable channel will

ie beginning of the fiscal year work was in progress extending th jetty tramway and connecting it with Parker's Wharf and in ig the Fahey Quarry near Randolph. At this time the tramway 2 feet long.

he required plant and materials were brought over from the ide, and in September, connection was finally made with Parker's by means of the tramway built across the sandy uplands on the side of the entrance. Parker's Wharf has been materially hened and enlarged, and a building over the hoisting machinery ilt.

he quarry, fender-piles were driven for convenience of landing oisting engine and derrick put into operation. During Sep-, 428.10 cubic yards of stone were quarried and moved to posi- the jetty.

ng October, 830 cubic yards of stone were added to the jetty and November 244.32 cubic yards of stone were added.

small buildings were erected, one for sheltering the locomotive e for the pile-driving machinery.

mber 18, 1891, funds being practically exhausted, all work ceased e machinery was housed. From that date to the end of the fiscal o more active work was done upon the project. The matter of extension of the work was studied and the country was explored etter quarry, as the Fahey Quarry turned out to be very poor. mber of photographic views were taken of the Coquille work, of which were sent to the World's Fair.

mmendations and remarks.—A careful consideration of the con-

dition of the work at the mouth of the Coquille led me to write the following letter:

UNITED STATES ENGINEER OFFICE,
Portland, Oregon, February 8, 1892.

GENERAL: I have the honor to forward herewith a blue print exhibiting the latest survey of the Coquille River entrance, which shows the extent of the jetties at the present time.

This work has been done in practical conformity with the project of Maj. J. M. Wilson in 1878, at which time the cost of the completed work was estimated at \$164,200.

The following are the appropriations which have been made for the work:

1880	\$10,000
1882	10,000
1884	10,000
1886	30,000
1888	25,000
1890	30,000
Total	105,000

Of this amount \$6,884 was expended in snagging on the Upper Coquille River, and there remains on hand at present, exclusive of outstanding liabilities, \$3,700. There has been expended on the project up to the present time \$94,416.

The work which has been done has been productive of great benefit, not so much by deepening the bar channel as by changing its direction from a southerly and variable course among the rocks shown on the plot to a more direct and more permanent course out to sea.

But the portions of the jetties shown on the plot are not completed in a satisfactory manner to be left with the assurance that they will be permanent.

There has never been a sufficient amount of money on hand at any one time to procure a plant suited to carry on the work properly and economically, and the design of the work and the method of carrying it on varies materially from those designs and methods which have been so successful and economical in other places where more money has been available.

I send herewith a series of photographs, which exhibit the improvements, at present existing, and their construction. The jetties are double rows of piles driven close together and surmounted by longitudinal and cross caps of heavy sawed timber. Upon these there is a single-track railway, upon which runs the revolving pile driver, and upon which are taken out the piles, rock, etc. The space between the rows of piles is filled with rock up to high water, and rock is to be dumped along the foot of the piles to give the necessary security when the piles shall be destroyed.

Photographs 167, 180, and 181 exhibit the condition of the south jetty. The outer portion of the pile tramway is in fair condition. The inner portion, built ten or twelve years ago, is nearly destroyed. The enrockment along the lagoon (the site of the old channel) has settled to about low water, and it is entirely within the realms of possibility that unless this portion of the jetty is strengthened the river may cut through this old channel. Before the work can be considered as completed this south jetty should be materially strengthened. The north jetty will require much more rock before it is finally completed.

I have made the following estimate for the permanent and thorough completion of the existing jetties, as follows:

South jetty:

Repairs to tramway, 1,920 feet, at \$2.50 per foot	\$4,800
Enrockment of south jetty, 1,920 feet, at \$12 per foot	23,040
Enrockment of north jetty, 530 feet, at \$12 per foot	6,360

Total..... 34,200

Jetty extension.—The project for the improvement of the Coquille Entrance is rather indefinite as regards the length to be given the jetties. Maj. Wilson's project was for the jetties to be extended out to a depth of 12 feet in order to get a 12 foot channel over the bar. It was subsequently changed on the recommendation of Capt. Powell to an extension capable of giving a depth of 8 feet in the bar channel, he believing that this is the most that can reasonably be expected. To reach a depth of 8 feet, according to the present survey, will require an extension of the south jetty of 740 feet and an extension of the north jetty of 1,067 feet.

I have estimated that the extension of the south jetty will cost \$51,090.75 and the extension of the north jetty \$78,998.40.

The total estimate for the completed project, in the light of past experience, is, therefore:

Completion of existing jetties	\$34, 200. 00
Extension of south jetty, 740 feet	51, 090. 75
Extension of north jetty, 1,067 feet	78, 998. 40
	<hr/>
	164, 289. 15
Add 10 per cent for contingencies	16, 428. 91
	<hr/>
	180, 718. 06

In round numbers, \$180,000 will be required to complete the project by extending the jetties to a depth of 8 feet at low water. The existing bar channel, shown by the survey of 1891, has a governing low-water depth of 4 feet. At times this has been increased to 8 and 10 feet.

The discrepancy between my estimate of cost and that of Maj. Wilson is due primarily to the fact that at the time the latter was made but little was known of the cost of doing work along the Oregon coast, and experience had not determined the strength and solidity required to withstand the storms of this section.

Also, there has never been an adequate amount of money to carry on the work economically, and the desire to produce beneficial results has caused the works to be extended to a greater extent than they could be fully completed, and from this has arisen the necessity for going over the work twice and three times at a large increase over what it would have cost if it had been fully and substantially completed as it progressed.

It is designed, if more money is appropriated for this project, to make a change in the plan of the work by adopting jetties more in conformity to those at Coos Bay and Yaquina Bay; that is, to build a tramway with pile bents 15 feet apart, and a jetty of brush and stone, instead of a close pile tramway with stone enrockment.

It is recommended that the estimates for the Coquille River be corrected to correspond with the figures above given.

Very respectfully, your obedient servant,

Brig. Gen. THOMAS L. CASEY,
Chief of Engineers, U. S. A.

T. W. SYMONS,
Captain, Corps of Engineers.

The recommendations contained in this letter were approved by the division engineer and yourself, and will be the guide in future operations.

The amounts of money appropriated for this work have always been inadequate and have resulted in greatly increasing the cost of the work.

The least appropriation which should be made for the work is \$50,000, the sum recommended.

Future operations.—It is proposed to expend any future appropriation, first, in strengthening the present north and south jetties, and, second, in extending them. To this end it will be necessary to procure another quarry, and it will probably be found best to buy or build two or three scows for transportation of rock and brush.

Report of Capt. R. S. Littlefield, in charge of the work at the Coquille, is herewith.

Money statement.

July 1, 1891, balance unexpended	\$21, 103. 49
June 30, 1892, amount expended during fiscal year	18, 403. 47
	<hr/>
July 1, 1892, balance unexpended	2, 700. 02
July 1, 1892, outstanding liabilities	110. 00
	<hr/>
July 1, 1892, balance available	2, 590. 02
Amount appropriated by act approved July 13, 1892	25, 000. 00
	<hr/>
Amount available for fiscal year ending June 30, 1893	27, 590. 02

Amount required for completion of existing project

Amount available for completion of existing project	153, 000. 00
Amount actually expended in fiscal year ending June 30, 1894	50, 000. 00
Balance available for completion of sections 2 of river and	
and 1867.	

the sea beach.

During the winter months, including March, 1891, a small constructing twelve side-dump cars of the Yaquina pattern months. April, May, and June, the Government pile-driver jetty on the south side of the river to the north shore by a high tide, which necessitated the building of 705 feet of track including track to land the driver, and to transport the same to the new jetty.

A small submerged crib was sunk at the end of an inclined into the river; so that piles could be floated onto a car truck and thence be taken over the tramway 525 feet in length jetty. This tramway had been the route for transporting the spit.

During the three months above named the jetty was built: driving and framing, a length seaward of 373 feet; 780 feet of lagoon on the south side, which formed the harbor for the use of the locomotive, rails, coal, and hoisting engine for the same, formerly used, which latter was applied to the construction of roadway across the north spit mentioned above. Also, during 1891, a tramway for transporting the stone from the Parker wharf with the inshore end of the jetty, a distance of over 3,600 feet, rails partly laid. A considerable amount of stripping was done to the prospective stone supply at Randolph, 6 miles up the river.

Three rocks in a cluster near mid-channel just inside the jetty, with 600 pounds of No. 1 giant powder, and the depth over them from 2 feet to 9 feet at low water.

The work from July to November, 1891, when the operation of the transfer, as needed, of the remaining parts of the plant from the lagoon south to the north shore of the river, the laying of the balance way, 3,786 feet in length, connecting the jetty with Parker's wharf, of this wharf to double its former size; the building of a pier for transferring the stone from the barges to the cars; the building of the Randolph Quarry for loading the barges, for which derrick was purchased, and the extension of the jetty 137 feet, including a breach made by the sea. The total length of the jetty built is 510 feet.

For the whole work 771 piles were used on jetty construction and 11 piles for a small wharf at the Randolph Quarry. The piles, after reaching open ocean water, were 36 feet in length. The piles 30 feet, were used on the beach approach, Rackliffe Rock to the jetty.

The penetration of the piles, when the sea was reached, at their points struck a layer of gravel boulders, and generally at a greater depth than the average stated, which, however,

ran parallel to the south jetty and 100 to 150 feet north of the same. Throughout the year, except during parts of December, January, and February, the channel had depths for the class of vessels employed in this trade, for which light-draft schooners are used. From the record of least soundings, as taken by the bar tug *Tri-um* in sounding the bar, the channel over the same shows 6 to 7 or more feet at low water, giving at ordinary high tide 10 to 12 feet as least depths, while the usual draft, that of the vessels trading from here ranges from 8 to 9½ feet. It may be stated in this connection that except when the bar is smooth no craft should draw within the depth of the least depth on the same, for, unlike lake harbor bars, or even those on the Atlantic coast, the seas on these Pacific bars, when rough, cause an abnormal pitch of the vessel, with the resultant effect of striking heavily on the bottom if the bar is shoal.

While the bar was in bad condition during the months named above, the southwest winds, accompanied by heavy rains, were prevalent. These heavy rains kept the river at a freshet stage, which caused the deposit, no doubt, of some of the detritus tied in suspension by the river. At any rate the conclusion has been arrived at by those engaged in commerce here that the immediate effect of a freshet is to shoal the bar, a conclusion contrary to the opinions formerly held upon this subject. The long continuance of a full, and in consequence a rapid stream, during this season, resulted in benefit to the upper part of the river, *i. e.*, the reach between Coles City and Myrtle Point. The steamboats plied with more regularity between these two places than for years, and the strong, steady current shortened up the three bars obstructing navigation for a mile or so below Myrtle Point, which is the end of navigation.

The end of the south jetty, built in 1877, was disrupted by the seas and drift trees to such an extent that by March the bulkhead piles at the extreme end and 14 feet from the jetty piling were carried away.

The Coquille is exceptional to all other rivers of the coast in the amount of drift, consisting of whole trees and logs, brought down and deposited on the beaches outside the entrance. To the south of the south jetty the high-tide line receded fully 100 feet by a "panning down" process resulting from the heavy seas during storms; the low-water line moved back correspondingly.

During the year there were built: near the north jetty, a tool house; at Randolph, a saw engine house; on Parker's Wharf, a shed over the hoisting engine, a house to store the locomotive in, and another for the protection of the pile-driving machinery.

For building the jetty tramway, wharves, houses above-mentioned, etc., 93,521 feet of new lumber was purchased and expended, and in addition there was used a quantity from the old tracks torn down on the south side.

The bulk of the steel rails used were on hand from a former appropriation.

There were quarried 1,501 cubic yards of stone and dumped in the jetty near the end on both sides for nearly its full length, as riprapping.

The north jetty stands without injury, and, for the greater part of its length, sand and drift equal its height, or, in other words, the deposit formed is 17 feet above the low water.

There are two rocks whose removal by blasting I would recommend; one just above town of Bandon on the right-hand side of the river, and the other on the left-hand side opposite Randolph Slough, where the steamboats turn in making a landfall at Randolph.

Both of these rocks are submerged, except at extreme low water, and are dangerous to navigation, being in or near the channel.

A good sized twin screw coasting steamer, the *Homer*, was built at Bandon during the present fiscal year. Tonnage, gross 433; net 330; length over all, 153 feet; beam, 32 feet 8 inches; depth of hold, 10 feet.

Mr. Adam Pershbaker has under construction at his mill a schooner for the Coles trade, designed to carry 300,000 feet, B. M., of lumber.

Respectfully submitted.

R. S. LITTLEFIELD.

Capt. T. W. SYMONS,
Corps of Engineers, U. S. A.,
(through Mr. J. S. Pollemus, U. S. Assistant Engineer.)

2068 REPORT OF THE CHIEF OF ENGINEERS, U. S. ARMY.

COMMERCIAL STATISTICS.

The mouth of the Coquille River is in the collection district of southern Oregon, Empire City, on Cass Bay, is the port of entry. The nearest light-house is on (Stragg, 12 miles northward.

The following returns relative to the commerce of the Coquille River for the year ending June 30, 1902, are furnished by Capt. R. S. Littlefield, in personal charge of the work.

SHIPPING—VESSELS ARRIVED AND CLEARED.

The number of vessels of all descriptions crossing in over the bar during the year was 92; the number crossing out was 93. Their names, registered tonnage, and number of trips made are as follows:

Name.	Trips.	Registered tonnage.
Steam schooner, <i>Star</i> (Capt. R. S. Littlefield)	15	71.15
Motor launch, <i>Star</i>	1	420.33
Motor launch, <i>Star</i>	9	117.79
Motor launch, <i>Star</i>	9	85.64
Motor launch, <i>Star</i>	5	95.67
Motor launch, <i>Star</i>	8	91.10
Motor launch, <i>Star</i>	10	83.20
Motor launch, <i>Star</i>	8	117.34
Motor launch, <i>Star</i>	10	97.93
Motor launch, <i>Star</i>	5	87.17
Motor launch, <i>Star</i>	7	117.74
Motor launch, <i>Star</i>	6	95.67

One steam schooner, the *Star*, was built at Bandon during the year. Her length is 124 feet; beam, 32 feet; draft, 10 feet; and registered tonnage, 420.33.

Arrivals and departures.

	Number.	Aggregate tonnage.
Arrivals	92	8.4
Departures	93	9.1
Total	185	17.5

owing to the high stage of water in the Coquille River, and as a result of season's steaming operations, the river steamers plying between Bandon and Mouth of the River have been able to reach the latter place with more regularity and less delay than during the present year than hitherto.

No new routes of travel have been established since date of last annual report.

Articles.	Quantity.	Articles.	Quantity.
Wheat	5,160.20	Lard pounds
Barley	16,222.4	Beeswax do
Oats	187.14	Sheep pelts tons
Flour	85.14	Shingles do
.....	Liquors do
.....	Brick do
.....	General merchandise do
.....	Cheese do
.....	Total
.....	<i>Imports.</i>	
.....	General merchandise tons
.....	Machinery do
.....	Household goods do
.....	Total

Estimated value of Imports \$28
Exports 18

T T 2.

IMPROVEMENT OF ENTRANCE TO COOS BAY, OREGON.

Description of original condition.—The obstructions which existed at entrance to this harbor before the works of improvement were begun consisted, first, of the outer bar, which is of sand and is shifting in character, and second, of the inner shoals formed by the sands which accumulate in the spring, summer, and autumn during the times when northwesterly winds prevail.

Under the action of these winds the spit on the north side advanced and the south, contracting the navigable passage under Coos Head to a very narrow width, and usually making the outer channel follow the west side of the spit in a long and tortuous course across the bar. The channel had at times broken through the north spit on a line, the general direction of which is from Fossil Point to a point just to the north of Coos Head. It was then direct, the depth of water was great, and vessels could enter or go out without trouble. The mean rise of the tide above the plane of reference was 5.6 feet.

Plan of improvement.—The project for this improvement, adopted in 1879, was to construct, at an estimated cost of \$600,000, a jetty of wood and stone, as may be found best, from a point 250 yards below the northern extremity of Fossil Point, on a line toward the east end of Coos Head, this line in plan curving so as to be directed at its outer end to the Head or a little to the north of it.

The object is to prevent accretion to the south end of the sand spit on the north side of the entrance and to open and maintain a deeper and more direct channel across the bar.

The present plan for the improvement of the entrance to Coos Bay, under which operations are progressing, consists in the abandonment of any further work on the Fossil Point Jetty and the building of two high-tide jetties of stone, one from Coos Head and one from the northern end of the north spit out toward the bar, ending at a distance of about 1,500 feet.

In addition to this the project includes the reclamation and holding the sands of the north spit, to prevent as far as possible their blowing over into the harbor.

The estimated cost of the work as approved is \$2,466,412.20.

APPROPRIATIONS.

of—	
March 3, 1879.....	\$40,000
March 3, 1881.....	30,000
August 2, 1882.....	30,000
July 5, 1884.....	30,000
August 5, 1886.....	33,750
August 11, 1888.....	50,000
September 19, 1890.....	125,000
Total.....	338,750

Amount expended to June 30, 1891.—The total amount expended in improving the entrance to Coos Bay to June 30, 1891, was \$269,840.48. Of this amount, \$210,317.74 was expended on the old project and \$59,522.74 on the new project.

Results obtained to June 30, 1891.—The work done up to June 30, 1891, consisted of the building of the Fossil Point Jetty under the old plan to a total length of 1,761 feet, but a considerable portion of this had been expended simply in a paving of heavy stone on the bottom in advance

of the jetty to prevent scour. The results of the work thus far have been to keep the channel over the bar from varying its position much as formerly and to commence an erosion of the north spit

Instead of this erosion continuing as expected, the water has been shoaled through the spit in a swash channel, thus greatly reducing the unevenness of the currents on the bar.

I quote the following from my last annual report as to the results obtained to June 30, 1891:

There has been practically no change in the results since the preceding report.

There has been considerable trouble with the bar, during the past year, the waters spreading out in an uncontrolled way and the prevalence of stones repeatedly filled with sand the bar channel made by the ebbing and flooding

The erosion of the north spit went on during the year, and a better direction of the outflowing and incoming currents has been thus secured. The troubles of the currents and eddies which formerly prevailed inside the bay, due to the waters of the main and south bays, have practically disappeared.

The bar has sometimes had a good depth of 12 to 15 feet at low water, but other times has shoaled to about an equal depth at high water. The works that have been done have not increased the bar depths, and no increased depth expected until the north jetty, now under construction, has been very considerably extended.

Amount expended during fiscal year ending June 30, 1892.—The amount expended during the fiscal year ending June 30, 1892, was \$6

Results obtained to June 30, 1892.—The north jetty and approach have been built to a total length of 4,800 feet from the wharf located inside of the north spit near its end. Of this length 1,808 feet beyond the low-water mark, 1,392 feet is between high and low water and 1,600 feet above the ordinary high-water mark, as high water existed at the time the work was commenced.

As a result due to this extension, or due to this and other works there has been a marked improvement in the entrance to the harbor during the past winter. Reports were constant during the winter that the bar depths were from 18 to 21 feet at low water. In order to determine the exact condition of the bar a survey was made during March and the plotted results show the bar some 2,000 feet inside the position of the bar at the time of the survey in August, 1891, and a channel of the bar with 18 feet of water on it at low water. This bar channel extends a little to the north of the bar channel existing the previous summer and has been in continual existence for about eight months.

The exceptionally great results from the small amount of work done is explained by the fact that the jetty cut off the swash channel from the north spit directly opposite Fossil Point, throwing a large volume of the ebbing waters over against Coos Head, and thus tending to shoal the waters upon the bar. It is not expected to maintain the depth shown by the present extension of the jetty at all times of the summer.

Report of operations.—At the beginning of the fiscal year 1892 a tramway had been built to a length of 1,600 feet. The work of the year was in progress and continued uninterruptedly until October 1, at which date there had been completed 303 trestle bents, or 4,800 feet. One thousand eight hundred and eight feet was built beyond the low water.

The work has been done by day's labor, with material furnished by contract.

A contract was entered into with Mr. Patrick O'Neil for furnishing brush fascines (1,000 cords, more or less) at \$2 per cord. The first load of fascines was received from him August 22, and he completed

ot in January, having furnished 1,158½ cords, while on account
y on his part 368½ cords were procured by hired labor.

tracts were entered into with George W. Loggie for furnishing
2,000, more or less) at 22 cents each, and with Mr. William E.
for quarrying and delivering rock (20,000 tons, more or less) at
ts per ton.

work of making mattresses and dumping rock was begun August
2.

he middle of December mattress work ceased for the season, and
t time the brush work consisted of—

	Feet.
mattresses from Bent 103 to 303	3,200
tier mattresses from Bent 100 seaward	160
mattresses on bay side from Bent 60 to 103	592
mattresses on sea side from Bent 140 to 164	384
r mattresses 20 feet square.	

siderable trouble was experienced in getting rock fast enough
ontractor Baines, but by the end of December the crest of the
ment had reached half tide, and by the middle of March it had
d ordinary high tide. The cost of unloading and dumping rock
und to average 8 cents per ton. During the year 23,923.4 tons
lumped on the jetty.

loyés were subsisted at the Government mess house while oper-
were being carried on.

cost of feeding employés per day per man at north spit was
to be 36.9 cents.

h miscellaneous work was done during the year, such as building
age platform, gridiron for brush and piles, constructing an addi-
scow, building a locomotive house and trestle sidings, assembling
st 12 cars of the 20 sent from Fort Stevens, overhauling and re-
g cars, etc. A small dwelling house for the superintendent in
e of the work has also been constructed.

amation of sand dunes.—As stated in my last annual report, a
ity of Holland grass roots was received from San Francisco and
d in several localities on the north spit.

ut one-third of the roots sprouted, and give promise of a vigorous
h.

ing April, 1892, another consignment of 100 sacks of roots was
ed, and these have been set out on the north spit.

ire fence has been built across the spit to keep cattle from the
portion of it, and during March, 1892, about 80 bushels of oats,
, and mixed grass seeds were sown.

ut 600 young pine and spruce trees were dug up and transplanted
north spit.

n Fort Stevens we secured a quantity of Scotch broom seed, which
anted early in the spring of 1892.

re was also gathered a large quantity of the seeds of plants grow-
turally about the locality. These consisted of marine pine cones,
nick berries, salal berries, and other plants, the names of which
t known. These were all planted, both as mixtures and sepa-

The knowledge gained by these experiments will be valuable
further prosecution of the work of the reclamation of the sand

vey.—A survey of the bar was made in March, 1892. For this
se the tug *Hunter* was secured. This survey was made by Mr.
F. Savage, assistant engineer.



The text in this section is extremely faint and illegible. It appears to be a dense block of printed matter, possibly a list or a series of short paragraphs, but the characters are too light to be read accurately. The text is arranged in several horizontal lines across the right half of the page.

the direction in which the littoral current is moving. There were a number of floats put in a couple of thousand feet outside the bar on a flood tide, and they drifted in toward the bar very slowly, not averaging 1,000 feet per hour, even with a wind from the northwest, and as the tide changed, they began to work south outside the bar, with very little increase in velocity, toward the inside of Yokum Point. Frequently three other floats were tried on the ebb from well inside of Coos Bay, and as a result of observing them, I should judge that the amount of water flowing out over the spits was not very great in comparison with the amount passing the channel. One float was put in well toward the end of the jetty and drifted directly over the north spit, while the other two, being more in the main channel, followed the same well out beyond Guano Rock, when one of them being drawn to the south, began to be drawn toward the south spit, and finally passed out over that spit, while the other followed the main channel for awhile and slipped out to the north of the bar buoy.

The ebb tide there is a very noticeable and interesting tide rip, as the current in the bar proper must run at the rate of 5 miles per hour, or perhaps faster, which, combined with the swell rolling in, makes an extremely rough-looking bar, and, however, there is really only a bad chop sea, but to anyone who did not know the channel from ranges on shore, it would be impossible to pick it out at a glance, on account of the nasty looking and ever-changing seas on the bar. During the ebb tide is running out this way the whole mass of water is seen to carry with it great quantities, which is probably deposited in the smoother water a short distance beyond the bar, as soundings show 2 or 3 feet less water just outside the bar taken on an ebb tide instead of on the flood.

It was told that frequently the greatest current runs directly under Coos Head, and during this survey the water seemed to have a greater velocity upon the bar.

The littoral currents would seem to be directly affected by the wind and its force, and during the summer months when the wind is from the northwest the general trend of the current is southerly, and in winter with the prevailing southwesterly winds the current is nearly always moving to the north.

From several conversations with the men on the bar tugs and with Capt. James E. of the *Hunter*, in particular, as I know he is a very observant man and one whose word and judgment reliance can be placed. All the bar men are unanimous in their opinions in regard to the action of the winds upon the littoral currents and their general directions, but they notice very little of a littoral current in the bar, but it is always noticeable at the whistling buoy where the current runs from 1 to 2 miles per hour. This alongshore current is usually perceptible from one-fourth to 15 miles offshore and its direction, at least on the surface, shifts with a change of wind, and oftentimes before the wind changes, in the same way, that the direction of the swell will change before the wind shifts. Sometimes a slight eddy has been noticeable in near the bar with a direction opposite that of the main littoral current. The presence of this eddy, or of so slight a local current in near the bar is due, I think, to the fact of the proximity of Cape Gregory, which helps to form the small bay in which the entrance to Coos Bay is situated.

At the time of this survey the prevailing winds were about shifting from the southwest to the northwest, and one day the littoral current would be in one direction and the next day perhaps running exactly opposite, on account presumably of the wind shifting. As an example, on Thursday, March 10, out at the whistling buoy there was a strong northerly current with the wind blowing strongly from the southwest, from which there was quite a heavy swell noticeable outside the protection of Cape Gregory, while on the following day, Friday, there was a rather strong southerly current with the sea very smooth indeed, and practically no wind, what there was coming from the northwest. On Saturday the conditions were very much the same as on Friday, only with less current and a little more wind.

Capt. Magee, of the tug *Hunter*, states that the best water and safest channel is to be found when the channel across the bar is in its most southern position, *i. e.*, about 500 or 1,000 feet south of the present position of the bar buoy. It is safest because it affords the shortest and most direct route out to the sea, and enables a vessel generally to take the swell head on, or nearly so. At one time, when the buoy was in the above position, there were 27 feet at low water across the bar, which would have been inconvenient to have plotted the lines as we ran, on account of the lack of a reliable recorder and proper drafting facilities, etc., on board the cutter. I did most of the recording myself, and Mr. Polhemus and Morton L. Tower acted as sextants during this survey. The leading was done by the deckhand of the cutter, who proved quite competent.

Very respectfully, your obedient servant,

J. R. SAVAGE.

Capt. T. W. SYMONS,
Corps of Engineers, U. S. A.

tion of the jetty tramway had been extended nearly 1,600 feet from wharf. Only four of the geared dump cars had been received from

last fiscal year operations at Coos Bay consisted chiefly in continuing work begun with the \$125,000 appropriated September 19, 1890, of which allowance was something over \$56,000.

Construction of the north jetty under the general plans approved by the United States Engineers October 27, 1889. From 30 to 40 men were employed on the jetty as completed consists of a ridge of pieces of rock dumped on a mattress of brush fascines 20 feet wide. Its crest is brought up to tide level, and the enrockment takes such slopes as the surges naturally assume about 2 to 1. The mattress is laid and the rock dumped from a pile-trestle tramway, constructed in advance by an overhang revolving on a trestle. In July the last of the four large scows for transporting the rock was completed. In the same month we finished the gridiron for brush, 50 feet by 12 feet, and a coal bunker 12 feet by 30 feet. Also a trestle spur track long to accommodate cars not in use. Finding our car shop not large enough for the work, we extended it into the locomotive house, and erected another building 20 feet by 30 feet.

Jetty tramway.—As before stated, at the commencement of the fiscal year had reached a point about 1,600 feet from the receiving wharf. It is a double track, 3-foot gauge railway, 13 feet from center to center resting on longitudinal stringers of fir 12 by 16 inches by 32 feet, supported by pile bents 16 feet apart, capped with fir timbers 12 by 12 inches by 3 by 12 inch planks are placed between the tracks for footwalks.

The elevation of the top of rail is 21½ feet above mean low water.

The tramway was extended by means of the large jetty pile-driver with its 10 by 10 inch Worthington steam pump and water jets. It is the same driver with the water jet part of the jetty tramway at the mouth of the Columbia River. A good general description of the machine and its working is given in the report to Maj. Handbury on page 3021, Report of Chief of Engineers that our machine has only single trucks of a somewhat different design from the one used at Coos Bay.

The piles were of fir from 45 to 56 feet long and about 17 inches in diameter and cost 4½ cents per foot. They were pumped into the sand from 20 to 27 feet by means of hydraulic jets were supplied by means of a 10 by 6 by 10 inch Worthington steam pump, with hose connection attached to two 1½-inch pipes temporarily attached to each pile with staples.

The piles were drawn up when the pile top had reached a point within a few feet of the required elevation, and it was then driven to grade by a few blows of the driver thus obviating the necessity of sawing it off.

Tracks, timbers, and rails were run out to the driver from the storage platforms at the wharf. Up to the present time we have not found it necessary to use the trestle, but in future this will be required.

The crew, including those employed preparing and loading piles and driving them, cost eleven men with daily wages amounting to \$29.50.

The average daily extension was four bents, or 64 feet of tramway. The greatest day's work was five bents, and during the month a total tramway extension of 1,280 feet was accomplished, but it did not work continuously on account of the weather.

The average cost for one 16-foot bent of tramway, being the average of the season,

1. Lumber at \$9.50.....	\$13.15
200 feet at 5 cents.....	10.00
Steel rail at \$50 per ton.....	16.00
.....	85
Road spikes at 3.6 cents.....	40
Hot bolts at 3 cents.....	1.80
(for season).....	9.83
<hr/>	
Cost one bent.....	*52.03
Per foot.....	3.25

The first 1,600 feet of tramway is located above extreme high water, the next 1,392 feet is between extreme high water and ordinary low water. Immediately after passing low-water mark, a small swash channel was crossed wide and from 6 to 8 feet deep at low tide. Beyond this channel on the

* No allowance is made for first cost of plant.

worked on the scows and four on the wharf, including engineer of hoist-

new loads of the poorer rock were dumped on the side mattresses on the side of-tranway for 592 feet between bents 66 and 103 to prevent the encroachment of the bay channel at this place.

Active operations.—Active operations closed soon after the delivery of stone want of sufficient funds to continue purchase, about the middle of April. Stone scows were moored alongside the boom at Yarrow, away from the in- the teredo, and the sheathing taken from their decks. The pile-driver on the turn-out prepared for it, the locomotive and hoisting engines and oiled, and the smaller tools and appliances put away.

Reclamation of sand dunes.—With a view to the future work of reclaiming the the north spit, to prevent the winds from carrying the sand into the bay, ornamental work was done to ascertain the most suitable plant growths and advisable time of the year for planting.

sacks of Holland grass roots were obtained from the Golden Gate Park smers, of San Francisco, and set out in the sands here April 30, over a year planted about 3 acres with it, but only one quarter of it lived; but I notice it survived has commenced to spread and grow.

ing we received 100 sacks of the same kind of grass roots, and set it out month earlier, planting 5 or 6 acres. We set the small bunches of roots inches deep in the sand and 2 or 3 feet apart. Up to date nearly every bunch to be growing well, and I believe it is the most suitable growth to start this th.

all we gathered about 10 bushels of seeds of plants native to the spit, and 30 of Marine pine and spruce cones, and shook out and planted the seeds this We also planted 50 bushels of oats, 25 bushels of barley, and 5 bushels of grass seed.

storms continued late this spring, and I fear most of the oats and barley is d.

so planted a sack of Scotch broom and other seeds, and transplanted 400 rine pine and spruce trees, and set out a number of willow cuttings.

posed spit is a very difficult place to get any growth started, and from experience I think the Holland grass is the best to begin with, supple- with seeds of native plants and Marine pine.

probably be necessary to cover the higher dunes with a protection of brush plant life gets started. We built a barb wire fence one-third of a mile on the spit from bay to ocean to prevent cattle from straying down and in- ; with the grass plantations.

r.—According to our contract with Mr. Baines, we were entitled to the use at when she was not employed in towing the rock scows. Although we a work out of the boat in this way, we were obliged to hire considerable wing to transport the brush, poles, piles, coal, etc. We employed the local r this purpose at an agreed price per trip. We often experienced delays and is method inconvenient, and I would recommend for another appropriation ter or purchase of a good-sized steam launch to tend on the works, tow small id piles, and deliver supplies.

supply.—The large and shallow well in the sands of the north spit furnished a supply of fresh water at the works. It was pumped into an elevated tank foot Aeromotor windmill.

—We received from the Government works at Fort Stevens 20 geared dump ich came knocked down for ease in shipment. These we assembled at the

ive on hand now the following plant, all in good order: Four large stone escribed in last Annual Report; one small scow 40 by 14 by 4; one 11-ton truck Baldwin locomotive; twenty geared dump cars (Fort Stevens pattern); cars; two mattress cars; four push cars; one jetty pile-driver; one double- 10 by 12 Lidgerwood hoisting engine; one small hoisting engine; an outfit boats, oars, pumps, blacksmith tools, saws, sledges, axes, hose, stoves, and all tools and appliances.

r accomplished and changes effected to date.—Immediately after the extension of h jetty across the swash channel the sands on the sea side began to accumu- have built up above low-water level nearly to its end. At the same time emity of the north spit commenced to cut away along its southern and east- , widening and straightening the channel. In October the depth on the bar rapidly improve.

ase for the year previous the least depth at mean low water in the bar chan- been about 10 or 11 feet, as soon as the jetty was extended to its present h attained a depth varying from 16 to 18 feet at the same stage of tide, which maintained ever since for a period of eight months.

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A survey of the bar was made last March, wh
 Housatonic.—While the work was in prog
 views was taken by Mr. Bratt, illustrating the
 A careful record of the commercial statistics f
 for the past fiscal year and forwarded to you m
 A survey of quarry sites on Coos River was fi
 of answer is now being built.

Very respectfully, your obedient servant,

Capt. T. W. SYMONS,
 Corps of Engineers, U. S. A.

COMMERCIAL STAT

Coos Bay is in the collection district of sou
 bay, in the port of entry. The nearest light-h
 side and west of the entrance.

The following returns are furnished by Mr. J.
 charge of works at Coos Bay, and are for the fi

Arrivals and departures

Arrivals.....
 Departures.....
 Total.....

Vessels built.

Schooner *Galveston*.....
 Schooner *Niagara De Paz*.....
 Steam launch *Fawn*.....
 Schooner *Boston* (rebuilt from old tug *Columbia*)
 Number of passengers arrived by sea.....
 Number of passengers departed by sea.....

No new routes of travel have been established
 report.

Article	Quantity	
<i>Exports</i>		
Lumber.....	1000	Butter
S. M.	40,500,727	Hides
S. M.	42,105	Dress
S. M.	3,414,509	Mixed
S. M.	1,457	Pickets
S. M.	11	Barrel
S. M.	11	Barrel
S. M.	2,126,424	Spars
S. M.	197	Machin
S. M.	36,155	Barrel
S. M.	2,474	Chicks
S. M.	2,309	Red wa
S. M.	20	Specie
S. M.	142	
S. M.	36	Ta
S. M.	27.5	
S. M.	22	
S. M.	74.5	General
S. M.	4	exp. 3
S. M.	235.9	Specie
S. M.	6.1	
S. M.	1,050	Ta

Estimated value:
 Exports.....
 Exports.....

TT 3.

IMPROVEMENT OF UMPQUA RIVER, OREGON.

description of original condition.—Just below Scottsburg, the head navigation on the Umpqua River, are five sandstone bars or ledges 15 feet wide and submerged from 1 foot to 2 feet at low tide on river stage. They are separated by pools about 150 feet wide from 5 to 10 feet deep at low water.

mode of improvement.—The honorable Secretary of War having authorized the expenditure of the balance remaining of the appropriation March 3, 1871, viz, \$4,685.89 in improving the river below Scottsburg a project was submitted and approved in 1885 for making at an estimated cost of \$4,056, a channel 50 feet wide and 3 feet deep at low water through the rock ledges above described, by drilling and blasting the rock and removing the broken pieces to deeper water in the

September, 1889, a survey was made of the Umpqua River from Scottsburg to its mouth, and a project prepared for its improvement was submitted January 16, 1890. This project provides for the removal of rock boulders and ledges in the wharf basin at Scottsburg, below this basin with the view of opening a channel way 50 feet wide and 4 feet in depth at low water. The project was approved on the date of October 28, 1890.

The estimated cost of completing the project was \$9,000.

APPROPRIATIONS.

March 3, 1871.....	\$22,500
August 11, 1888.....	2,000
September 19, 1890.....	9,000
Total.....	33,500

Amount expended to June 30, 1891.—The amount expended on the project to June 30, 1891, was \$6,992.42.

Results obtained to June 30, 1891.—The result obtained was a navigable channel 50 feet wide and 2 feet deep at low water, extending nearly through the reefs below Scottsburg, where formerly there was only a bar less in depth.

Amount expended during fiscal year ending June 30, 1892.—The amount expended during fiscal year ending June 30, 1892, was \$7,252.74.

Results obtained to June 30, 1892.—The project was nearly completed. While it could be entirely completed, however, the water in the river had become muddy, so that the finishing touches could not be made. There are in consequence some points of rock which will have to be removed before the project can be considered as fully completed.

Mode of operations.—Mr. C. M. Carlson was placed in charge of the work of rock removal on the Umpqua. This work commenced in July, 1891, and continued until November, 1891.

During this time, 955 tons of rock were blasted, hoisted, and loaded on scow and removed to a convenient locality where it could be used without injury to navigation.

The holes were drilled by means of an Ingersoll steam drill. They were loaded with dynamite and exploded by fuse and sympathetic detonation.

A rock broken was raised by means of a diver and a steam hoisting engine and dumped by hand.

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Conclusions and remarks.—It is believed that sufficient money has been appropriated to complete the project, and therefore no recommendation for further appropriation is made.

Future operations.—As soon as practicable the work of completing the rock removal from the selected channel will be commenced.

Money statement.

July 1, 1891, balance unexpended	\$8.63
Amount refunded February 9, 1892	
	8.63
June 30, 1892, amount expended during fiscal year	7.25
July 1, 1892, balance unexpended	1.44

COMMERCIAL STATISTICS.

Umpqua River is in the collection district of Southern Oregon. Empire City Coast Bay, is the nearest port of entry. The nearest light-house is at Cape At entrance to Coos Bay, distance 22 miles.

The following returns, furnished by Mr. O. B. Hinsdale, of Gardiner, Oregon, for the year ending June 30, 1892:

VESSELS ARRIVED AND CLEARED.

The number of vessels of all descriptions crossing in over the Umpqua River during the year was 16; the number crossing out was 16. Their names, registered tonnage, and draft, are as follows:

Name.	Trips.	Registered tonnage.	Draft.
Boat No. 1, S. Leeds	8	217.70	1
Boat No. 2, S. Leeds	8	197.22	
Boat No. 3, S. Leeds	11	135.40	
Boat No. 4, S. Leeds	1	235.56	
Boat No. 5, S. Leeds	6	71.17	
Boat No. 6, S. Leeds	1	70.00	
S. S. S. M. M. M.	1	451.84	
Boat No. 7, S. Leeds	5	295.00	
Boat No. 8, S. Leeds	8	294.00	
Boat No. 9, S. Leeds	4	339.14	
Boat No. 10, S. Leeds	1	368.48	
Boat No. 11, S. Leeds	1	343.69	
Boat No. 12, S. Leeds	1	135.40	
Boat No. 13, S. Leeds	20	52.25	
Boat No. 14, S. Leeds	20		
Boat No. 15, S. Leeds	2	53.38	

There were no vessels built here during the year, and no new routes of travel established.

River traffic.

Steamer (propeller), 22.28 registered tonnage:	
Miles run during year	8
Passengers carried during year	2
Freight carried during year	tons..
Steamer (sloop, stern-wheeler), 101.02 registered tonnage:	
Miles run during the year	5
Passengers carried during the year	1
Freight carried during the year	tons.. 1

Articles.	Quantity.	Value.	Articles.	Quantity.	Value.
<i>Exports.</i>					
B. M.	12, 142, 000	Hides and furs.....tons..	3	750
.....tons..	24, 284	\$194, 272	Grain.....do.....	234	5, 850
.....do.....	733	7, 335	Hops.....do.....	3	1, 050
.....do.....	2	1, 000	<i>Imports.</i>		
.....do.....	16	6, 409	General misc.....tons..	1, 880	150, 000
.....do.....	5	3, 000			
.....do.....	5	100			

ports (tons, 25,285½) \$219, 757
 ports (tons, 1,880) 150, 000
 not exported, as cannery did not run this year.

T T 4.

IMPROVEMENT OF MOUTH OF SIUSLAW RIVER, OREGON.

ption of original condition.—The Siuslaw River enters the ocean midst of a vast shifting sandy beach, without any headland or point to determine or aid in determining the location of the entrance channel.

The unconfined channel has a range of about 1 mile, over which it is in making connection with the ocean. In consequence of this spreading and spreading out of the outgoing and incoming waters, the shoals shoal very badly, while at other times it has a depth equal to the controlling depth inside the entrance. The depth on the bar is from 5 to 12 feet at low water, and the bar channel changes very much in position and direction.

When a channel is developed inside the bar which runs nearly parallel with the coast. This channel is narrow, badly defined, and is, as boats have to pass through it in the trough of the sea. At other times there are two channels.

Plan of improvement.—The plan of improvement for the entrance to the river is to confine the outgoing and incoming waters between two stone jetties, so located as to direct the currents upon the river in a direction practically perpendicular to the coast, these jetties converge till they are 600 feet apart at the crest of the bar. The location of the jetties is approximately shown upon the map published in the last annual report from this office. (Report of Chief of Engineers, page 3178.)

The north jetty leaves the mainland about half a mile north of Cannon Beach and is 4,500 feet long. The south jetty extends from the end of the rocky peninsula and is 3,200 feet long.

The wharf for the north jetty is located just below Cannery Hill, and the length of the wharf to the jetty is 3,000 feet long.

The wharf for the south jetty is opposite the north jetty wharf, and the length of the wharf is 2,400 feet long.

The jetties are to be built of brush and stone from a pile tramway.

APPROPRIATION.

September 19, 1890 \$50, 000

Amount expended to June 30, 1891.—The amount expended during the last year ending June 30, 1891, was \$601.96.

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Results obtained to June 30, 1891.—No work was obtained.

Amount expended during fiscal year ending June 30

Results obtained to June 30, 1892.—The work preliminary character and nothing has been done of any effect on the entrance to the river, and no results have been obtained.

Report of operations.—The report of the Board is filed in my last annual report (page 3175, Report 1891), was approved August 4, 1891, by the Chief of Engineers. The following is the endorsement on the report:

[First endorsement.]

OFFICE CHIEF

Respectfully submitted to the Secretary of War.

By endorsement of the 19th February last, the Acting Surgeon-General recommended that the work be held to await further appropriation by Congress.

By authority of the Secretary of War a Board of Engineers was organized on the 20th of February, 1891, and the report of this board is filed in my last annual report (page 3175, Report 1891), was approved August 4, 1891, by the Chief of Engineers. The following language is in reference to beginning the work:

"It would seem to be imperatively necessary, in order to commence the work, that it be not commenced until an amount of money has been appropriated which will cover about three-fourths of the total cost of that jetty. It would seem to be inadvisable to make so large an amount as this in one sum, or to await its accumulation by small appropriations. A plan of operations might be adopted by which these smaller amounts of money might be applied to the channel over the bar."

The change suggested would be to provide the necessary amount of money for the construction of a dike on the north side, at the northern end of the sandy peninsula, and thus forming a west bank for the river which will keep the water from passing around the north end. The result of this is to prevent the water from breaking through and flowing over the bar. No permanent relief, however, can be expected until the dike and the water across the bar are constructed.

It is recommended that the change of plan suggested by the procurement of the necessary plant and appliances for the construction of a dike on the sandy peninsula, be authorized, and that the work be commenced in this direction.

Major, Corps of Engineers

This was approved the same day by the Secretary of War. The project under which work is to be carried out immediately upon receipt of the requisite authority was approved and the work began.

On account of his experience at Yaquina Bay, Oregon, a lieutenant engineer, was designated to take charge of the work at Siuslaw.

On September 5, 1891, Mr. Lyell arrived at the mouth of the river to commence operations.

The land necessary for a wharf, storehouses, the tramway approach to the north jetty was leased to the Government for a term of years sufficient to complete the work for \$100 per year.

The end of the sandy peninsula from which the south jetty must start, and upon which must be located the wharf, storage platforms, etc., for the south jetty, was purchased from Mr. S. J. Beswick for \$5 per acre.

It was necessary to build a floating pile driver in order to construct the wharf at the mouth of the river. A locomotive was purchased from H. K. Porter & Co., of Pittsburg, Pa.

Cars of the Yaquina Bay type were procured from New Jersey, and two hoisting engines, steel rails, and the multifarious tools and materials needed in the prosecution of the work were procured.

Two rock scows have been built, and the work is fairly under way.

The wharf under Cannery Hill has been completed.

During September, 1891, a new survey of the entrance to the Siuslaw was made by Mr. John R. Savage, assistant engineer. This was forwarded to the Department with the following letter advocating a change in the project as approved by the Secretary of War:

PORTLAND, OREGON, *December 28, 1891.*

GENERAL: I have the honor to send herewith a blue print of the survey of the Siuslaw River entrance made in September, 1891.

Upon this I have marked, in full red lines, the project for the improvement of the entrance recommended by the Board of Engineers, consisting of Maj. Handbury, Lieut. Burr, and myself.

The position of the main-entrance channel varies naturally in location from the end of the south spit (so marked) to its present position 1 mile to the northward.

In its report, after recommending the project above named, the Board used the following language:

"It would seem to be imperatively necessary, in order that it may be efficiently and economically done, that it be not commenced until an amount sufficient to build the north jetty to its full length up to at least low water be made available. This involves about three-fourths of the total cost of that jetty, or \$300,000. Should it be found inexpedient to make so large an amount as this available for the work in one sum, or to await its accumulation by small appropriations, a change in the plan of operations might be adopted by which these smaller amounts could be expended with advantage perhaps to the channel over the bar.

"The change suggested would be to provide the necessary plant and appliances, and then commence the construction of a dike on the sand spit near the mouth of the river, at the northern end of the sandy peninsula, and extend this to the north, thus forming a west bank for the river which will keep the ebb water from spilling over until it passes around to the north end. The result of this will be to hold the channel to the northward and prevent the breaking through and formation of a south channel. No permanent relief, however, can be expected until the two jetties that are to direct the water across the bar are constructed."

This language was the result of a compromise on the part of the members of the Board. The work, as approved, is shown by the dotted line on the blue print.

The recent survey, made in September last, the results of which are incorporated on the blue print sent herewith, indicates very strongly that the work outlined and approved will be of scarcely any advantage, and give no permanent relief, and that it will form no part of the best plan for the improvement of the river entrance.

The work is so contrary to my sense of what is proper to be done that I feel it to be my duty to protest against it.

The building of the jetty on the dotted line will simply be extending the sand spit to the north and changing the locus of the channel variations to the north. It will do practically no good to the outer channel and bar. If it did, the benefit must of necessity be temporary. I can not but regard it as a waste of money.

Upon the chart can be noticed the southerly trend of the outer channel after it gets by the harbor throat, if it can be so called. This direction compels boats, desirous of entering or leaving, to run in the trough of the sea in a narrow channel only 5 feet in depth at low water, and makes the entrance a fearfully risky one. Any extension of the jetty on the dotted line would not materially better this direction.

If any benefit to the entrance to this river is to result from improvement works, this benefit will arise from doing away with the trough channel and making the entrance channel run directly out to sea, rather than from any increased depth which must remain slight and uncertain.

It is also more than likely that before work could be fairly started, the main channel will have shifted to the southern position, now indicated as a secondary channel.

enced by building the west or south jetty. It will serve as a directrix to the currents and it alone must have some influence in determining the new proposed channel. This influence will be aided by the partial obstruction of the north channel already proposed.

If, during the period of construction, the new channel shall be developed, then it would seem to be good policy to suspend work on the west or south jetty and begin on the north jetty, with a view to holding the new channel in place. It will at this time be expedient to entirely close the channel supposed to have been already partially obstructed.

The lines of the adopted project suggest remark:

The distance between these lines is as much as 1,600 feet. The river maintains a normal depth of about 12 feet where the width is 900 to 1,000 feet.

It is not understood how an adequate channel can be developed and maintained between jetties spaced farther apart than about 900 feet. It is suggested that the alignment of the jetties be reconsidered.

If this be done, it is thought that a modification will be advantageous in the way of softening the right angled change of direction which is observed at the head of the jetties.

The inclosed tracing shows an alternative alignment which eases the curvature by making the bend longer, and by shifting the entrance several hundred feet to the northward.

This alignment is proposed in the way of illustration. It may be varied to suit any local conditions or requirements not mentioned in the report of the Board.

The channel may be trained, if necessary, to a needful distance from the concave face by a few short groins or by paving with brush mattresses.

Reference may be made to indorsement of January 15, 1891, upon a project submitted by Capt. Symons.

G. H. MENDELL,
Colonel, Corps of Engineers, Division Engineer.

[Third indorsement.]

OFFICE CHIEF OF ENGINEERS,
U. S. ARMY,
February 7, 1892.

Respectfully returned to Capt. Symons for remark, attention being called to the second indorsement.

To be returned through division engineer.

By command of Brig. Gen. Casey:

H. M. ADAMS,
Major, Corps of Engineers.

[Fourth indorsement.]

U. S. ENGINEER OFFICE,
Portland, Oregon, February 8, 1892.

Respectfully returned to the Chief of Engineers, U. S. Army, through Col. G. H. Mendell, Division Engineer.

I am in substantial accord with Col. Mendell as shown in the preceding indorsement.

In the Board's project the crossing of the channel by the north jetty was determined in location principally on account of the soft sandstone which there juts out from the main shore and upon which a stone jetty could be founded with more surety than upon the sands elsewhere.

As this turn is a critical point with the jetty, it was deemed best to take advantage of this hard bottom:

In order to utilize and take advantage of this hard bottom, it is suggested that the adopted north jetty start from the main shore at the Board's location and end at Col. Mendell's location. This would tend to somewhat ease the curve.

This, with the south jetty location suggested by Col. Mendell, would be entirely acceptable to me.

I inclose a second blue print upon which I have laid down the lines as above outlined. I believe it will be advisable to leave an increased width between the jetties at the turn. Further contraction here seems unnecessary as natural causes will cause a deep channel to exist along the concave side of the north jetty, and if this channel be narrowed, there might be an undue tendency to undermine the jetty.

As regards the sequence of the work, I would recommend that the tramway to the north jetty along the main shore be first built and the jetty extended out into the river, but stopped at such a point as to leave in the present channel ample room for

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the passage of boats. Then, in the open part of the channel, to put down the timbers suggested by Col. Mendell.

The north jetty so built would be very valuable in handling these mattresses. being done, work to be stopped, and the south jetty commenced.

With the north jetty in the state outlined we would be prepared to take advantage of the breaking through of the south channel at any time, to push the sand and hold the channel in its southern position.

In this I believe I am substantially in accord with Col. Mendell's views.

THOS. W. SYMONS,
Captain, Corps of Engineers.

[Fifth indorsement.]

U. S. ENGINEER OFFICE,
San Francisco, Cal., February 12, 1892.

Respectfully returned to the Chief of Engineers, U. S. Army.

The existence of the ledge of sandstone not, however, mentioned in the B report, is a good reason for the location of the north jetty proposed by the B report, and this location is for this reason recommended.

The order of work proposed, fourth indorsement, is recommended.

I am not at present prepared to recommend the location shown on blue of the south jetty, the interval between the two jetties at some points appears rather great. The work recommended for north jetty will, it is supposed, be as can be done with the present appropriation, and, if the Department shall so direct, it is suggested that the final location of the south jetty be deferred for consideration at a future time.

G. H. MENDELL,
Colonel, Corps of Engineers, Division Engineer.

[Sixth indorsement.]

OFFICE CHIEF OF ENGINEERS,
U. S. ARMY,
February 13, 1892.

Respectfully returned approved as recommended by division engineer in indorsement.

When such record as may be necessary has been made, this paper will be returned to the Chief of Engineers.

By command of Brig. Gen. Casey:

H. M. ADAMS,
Major, Corps of Engineers.

[Seventh indorsement.]

U. S. ENGINEER'S OFFICE,
San Francisco, Cal., February 13, 1892.

Respectfully transmitted to Capt. Thomas W. Symons, Corps of Engineers, A. Portland, Oregon, attention invited to preceding indorsement of Chief of Engineers.

G. H. MENDELL,
Colonel, Corps of Engineers, Division Engineer.

These indorsements convey an order changing the project for the main project of the Board of Engineers, and upon this project work is in progress.

Early in 1892, the south channel opened, and since has been the principal channel used by boats entering and leaving the river.

Recommendations and remarks.—The south channel having opened through and become the best channel, all questions, for the present, concerning the stoppage of the north channel to navigation, the building of the north jetty across the river can be left out of consideration.

If the north jetty is once completed across the river, forming an artificial headland to turn the waters into the south channel, it

that this main entrance channel will be maintained generally in its original position, although until the completion of the jetties it undoubtedly wanders about more or less.

The general methods of procedure in the construction of the north jetty are applicable, depending upon the amount of money available:

1. If there should be a sufficient amount of money available, this jetty could be built out for its full length as rapidly as possible, the channel being brought up to at least low-water level, and to subsequently have rock added until it reaches full high-water level.

2. To carry out the project by this method a sum of \$250,000 can be profitably expended during the next fiscal year.

3. The second method of procedure is to push the jetty toward the sea, by bringing it as it progresses by bringing the enrockment up to full high-water level.

4. To carry out the project by this method a sum of \$50,000 can be profitably expended during the next fiscal year.

It is probable this will have to be the method adopted, \$50,000 is the amount specified in the money statement that can be profitably expended.

Future operations.—Future operations will consist in the extension of the north jetty to as great an extent as possible with the money available.

The report of Assistant Engineer G. A. Lyell, which accompanies this report, gives in more full detail the operations and history of the project.

Money statement.

1891, balance unexpended	\$49,398.04
1892, amount expended during fiscal year.....	29,816.90
	19,581.14
1892, balance unexpended.....	19,581.14
1892, outstanding liabilities.....	2,300.00
	17,281.14
1892, balance available	17,281.14
Appropriated by act approved July 13, 1892	20,000.00
	37,281.14
Amount (estimated) required for completion of existing project.....	630,000.00
Amount that can be profitably expended in fiscal year ending June 30, 1893	50,000.00
Amount expended in compliance with requirements of sections 2 of river and harbor acts of 1866 and 1867.....	50,000.00

REPORT OF MR. G. A. LYELL, ASSISTANT ENGINEER.


UNITED STATES ASSISTANT ENGINEER'S OFFICE, Florence, Lane County, Oregon, June 14, 1892.

SIR: I have the honor to submit the following report of operations on the improvement of the entrance to Siuslaw River, Oregon, for the year ending June 30,

under the act of Congress passed September 19, 1890, \$50,000 was appropriated for the improvement of the entrance to Siuslaw River, Oregon, for the year ending June 30, 1892, and a Board of Engineer Officers appointed April 15, 1891, by Order No. 23, Headquarters Corps of Engineers, to formulate and submit a report for the improvement.

In compliance with instructions contained in your letter of August 5, 1891, directing me to proceed to the Siuslaw to start the work, I turned over the works at Yaquina Bay to Mr. Ricksecker, and then proceeded to Portland to gather data in connection with the project for improving Siuslaw River, and make estimates of the amount of money requisite for construction of plant for carrying on the work.

On September 5, 1891, following your letter of instructions, I arrived at Florence, Oregon, and proceeded to find quarters suitable for an office, and succeeded in ob-



most of them come down the river and are floated by high t river. Some of them float as far up as Acme, about 7 miles, and a structure of any kind built near the mouth of the river must to resist the constant hammering and pounding of these large li

The best location for the tramway leading out to the propose along the beach, between the high and low water line, as to foll of the bluff will prove very expensive on account of the large that lie buried under the sand, and will have to be removed piles that support the tramway. The beach, for the most ps sandstone of a soft nature, but apparently not subject to sco masses of rock can be placed without making an expensive m support the rock.

When once the tramway is constructed to the root of the nor rapped, sand will soon fill in behind the tramway and prevent t again.

At the time of my arrival quite a channel had developed at th current, and this channel was quite wide on the inside; the were found to have encroached to a very great extent on the riv Hill and signal station "Gulch." Quite an elevation had been f ground since the survey of 1889, and quite an area of drift lo The deepest water, about 7 feet, was found in the south chan close to the middle sands; this at low water. The trend of th decidedly to the southward, just as the north channel has al direction; and my observations lead me to the belief that the l south along this part of the coast.

The prevailing winds are from the northwest, as evideced sand dunes are higher to the south than north of the entrance, a clination on their south than on their north sides. The trees a southeast and have fewer limbs on their north than on their so

If the works of improvement are ever completed I believe th will fill in entirely with sand from the northwest and will work along the north jetty to its top, just as the sands have done a north side of the north jetty.

On the 3d of last December a furious gale set in from the we tained a high velocity and lasted for several hours; trees were up of sand blew all the way across the river from the sand dunes and a very high tide occurred, being about 4 feet above the a immense drift logs. Considerable shoaling occurred all along river to its mouth and choked up very perceptibly the inco shoaled the bar at the north channel. At one time the sand du so low that a vessel could be seen at sea, standing on the lowe

No damage was sustained by the storm.

The past spring was very stormy and much rain fell.

So much has the south channel worked to the south that on the 8th of small spit had formed just inside the south channel, having been washed off of the south spit.

urf has laid bare all the rocky beach in the river opposite the south channel. has been formed just north of the wharf which holds the sand in suspension, ch sand has been deposited at the north side of the wharf, making quite a acc. This is helped to a great extent by the projecting nature of the river in the vicinity of Cannery Hill, which deflects the current toward the south t as the river widens just below, the force of the deflected current is lost by ng so much, and the river shoals from 20 feet opposite Cannery Hill to 9 feet opposite shore and at the wider part of the river.

nap of the 1889 survey, and that of the 1891 survey, show very nearly the nfiguration of the bed of the river just below Cannery Hill, i. e., deep water east side, and shoaling up in a northwesterly direction toward the west bank. to me these conditions ought to govern the distance of the jetties apart of their curves, and they should not be over 1,000 feet apart, converging to at their outer ends. These distances of the jetties apart would not create a velocity in the ebb and flood currents than now exists at the narrow por- the river above Cannery Hill, where the banks of the river are comparatively

ght be urged that the distance of 1,000 feet between the jetties on the inside end to throw the current against the north jetty, and undermine it; but, as re stated, the point at Cannery Hill deflects the current toward the west hich breaks its force below this point. The strongest currents are developed ast two hours of the ebb; the flood-tide currents are not so strong.

iver bed in the vicinity of the proposed jetties is, for the most part, sand- f a soft nature, not however liable to be cut out by the currents. The map 889 survey shows the north channel at its best when fully developed, as it en the survey was made.

orth or right bank follows the direction of a curve with a radius approxi- 3,000 feet, and very regular. I observed, at the time the 1889 survey was trong currents on the ebb, but the right bank was so uniform (fine sand) was scarcely disturbed to any great extent before the tide would change to nd counteract any tendency to much scouring. A study of these conditions the guide to a very great extent for the final location, degree of curvature, the jetties.

7.—On the 9th of September the steamer *Gen. H. G. Wright* arrived at Flor- ith Mr. J. R. Savage in charge, to make a survey of the bar at the entrance river; having completed the survey, the steamer departed on the 20th for y.

tions.—Owing to infrequent trips made by the steamer *George H. Chance* to e, much time was lost in getting the work started, and this has caused many ptions in carrying on the work since its commencement.

l house about 14 by 16 feet was built of rough boards near the cannery at e. This house has been used for storage purposes, and served well as a place h to spin oakum for calking, and also for light jobbing work.

ill landing place for receiving lumber for construction of pile-driver scow o built in front of the tool house.

ch ways were laid on the flat just west of the cannery, upon which the pile- scow was built.

ing pile-driver.—On the 7th of October work was commenced on a derrick for ting driver, and on the 9th the steamer *Chance* arrived, bringing materials for k.

cow is built of sound fir lumber; length, 52 feet 8 inches; width, 22 feet 8 depth of hold, 4 feet. Deck and bottom planking, 3 inches thick; gunwales, s thick; two longitudinal bulkheads, 6 inches thick, with three interme- ngitudinal trusses and three equidistant cross trusses all strongly braced, and bolted.

are three wooden box pumps; three stout cavils each side, and three hatches. rick with leaders 47 feet long is provided. This derrick has double sheaves head block, one for hammer rope and the other for pile-hoist. Five platforms vided upon which to handle the piles while being driven. A movable s provided at one end on the fourth platform upon which the hammer rests ot in use, and this block can be worked in and out of position by a small rope ; down the back brace nearly to the deck of the scow and obviates the ne- of sending a man aloft to chock the hammer. The leaders of the derrick are i their inner edges with $\frac{1}{4}$ by $2\frac{1}{4}$ inch strap iron to prevent abrasion by the r. The hammer is of cast iron and weighs 3,600 pounds, and has a total ea- ders of about 30 feet.

cow and derrick were completely finished and launched on the 28th of No-

was used to get up to assist the arrival of the hoisting engine when actual construction of the scow, as there was no shed available at work.

Ship.—On the 27th of November a large skiff was completed for use in connection with the floating pile-driver in his work, and has been used by the pile-driver crew in tow with timbers, also in towing piles and carrying other material down. It is provided with benches for six oars and has a cabin 4 feet. The boat has proved of valuable assistance to

Construction.—On the 28th of December the yard where the float will be constructed by the usual way, landing, etc., and everything was done to open Point about 1 mile below Florence, when started to utilize ground near (Line & Smith's new sawmill) which is constructed rock across, the ground being above high water in the scow, when arrangements had been made for the ground was cleared and a shed erected, under which to build the float 25 to 30 feet wide with rough boards a frame, and set apart in the ends and sides.

Two water a heavy pole of wood hlew off a portion of one side of the float.

A trail was laid down about 20 feet from the shed to the saw float, and was placed open it for transferring the scow it and the ground a series of landing the heavy timbers.

Structures.—The logs and scows have been built. They are 12 feet wide at deck, depth of hold, 5 feet. Gunwales are 6 in.

There are three longitudinal bulkheads 6 inches thick, an iron cross. Bottom and deck planking is 3 inches thick. It is covered with galvanized iron boat spikes and painted with paint. The sides are painted with two coats red metallic paint has been found to stand the action of salt water better than iron and copper.

The deck was given one coat of hot coal tar. These scows are provided with a gypsy winches, bits, and chocks are. These logs within scows are provided on each side. The hatchways, and have a capacity of about 200 tons on a 41-foot

Logs and brush house.—On the 28th of December last a shed was erected on the ground of logs and brush preparatory to house. This house is located at the base of Cannery Hill, a scow was placed under it. The floors of each building are made of logs. The posts were well braced all around, as it built in the mouth of the river during storms.

Scow.—The scow frame is 25 by 22 feet inside, covered with galvanized iron, and is braced to brace the building. On the top level, a deck is being. The roof is covered with cedar shingle and is 10 feet high.

Posts are provided to accommodate 50 men. Four small rooms are provided for men. A public room is provided at one end.

House.—The new house is located 114 feet from the bu building. It has a kitchen and pantry on one side of way, and on the other side two dining rooms, one public and the kitchen along room has a small bedroom adjoining.

The house has a framed ventilator in the ridge of the roof for the purpose of January the steamer Chase brought in the hoisting pile-driver, and on the 9th it was placed aboard the scow, and setting up the machinery began.

The weather was stormy and little progress could be made with the pile-driver. On the 10th of January logs were received for furnishing the float, which at the mouth of the river, and a contract was made for the logs to be delivered at Florence.

On the 11th of January a raft of 100 piles was received, and on the 12th it was placed at the mouth of the river, where 8 piles were laid in the river opposite the old cannery, to serve as a bet during when not in use. The machinery worked very well, altho

Structure.—The receiving wharf is located with its front beam in a narrow hill with an approach 20 feet wide. Dimension length, 100 feet; width, 40 feet.

There are 3 longitudinal rows of 10 piles driven 10 feet apart, and 10 transverse rows of 10 piles driven 10 feet apart. Braces and

inches were placed over each transverse row of piles, and between the 8 by 12 inch joists were placed.

oring is composed of 2 by 12 inch planks spiked to stringers and joists.

ine was placed at the south end of the wharf for landing lumber and other

ing, etc.—Pile-driving for the wharf was attended with some difficulty on of the heavy swells from the ocean making it difficult to keep the pile-driver in; the crew, too, were untrained for such work. The river bed being rather ring was very slow work. The average penetration was about 8 feet through f sand, then into decomposed or soft sandstone, and there will be no danger scouring effect of the current. The front row of piles for the wharf was water 10 to 12 feet deep at low tide.

g engine for wharf.—The hoisting engine for wharf has been received and position on the wharf, but the boiler is not yet in position. This engine t at the works of J. S. Mundy & Co., Newark, N. J. Has two cylinders 9 hes; single drum; 50 horse power, and substantially built.

rick machinery for wharf has been received, but has not yet been set up. tire.—The locomotive "Siuslaw" has been received and stored on the wharf. motive was built at the works of H. K. Porter & Co., Pittsburg, Pa. It is ngine with 4 drivers; a saddle tank and trailing tender with 2 wheels. nt brakes, and altogether is very substantially built.

ars.—Irons for 12 dump cars have been received. These have been over- d given a coat of metallic paint. Not having the wharf ready upon which the cars, the irons were stored under the mess house. The cars will be o those in use at Yuquina Bay.

r for all the cars is now being prepared at one of the sawmills and they will as soon as practicable. They will have a capacity of 8 tons each, and with comotive on each jetty long trains of cars can readily be handled and the editiously done.

nd fastenings.—Thirty-four tons of 30-pound steel rails with the necessary frogs, and fastenings, have been received. They have all been given a coat stalic paint in boiled linseed oil, to protect them from the weather.

quarry.—The site selected for the rock quarry is located at Point Terrace e, about 12 miles above Florence. The rock crops out in bold pinnacles e river bank with very little vegetation growing on top of it, and can be arried. This rock is of a very fine grain, hard, and takes a polish, and suitable for all kinds of building purposes. Nearly 2½ acres have been in- survey and comprise the best part of the quarry. Water for engine pur- l for mess and bunk houses is easily accessible. The river at quarry site is o with steep banks.

R. Stiles is the owner of the quarry, and has consented to make a lease to nment for all rock needed on the works of improvement at 2½ cents per ton. ild carry with it also the privilege of erecting buildings, collecting water rious purposes of the work, and erecting a wharf and tank for the tow- c.

ire other good ledges of sandstone rock on the river, but none so well located ying as that at Point Terrace.

Cost of plant, etc.

.....	\$3,491.23
live	2,824.99
; pile-driver (complete)	2,921.32
oat	45.00
.....	81.00
cars (irons for).....	2,000.00
rs	250.00
for tramway driver.....	667.00
g engine	2,000.00
.....	1,615.00
(incomplete)	260.00
s, frogs, switches, and fastenings.....	2,250.00
ions (small tools, etc.).....	431.00
tal	18,836.54
mess house and bunk house.....	1,620.00
wharf approach, etc.....	1,760.00

most exclusively for shingles, but none are shipped. Some cedar lumber is manufactured and shipped.

Spruce and hemlock abound, but are not manufactured into lumber to any extent. **Coal** is said to exist in the hills to the southeast of Florence.

Rock has been found in the mountains back of Hecata Head indicating the presence of gold.

Clay of a fine quality abounds in the vicinity of Florence, but to what extent is not yet known. Samples have been fired and compare favorably with the manufactured article brought from California, and will make drain tiles, chimney flues, etc.

The run of salmon in the river is not as plentiful now as in former years when two or three canneries could be kept running each season, where only one runs at present with a very light catch. The product is shipped to San Francisco.

Shipping.—The following is a list of the steamers and sail vessels running to Florence during a portion of the year, with their tonnage:

	Tonnage.
Steam schooner <i>George H. Chance</i>	71.17
Steam schooner <i>Mischief</i>	48.43
Schooner <i>Free Trade</i>	86
Schooner <i>Albion</i>	70
Schooner <i>Amethyst</i>	71
Schooner <i>Helen Merriam</i>	70
Schooner <i>Mary De Leo</i>	50

The schooners were all from San Francisco, and brought merchandise, and returned loaded with lumber.

The small steamer *Lillian*, belonging to Florence, runs as a towboat on the river and tows the schooners to sea, but she is hardly large enough for the service. The trade does not as yet justify the employment of a regular tug.

The bar, with an average depth at present of about 7 feet at low water, does not justify the employment of large craft in the trade. The small schooners now running here bring loaded only 15 or 20 tons of merchandise, and therefore run very light. They can cross in at half tide, and the practice has been to station the steamer *Lillian* close down to the bar and let the vessels sail in to her; then they are taken in tow and brought up the river.

There are three small steamers running on the river; they are the *Lillian*, *Coos*, and *Mink*. The latter was engaged until recently in carrying the mails and passengers between Florence and the head of tide.

The *Coos* has now taken the place of the *Mink* on the route, as she is a side-wheeler and has better accommodations.

The question of building a railroad between Eugene and Florence has been agitated for sometime past, but no definite arrangements looking to its early commencement have as yet been effected. Such an expensive undertaking does not, at the present time, seem to offer inducements for the investment of capital. A good well-graded wagon road, built from the head of tide to Eugene, would suffice for the present needs of trade.

Should the bar ever be improved, Florence would become quite a shipping point, and a railroad built from here to Eugene and on to a connection with some eastern line would offer another competing line for carrying the products of the Willamette Valley and the country east of the Cascade Range to market.

On the 14th instant the steamer *Lillian* towed the schooner *Amethyst* to sea loaded with lumber for San Francisco. The bar was smooth and the master of the *Lillian* reports that he found 12 feet at high water with a 6½-foot tide; the inner channel showed 19 feet close to the bar, which is in the form of a ridge, with a steep slope outside running at once from 15 to 20 feet. It is to be regretted that a careful survey can not at the present time be made.

Northwest winds have prevailed of late with fog, but if an opportunity presents I will endeavor to run a few lines with the *Lillian* out over the bar.

Capt. S. R. Babbidge was connected with the work for several months as foreman, but recently resigned to accept a position at Newport. He proved of valuable assistance to me in getting the work started.

In compliance with regulations the men work 8 hours a day, but under emergencies, and to take advantage of the weather, tides, etc., this time is increased.

In conclusion I desire to acknowledge the prompt attention given by all employed

your obedient servant,

GWYNN A. LYELL,
Assistant Engineer.

COMMERCIAL STATISTICS.

Florence is in the collection district of Yaquina, and was settled in 1876. The commercial importance of Florence now depends solely upon the steamer *Chance*, which makes irregular trips, and the small coasting schooners which have been running to the place.

There is no line of steamers running direct to Florence at this time. No outlet exists to the valley except over a very rough mountain road almost impassable in winter, and down the beach via the Umpqua River, over both of which routes mails and passengers are carried.

A mail now goes twice a week to Waldport on Alsea Bay, and Newport on Yaquina Bay via Hecata and Cape Perpetua, formerly on horseback; now, however, it goes as far as Hecata over the new light-house road.

The following table has been compiled from the records of this office and is approximately complete for the past year.

Commercial statistics for past years are not attainable, other than those already published in reports relative to this place.

Shipping, 1891-'92.

	No.	Tons.
Arrivals.....	22	70
Departures.....	23	1,527
Total.....	45	1,597
Boats.....	1	20
Steamers and vessels running to Florence.....	9	40

Arrivals and departures of passengers by steamer, 36.

Articles.	Tons.	Articles.	Tons.
<i>Exports.</i>		<i>Imports—continued.</i>	
Saltine.....		Hay.....	10
Salt (740 barrels).....	22	Lime.....	16
Canned (2 1/2 cases).....	74	Cement.....	147
Spices, green.....	2	Flour and feed.....	5
Miscellaneous.....	3	Shingles (25,000).....	1
Lumber ft. and vol. (888,000 feet, P. M.).....	1,529	Boiler and fixtures.....	2
Total.....	1,629	Powder.....	14
		Tin plate.....	4
		Salt.....	3
		Pig tin, and lead.....	3
		Total.....	191
Miscellaneous merchandise, etc.....	345		
Steel rails, locomotive building material, heavy iron, etc., for port construction.....	160		

T T 5.

IMPROVEMENT OF ENTRANCE TO YAQUINA BAY, OREGON.

Description of original condition.—The usual prevailing depths over the bar of the bay, before improvement, were from 7 feet to 8 feet. There were three channels existed, known as the north, middle, and south channels. The north channel was the one mostly used, but was rendered impassable by the presence of rocks. The middle channel, though the shallowest, was usually the shallowest of the three, and so was little used. The south channel, besides being long and tortuous, was so shallow that it was considered un navigable. Owing to the shifting nature of the bar, these channels were constantly changing.

~~It is proposed to improve the bar by the construction of a breakwater across the mouth of the bay, and the removal of the rocks from the channels.~~

Plan of improvement.—The project adopted in 1881 was to run out a pier or jetty on the south side of the entrance so as to cause the south channel to shoal up and the flow to be deflected northward, with a view to opening and maintaining the central channel with a least depth of 17 feet at high water.

As originally projected the dike was to start from near low-water mark and run out into the sea a distance of 2,500 feet, and was to rise to a height above mean low water. The beach on the line of the jetty from high-water level was to be protected by sinking gabions in the beach with sand heaped over them, and by covering the surface with a layer of mattresses and stone.

The stone for the jetty was to be deposited in place from barges, and cribs were to be used if practicable. It was found, however, that cribs could not be used on account of the strong currents and high seas, and that the ocean swell, even in quiet weather, was too great to permit the use of stone barges. Accordingly the jetty had to be built from the shore end by carrying the stone out over a tramway, and was begun at high-tide line instead of at low-tide line, thus making the total projected length of the jetty in 1881, 3,700 feet.

In 1884, the jetty was extended shoreward a distance of 316 feet, to close a gap that had been washed out by the sea, making the total projected length at that time about 4,000 feet.

The present project, adopted in 1888, is to raise the south jetty to full high water without extending it seaward, thus making its total length 48 feet, and to construct a mid-tide jetty on the north side of the entrance about 2,300 feet in length, extending from the north head pier and behind the reef putting out from the head to a point opposite the end of the south jetty and distant from it about 1,000 feet.

APPROPRIATIONS.

of		
June 14, 1880	\$40,000	
March 3, 1881	10,000	
August 2, 1882	60,000	
July 5, 1884	50,000	
August 5, 1886	75,000	
August 11, 1888	150,000	
September 19, 1890	165,000	
Total.....		550,000

Amount expended to June 30, 1891, was \$451,239.57.

Results obtained to June 30, 1891.—The following extract from my last annual report gives the results which had been obtained to June 30, 1891:

The work during the past fiscal year has been chiefly confined to the extension of the north jetty. The tramway of this jetty has been extended 600 feet, and 30,327 cubic feet of rock have been placed upon this new extension and the old portion of the jetty.

Mr. Lyell, the assistant in charge, reports that there has been a general deepening of the bar throughout the year of 10 to 12 feet at low water.

The tendency of the channel to run straight out has been materially increased by the extension of the north jetty. The full results of the work done can not be realized until the north jetty is built out to an equal length with the south jetty.

Under the influence of the southerly winds, sand is moved and accumulates under the lee of the south jetty, and the ebb current not being strong enough to scour it away it is deflected to the north and finds its way to the ocean without doing much good on the bar. This accumulation of sand on the channel side of either jetty will, in all probability, be possible when the north jetty is completed.

The recent careful survey of the Yaquina entrance shows a low-water depth of 11 feet.

The results obtained by the work done during the year have been highly satisfactory. The extension of the north jetty prevented the formation of a sand bank under the lee of the jetty and the entrance channel has been continuously straightened.

The bar depth of 11 feet at low water reported last year has materially increased. During the past winter it was reported by the pilot in charge of the bar tug that the depth was 15 feet on the bar at low water. Recent soundings indicate even a greater depth than this, and it is believed that the channel now has a least depth of 16 to 18 feet.

It is intended to make a careful survey during the next year as the north jetty is extended to its full length to determine the conditions on the bar.

Report of operations.—Mr. Eugene Ricksecker assisted in the work at Yaquina Bay on August 18, 1891, and Mr. A. Lyell, who was placed in charge of the improvement of the mouth of the Siuslaw River.

The extension of the north jetty tramway was continued during the fiscal year until early in October, 1891, when on September 8, having torn away two completed bents and partially completed one, and further damage being threatened, it was recommended September 21 that work on the north jetty be temporarily suspended and not resumed until the weather should improve and that in the meanwhile operations be transferred to the south jetty. This was approved under date of October 1, 1891. The bents carried away had been replaced and the tramway extended farther, when operations ceased.

The amount of tramway built since March 28, 1891, is 1,035 feet, making a total length of 4,153 feet, of which 3,118 feet were constructed after June 30, 1891. The approach to the jetty is 2,100 feet in length.

The work of dumping rock on the north jetty was suspended on February 13, 1892. By this time 41,530 tons had been dumped on the jetty since February 1, 1891, and the enrockment was about 100 feet in length. As the rock composed of

m elevation of 19 feet to a point where the sand dunes meet the jetty miles from the new wharf. Storms during December were unusually severe and the south tramway suffered considerable damage in consequence. During February the tramway was further repaired for distance of 500 feet.

During the year, 12,519.51 tons of rock were placed on the jetty.

The lower quarry was abandoned February 17th, another slide having occurred there in January. The upper quarry was worked during the entire year. The removal of debris from the upper quarry proving troublesome, it was finally decided to use cars for carrying it away; 10 feet of track were laid. In February, the north side locomotive and cars were transferred to the quarry, and debris was dumped along the river bank on land purchased by authority for that purpose.

Two new scows were built during the winter at an approximate cost of the two of \$3,400. They were launched on November 27 and on January 1.

Much miscellaneous work was done during the year, including the overhauling and repairing of several scows; the repairing of the locomotive, cars, and rock boxes; the building and fitting out of a blacksmith shop, and a large amount of general repairing. The plant has been kept throughout in good condition.

Authority having been granted therefor, a naphtha launch for the use of the assistant in charge at Yaquina Bay was purchased at a cost of \$937.50. She arrived at Newport on February 11, and is of great service.

By agreement with Mr. Samuel Case, the lease of a strip of land occupied by the tramway leading out to the north jetty was renewed from September 17, 1891, for a period of three years, at a rent of \$100 per year, payable semiannually.

The lease of wharf and buildings at Newport expired December 15, 1891, and was then renewed by agreement with Messrs. Winant, Buckley and Warren for a period of three years, at a rent of \$180 per annum, payable quarterly.

The lease from L. E. Davis of land on the south side of Yaquina Bay occupied by the tramway, which expired January 1, was renewed for a period of five years, at a rent of \$15 per annum.

Recommendations and remarks.—The extension of the north jetty is fraught with much uncertainty. It is founded upon rock. The tramway piles do not penetrate this rock more than a foot or two, and are secured in place by bracing and as soon as possible by dumping rock about them. If, before they are finally secured by the enrockment, a severe storm occurs, the piles are very apt to be washed away. Such has been the case on several occasions. It is hardly possible to estimate what will be the cost of completing this jetty owing to this circumstance.

Last year the amount estimated for the completion of the existing project was \$165,000. The river and harbor bill now in Congress carries \$75,000 for this work. This leaves a balance on the estimate of \$90,000. This is the amount stated that can be profitably expended during the next fiscal year for the completion of the project. It is possible that this amount is in excess of what will be required for the completion of the work. A close estimate of the cost of completing the work can be made one year from the present time.

Future operations.—It is expected to continue work on the north jetty now until it is finally and fully completed in accordance with the

plan of the Board of Engineers. When this is south jetty will be commenced and carried to

The report of Lieut. G. D. Fitch, Corps a charge of the work, is herewith.

Money statement.

July 1, 1891, balance unexpended
Amount refunded March 27, 1892

Amount expended by Treasury Department
June 30, 1892, amount expended during fiscal year

July 1, 1892, balance unexpended
July 1, 1892, outstanding liabilities

July 1, 1892, balance available
Amount appropriated by act approved July 13, 1892

Amount available for fiscal year ending June 30, 1893

{ Amount (estimated) required for completion of exist-
Amount that can be profitably expended in fiscal year of
Submitted in compliance with requirements of sec-
larber acts of 1896 and 1897.

REPORT OF LIEUTENANT G. D. FITCH, CORP

UNITED STATES
NAVY

SIR: I have the honor to submit the following report ment of the entrance to Yaquina Bay, Oregon, for the fis I assumed local charge of this work on April 18, 1892 sector, who had been in charge since August 18, 1891, I needed Mr. G. A. Lyell.

At the beginning of the fiscal year work was in activ Removing debris from the upper quarry preparatory Quarrying at the lower quarry.

Constructing a new wharf and approach on the south Placing rock on the north jetty and driving piles for Quarry operations.—The lower quarry was worked out 1892, upon which date it was abandoned on account of in January, covering the greater part of the workable

The upper or Government quarry, which had been re count of a former slide in the lower quarry during Mar the entire year. The rock from this quarry is much ha quarry. There was a large quantity of debris to remov first this was placed on scows and dumped along the b ity. This method proving objectionable, it was decide Four hundred feet of track was accordingly laid in Fel motive were transferred from the north side to the dumped on land purchased for the purpose just above By this means the earth could be disposed of rapidly. quarry, except a small part at one end, is free from déb

During the year the lower quarry furnished 141 scow QUARRY 110, averaging about 250 tons per scow. About removed from the upper quarry.

The force employed at the quarry consists of: One 1 one engine tender, at 37½ cents per hour (he tends b engine and has no fireman); one blacksmith, at 35 cen cents per hour, and 24 quarrymen, at 25 cents per hour

South side.—At the beginning of the fiscal year the 1 receiving wharf had reached a length of 2,088 feet, and During July, 1891, 1,200 feet of trestle was built. Dur

cranes for the approach and wharf were driven and the work completed in September. The new wharf is 3,500 feet from the old one.

Early in October operations were begun again on the south side, and during that month an engine and tool house was completed and a number of old dump cars were repaired. During November and until the middle of December repairs were made to the old tramway. Then the work of unloading and dumping rock on the north jetty was started.

By the middle of February the enrockment had reached a nearly uniform elevation of 10 feet from a point where the sand dunes meet the jetty out to the old crib, which is about $1\frac{1}{2}$ miles from the new wharf.

On December 16 the wreck of an old vessel, entering the bay under a heavy sea and high tide, struck the south tramway a few hundred feet inside the old wreck in Virginia Bay, and tore away about sixteen bents of tramway and superstructure.

Storms during December were unusually severe, and the south tramway suffered the consequence considerable further damage.

During February the south tramway was repaired over a length of 500 feet.

During the latter part of March all work ceased on the south side, and preparations were made for resuming work on the north jetty tramway.

During the year about 12,500 tons of rock were dumped on the south jetty. The only employé on the south side at present is a watchman, at \$60 per month.

North jetty.—At the beginning of the fiscal year the north jetty tramway had reached a total length of 3,720 feet, and the work of tramway extension was in active progress.

A heavy storm on September 8 having torn away two complete bents and one partially completed one, and further damage being threatened, authority was obtained to stop work temporarily on the north jetty tramway until the weather should be more favorable. By October 8 the two bents carried away had been replaced and the tramway extended one bent farther.

Operations, except that of dumping rock, were then transferred to the south side, and the extension of the north jetty tramway ceased for the season, twenty-nine bents, or 435 feet, of tramway having been constructed since June 30, 1891.

Work was resumed on the north jetty tramway on April 4, 1892, and by April 21 ten bents had been built. A severe storm on April 24 and 25 carried away six of these bents; three of them had been replaced when another severe storm on April 28 carried away the last bent replaced, and did more or less damage to the six preceding bents. Work had been resumed too early in the season. Since the second storm the weather has not caused further damage. The work of tramway extension has, however, proceeded at a slow rate necessarily, owing to the fact that the bottom is of rock, and as the piles can obtain no penetration, it is necessary to secure each bent by dumping rock before proceeding to the next.

From April 4 until the present time (June 16), 27 bents, or 405 feet, of tramway, have been constructed; making a total length of 4,560 feet, of which 840 feet were constructed during the fiscal year. The tramway will probably be completed by the latter part of July.

The work of dumping rock on the north jetty was continued from the beginning of the fiscal year until February 13, 1892. By this time about 41,000 tons had been placed on the jetty, and the enrockment of the portion already constructed was fully equal to that called for by the project. Rock dumping therefore ceased until April 4, since which date about 8,800 tons of rock have been dumped on the portion of the jetty since then constructed.

The total amount of rock placed on the north jetty during the year was about 49,800 tons. Little settlement is expected, as the dumping was practically on a rock bottom.

The portion of the jetty built this season has reached a height of about 2 feet above mean lower low water, except that the last 60 feet is somewhat lower. Sand is being deposited in large quantities along the ocean side of the north jetty.

The force employed on the north side consists of 1 clerk, at \$90 per month; 1 pile-driver foreman, at \$105 per month; 1 foreman, at \$80 per month; 1 watchman, at \$60 per month; 1 storekeeper, at \$60 per month; 1 locomotive engineer, at 35 cents per hour; 1 engine tender (pile-driver), at 32½ cents per hour; 1 blacksmith, at 30 cents per hour; 1 engine-tender (hoisting engine), at 27½ cents per hour; 1 train foreman, at 27½ cents per hour; 1 scow foreman, at 27½ cents per hour; 1 dock foreman, at 27½ cents per hour; 4 riggers on pile-driver, at 27½ cents per hour; 3 laborers, at 25 cents per hour, and 6 laborers, at 22½ cents per hour.

Miscellaneous.—Much miscellaneous work was done during the year, including the overhauling and repairing of several scows; the repairing of the locomotive and cars; the construction of a large number of dirt boxes; the building and fitting out of a blacksmith shop, and a large amount of general repairing.

The only additions made to the plant during the past year were two scows built at Toledo by hired labor, and a naphtha launch purchased for the use of the assist-

By agreement with Mr. Samuel Case, the lease of a strip of tramway leading out to the north jetty was renewed from September period of three years, at a rental of \$100 per year, payable semi-

The lease of wharf and buildings at Newport expired December then renewed with Messrs. Winant, Buckley & Winant, by agreement of three years, at a rental of \$180 per annum, payable quarterly

The lease from Mr. L. E. Davis of some land on the south which expired January 1, 1892, was renewed for a period of five \$15 per annum.

A table of commercial statistics for the year will be sent month.

Very respectfully, your obedient servant,

GR
First Lieut.

Capt. THOMAS W. SYMONS,
Corps of Engineers.

COMMERCIAL STATISTICS.

Yaquina Bay is in the collection district of Yaquina. Yaquina entry. The nearest light-house is at Cape Foulweather, 4½ miles distance. Yaquina Bay forms (with the Oregon Pacific Railroad) a route from Portland and Willamette Valley to San Francisco.

The following returns were collected by Lieut. G. D. Fitch in local charge of the work at Yaquina Bay, and are for the five months ending March 30, 1892:

Arrivals and departures of vessels.

Arrivals.....
Departures.....
Total.....

Exports and imports.

Article.	Quantity.	Article.	Quantity.
<i>Exports.</i>		<i>Exports—Continued.</i>	
Wool.....	tons 7,390	Seal furs.....	do. 1.9
Wheat.....	do. 9,202.1	Miscellaneous merchandise.....	do. 753.8
Woolen fiber.....	{ feet, B. M. 214,500	Oil.....	do. 1.5
Woolen fiber.....	{ tons 429.1	Bones.....	do. 10
Wool and feed.....	do. 981.5	Trees.....	do. 3
Wool.....	do. 1,370	Total.....	do. 20,516.8
Wool.....	do. 62	<i>Imports.</i>	
Wool.....	do. 53.8	General merchandise.....	tons 4,792.1
Wool.....	pounds 200	Machinery.....	do. 47.7
Wool.....	number 75	Coal.....	do. 201.7
Wool.....	tons 180	Hardware.....	do. 1,437.8
Wool.....	do. 66.9	Malt.....	do. 58.4
Wool.....	do. 1.2	Total.....	do. 6,537.7
Wool.....	do. 27.2		
Wool.....	do. 18.8		
Wool.....	do. 6.9		
Wool.....	do. 167.5		
Wool.....	do. 20		

Estimated value:	
Imports.....	\$515,558
Exports.....	317,294

T T 6.

IMPROVEMENT OF TILLAMOOK BAY AND BAR, OREGON.

Description of original condition.—The channel over the bar is direct shifts but little, so that the bar is considered one of the safest on the Oregon coast. A fair depth of water is maintained throughout most of the year, and vessels drawing as much as 13 feet can usually enter without difficulty. The bay at low tide consists of three channels running through sand and mud flats. These channels are of fair depth near the entrance, but gradually shoal up, giving depths of only 9 or 2 feet near the head of the bay. Tillamook, the principal town of the region, is situated on a tidal slough above the head of the bay, and can only be reached by light-draft vessels at high tide.

Plan of improvement.—No general plan and project for the improvement of the bay and bar have yet been approved. A special project for the expenditure of the money appropriated by the act of August 1, 1888, has been approved. This consisted in making a survey of the bar and entrance and the construction of longitudinal and spur dikes and shore protection works to deepen the water over Dry Stocking Bar at the mouth of Hoquarton Slough and to remove snags and overhanging trees from Hoquarton Slough.

APPROPRIATIONS.

Amount of—	
August 11, 1888.....	\$5,200
September 19, 1890.....	500
Total.....	5,700

Amount expended to June 30, 1891.—The amount expended to June 30, 1891, was \$4,922.95.

Results obtained to June 30, 1891.—No work was done during the year

1890-91, and the condition of affairs remained practically the same at the end of the previous fiscal year, June 30, 1890.

During the year ending June 30, 1890, a survey of the bar and entrance to the bay was made. Hoquarton Slough was cleared of snags as far as Tillamook City.

During the high water of November and December, 1889, the longitudinal dike and three spur dikes adjacent thereto were washed away. So no results of any practical importance or permanence had been obtained up to June 30, 1890.

The amount expended during the fiscal year ending June 30, 1892, was \$709.90.

Results obtained to June 30, 1892.—No work was done beyond the survey hereinafter mentioned, and no results were obtained during the year.

Report of operations.—During July, 1891, a survey of the entrance to Tillamook Bay was made by Mr. John R. Savage. The results of this survey and report thereon are published in House Ex. Doc. No. 35, Fifty-second Congress.

Recommendations and remarks.—A special project for the improvement of Tillamook Bay having been submitted, no further recommendations and remarks are now deemed necessary.

Money statement.

July 1, 1892, balance unexpended	\$770
June 30, 1892, amount expended during fiscal year	70
July 1, 1892, balance unexpended	6
Amount appropriated by act approved July 13, 1892	15, 00
Amount available for fiscal year ending June 30, 1893	15, 06

COMMERCIAL STATISTICS.

Tillamook Bay is in the collection district of Oregon. Astoria, near the mouth of the Columbia River, is the nearest port of entry. The nearest light-house is at Meares, about 4 miles south of the entrance.

The following returns furnished by Mr. Charles Lamb, of Tillamook, are for year ending June 30, 1892:

The number of vessels of all descriptions crossing in over the bar during the year was 74. The number crossing out was 73. Their names, draft, and registered tonnage are as follows:

Vessels arrived and cleared.

Name.	Trips.	Registered tonnage.	Draft.
Steam schooner Tru-koe	20	350	
Steam schooner Socia	12	200	
Steamer Humer	1	420	
Steam schooner R. P. Emore	12	30	
Steam schooner Augusta	21	87	
Steam schooner W. H. Harrison	5	70	
Steam schooner Geo. H. Chance	2	71.17	
Steam saw Improvement	1	13	

Vessels built during the year.

Name.	Tonnage.	Draft.
Gen. Garfield, rebuilt (runs on Tillamook Bay, towing and freight busi- ness).....	10	Feet. 7

Articles.	Quantity.	Value.	Articles.	Quantity.	Value.
<i>Exports.</i>			<i>Imports.</i>		
Wool.....	12,650,000	\$164,450	Sawmill machinery†		
Do.....	25,800	 tons	50	\$14,000
Wool, canned.....	684	90,250	Electric-light plant.....	30	5,000
.....	30	12,000	Stave factory.....	30	5,000
.....	6	1,500	General merchandise,		
.....	25	5,000 tons	5,000	420,000
.....	5	750	Agricultural imple-		
Wool stock*.....	100	1,000	ments.....	150	7,500
General merchandise,			Flour.....	380	11,400
..... tons	175	13,125	Feed.....	350	7,000
Total.....	26,325	288,075	Shingles.....	125	5,000
			Brick.....	120	520
			Total.....	6,895	470,820

* Stave mill just started.

† Now under construction.

T T 7.

IMPROVEMENT OF ENTRANCE TO HARBOR AT NEHALEM BAY, OREGON.

Description of original condition.—Nehalem Bay at high water is about 1½ miles long by 1½ miles wide at widest part, and is connected with the main bay by a narrow channel extending in a southerly direction about 3 miles. This channel varies in width from 1,000 to 2,000 feet at high water. A low, sandy peninsula, about 2½ miles long and one-fourth mile wide, lies to the west of this channel and separates it from the ocean. At the southern end of this peninsula forms the northern shore of the bay. Between this and the main shore, limiting the entrance on the south, is a distance of 1 mile. The entrance channel shifts within the limits of 1 mile, occupying sometimes a southerly position and sometimes a northerly one, and sometimes there are two channels separated by a sand island bare at low water. At the time of the survey in 1889, a depth of 6 feet was found on the bar at the northern channel. The ordinary bar depth may be stated to be about 5 to 6 feet at low water.

Plan of improvement.—The plan of improvement recommended by Capt. Young was to build two high-tide stone jetties converging until the distance apart is about 500 feet, and then, if necessary, running to sea parallel to each other to such distance as will insure a lower depth on the bar of at least 8 feet; the north jetty to rest on the edge of the sandy peninsula and the south jetty upon the mainland, the object being to hold the channel in its southerly position.

The cost of the work, as estimated by Capt. W. Young, Corps of Engineers, was \$325,927.50.

Subsequent estimates made in May, 1891, show that the cost was reestimated by Capt. Young, and that the work will cost \$712,388.

APPROPRIATION.

Act of September 19, 1890, "for commencement of jetty construction," \$10,000.

Amount expended to June 30, 1891.—There were no expenditures for the year ending June 30, 1891.

Results obtained to June 30, 1891.—No work was done and no results obtained.

The amount expended during fiscal year ending June 30, 1892, was \$415.08.

Results obtained to June 30, 1892.—No work other than the survey hereinafter mentioned was done and no results were obtained.

Report of operations.—The sum of \$10,000 being inadequate even to procure the necessary plant to commence jetty construction at the mouth of the Nehalem, no final project was made or approved, and all work of actual construction was postponed.

During May, 1891, a reëxamination of the entrance was made, and it was found that the north channel had shoaled up, and only the south one was open.

A careful study of the project was made, and the cost of its carrying out estimated, with a result of forcing a conviction that the original estimate was far too low, in fact, not one-half of what the work would actually cost. This was laid before you in my letter of June 11, 1891.

Under date of June 25, 1891, I was informed that the Secretary of War had directed the withholding of the expenditure of the appropriation until Congress shall have taken further action in the matter.

In July, 1891, a resurvey of the Nehalem entrance was made by Mr. John R. Savage, on the steamer *Gen. H. G. Wright*.

Mr. Savage's report accompanies this report.

NEWPORT, OREGON, July 28, 1891.

CAPTAIN: I have the honor to submit the following report of the survey of the entrance to Nehalem River, Oregon:

Arrived off the Nehalem River on the steamer *Gen. H. G. Wright* on the morning of Monday, July 20, 1891, and crossed the bar at once and anchored about a quarter of a mile upstream. I found that none of the signals used in former years remained standing, so we started out at once and ran a meander line about 9,000 feet in length along the beach near high-water line, from which the various signals for sounding were located, either by offsets from the line, or, as in the case of the two signals on the spit, by cuts from the adjacent plugs on the meander line.

About two days were taken up in locating and erecting the necessary signals and about half a day in locating the high and low water lines along the shores as far as shown on the map, in each case the mean tide being taken.

The weather was very favorable for work on the bar while we were in the Nehalem, but on account of the shoalness of the water on the bar it was impossible to work more than two hours per day while sounding in the channel, and then only on the two hours of the flood tide preceding high water.

We worked two days on the bar and ran in all twelve lines in and out over the bar, sufficient to give a good idea of its shape and the depth of water to be found on the same, which at mean high tide is about 11 feet, or, in other words, 4 feet at low water with a narrow bar channel 5 to 6 feet in depth, as shown upon the plot.

While sounding upon the bar there were about 115 observations taken with the sextant and about 475 casts of the lead made.

About a day was spent in developing roughly the river for about a mile inside the entrance.

The channel has changed its position very much since the last survey two years ago, and it is now very much more to the south, and what was then the channel is now entirely filled up with sand above the low-water line. The change in the channel is by no means unusual, as I was told that the channel had shifted from the north channel across the spit to its present position and back again two or three times in the last two or three years. The last change occurred some time during this last spring, either in March or April. When Capt. Schrader, of the steamer *Augusta*, went in he used the north channel across the spit, and when he crossed the

again a week later the channel had shifted to the southward, about where it is

two or 3 miles up the river, at a cannery wharf, I found the gauge used by Lyell in his survey of two years ago, and set up a new gauge with its zero 2 feet above the zero of Mr. Lyell's gauge, which was set to the mean of the lower low tides.

I set the gauge lower so as to catch the extreme low tides that were running while I was in the Nehalem. I also checked the readings taken by comparing them with the times and heights of the tides for the Pacific Coast, which all in the mean were 2 feet less than the readings of the gauge; there was quite a noticeable difference in the times of the low waters, which, however, can easily be accounted for by the gauge being nearly 3 miles from the ocean.

Very respectfully, your obedient servant,

J. R. SAVAGE.

Capt. T. W. SYMONS,
Corps of Engineers, U. S. A.

Under date of February 17, 1892, by indorsement, I was directed to prepare a project and estimate for the improvement of the Nehalem River entrance, including therein a historical account of the variations of the entrance.

My report was made under date of February 29, 1892, and was as follows:

PORTLAND, OREGON, February 29, 1892.

GENERAL: I have the honor to acknowledge receipt, by indorsement, of orders to prepare a project and estimate for the improvement of the Nehalem River entrance, including therein a historical account of the variations of the same.

Nehalem Bay, at high water, is about 2 miles long by $1\frac{1}{4}$ mile wide at its widest part, and is connected with the ocean by a narrow channel extending in a southerly direction about 3 miles. This channel varies in width from 1,000 to 2,000 feet at high tide and from 500 to 1,500 at low tide. A low, sandy peninsula, about $2\frac{1}{4}$ miles long and one-fourth mile wide, lies to the west of this channel and separates it from the ocean. The southern end of this sandy peninsula forms the northern shore of the entrance; between this and the main shore, limiting the entrance on the south, is a distance of 1 mile.

The entrance channel shifts within these limits of 1 mile, sometimes occupying a northerly position and sometimes a southerly position, and sometimes there are two channels separated by a sand island, bare at low water. The channel change from a northerly to a southern position is a very common occurrence.

Mr. Savage, who made the last survey in 1891, states that the people told him that a channel change had taken place two or three times within the same number of years. The last change was from a northerly to a southern position, previous to the survey of 1891, which occurred in March or April of the same year, within the space of a week. He also states that most of the residents on the Nehalem are of the opinion that there is better water over the bar when the channel occupies the northern position.

In the Pacific Coast Pilot, published by the Coast and Geodetic Survey, it is stated, giving an account of this survey of the river in 1875, that "at the time of the survey in 1875 the low-water channel of the river at the entrance was 160 yards wide, at the bend, 3 miles inside, it was 650 yards wide. The bar of the river was not marked, but it was sufficiently near to the observers to be watched from the shore, for five months they never saw it without a break entirely across it. It was believed to have less than 6 feet of water upon it. The channel evidently shifts with heavy storms. * * *

In July, 1867, when we were passing close along the coast, we found the Nehalem lying abreast of the position of the north point in 1875. * * *

In 1868, during an examination of the river, the bar broke continuously and was unapproachable. * * *

At high water the sea breaks entirely across the narrow channel as far northward as the sand point, effectually closing the river."

The river and harbor act of March 3, 1875, directed that an examination should be made of the mouth of the Nehalem River. This was not done directly by the Corps of Engineers, but was done by Capt. Gilbert, an assistant of the Coast and Geodetic Survey, who was at work in that vicinity, and he made the examination in accordance with orders from the Superintendent of the Coast Survey.

The quotation just given from the Pacific Coast Pilot is based upon Capt. Gilbert's examination.

The mouth of the Nehalem was next examined by Mr. Philip G. Eastwick, under the orders of Capt. Powell. No boat could be obtained for taking soundings on the

The south jetty should not break away from the shore so abruptly, as shown on Mr. Young's plan, but should leave it with a gentle curve.

Send herewith a blue print, upon which I have indicated two possible locations for a pair of jetties.

The extension of the jetties seaward is rather problematical. I have indicated on the plan the extent to which they should go to secure the maximum benefit under existing conditions. The building of the jetties would, however, change existing conditions, and because of the vast volume of sands in the vicinity, and the fact that the Nehalem entrance is in a shallow bight of the coast, it is altogether probable that they will have to be extended farther on account of the accumulation of sands in front of them.

It is probable that by confining the inflowing and outflowing waters between jetties 600 feet apart a bar depth of 8 feet at low water may ordinarily be expected. This would by no means be permanent. Exposed to the full force of the ocean waves, the movable sands at a depth of 8 feet or less would be stirred up at every moderate storm, and the bar channel reduced temporarily to the ordinary depth before any improvements were undertaken.

It is also safe to predict that there would be times when the bar would exceed 8 feet temporarily.

With such an entrance as the Nehalem it is in my opinion unsafe to predict any permanent bar depth.

Estimates.—Any estimate of the cost of the jetties based upon existing conditions must be approximate and subject to change. These conditions and depths are subject to change naturally, and the construction of the jetties would cause still more marked changes.

In making the following estimates I have assumed the present bottoms, as shown on the last survey, as the basis. The jetties are planned to be rubble mounds, on both mattresses, built to full high-tide level, 10 feet wide on top, with a slope of 1 on 3 on the channel side from high to low water, and a slope of 1 on 1½ on the side away from the channel between high and low water, and slopes of 1 on 1 on both sides below the water.

In order to provide for settlement of the rubble mound into the sand, and due to scouring in the deeper contracted channel, and to abrasion, an allowance of 6 feet increased depth is made in all estimates except the estimate for the jetty from the end of the North Spit extending southerly and limiting the river on the west. For this portion, from A to B, an allowance of 4 feet is made.

North location.

estimate:	Feet.
South Jetty, approximate length.....	4,500
Rubble required, 108,308 cubic yards, at \$4.....	\$433,232
North Jetty, approximate length.....	4,200
Rubble required, 93,367 cubic yards, at \$4.....	373,468
Total.....	806,700

South location.

Estimate:	Feet.
South Jetty, approximate length.....	2,700
South Jetty, approximate length of tramway approach.....	5,500
Rubble required, 69,760 cubic yards, at \$4.....	\$279,040
North Jetty, approximate length.....	6,800
Rubble required, 115,205 cubic yards, at \$4.....	460,820
Tramway approach, 5,500 feet, at \$4.....	22,000
Total.....	761,860

The estimated cost of completing the project wherever the jetties are located will be about \$800,000.

The cost of the plant necessary and the expenses attendant upon the commencement of this work in a manner commensurate with its magnitude is \$50,000.

On account of the lack of any fixed, rocky headland, and the unstable character of the sands upon and in which this work must be founded, and the great changes which are liable to occur at any time, it is particularly necessary and desirable that, once commenced, it be prosecuted with vigor and with ample funds.

LETTER OF THE CHIEF OF ENGINEERS, U. S. ARMY.

...cation for this improvement being undertaken is, as stated by Capt. the lumber business by furnishing a harbor from which the lumber tributary to the Nehalem could be shipped to market. not be obtained at the Nehalem sufficient in capacity to permit the lumber to be carried on profitably, the improvement seems unjustifiable, as the interest involved are comparatively insignificant. to carry an even a coast-wise trade in lumber profitably at present prices, or prices which could confidently be expected for years to come, requires a harbor with a greater depth of water on the bar than can be hoped for at the Nehalem, and a harbor where the vessels are not liable to be bar bound for weeks at a time.

It is not possible for me to believe that such a harbor can be obtained at the Nehalem. The lumber of the Nehalem must be shipped out of the country by some other route.

I entirely coincide with the opinion expressed by Capt. Powell, that the problem of providing this district with means of transportation would be satisfactorily solved by the construction of a railroad to the Columbia River, and by the establishment of a more convenient line of communication to Tillamook Bay.

Very respectfully, your obedient servant,

THOMAS W. SYMONS,
Captain, Corps of Engineers.

Brig. Gen. THOMAS L. CAS
Chief of Engineers,

Recommendations and remarks on the Nehalem having been defined, the work of construction having been authorized, from making any recommendation as to the amount that could be expended during the next fiscal year.

As the project for the improvement of the bar has been formally decided upon, and no work has been authorized or commenced, I refrain from making any recommendation as to future appropriations, or as to the amount that could be expended upon the work during the next fiscal year.

ment.

July 1, 1891, balance unexpended	\$10,000.00
June 30, 1892, amount expended during fiscal year.....	415.06
July 1, 1892, balance unexpended	9,584.94

COMMERCIAL STATISTICS.

The following returns, furnished by Mr. E. G. Wist, of Nehalem, are for the year ending June 30, 1892.

The number of vessels of all descriptions crossing in over the bar during the year was 16. The number crossing out was 16. Their names, registered tonnage, and draft are as follows:

Vessels arrived and cleared.

Name.	Trips.	Registered tonnage.	Draft loaded.
Steam schooner Augusta.....	9	87	7 9
Steam schooner W. H. Harrison.....	7	70	6 0

As there were no winter freshets last season and the bar channel having shifted, vessel masters were very cautious and refused to come in; only 1 vessel came in during the month of May.

Articles.	Quantity.	Value.	Articles.	Quantity.	Value.
EXPORTS.			IMPORTS.‡		
Box lumber*.....	tons.. 255	\$14,000	Two sawmill outfits.....	tons.. 150	\$25,000
Salmon†.....	do... 5	25	One pile-driving outfit.....	do... 15	2,000
Water.....	do... 15	6,000	Merchandise and logging machinery.....	tons.. 650	50,000
Salmon;.....	do... 8	500	Cannery supplies.....	do... 200	10,000
Salmon, canned.....	do... 144	20,000			
Skins and furs.....	do... 10	10,000			
Whorls and beeswax.....	do... 3	1,200			
Whitten bark.....	do... 3	300			
			Total.....	1,015	87,000
Total.....	443	52,025			

* Double amount on hand, but unable to ship.
 † Sample.
 ‡ Off year for salmon; double the amount the year previous.
 § The bar channel having shifted, vessel masters were cautious and refused to enter.

T T 8.

IMPROVEMENT OF UPPER COLUMBIA AND SNAKE RIVERS, OREGON AND WASHINGTON.

Description of original condition.—Under the above head it has been deemed proper to officially include the continuous Columbia and Snake rivers from Celilo, at the head of The Dalles, to Lewiston.

The Upper Columbia and Snake form a continuous line of navigable river, but broken by many rapids, rendering navigation difficult and dangerous.

These rapids are in nearly every instance caused by rocky bars with occasional boulders, and the channels were crooked and narrow.

Before improvement the ruling depth at low water was 2 to 3 feet in many of these bars, some of which were practically impassable at low water.

The Columbia was navigable all the year round, except when closed by ice. The Snake was, however, only navigable during high water and for three to four months in the spring of the year.

Plan of improvement.—Previous to 1877 \$120,000 had been appropriated for the work of improvement, and had been expended in survey and rock removal at the principal rapids.

In 1877 Maj. J. M. Wilson made a project for the improvement of the upper Columbia and Snake rivers, consisting of removing rock boulders, and rocky reefs, and scraping gravel bars in the Columbia and Snake as far as Lewiston. The estimated cost of this work was \$132,000.

The present project or plan of improvement consists in removal of boulders, gravel bars, and rock ledges, and putting in such contraction works as may be necessary.

The estimated cost of the work has not been accurately determined because of the lack of continuous and full surveys, because from the nature of things it can not be, as many of the obstructions which it is necessary to remove are continually recurring.

APPROPRIATIONS.

Upper Columbia River:	
Act June 30, 1873	850.00
Act June 23, 1873	20.00
Act March 3, 1875	35.00
Total	100.00
Upper Columbia and Snake rivers:	
Act August 14, 1876	15.00
Act June 24, 1878	20.00
Act March 3, 1879	20.00
Act June 24, 1880	15.00
Act March 3, 1881	15.00
Act August 2, 1882	5.00
Act of July 5, 1884	20.00
Act August 2, 1886	10.00
Act August 11, 1888	10.00
Act September 24, 1890	20.00
Total	150.00
Aggregate	250.00

The amount expended on the project to June 30, 1891, was \$134.44. Results obtained to June 30, 1891.—Up to this date no work had been done in the river since the report of Maj. Jones, in 1889, was written and consequently no additional results had been obtained.

Ice and heavy freshets had brought into the river channel a number of great boulders, which were an additional annoyance and danger to boats navigating the river.

Although the work heretofore done on the river has been very beneficial to navigation by lessening its dangers, it has not increased navigable depth and capacity, particularly of the Snake River.

As stated in Maj. Jones's report, the general result of all work and of natural forces at work has been to lower the bed of the river by 2 feet, but it is found that the water surface has also been lowered, so that the resulting navigable depth is about the same as which formerly existed.

The amount expended during the fiscal year ending June 30, 1892, was \$22,754.

Results obtained to June 30, 1892.—During the fiscal year ending June 30, 1892, a large amount of work was done in removing bed and boulders from the river between Riparia and Lewiston. The amount of rock removed was 286 cubic yards. The removal of this of very great benefit to navigation, permitting the boats to carry their double the loads that they could have carried without the present and with greater safety.

Report of operations.—Mr. W. H. Wood was placed in charge of work on the Snake, and left Portland August 6, 1891, and proceeded to Riparia, where the drill scow constructed the previous fiscal year was moored.

A crew and full outfit of tools and supplies were secured, and August 23 the work commenced.

The plan of the work was to go from point to point up the stopping long enough at each bad place to remove the worst and dangerous rocks. The drill scow was towed from point to point the steamers plying on the river.

Capt. Baughman, who has had charge of the steamers running the river, was engaged to designate the points where work was

and to buoy the rocks of which the removal was most desirable. Practical and close knowledge of the river, gained by running on its stages for many years, was of the greatest benefit to the work, being more to be accomplished for the benefit of navigation with the expenditure of time and money than could have been accomplished by any other method.

The work continued until November 28, 1891, when the scow was fast in winter quarters at Selkirk's old ferry, 6 miles below Lewis-

and for this several days were spent in repairing the old crib at Log Cabin Rapids, and everything about the scow was made snug, and Mr. L. returned to Portland December 15, 1891.

Recommendations and remarks.—Attention is invited to my last annual report under this head (p. 3212, Report of Chief of Engineers).

The only portion of the section of river under consideration which is reported is the Snake, from Riparia to Lewiston. Upon this portion of the river is transported a large amount of grain, and it furnishes at present the most convenient line of travel for a great section of country. In this section of the river are many rapids, swifts, and shoals, where it is necessary to secure the best available water. During periods of high water great bowlders are moved from places where they do not go to places where they are in the way of steamers, and these rollers, like snags in some rivers, require to be repeatedly removed. This work requires yearly attention. A drill scow with an equipment of tools has been provided, which will enable the work to be done easily and expeditiously.

In addition to this work of rock removal, there are three places between Riparia and Lewiston where other work is necessary. These are at Goose Island, 9 miles above Riparia; Diamond Crossing, 13 miles above Riparia, and Log Cabin Rapids, 38 miles above Riparia.

At Wild Goose Island the river is very wide and a number of gravel bars have been formed, which divide the waterway into a number of channels, which at low water become very shoal, crooked, and swift.

At low water now steamboats are compelled to use the south channel, which is hewn on the flat. This is crooked and shoal above the rapid. The straight middle channel becomes worse than the south channel at low water and can not be used. It is proposed to overcome the difficulty by building a stone dike across the south channel, as shown on the plat, this dike to extend to 2 feet above low water. This, it is believed, will force the water through the middle channel, thereby scouring it to greater depth, lengthening the rapid and lightening the current.

The estimated cost of this dike is \$8,068.50.

At Diamond Crossing there is a long shoal extending diagonally across the river, covering a distance of a half mile in the length of the river. The river is wide, the current light, and there is no defined channel. The present line of steamboat channel is very hard to navigate. The boats have to drift nearly across the river in very shoal water, which in windy weather makes it extremely difficult to keep from being driven on the bars.

It is here proposed to build a stone dike 750 feet long to a height of 2 feet above low water from the end of the gravel bar on the south side of the stream, and thus force the water around same. This it is believed will give a narrower and deeper channel.

The estimated cost of this dike is \$9,330.75.

At Log Cabin Rapids the river divides into two channels—the north channel and the south channel. The north channel is very shoal and crooked, and the south channel is very shoal and crooked. It is proposed to build a stone dike across the north channel, as shown on the plat, this dike to extend to 2 feet above low water. This, it is believed, will force the water through the south channel, thereby scouring it to greater depth, lengthening the rapid and lightening the current.

The estimated cost of this dike is \$9,330.75.

At Log Cabin Rapids the river divides into two channels—the north

and south. Steamboats use the south channel, the north one being crooked, and with the fall so concentrated as to prevent boats going up it without lining. The south channel is only 4 feet deep in places at low water, and is constantly getting shoaler. It has a current of 6 miles per hour. The channel is full of bowlders and large gravel and is so narrow and crooked that in windy weather it is hard to navigate.

It is proposed to remedy the difficulty at this point by building a stone-dike 500 feet long to 2 feet above low water, closing up the north channel and thus forcing the water at low stages through the south channel, giving a greater volume, which it is believed will both deepen and widen it.

The estimated cost of this dike is \$5,810.75.

At this point there is a crib work built of timber and filled with stone, the object of which is to prevent boats drifting downstream from running on the rocky shore. This crib should be extended 50 feet farther downstream to more perfectly fulfill its mission.

The estimated cost of this extension is \$1,016.63.

Plans showing the three bad places above mentioned and the proposed improvements are herewith.

The amount of money recommended for the next fiscal year is as follows:

Removing bowlders, rocks, etc.....	85,000.00
Building dike at Wild Goose Island.....	5,810.75
Building dike at Diamond Crossing.....	9,330.75
Building dike at Log Cabin Rapids.....	5,810.75
Extending crib.....	1,016.63
Total.....	107,773.33

In round numbers, \$100,000.

It is further recommended that the appropriation be made available as far as Astoria, above 7 miles above Lewiston, for reasons set forth in my letter of December 21, 1891, and published in House Ex. Doc. No. 77, Fifty-second Congress, first session.

Report of Mr. W. H. Wood, the assistant engineer in personal charge of the work, is herewith:

UNITED STATES ENGINEER OFFICE,
Portland, Oregon, December 24, 1891.

CAPTAIN: I have the honor to submit the following report of the work of improving the Snake River between Riparia, Wash., and Lewiston, Idaho, during the months of August, September, October, November, and December, 1891.

According to your verbal instructions I was placed in charge of this work August 2, 1891. I left Portland August 6 and proceeded to Riparia; thence to Lewiston.

I found the work most necessary to be done was the removal of numerous rocks or bowlders which had been carried down by ice and high water and deposited on the bars and shoals, thereby obstructing navigation very much, as they invariably lay in the deepest water, thus causing the boats to leave the channel to avoid same.

The United States drill scow *Tacoma* was tied up at Riparia in charge of Capt. E. W. Baughman. The scow was turned over to me August 13, at which time the water in the river was 3 feet above low water, but falling rapidly.

It was deemed advisable to delay the work some days awaiting lower water. A part of a crew of men who were hired in Pasco, Wash., arriving at Riparia August 15, were put to work repairing and equipping the scow, taking aboard provisions, wood, etc. This work continued until August 28, when, having secured a crew of 10 men, and the river having fallen to 1 foot above low water, I started to tow the scow up the river.

Arrangements had previously been made with Capt. Baughman to have the steamer *Gene Pava* tow the scow from point to point as required.

Owing to high winds the steamer was unable to tow, and the scow was landed 1 mile above Riparia, from which point it was lined to Little Goose Island, 3 miles away, which consumed four days.

The actual work of drilling began here on September 3 and progressed until September 12. Forty-two and one-half linear feet of holes 2 inches in diameter were

rilled in seven bowlders. The holes were fired and the bowlders completely broken up. Several smaller bowlders were also removed by grapping tongs. A channel was thus secured 100 feet wide, with a minimum depth of $3\frac{1}{2}$ feet at low water; stage of water, +0.25. (All stages of water referred to Union Pacific Railroad gauge on abutment of railway bridge at Riparia.)

September 14, towed by steamer to mouth of Deadmans Gulch, 18 miles above Riparia. Here two large bowlders lay directly in channel. Four holes, each 3 feet deep, were drilled, loaded, and fired, and the bowlders completely removed.

September 18, towed from Deadmans Gulch to a point 2 miles above Almota, and 7 miles above Riparia. One large rock lay in channel here. Four holes, each 3 feet deep, were drilled and fired and the rock completely broken up, thus giving 6 feet of water at low-water stage.

September 20 and 21, the scow was being lined from the above point to rapid at Log Cabin Island, 4 miles above, where several bowlders lay in channel, and a small gravel bar formed at the lower end of the rapid was obstructing the channel. The rock was removed by the usual process of drilling and blasting. Twenty-two feet of holes were drilled in four rocks, which, when fired, were completely removed. A number of smaller bowlders were picked up and broken or removed from the channel.

The gravel bar was blasted by driving 3-inch iron gas pipe to a depth of 5 feet in gravel, and when heavily loaded and fired the gravel was so loosened that since that time the current has washed the greater portion away.

September 24, towed from Log Cabin Island to Kelleys Bar, 23 miles below Lewiston, where one large rock, submerged 2 feet, was directly in the channel. Nine feet of holes were drilled, loaded and fired, breaking up rock and leaving a depth of 6 feet at low water.

September 25, rafted scow 4 miles down the river to Truax Landing, where one bowlder lay in channel. Two holes, each 3 feet deep, were drilled and fired, completely removing the bowlder. Owing to a slight accident caused by the parting of a line, one of the spuds on the scow was broken and we lay at Truax Landing repairing the same until October 1, when the steamer towed the scow to Little Pine Tree rapid, 13 miles below Lewiston. Here a quantity of work was to be done. Just above the rapids the river is wide and shoal, and what small channel there was was obstructed by submerged bowlders that it was very difficult for boats to get through.

About 80 of these bowlders were removed. One hundred and three and one-half linear feet of drilling was done in thirty-nine holes, which were fired with good results, while a number of the smaller rocks were picked up with a derrick and tongs. There now remains a clear straight channel 150 feet wide, with a least depth of 5 feet at low water, which the river boats now go through without difficulty.

October 18, in tow of the steamer, we moved to Lewiston, where from this date until November 19, our time was spent in removing bowlders in this vicinity.

A total of 191½ feet of holes were drilled and all holes fired, and 132 cubic yards of rock removed by blasting, and a large number of small rocks removed with derrick and tongs, so that at present boats can load at Lewiston with 2,500 sacks of grain, where formerly 1,000 sacks were all they could get out with.

November 21, rafted scow down river to a bar 4 miles below Lewiston, and there removed one large rock from the channel. Three holes, each 4 feet deep, were drilled and fired with good results, and there is now 8 feet of water over the rock.

A heavy wind storm of three days' duration held us here unable to move. All through this work considerable time was lost on account of the wind, which frequently blows a gale. At such times we were not able to move or work.

November 28, the scow was rafted to winter quarters at Selkirk's old ferry, now known as Evans, 6 miles below Lewiston.

Repairing crib.—At Log Cabin Island a crib had formerly been built to aid in navigating the channel at that point, but it had been damaged to some extent by high water, and according to your instructions, I was to repair the crib. I purchased sufficient lumber at Lewiston, and on November 28 I started with lumber, raft, and men to repair the same, and reached the work the same day. The Union Pacific Company allowed me the use of their scow which lay at that point to live on. The weather turned very stormy and this work was not completed until December 8.

Following is a statement of the cost of the same:

Lumber	\$99.36
Labor	159.80
Iron bolts, etc	11.45
subsistence	56.79
Total	327.40

Having completed the above work and returned to the scow at Evans, and made some needed repairs, painting roof, etc., I took an inventory of all equipment; discharged all the men December 12, and returned to Portland December 15, 1891.

MUCH valuable aid was given me by C. A. the river for years) in pointing out all the might have been overlooked, and in many i were hard to discover, and otherwise aiding

All the equipment of the scow was store the keys left with Capt. Baughman, who ne a quantity of powder that was left at Little

The stage of water throughout this wor than 1 foot from zero of gauge at Riparia, i clearing again.

Summarized

Total expenditures:

Labor	
Equipment	
Supplies (work)	
Supplies (subsistence)	
Repairs to boat	
Towing boat	
Engineering	

Total

Summary of work:

Work began August 19, 1891.	
Work ended December 12, 1891.	
Labor performed	
Holes drilled	
Holes drilled	
Holes fired	
Explosives used	
Rock removed	

Cost of rock per cubic yard, \$15.43.

All labor was hired by the month and by

Future work.—The removal of bowlders fr have yearly attention. From the reports c ice and high water carry down and deposit

Estimated cost of this work yearly, \$5,000

Aside from the above there are three p Lewiston that I think could be greatly in advisable.

(1) Wild Goose Island, 9 miles above I three channels, and shoals badly in each. now go through the south channel, whic rapid. The middle channel is the straight it shoals at head of rapid, which shoal a the rapid and forming so strong a current t

I would suggest a stone dike, 900 feet lo on the sketch, closing up the south chan middle channel, thereby scouring it to a j lengthen the rapid and lighten the current, which will be easy to scour.

Estimate: 2,934 cubic yards rock, at \$2.75

(2) Diamond Crossing, 13 miles above R diagonally across the river, and about a point is wide and the current light, with lo nel. It is here proposed to build a stone d from the end of the gravel bar on the south water around the same, thereby forming an on the north side of the river, which it is l

The present line of the steamboat chann Steamers have to drift nearly across the ri weather makes it difficult to keep from bei

Estimate: 3,393 cubic yards rock, at \$2.75

(3) Log Cabin Rapids, 38 miles above R north and south channels. Steamboats use so rapid as to always cause them to line, channel is only 4 feet deep at low water shouler. It has about a 6-mile current.

The bottom is composed of bowlders and

* This does not include th

more closely and is so narrow and crooked that in windy weather it is hard to

ould suggest a stone dike 500 feet long, built to 2 feet above low water, closing the north channel and forcing all the water through the south channel, a greater volume of water which I believe will both deepen and widen the

ated: 2,113 cubic yards rock, at \$2.75 per yard in place, \$5,810.75.

is place I would also recommend the extension of a crib, same as is shown on here in sketch and used as a sheer for steamers.

needs to be extended about 50 feet on the west end. To be built of rough and filled with suitable rock.

ated cost complete: \$1,016.63.

Summary of future improvements.

ing bowlders from channel yearly.....	\$5,000.00
g dike at Wild Goose Island.....	8,068.50
g dike at Diamond Crossing.....	9,330.75
g dike at Log Cabin Rapids.....	5,810.75
ion of crib.....	1,016.63
Total.....	29,226.63

above to include cost of plant together with placing of rock.

Very respectfully,

W. H. WOOD,
Assistant Engineer.

F. SYMONS,

Capt., Corps of Engineers.

ure operations.—Future operations with the money available will t in placing the drill scow in commission, and removing rocks owlders. This work can only be done advantageously when the is low and the water is clear, which is from about September 1 to aber 1 each year.

Money statement.

1891, balance unexpended.....	\$16,557.77
, 1892, amount expended during fiscal year.....	8,275.01
1892, balance unexpended.....	8,282.73
1892, outstanding liabilities.....	25.00
1892, balance available.....	8,257.73
t appropriated by act approved July 13, 1892.....	15,000.00
t available for fiscal year ending June 30, 1893.....	23,257.73

unt that can be profitably expended in fiscal year ending June 30, 1894 30,000.00
itted in compliance with requirements of sections 2 of river and
bor acts of 1866 and 1867.

COMMERCIAL STATISTICS.

following information is furnished by Capt. B. R. Pegram, superintendent, Lines, Union Pacific System:

ing.—Steamers operated by Union Pacific Railroad upon Snake River between and Lewiston and intermediate points during fiscal year July 1, 1891, to), 1892, with tonnage, draft, and amount of freight carried:

Name.	Registered tonnage.	Draft.	Freight carried.
		<i>Inches.</i>	<i>Tons.</i>
Annie Faxon.....	564	22	9,029
Spokane.....	531	20	5,594
Almota.....	305	18	4,544
tal.....			19,167

value about \$600,000.

T T 9.

IMPROVEMENT OF COLUMBIA RIVER BETWEEN HEAD OF ROCK ISLAND AND FOOT OF PRIEST RAPIDS, WASHINGTON.

Description of original condition.—The portion of the Columbia from the head of Rock Island Rapids to the foot of Priest Rapids is about 60 miles in length. The river flows generally through a deep cañon. The banks for the greater part of the way are nearly precipitous bluffs from 1,000 to 3,000 feet high, composed of columnar black basalt. The country bordering the river is rocky and sterile.

The three principal obstructions to the navigation of this portion of the river are Rock Island, Cabinet, and Priest rapids. At Rock Island Rapids the river has cut around both sides of a large island of rock. The channel, however, on each side is very much obstructed with reefs, rocks, and points sharp projecting from the shore; The result is a waterway so much restricted as to dam up the waters to such an extent that the river runs in lines of very steep slope and amidst great masses of rock. A high projecting island, at the head of the island which has caused extensive flooding, nearly choking the channel at low water.

The river here has a fall of 3,000 feet, and 12½ feet in 8,000 feet, at a stage as near as ascertained of 4 feet above low water.

At Cabinet Rapids a large rock projects from the left shore nearly two-thirds of the channel of the river, deflecting the current and causing it to run in a nearly vertical basaltic bank. Large masses of rock exist in the channel in and about the main rapids. The fall here is 10 feet in a distance of about 8,000 feet.

At Priest Rapids there are seven principal rapids extending over a length of 10 miles of river.

The total fall in this distance is, at low water, 72 feet, and at high water about 63½ feet.

In all these rapids the river runs through and over hard, rough, and jagged basaltic rocks.

Plan of improvement.—The plan of improvement under which existing work has been carried on consists in removing obstructing rocks at Priest, Cabinet, and Rock Island rapids, and putting in at all locations where they would be convenient, iron posts and ring bolts, to which ascending boats could make fast their lines and then wind themselves up over the rapids by means of their steam capstans, with which all such boats would naturally be provided.

The estimated cost of the work proposed was \$550,000.

In addition to this it was proposed to continue the survey of the Columbia from the head of Rock Island Rapids to the boundary line. This project was approved by the Secretary of War October 18, 1890.

APPROPRIATION.

Act of September 19, 1890 \$70,000
Of which \$10,000, or so much thereof as may be necessary, may be used in the survey of the Columbia River from the international boundary line to Rock Island Rapids.

The amount expended to end of fiscal year June 30, 1891, was \$31,291.13.

Forsurvey of river from international boundary line to Rock Island Rapids. \$2,691.91
For improvement of river at Priest, Cabinet, and Rock Island rapids 28,599.22

Results obtained to June 30, 1891.—The following extract from my last annual report shows the condition of the work to June 30, 1891:

The work so far done on the river at Priest and Cabinet rapids has not produced any results of a marked nature. All rocks removed tend to the final result and would benefit and render safer navigation on the river if there was any navigation, which there is not.

The survey of the river from the international boundary line to the head of Rock Island Rapids has progressed under Mr. William Cuthbert, so that the work is nearly done from the boundary line to the mouth of the Spokane.

The total amount expended during the fiscal year ending June 30, 1892, was \$28,195.29, of which there was expended for survey of river from international boundary line to Rock Island Rapids \$7,227.95, and for improvement of river at Priest, Cabinet, and Rock Island rapids, \$20,967.34.

Results obtained to June 30, 1892.—The work of improvement during the fiscal year has been confined to Cabinet and Rock Island rapids.

At Cabinet Rapids a number of projecting points along the right bank have been removed, and some rocky island masses near the left bank were honeycombed with drifts and drill holes, loaded with dynamite, and blasted, dividing the rock into small fragments which it is expected will be washed away at high water.

If this is the case, Cabinet Rapids will be improved to as great an extent as appears necessary in the way of rock removal.

A number of ring bolts have also been put in at various points along the river for convenience of boats in lining over the rapids, if any should attempt it.

The work at Rock Island Rapids has been conducted with the object in view of removing obstructions which existed to boats lining up along the right bank during the higher stages of the river, and which endangered boats in running down.

That which has been done at both Cabinet and Rock Island rapids would be of great benefit to navigation if any existed.

Report of operations.—At the beginning of the fiscal year the operations in progress in connection with the improvement were the taking of the drill scows up to Cabinet and Rock Island rapids. On July 17, 1891, the foot of Cabinet Rapids was reached, and on July 31 the boats had been lined over the rapids and taken to Moses Coulee Ferry, between Cabinet and Rock Island rapids. The high water continuing, work on rock removal was not commenced until September 10. Upon this latter date operations were resumed on Cabinet Rapids and continued until February 15, 1892, with an average daily force of thirty-six and one-half men.

During this time there was used at Cabinet Rapids 11,050 pounds of dynamite No. 1; there were drilled 472 2-inch holes, aggregating a length of 3,002 linear feet, and twenty-two drifts were run to a total length of 410 feet. The last blast was February 12. The result of the work was the removal and breaking up of 12,199 cubic yards of rock. That which was not fully removed it is expected will be removed by the high water.

On February 25 work was commenced on Rock Island Rapids, and continued until May 5, with an average force of fifteen men. During this time there was removed rock aggregating 1,894 cubic yards; 256 holes 2 inches in diameter were drilled to a total length of 1,414 feet.

A permanent gauge was erected at Rock Island Rapids.

On May 5 the boats were taken back to Moses Coulee and left in charge of a watchman.

The tracings sent herewith show the work which Cabinet and Rock Island rapids.

Recommendations and remarks.—I desire to invite statements under this head in my last annual report of the Columbia from the head of Rock Island Rapids to Cabinet Rapids, and the "Upper Columbia and Snake," published in the Report of Chief of Engineers, U. S. Army.

If the work on the project is to proceed, the amount that can be profitably expended the next fiscal year is \$100,000.

Future operations.—As there is a practical certainty that there will be no further appropriation at present for the improvement of this stretch of river, no further operations of any magnitude are contemplated.

With the money remaining on hand, it is expected that the plant, and if authority is granted it is expected that the tools in the improvement of the Snake River, for which provision is made in the river and harbor bill.

The report of Mr. J. G. Holcombe, in charge of the work, is herewith submitted.

Money statement.

July 1, 1891, balance unexpended.....
June 30, 1892, amount expended during fiscal year:	
Improvements
Survey
July 1, 1892, balance unexpended.....
July 1, 1892, outstanding liabilities
July 1, 1892, balance available

REPORT OF MR. J. G. HOLCOMBE.

PORTLAND, OR

SIR: I have the honor to submit the following annual report upon this work under your charge for the year ending June 30, 1892.

At the close of last year work at Priest Rapids had been advanced to the point where the scows were being lined to Cabinet Rapids, 54 miles above Priest Rapids.

On June 30 they had been taken to a point about 13 miles below Cabinet Rapids, and July 17, having reached the foot of them, I built a magazine of dynamite in it. The scows were then lined through the rapids, and finally reached the head, having been considerably delayed during the summer months.

Having reached the head of the rapids I hired the steam ferry to take the boats to her landing at Moses Coulee, where they were to be sufficiently to permit work on Cabinet Rapids.

As there were no steamers upon this section of the river (between Cabinet Rapids), the moving of the boats had to be by hand. It was possible, except for a short distance, because of the rough and uneven banks.

Having reached the ferry landing on July 31, I laid off 5 men for a special mail-carrier who had been employed since March 1, 1891, twice a week from Ellensburg, the nearest post-office to the river. On August 1, after the visit of inspection by Col. Mendell and your men, keeping 3 men to finish certain repairs to the plant.

On the 22d of August I left for Ellensburg to engage miners to work on the river being low enough by the 10th of September to resume work.

On that date the drill scows were dropped down stream to the rapids and work again commenced.

Plan of improvements.—The project upon which this work has been done was recommended by you as a modification of Maj. W. A. Jones's project, and it consisted of the removal by blasting of the worst rocks obstructing navigation and the placing of ring bolts and deadmen, to which the steamers could make fast and line themselves over by means of their steam capstans. This was recommended, not as a permanent improvement, but as an aid that would become at once available to any steamer that might desire to use this stretch of river.

Cabinet Rapids.—During the previous year a small force was sent from Priest Rapids with small boats and tools to blast away certain points and a reef along the right bank at this place. This force worked from April 15 to May 15 and succeeded in blasting at points (marked on the map) H, I, K, and the reef G. On May 15 this work had to be stopped because of the high water.

As the river fell during the year, it was found that the high water had carried away all of the débris of last year's blasting, leaving in most instances a clean rock bed at or below the low-water level.

The work of the present year was commenced by the removal of the point F, which was cut down as low as the then stage of the river would permit. After this was completed work was started upon the mass of rocky islands in the right-hand channel, and the rocks A, B, C, D, N, O, and P have been blasted down to and below the low water of this year.

The rocks B, C, D, P, and parts of A and E were blown up by the usual method of drilling 2-inch holes down to the low-water level, loading with dynamite, and firing by electricity. For the rest, parts of A, E, N, and O, drifts were driven in on the low-water level, these drifts charged and tamped, and then fired by electricity, thus shattering the rock. This method was used in every case where feasible and when the dimensions, especially in height, would allow, and proved a quick and cheap method of blasting the larger rocks.

Results.—As previously stated, the débris of last year's work here had been washed away by the high water.

During this year there have been blasted on three rapids 12,199 cubic yards of basalt rock, using 11,050 pounds of dynamite. To blast this quantity of rock, 472 2-inch holes were drilled, having a total linear measurement of 3,002 feet. There were also driven twenty-two drifts, having a total linear measurement of 410 feet, and one shaft measuring 10 feet to low water, from which three of the above drifts were driven.

The last blast at this rapid was on February 12, when the rock E, containing 6,445 cubic yards was blown up, thus completing the project of rock-blasting at these rapids.

I then had three sets of ring bolts placed at convenient points for ascending steamers to make fast to and line themselves up over these rapids.

The work at Cabinet Rapids extended during this year from September 10, 1891, to February 15, 1892, during which period 6,103 days of labor were expended upon the rock work; this includes Sundays and holidays, upon which no work was done, but which are here included because the force were all employed by the month and subsisted. This employment of labor by the month was necessary because of the comparatively short seasons of good weather in which work could be done, making it necessary to work early and late to accomplish any valuable results.

The average daily force employed during this period was 36.54 men, the largest force 43 men, the smallest force 15 men. On January 31 the force was reduced from 43 to 15 men, this being as large a force as could be conveniently employed at the then existing state of the work.

The work of drilling, etc., was carried on upon one hundred and thirty-four different days, and having a mild winter no time was lost by bad weather.

Débris.—All of the rock is broken small and it is expected that the coming high water will wash considerable of it away.

Condition of the rapids.—While this rapid has at all stages of the river rough water, still steamers, it is believed, can pass them in safety by using the channel along the left bank in high water and the channel along the right bank in medium and in low water. It has been this medium stage that has been the impassable one at this place.

Future improvements.—It is believed that the work already carried out at these rapids is all that is required for the near future, and until the surveys of the upper river are completed and a final scheme of improvement adopted no further work will be required.

Rock Island rapids.—On the completion of the operations at Cabinet Rapids, the drill scows were lined to Rock Island Rapids. We left Cabinet Rapids on February 15, and on the 25th of the same month we reached Rock Island Rapids. A slight delay was caused at the Nixon Rapid by the low stage of the river, and we lost

about 6 f 44-inch line by one of the small boats swamping. Beyond this, day as is.

of improvements.—For Rock Island Rapids, the plans were the same as Cabinet Rapids, but, on account of the short time before high water, I was directed to confine the work as far as possible to opening a channel through which boats could be lined on an upgoing trip.

On February 25, drilling was commenced and continued until May 5; the first work started was the removal of rock T (see map); this was followed by the clearing of the reef between the right bank and rock O, the blasting of a part of rock that nearly closed this channel at its head, and the blasting of the point S. While this work was being carried out, I received information that the steamer *City of Ellensburg*, then tied up above Rock Island Rapids, was to be placed in commission and run between Port Eaton (below Cabinet Rapids) and the Okanogan River, so 80 miles above Rock Island Rapids.

I then, at the request of the owners of the steamer, removed as much of the point R and rock e as the rising river would permit, so as to give as straight channel possible for the descending steamer.

After blasting all the rock possible this season, I placed ten sets of ring bolts at all feasible points for use of ascending steamers.

Results.—At Rock Island Rapids 3 cubic yards of basalt rock have been blasted using 1,325 pounds of dynamite. 256 2-inch holes were drilled having a total length of 1,414 feet. As stated above, the work at Rock Island Rapids was carried out from February 25 to May 5, 1892, during which period 1,173 days' labor was expended, including Sundays and holidays; 15 men were employed in most of these were employed in most of the work was 15 men.

The average daily force employed was 15 men. Having completed all that was possible, I dropped the drill screws back to the original level before high water on May 6, 1892, and the work was 15 men.

On the 7th instant I paid off and discharged the men and having packed all tools and equipment on the steamer, I left the rest of the force and, leaving a watchman at the mill, returned to the Portland office. I paid off the men and cleaned the boats, I paid off the watchman at the mill, I reported to you in charge of the scow, I reported to you

Before leaving Rock Island I erected a permanent river gauge reading from low water this year to 40 feet above.

Condition of plant.—During the last month of the work at Rock Island Rapids I employed a carpenter, who was engaged in repairing and painting all the small boats used upon this work and the survey of the upper river, which had been left without a survey when that survey was stopped. These boats (seven in number) have been placed in good order and the entire plant is in good condition, with the possible exception of the lines and oars, the work in the rocky channels having been the cause of breakings and losing a quantity of these articles.

Future improvements.—This rapid is still a dangerous one for boats to pass down over. I would recommend the removal of rocks a, b, c, i, k, and a part of rock o, at U, V, and R, so as to give a safe channel for boats to descend during high and medium water. This work can be done in a single season and would afford relief while the permanent improvements are being planned and carried out upon the upper river.

In this connection I deem it right to call attention to the fact that the Great Northern Railroad is now building along the left bank of the river from below to point above Rock Island Rapids, where it will cross the river on its way to the Pacific Ocean, thus forming an outlet to this upper country above Priest, Cabinet, and Rock Island Rapids. The only other railroad that now reaches the river below Kettle Falls is the Northern Pacific, crossing at Pasco, but this place can not be made available as an outlet for the upper country until such time as the improvement of Priest Rapids is carried out. There is a possibility of a railroad being built from Ellensburg, on the Northern Pacific, to Port Eaton. If this road is built it will be for the purpose of developing the steamboat trade upon the upper river, and the clearing of Rock Island Rapids becomes of vital importance and the clearing them would be for the benefit of that stretch of country that is now without a permanent connection with the marts of this coast.

This and Priest Rapids will always be a difficult place for boats to pass over unless either a system of locks or boat railways are built at these places, because of the great fall that takes place in the river profile at these points, and until such time as the obstructions at the Dalles and the Cascades are overcome, the steamboat traffic up this river will be in connection with the railroads.

Priest Rapids.—During the past year no work of any sort was carried out at this place, work having stopped before the end of last year.

Commerce.—During the past year there were no steamers or other vessels navigating this stretch of river. On the 1st of May the *City of Ellensburg* was placed in commission and began to make two trips a week from above Rock Island to the Okanogan River.

It is not out of place to call attention to the fact that this boat passes over all the riffles and rapids to, and including the Methow, without the use of lines, and before she was overhauled she lined at six different places on the route.

The building of the Great Northern Railroad has called renewed attention to this large section of country, and settlers are coming in from all points.

During the past year the fall of the river from high to low water at Moses Coulee Ferry was 40 feet, the low water uncovering rock c, at the foot of Rock Island Rapids, which I am informed had not happened for five years.

Visits of inspection have been paid to this work by Col. Mendell, yourself, Lieut. Fitch, and Lieut. Shunk.

There were also taken during the month of September, 1891, photographic views of Cabinet and Rock Island rapids, which have been transmitted to you.

Accidents.—During the past year there has been no accident or other mishap of a serious nature.

Very respectfully, your obedient servant,

J. G. HOLCOMBE.

Capt. T. W. SYMONS,
Corps of Engineers.

SURVEY OF THE COLUMBIA RIVER FROM THE INTERNATIONAL BOUNDARY LINE TO THE HEAD OF ROCK ISLAND RAPIDS.

The amount expended on this work to June 30, 1891, was \$2,691.91, with the result of nearly completing the survey from the boundary to the mouth of the Spokane River.

The amount expended during the year ending June 30, 1892, was \$7,227.95, making the total amount expended on the project to the latter date, \$9,919.86.

The work has been practically completed as far as the mouth of the Okanogan River. The maps, plans, and profiles of the river, as far as surveyed, are well under way, and will be completed in a short time.

Report of operations.—At the beginning of the fiscal year the party under Mr. William Cuthbert engaged on the survey, were in camp at the mouth of the Spokane, working up their notes and awaiting the subsidence of the waters.

On the 13th of July they started from camp and went back to the boundary line and commenced a line of levels which had not been taken in first traversing the river, and getting the topography, soundings, etc.

From the mouth of the Spokane down, the line of levels was run at the same time with the other survey.

Twenty-eight bench marks were cut in the solid rock, and one was marked on a cedar post where the survey terminated at the mouth of the Okanogan. (A list of these bench marks is given in Mr. Cuthbert's report.)

The latter part of August the running of the levels had been completed and the party were again in camp at the mouth of the Spokane.

It was not until September 26 that the river had fallen to a stage fitted for the work of continuing the survey. Upon this date Mr. Cuthbert with his party of eleven men started from the mouth of the Spokane and the field work of the survey continued until December 20, when the Okanogan was reached. The party then proceeded down the river, some of the men leaving at Wenatchee, until the drill scows at Rock Island Rapids, under Mr. J. G. Holcombe, were reached. Here were left the boats, camp outfit, etc., and after discharging his men, Mr. Cuthbert came on to Portland and has been at work ever since in plating his work.

In making this survey, the topography has been worked in by stadia, and connection has been made, wherever possible, with land office stakes;

the levels have been run by means of a regular Y level and the soundings taken in the ordinary manner with lines and poles, located by the staffs and cross sights.

Besides the instrumental survey which forms the basis for the map, a large number of photographs have been taken all along the river from the boundary line as far as the survey has progressed. These photographs are particularly of the bad places in the river.

There were taken seventy-one views from the boundary to the Spokane, and sixty-four from the Spokane to the Okanogan. Of these views eighty-nine in quintuplicate were sent for exhibition at the World's Fair at Chicago. There were also sent with these fifteen photographs in quintuplicate, of Priest, Cabinet, and Rock Island rapids.

It is believed that a very correct idea of the river can be obtained from the maps, profiles, and these photographs.

At a number of places the river was gauged.

Near Marcus, which is just above Kettle Falls, the measurements were made at a stage of 8 feet below high water, and gave a discharge of 196,000 cubic feet per second. By allowing for an increased height of 8 feet to bring the river up to high water and with the observed rate of current, the discharge is 250,000 cubic feet per second.

At Bickers, below Grand Rapids, the measurements were taken at a stage 6 feet above low water, and gave a discharge of 112,000 cubic feet per second. Taking 6 feet in elevation away from the cross section gives a discharge at low water of 53,000 cubic feet per second.

As the velocity at high water would undoubtedly be increased over the velocity measured at 8 feet below the high-water stage, and the velocity at low water would be decreased below that observed at stage 6 feet above low water, it may be taken as an approximate general summary that in this upper part of its course, the Columbia has low-water flow of about 50,000 cubic feet per second, and a high-water flow of about 300,000 cubic feet per second.

The total length of the Columbia from the boundary line to the mouth of the Okanogan was found to be 214 miles, and in this distance the fall was found to be 524.4 feet, being an average fall per mile of 2.45 feet. The average fall from the boundary to the Spokane is 2.2 feet per mile and from the Spokane to the Okanogan is 2.8 feet per mile. In the stretch of possibly navigable water from the Okanogan to Kettle Falls the fall is 1.97 feet per mile, and from below Grand Rapids to the Spokane the fall per mile is 1.62 feet.

As nearly as can be ascertained the average velocity of the water from the boundary line to the Okanogan is 3.48 miles per hour.

The maximum velocity observed was at the upper part of the Spokane Rapids, where it was 15.38 miles per hour. There were a number of places where the velocity was found to be 10 to 12 miles per hour. A list of places where the most rapid flow was observed is given in Cuthbert's report.

Recommendations and remarks.—It is very much to be hoped that this work can be continued until the whole of the Columbia River surveyed.

It is not probable that this river for its whole length in our territory will ever be used as a through highway of commerce, but unquestionably certain portions of it will be used as feeders to railroads, and it will become of great importance. The survey will enable correct conclusions to be arrived at regarding these portions, the kind of boats necessary and proper to put on, and the improvements which may be necessary to utilize the river for the greatest benefit of the people.

f the amount named in the river and harbor bill now before Congress is appropriated, it will complete the survey to Rock Island Rapids, and no more money will be required for the survey of this portion of the river.

If it does not, I would recommend an appropriation of \$10,000 to complete the survey to Rock Island Rapids.

Report of Mr. Cuthbert is herewith:

UNITED STATES ENGINEER OFFICE,
Portland, Oregon, June 3, 1892.

CAPTAIN: I have the honor to report that in pursuance of instructions contained in your letter dated June 17, 1891, "to go back to the boundary line and run a line of levels down the river, * * * " and of verbal instructions received from you at Spokane City on July 2, 1891, I proceeded to camp at the mouth of the Spokane River, and the Columbia River having fallen sufficiently to allow of it, started from there on Monday, July 13, 1891, with four men to tow one of the boats in my charge to Rickey's Landing, a distance of 55 miles.

This was accomplished by 3 p. m. on Saturday, the 18th, having made an average of 10 miles per day with a loaded boat.

On Sunday, July 19, the boat and baggage was hauled by team to Meyers Siding, station on the Spokane Falls and Northern Railway, and the boat loaded onto a flat car which had been previously arranged for.

On Tuesday, July 21, started from Meyer's Siding at 5:30 a. m., reaching Corbin's Landing on the Columbia at 10 a. m., where the boat and baggage were transferred to the steamboat *Lytton*, and the international boundary line was reached at 12:30 p. m.

Bench marks.—Twenty-eight bench marks (a list of which, giving their positions and elevations, is attached) have been cut in the solid rock. The twenty-ninth is a star post near the mouth of the Okanogan River. I could not find any rock in that immediate locality.

Levels.—Establishing the elevation of bench mark No. 0 at the international boundary line from the levels of the Spokane Falls and Northern Railway, which are connected from the sea level datum of the Northern Pacific Railway, as 1,356 feet above sea level, I proceeded leveling down river, taking the water surface and all well defined high-water marks. The river at this time was 7 feet below the high-water stage.

Connections with the levels already taken at all important places during the low-water stage were made. (A list of levels and falls is attached.)

On July 30 I got down to Little Dalles. There I had to portage the boat around, the high stage of water rendered the river unnavigable, and Marcus was reached on Saturday, August 8, 1891.

Soundings.—From Marcus downwards I re-sounded the river, the depths being related to the water-surface line of levels and a good check on the soundings of the former survey.

Volume of flow.—At a very favorable spot about 2 miles above Marcus, I tested the volume of flow of water. This I have done at several other places and at various stages of water.

All of these, allowing for the different heights of water, compare very well. I took out two of them, those nearest to high and low waters:

Marcus:	Cubic feet per second.
Eight feet below high water.....	195, 621
Allow 8 feet higher water, but no increase in rate of current, and it will give	249, 853
Rickey's Landing:	
Six feet above low water	112, 149
Taking 6 feet in elevation from cross sections, but not reducing rate of current	53, 528
This will give an average of.....	151, 600

From this average the flow during the average day would be 13,106,050,200 cubic feet.

Rates of flow.—With a view of ascertaining what rate of speed a steamboat to successfully navigate this river should be capable of, I tested the rate of surface flow in places where it is quicker than usual.

From these tests and observations of slower currents, I ascertain as nearly as possible that the average rate of surface flow from the international boundary to the

mouth of the Okanogan River is 3.48 miles per hour. The particular places where the flow is most rapid are as follows:

Names of places.	Miles from boundary line.	Miles per hour.
Pingstone Rapids.....	31	8.25
For 2 miles above Rogers Bar.....	80	8
1 mile below Rogers Bar.....	83	8
Deer Creek.....	94	7.15
Upper end of Spokane Indian Reservation.....	97	5
Miter Rock.....	100	6
Spokane Rapids, upper part.....	105	15.20
China Rapids.....	109	12.7
Above Hawk Creek for 4,000 feet.....	110	11
Below Hawk Creek:		
For 6,000 feet.....	111	19
For 3,000 feet.....	112	17
For 3,000 feet.....	113	19
For 2,000 feet.....	114	12
Friedlander's store.....	124	11
Below mouth of Sans Poil River.....	133	9
Mammoth Spring.....	136	6
Cayuse Rapid.....	142	7
Nespillem Rapid.....	165	7.5
Mah-kin Rapid.....	175	10.21
Parsons Rapid.....	182	6.40
7,000 feet below Island Shoal.....	185	7
Long Rapids:		
First Riffle.....	190	9.28
Second Riffle.....	191	10.34
Third Riffle.....	191	10.50
Eagle Rapids.....	193	7.25
Foster Creek Rapids.....	201	6.25
Bend above Port Columbia.....	211	6

On the evening of Saturday, August 29, I had got to within 2 miles of the head of Spokane Rapids. Here I stopped until I had a full crew, to take cross soundings at frequent intervals above these rapids, so as to determine what the result to this part of the river would be were the boulders at present causing the obstruction blasted away.

Survey resumed.—The river having fallen sufficiently to allow of the regular survey being gone on with advantage, I recommenced work with a full party of 11 men, including Mr. O. C. Yocum as photographer, on Saturday, September 26, and found that there is very deep water above the Spokane Rapid, ranging from 51 to 69 feet, with a sluggish current of about $2\frac{1}{2}$ miles per hour.

Hell Gate was reached on Sunday, October 25. At this place, the current at low water is not strong nor would it be hard to navigate; the difficult stages are at middle and high waters on account of the tortuous shape of the channel. If the middle high-water channel was blasted out to low-water level, this trouble would be very much reduced if not done away with entirely.

From this down to Mah-kin Rapid, which in character very much resembles the upper part of Spokane Rapid, the river is comparatively good. Improvements would consist of blasting away lone boulder rocks, to straighten or widen the channel as the case may be.

Mah-kin Rapid was reached on the 6th day of November. There is a shoal 3,000 feet above it, which I imagine will render it inexpedient to clear out and thereby lower the head of the rapid, but the foot of it could be dammed so as to lengthen and ease the fall and current.

A steamboat to get up here and similar places without lining would have to be capable of making 18 miles an hour in dead water and of keeping up her steam pressure.

On November 24 I got down to the head of Long Rapids in Nespillem Cañon. The lower and narrower part of this rapid is locally known as Box Cañon, the banks on both sides being composed of perpendicular rock 60 to 100 feet in height. At high water it is full of cross currents and whirlpools, owing to the crookedness of the passage. A very large quantity of rock must be removed from the jutting out points all the way through. The low-water depth will average all of 60 feet.

Permanent snow.—Although there was a fall of snow and sleet on November 9, it did not remain. The commencement of permanent snow was on Sunday, December 6. I was then 3 miles above Foster Creek Rapids. From this on, snowstorms and dense fogs greatly impeded the work and rendered it disagreeably cold.

Creek Rapids at low water is a mass of rocks; 34 can be counted above and there is an equal number within 3 feet of the surface.

This down to the mouth of the Okanogan River, which is 214 miles from the original boundary line, the river is good and navigable at any stage of water.

Work at this place on December 20, and proceeded down river with the view of sending the men to their homes from Pasco.

Left at Chelan December 22; went up the lake with Mr. Yocum the photographer, and came down again on the 23d, having obtained sixteen views of the outlet lake. I left the mouth of the Chelan River on the 26th and arrived at the mouth of the Wenatchee December 29, with a snow storm blowing very hard up

here the Spokane men asked to be allowed to go home; so, retaining as my own (four) as would enable me to handle the two boats, I let the Spokane men with the two boats go down through Rock Island Rapids to the scows of Mr. Holcombe, at Cabinet Rapids, on January 1, 1892.

Holcombe informed me that I could not take the boats down through Priest Rapids. I turned over everything I had brought down to him, discharged the four men, and came to this city via Ellensburg, arriving on the 6th day of January, 1892.

Since my arrival here I have been engaged in preparing plans and maps of the part of the river surveyed. These are not yet quite completed.

The photographic work done consisted in seventy-one views of the river.—The photographic work done consisted in seventy-one views of the river being taken at points above the Spokane River by Mr. Warren, and forty-two views of the Spokane River by Mr. Yocum, besides the sixteen he took of Lake

I am, very respectfully, your obedient servant,

WM. CUTHBERT,
Assistant Engineer.

T. W. SYMONS,
Lieutenant of Engineers, U. S. A.

List of bench marks.

Positions.	Elevations.
	<i>Fet.</i>
right bank at International Boundary	1,356
left bank at Murphys Rapid	1,333.68
left bank at Steamboat Rock	1,336.56
left bank at Sheep Creek Bend	1,360.85
left bank at Little Dalles	1,335.06
right bank 21 miles from boundary	1,304.64
left bank 27 miles from boundary	1,290.52
right bank at Pingstone Rapids	1,267.58
right bank at Marcus Rapids	1,265.90
left bank at Kettle Falls	1,241.91
left bank at Grand Rapids	1,201.68
left bank at Driftwood Rock Islands	1,193.11
left bank at Giffords Rocks	1,173.16
left bank at Turtle Rapid	1,157.35
left bank at Spencers Bar	1,168.95
left bank below Rogers Bar	1,151.49
right bank below Elbow Bend	1,148.45
left bank at Miter Rock	1,128.24
left bank at Spokane Rapids	1,118.15
left bank at foot of China Rapids	1,095.69
left bank at Whitestone Rock	1,075.78
left bank at Hall Gate	1,051.24
right bank below Neahkwa Creek	1,020.70
left bank opposite Neapilem River	969.30
right bank at Wild-geese Bills Ferry	947.17
right bank at Gaviota Bend	926.84
right bank at Box Cañon	899.30
left bank at Foster Creek Rapids	853.24
left bank at Port Columbia	817.21

Memoranda of levels and falls at a

	Elevat
International Boundary.....	<i>Fee</i> 1, 30
to	
Above Kettle Falls.....	1, 22
Below Kettle Falls.....	1, 15
Below Grand Rapids.....	1, 16
to	
Mouth of Spokane River.....	1, 07
Boundary to Spokane River.....	
Spokane to Okanogan River.....	
Boundary to Okanogan River.....	

COMMERCIAL STATIST

There is no commerce on the portion of the Colu Rock Island Rapids and the foot of Priest Rapids.

From the head of Rock Island Rapids to the O steamer running, and as soon as the Great Northern will probably be others.

Although this stretch of river between the Okar not that under improvement, it is deemed proper t ties " of it as furnished by the Ellensburg and Oka

Steamer *City of Ellensburg*, plying on this route, and June. Her registered tonnage is 85.15, and dr

Amount of freight carried during May and June Okanogan River and intermediate points was 238 chandise and lumber.

On the down trip the amount of freight carried merchandise and lumber.

Average number of passengers carried on each tr

T T 10.

IMPROVEMENT OF CHEHALIS RIV

Description of original condition.—The p that it is possible to navigate, is about 96 quato to its mouth. This may well be cons

First. From the mouth to Montesano, a d about 18 feet of water at high water, and ce portion of the river.

Second. From Montesano to Elma, 16 affected by the tides, and has in general su boats. But navigation is obstructed by lc summer by a scarcity of water on the bars.

Third. Above Elma the river is practic summer and fall by snags and a general lc the river is a succession of shoals and poo the depth is reported to be only from 6 to 1

Plan of improvement.—The plan of imp removal of snags, overhanging trees, dams, d obstructions to navigation.

APPROPRIATIONS.

Set of—		
August 2, 1883		\$3,000
July 5, 1884		2,500
August 5, 1886		2,500
August 11, 1888		2,000
September 19, 1890		3,000
Total		13,000

The amount expended to June 30, 1891, was \$10,614.60.

Results obtained to June 30, 1891.—During October and November, 1890, 51 snags, 9 trees, 45 piles, 6 stumps, 4 roots, and a quantity of drift were removed from the river between Montesano and Gray Harbor. The river between these places was put into good condition for navigation.

The amount expended during fiscal year ending June 30, 1892, was \$100.68.

Results obtained to June 30, 1892.—The river from Montesano to the bay has been kept open and all troublesome snags have been removed.

Report of operations.—During October, arrangements were made with Capt. Tew for the use of the steamer *Aberdeen*, and 3 troublesome snags, averaging 30 feet in length, were removed, and 5 fallen trees, averaging 90 feet in length and 6 feet in circumference, were sawed off and towed out of the channel.

Recommendations and remarks.—Above the town of Montesano there is practically no navigation on the Chehalis except the floating down of logs and an occasional trip of a steamer during high water.

The river between Montesano and the bay is now navigated without trouble by the steamers suited therefor.

A project has been submitted for the improvement of the Chehalis River to allow ocean-going craft to ascend to Montesano. A railroad has been built from Gray Harbor along the Chehalis to Olympia and to Centralia, and is now operated from Aberdeen eastward.

In view of the present good condition of the river from Montesano to the head of the bay, of the special project heretofore submitted for its deepening to permit the passage of ocean ships, and of the railroad now in operation which will still further lessen the importance as a navigable stream of the upper Chehalis, I do not recommend another appropriation for the Chehalis at present.

There is available from the last appropriation \$2,284.72, which will suffice to remove any dangerous snags which may appear in the lower river another year at least.

Future operations.—With the existing appropriation it is expected to clear the river below Montesano of snags as soon as they are found to exist and be troublesome.

Money statement.

July 1, 1891, balance unexpended	\$2,385.40
June 30, 1892, amount expended during fiscal year	100.68
July 1, 1892, balance unexpended	2,284.72

COMMERCIAL STATISTICS FOR GRAY HARBOR AND CHEHALIS RIVER.

The following returns relative to the commerce of Gray Harbor and the Chehalis River are furnished by Mr. W. B. Mack, secretary, Aberdeen Chamber of Commerce, and are for the fiscal year ending June 30, 1892.

2728 REPORT OF THE CHIEF OF ENGINEERS, U. S. ARMY.

Vessels arrived and cleared.—The number of vessels of all descriptions crossing over the bar during the year was 250; the number crossing out was 255.

Most of these vessels are regular traders. The maximum lumber cargo taken out 950,000 feet, and minimum 200,000 feet. Maximum draft of water, when loaded, feet 6 inches; minimum draft of water, when loaded, 12 feet.

EXPORTS.

Articles.	Quantity.	Value.	Remarks.
Lumber..... feet, B. M.	75,682,000		Canneries not running.
Do..... tons	151,364	\$1,097,400	
Salmon in bulk..... do	(*)		
Fruit..... do	(*)		
Hides..... do	210	31,500	
Lath..... do	(*)		Large amount shipped as breakage Do.
Shingles..... do	(*)		
Total.....	151,574		

* No record.

Lath and shingles are shipped as breakage and no record is kept. The year 18 was a poor year for the lumber business. About 25,000,000 feet was used on the harbor, besides amount shipped as above. No record kept of amount shipped by rail. Canneries did not run last year. A large amount of salmon was shipped in bulk of which no record was kept.

IMPORTS.

Articles.	Quantity.	Value.	Remarks.
General merchandise, machinery, pig iron, hay and feed, and live stock.	Tons. 41,000	\$2,343,150	No record of separate articles.

The above shipments were made by steam and sailing vessels; no record available of freight receipts by rail.

Farmers on Gray Harbor are beginning to supply the home market with products thus reducing the incoming shipments.

Vessels built during the year.

Name.	Registered tonnage.	Draught
Steamer City of Aberdeen.....	244.84	Feet
Steamer Clan McDonald.....	230.48
Four-masted barkentine Chehalis.....	686.00
Four-masted barkentine Gleaner.....	392.68

CHEHALIS RIVER TRAFFIC.

Chehalis River is in the collection district of Oregon. The nearest light-house on Toke Point at the entrance to Shoalwater Bay, 16 miles south of Gray Harbor into which the river empties. Aberdeen is the support of entry.

Number of river steamers plying upon Chehalis River to Gray Harbor, 9.

Estimated amount of freight carried by same, 20 tons per day besides lumber, etc. moved on scows and lighters. (Cattle and other live stock are also delivered in this way.)

Steamer *Tillie* makes regular trips to Oychut and Damons Point.

Steamer *Aberdeen* makes regular trips from South Aberdeen to Hoquiam.

Steamer *Typhoon* makes regular trips from South Aberdeen to Petersons Point.

Steamer *Clan McDonald* makes regular trips from Montesano to Petersons Point.

Steamers *Progress*, *Jessie*, and *Chicago* make regular trips from Aberdeen to South Aberdeen.

The naphtha launch *Sun Beam* makes daily trips with the mail up the Wishkawer, 14 miles. These boats all touch at way landings.

One new shingle mill has recently been built and is now in operation, and six more are in course of construction at different points on the river and harbor. Two box stores also established, one in operation, and the other about finished. At least six more sawmills will be built during the coming year. The Northern Pacific Railroad is now in operation to South Aberdeen, and lumber and shingles are being loaded over this line.

Acres under cultivation have increased about 100 per cent during the year, and a corresponding increase is anticipated during 1892.

T T II.

IMPROVEMENT OF SKAGIT, STILLAGUAMISH, NOOKSACK, SNOHOMISH, AND SNOQUALMIE RIVERS, WASHINGTON.

Description of original condition.—The waters of these rivers all rise in the Cascade Range and flow to the west into Puget Sound between the forty-eighth and forty-ninth parallels of latitude.

In their original condition they carried water enough for the purpose of steamer navigation, but were obstructed by snags, fallen trees, and jams. They were the great highways of the country, traveling on them being almost impossible on account of the heavy timber, dense underbrush, and fallen logs.

The Snoqualmie is a branch of the Snohomish.

The aggregate length of all is estimated to be about 250 miles.

Plan of improvement.—The plan of improvement contemplates the removal of snags, logs, trees, and other obstructions to navigation. For this purpose a snag boat with an outfit of tools and appliances has been provided. This boat passes from one river to another, doing service in each as far as the necessities of its commerce require and as an amount of the funds appropriated will admit.

With the funds that have been appropriated this snag boat and outfit have been provided, and a large number of snags, jams, overhanging trees, and other obstructions to their navigation removed. These, with the trees that from time to time fall into the rivers, will furnish constant employment for the snag boat for an indefinite period.

APPROPRIATIONS.

Cost of—

June 14, 1880	\$2,500
August 2, 1882	20,000
July 5, 1884	10,000
August 5, 1886	10,000
August 11, 1888	15,000
September 19, 1890	12,000
Total	69,500

The amount expended to June 30, 1891, was \$62,118.51.

Results obtained to June 30, 1891.—In this class of work no permanent results are aimed at or obtained. The constantly recurring snags and fallen trees, etc., are required to be constantly removed. The rivers have been kept open for navigation and many of the perils of navigation have been removed.

The amount expended during fiscal year ending June 30, 1892, was \$377.07.

Results obtained to June 30, 1892.—The results obtained were to greatly benefit navigation.

Report of operations.—In July, 1891, the snag boat was put in commission and after receiving her supplies at Seattle proceeded to the Snohomish, where snagging operations were carried on until September 11.

On September 11 the boat started for the Skagit, stopping on the way at the Stillaguamish to do some necessary work.

Work was continued on the Skagit from September 15 to September 29, at which date the boat started for the Nooksack River.

Work was continued on the Nooksack until November 6.

On this date the boat started for its winter quarters, stopping, however, in the Skagit and Stillaguamish to do some needed work.

The boat was laid up in Deadwater Slough of the Snohomish River November 15, 1891.

During the working season 1,175 snags were removed and 144 leaning trees cut from the banks to prevent them from falling into the stream.

Capt. Jefferson's report, which is herewith, gives details of the work done.

Capt. Jefferson has been employed during the time the snag boat has been laid up on duties connected with the building of bridges and obstructions in the navigable waters of western Washington. In this connection his services have been of great value.

His report giving a summary of the work done is herewith.

Recommendations and remarks.—The minor waterways of western Washington, consisting of the tributaries of Puget Sound, are of great importance in the development and prosperity of the country, and they should be kept as free from snags and other obstructions as possible.

The snag boat is on hand and can be kept employed almost constantly to good advantage.

An appropriation of \$30,000 is recommended for this work.

Future operations.—The snag boat will be put to work and kept at work to as great an extent as possible with the funds available.

Money statement.

July 1, 1891, balance unexpended	\$7,381.49
June 30, 1892, amount expended during fiscal year	5,377.07
<hr/>	
July 1, 1892, balance unexpended	2,004.42
July 1, 1892, outstanding liabilities	200.00
<hr/>	
July 1, 1892, balance available	1,804.42
Amount appropriated by act approved July 13, 1892 *	15,000.00
<hr/>	
Amount available for fiscal year ending June 30, 1893	16,804.42
<hr/>	
{ Amount that can be profitably expended in fiscal year ending June 30, 1894	30,000.00
{ Submitted in compliance with requirements of sections 2 of river and	
{ harbor acts of 1866 and 1867.	

* This appropriation was made under the title "Improving Puget Sound and its tributary waters, Washington."

REPORTS OF MR. E. H. JEFFERSON.

(1)

SEATTLE, WASH., June 8, 1892.

SIR: As master of the United States snag boat *Skagit*, I have the honor to submit the following annual report of operations for the seasons of 1891 and 1892, at improvement of *Skagit* and other rivers, Washington:

On July 21, 1891, instructions being received, preparations were commenced for placing the boat in commission again, she having been laid up for several months.

A crew was engaged, the boat got in readiness, taken to Seattle to receive supplies, and on the 31st of same month commenced snagging operations at Clarks Bend in the Snohomish River, continuing work in that vicinity till August 8. On that date started for a trip of examination to the different rivers named in the appropriation bill, with Col. G. H. Mendell and yourself on board. The Stillaguamish, *Skagit*, and *Nooksack* rivers were visited and examined as far up as was deemed practicable.

Returned again to the Snohomish River on the evening of the 10th, and resumed snagging operations the following day, continuing on that river working between its mouth and a point about 5 miles above Snohomish City until September 11, during which period Clarks Bend, a very bad place and dreaded by steamboat men, was thoroughly cleaned of obstructions, and the remains of an old abandoned log boom, a serious obstruction at low tide, was also removed.

September 11 departure was taken for the *Skagit* River, stopping at the Stillaguamish on the way, doing some necessary work there.

On the 15th of the same month commenced work on the *Skagit*, continuing there till the 28th, working between its mouth and the "Cut Off," a point about 35 miles up.

On the 29th of September departed for the *Nooksack* River, commencing work on that stream on October 1, continuing between its mouth and Lynden, the practical head of navigation, until November 6, removing many dangerous snags and pretty thoroughly clearing the channel to the point named.

November 7 passed out of the *Nooksack* River, bound for the *Skagit*. We were storm bound at Fairhaven two days, and arrived at Mt. Vernon on the *Skagit* on the 10th, finding the river banks full and too high for work.

November 12 worked near *Skagit* City, and in the evening ran over to the Stillaguamish.

November 13 worked in the south channel of Stillaguamish till high tide, then left for the Snohomish River to lay the boat up for the winter, as per orders previously received.

Laid the boat in Deadwater Slough near the town of Everett; took apart machinery, pipes, etc., properly cared for same and stowed away tools, gear, etc., finishing, and discharging the crew on the 15th of November, 1891.

Following is a summary of the snag boat's operations for the period stated:

Snags and drift removed:

Snohomish River.....	528
Stillaguamish River.....	43
<i>Skagit</i> River.....	176
<i>Nooksack</i> River.....	428
Total	1,175

Largest diameter.....	feet..	8
Smallest diameter.....	inches..	4
Total length.....	feet..	25,220

Leaning trees chopped from the banks:

Snohomish River.....	78
<i>Nooksack</i> River.....	66
Total	144

Largest diameter.....	feet..	4
Smallest diameter.....	inches..	3½
Total length.....	feet..	5,760

In addition to the above, the boat machinery, tools, gear, etc., were kept in repair. The snag boat has undoubtedly been of great service to the navigable interests of these rivers, and the comments of the steamboat men who know what has been done are generally of a favorable character. The only regrets seem to be that the appropriations are inefficient to keep the snag boat in commission the greater part of the time.

There will always be some complaint by people living on these streams from the fact that when the water is low steamers can not reach them, and of course the Government and its representatives are held responsible, or, rather, blamed for it, notwithstanding that at some of the shoal places a canoe can scarcely navigate.

The fact is our snag boat is rather too large for the upper waters of these streams; she can not get up to these shoal places when the water is low, and when she can get up, the water is too high and swift to do any work. For these swift shoal places she should be equipped with additional appliances, such as a spud, or groner, placed so that it could be dropped on the river bed through the deck and bottom of the boat, so as to hold her on these riffles, thus affording a chance to do work. A good hydraulic pump should also form part of her equipment, so the sand and gravel could be washed away from obstructions and thus permit them to be removed. A good heavy drag of some kind would also be useful.

Two regular steamers are kept employed on the Skagit River route from Seattle, and one locally from Mt. Vernon to "up-river" points when stage of water will permit. Occasional trips are made by other steamers.

The Nooksack River has two small steamers running from Bellingham Bay points when business demands.

On the Snohomish there are five regular steamers at present running from Tacoma and Seattle and six more that are engaged locally on it and its tributaries, the Snoqualmie and Skykomish. Tugs with lighters containing building material and machinery and steam barges with same made frequent trips to the Snohomish. In fact, there is more steamboat business done at the present time on the Snohomish and tributaries than on all the other rivers combined. Business on this stream has doubled and trebled during the year, on account of the establishment of the new town of Everett and the manufacturing enterprises connected therewith, as well as the boom given business by the construction of the Great Northern Railroad and the Everett and Monte Cristo Railroad of 61 miles to the mines of the Cascade Mountains.

Steamboat business has fallen off considerably on the other rivers, owing to the dull times and railroad competition to many river points.

Accurate commercial statistics are almost impossible to obtain for these rivers. The following, for merchandise in tons and passengers in numbers, are estimated to be nearly correct as representing the steamboat business on these rivers at the present time, and which has prevailed for the greater portion of the year. The figures given are monthly statements:

Route.	Freight.	Passengers.
	Tons.	No.
Skagit River	1,200	1,000
Nooksack River	300	100
Snohomish and tributaries	3,000	3,500

Very respectfully, your obedient servant,

Capt. T. W. SYMONS,
Corps of Engineers, U. S. A.

E. H. JEFFERSON.

(2.)

SEATTLE, WASH., June 8, 1892.

SIR: I have the honor to submit the following report of my operations in connection with the examination and inspection of the bridges and obstructions pertaining to the various rivers emptying into Puget Sound, Gray Harbor, and Willapa Bay, Washington, for the fiscal year about ending.

During the period stated the following bridges and structures have been examined, inspected, and reported upon:

A bridge over the Snohomish River at the city of Snohomish, built by the city and completed.

Bridges of the Snohomish, Skykomish and Spokane Railroad Company (now called the Everett and Monte Cristo Railroad) over the Snohomish River and Ebey Slough, completed.

Seattle and Northern Railroad Bridge over the Swinomish Slough, completed.
 Valley Street Railroad Company's bridge over the Duwamish River, completed.
 Northern Pacific Railroad Company's bridges over the Chehalis and Johns
 completed.
 county bridge over the Snoqualmie River, not yet commenced.
 county bridge over the Skagit River at Mount Vernon, not yet commenced.
 county bridges over the Willapa and Bear rivers, in course of construction.
 Everett Land and River Improvement Company's bridges over the Snohomish
 and Ebey Slough, in course of construction.
 Snohomish and Port Gardner Electric Motor Company's bridges over the Sno-
 homish River and Ebey Slough, in course of construction; and the Skagit County
 bridge over Swinomish Slough, in course of construction.
 Several trips have been made to the sites of some of these bridges, first, to examine
 and determine if they were properly located so as not to interfere with the inter-
 navigation, then to see that they were being built and completed according
 to plans submitted and approved.
 During the same period the following obstructions to navigation complained of
 have been examined and reported upon:
 obstructions of the entrance to the Nooksack River by the Bellingham Bay
 Company.
 obstructions of the Wishka River by the log boom of one Frank Miller.
 obstructions to the entrance to the Stillaguamish River by the Chinook Boom Com-
 pany.
 obstructions to Ebey Slough by the boom of Stimpson Mill Company.
 obstructions of Steamboat Slough (Snohomish River) by certain parties fell-
 ing trees in the stream while pursuing the work of diking land.
 obstructions of free and safe navigation of the Snohomish River by the sheer
 banks of Pearl & Tompkins.
 obstructive character of the bridges of the Seattle and Montana Railroad over
 Snohomish River and Steamboat Slough and a subsequent felling of trees into
 Steamboat Slough by the parties engaged in building a dike around Ebey Island.
 At most of these places measurements and soundings were taken.
 Very respectfully, your obedient servant,

E. H. JEFFERSON.

Wm. T. W. SYMONS,
Corps of Engineers, U. S. A.

T T 12.

PRELIMINARY EXAMINATION AND SURVEY OF OLYMPIA HARBOR, WASH-
 INGTON, FROM DEEP WATER IN BUDDS INLET TO FOURTH STREET
 BRIDGE, IN THE CITY OF OLYMPIA, AND SEPARATELY FROM SAID
 BRIDGE TO THE MOUTH OF THE DES CHUTES RIVER AT TUMWATER.

[Printed in House Ex. Doc. No. 32, Fifty-second Congress, first session.]

OFFICE OF THE CHIEF OF ENGINEERS,
 UNITED STATES ARMY,
 Washington, D. C., November 20, 1891.

SIR: I have the honor to submit herewith copy of report dated Janu-
 ary 12, 1891, on preliminary examination of "Olympia Harbor, Wash-
 ington, from deep water in Budds Inlet to Fourth Street Bridge in the
 city of Olympia, and separately from said bridge to the mouth of the
 Chutes River, at Tumwater," and copy of report, with map,* dated
 November 19, 1891, on survey of Olympia Harbor, Washington, from
 deep water in Budds Inlet to Fourth Street Bridge, Olympia, made by

* Not reprinted; printed in House Ex. Doc. No. 32, Fifty-second Congress, first ses-

Capt. Thomas W. Symons, Corps of Engineers, in compliance with provisions of river and harbor act approved September 19, 1890.

For the facts and reasons given in report of January 12, 1891, Capt. Symons does not consider Des Chutes River above the Fourth Street bridge, Olympia, to Tumwater, worthy of improvement. I concur in opinion.

The proposed improvement of Olympia Harbor below the Fourth Street bridge contemplates dredging a channel from deep water in Budds Inlet to the wharves near the bridge, 12 feet deep at low water, 250 feet wide, between pile and brush bulkheads built 400 feet beyond which the dredged material is to be deposited; and the construction of a basin at the upper end of the cut, near the bridge, 12 feet deep at low water, with an extreme length of 1,600 feet and maximum width of 500 feet, the bulkhead to be continued around the sides and ends of the basin, so as to deflect the current of the Des Chutes River, preventing the same from being made, however, to allow small boats to pass through the channel at the bridge. The cost of this work is estimated at \$275,000.

Very respectfully, your obedient servant,

THOS. LINCOLN CAS
Brig. Gen., Chief of Eng

HON. REDFIELD PROCTOR,
Secretary of War.

**PRELIMINARY EXAMINATION OF OLYMPIA HARBOR, WASH
FROM DEEP WATER IN BUDDS INLET TO FOURTH STREET
IN THE CITY OF OLYMPIA, AND SEPARATELY FROM SAID
TO THE MOUTH OF THE DES CHUTES RIVER, AT TUMWA**

UNITED STATES ENGINEER OFFICE
Portland, Oregon, January 1.

GENERAL: I have the honor to state that, in obedience to the instructions contained in your letter of September 20, 1890, a preliminary examination has been made of "Olympia Harbor, Washington, from deep water in Budds Inlet to Fourth Street bridge, in the city of Olympia, and separately from said bridge to the mouth of the Des Chutes River at Tumwater, and to report as to the most practical and economical channel, and the most feasible, economical, and suitable plan for improving the same for navigation by the class of vessels employed on Puget Sound, and also to cause to be made an estimate of the cost of such improvements."

In this connection attention is invited to the fact that in 1885 C. F. Powell, Corps of Engineers, submitted a report on Olympia Harbor with plan and estimate for its improvement, he having reported it was worthy of improvement. Captain Powell's report is contained in the Chief of Engineers' Report, 1885, page 2413.

In order to obtain the latest information in regard to the harbor of Olympia, Mr. A. J. McMillan was dispatched to the locality and his report which is herewith.

In my opinion Olympia Harbor from Fourth Street bridge to deep water in Budds Inlet is worthy of improvement, to make it navigable for the class of vessels employed on Puget Sound.

This opinion is based upon the facts set forth in Mr. McMillan's report. These may be summarized by stating that Olympia is the largest city of Washington and is a city of considerable commercial and manufacturing importance, which must naturally develop rapidly now

ilroads are getting there. The harbor is rendered very incon-
by reason of the extensive shoaling at the head of Budds Inlet.
ited States Coast and Geodetic Survey chart of Olympia Harbor
he situation very well.

gard to that portion of the inlet from Fourth street bridge to
ter, I do not deem it worthy of improvement for the reason set
Mr. McMillan's report.

ly all the data necessary for the preparation of plans and esti-
or the works of improvement recommended are available now in
ica. Only data showing recent changes will have to be gathered.
imated that this and the preparation of plans, estimates, and
will cost \$250.

Very respectfully, your obedient servant,

THOMAS W. SYMONS,
Captain, Corps of Engineers.

Gen. THOMAS L. CASEY,
Chief of Engineers, U. S. A.

ugh Col. G. H. Mendell, Corps of Engineers, Division Engineer,
Division.)

[First indorsement.]

U. S. ENGINEER OFFICE,
San Francisco, Cal., January 17, 1891.

ectfully forwarded.

asons herein stated, Olympia Harbor is worthy of improvement
e portion extending from Fourth street bridge to deep water in
Inlet.

G. H. MENDELL,
Colonel, Corps of Engineers, Division Engineer.

REPORT OF MR. A. J. McMILLAN.

OCTOBER 29, 1890.

IN: I have the honor of submitting the following report of a preliminary
ion of Olympia Harbor at the head of Budds Inlet, Puget Sound, State of
ton, made in accordance with your instructions.

he mouth of the Des Chutes River to 4,500 feet north of Fourth Street Bridge,
y of Olympia, is bare mud flat or tide land at low water, with the water of
hutes meandering through it, in a channel not deep enough in places to
at, making navigation impossible at that stage of the tide. It takes some-
r quarter tide for the stern-wheel steamers to reach Fourth Street Bridge.
a wharf piled out over the tide land for over a mile, but even that distance
reach water deep enough for the class of steamers that come to Olympia.
e people want is a channel deep enough for ocean steamers to reach the
of Fourth street; and that will necessitate the dredging of about 2 miles.
rial consists of fine gravel, sand, and clay, making easy dredging.

viewed several members of the Chamber of Commerce in regard to the item
er and harbor bill for making a channel from Fourth Street Bridge to Tum-
ed they all stated that they did not want or expect the Government to do
se it would cost a large amount of money, and would be of no benefit if it

rd to shipping, railways, manufactures, etc., I obtained the following in-
a:

are nine passenger steamers making regular trips from Olympia to points on
d.

nion Pacific, Northern Pacific, and the Port Townsend Southern Railway
s are building their lines within the city limits. The Northern Pacific
have a dredger at work at present at the foot of Seventh street, filling in

some 7 acres for depot grounds. The Union Pacific Company are going to fill in some 15 acres for the same purpose at another part of the harbor. The Port Townsend Southern Company are driving piling for coal bunkers. The coal will come from the company's large coal fields about 25 miles south of Olympia, near the Portland-Tacoma branch of the Northern Pacific Railway.

Everything points to Olympia becoming quite a railway and shipping center in the near future.

The manufactories of this place at present consists of two sawmills, one planer mill, one wooden water-pipe factory, and one foundry and machine shop. This place has until lately been without railway or shipping facilities, and there was no inducement for capitalists to invest in any kind of business.

Although Tumwater is about 2 miles from Olympia, it is practically a part of it. Tumwater has one sawmill, one sash and door factory, and one box factory. There is being built here a flouring mill of 100 barrels capacity; also a large dam for supplying power to electric-light and street-railway plants.

Tumwater has a population of about 400 and Olympia has about 6,000. Olympia has increased in population about 300 per month since last spring.

Very respectfully, your obedient servant,

A. J. McMILLAN.

Capt. THOMAS W. SYMONS,
Corps of Engineers, U. S. A.

SURVEY OF OLYMPIA HARBOR, WASHINGTON, FROM DEEP WATER IN
BUDDS INLET TO FOURTH STREET BRIDGE IN THE CITY OF OLYMPIA.

UNITED STATES ENGINEER OFFICE,
Portland, Oregon, September 19, 1891.

GENERAL: I have the honor to submit the following report upon the "improvement of Olympia Harbor":

The river and harbor bill approved September 19, 1890, contained an item for making a preliminary examination and estimate for "Olympia Harbor, Washington, from deep water in Budds Inlet to Fourth Street Bridge in the city of Olympia, and separately from said bridge to the mouth of the Des Chutes River at Tumwater, and to report as to the most practical and convenient channel and the most feasible, economical, and suitable plan for improving the same for navigation by the class of vessels employed on Puget Sound, and also to cause to be made an estimate of the cost of each of such improvements."

Being charged with the duty of making this examination and report I submitted a report thereon, under date of January 12, 1891.

Under date of January 23, 1891, I was charged with the survey of "Olympia Harbor, Washington, from deep water in Budds Inlet to Fourth Street Bridge, in the city of Olympia."

In 1885 a survey was made of Olympia Harbor, and plans and estimates for its improvement made by Capt. C. F. Powell, Corps of Engineers. The map made at this time was used as the basis of the present plans and estimates, the only changes being the necessary corrections to bring it up to date. The tracing submitted* herewith shows the plan recommended for the improvement of the harbor.

Captain Powell's report, page 2413, Report of Chief of Engineers for 1885, the preliminary report already submitted by myself, Mr. DeCourey's report, which is herewith, and the map submitted, are, it is believed, sufficiently descriptive of the locality to need little further in this line. The situation, however, may be summarized as follows:

Olympia, the capital of the State of Washington, is situated near the head of Budds Inlet, which at this point is about 2,000 feet wide. At Olympia, and for about 4,000 feet below, the inlet is a tide flat, more bare at low water. The general range of the tides is about 18 to feet. At high water boats can go up to Fourth Street Bridge, but

* Not reprinted; printed in House Ex. Doc. No. 32, Fifty-second Congress, first session.

r they are compelled to land passengers and freight at the end of wharf about 1 mile from the city.

an proposed for the improvement of the harbor consists in a channel from deep water, so that "the class of vessels employed on Puget Sound" can, at nearly all times, reach the wharves at Street Bridge.

interpreted the phrase "the class of vessels employed on Puget Sound" to mean those vessels which ply from port to port on the sound, in distinction to the deep-draft, seagoing vessels which ply to and from the sound.

Following list gives the dimensions of the principal boats of the

Name.	Length.	Beam.	Net tons.	Draft.	Description.
	Feet.	Ft. in.		Ft. in.	
.....	261	40	1,083.20	9	Iron, side-wheel.
le	254	40	913.73	13	Iron, propeller.
ston	246	33 5	866.35	12 5	Do.
.....	200	42	602.00	9	Do.
on	200	34	577.81	10	Do.
.....	141	26	158.77	16	Wood, propeller.
.....	107	22	92.77	10 6	Iron, propeller.
.....	107	21	80.48	11	Do.
.....	230	35	589.00	10	Wood, side-wheel.
.....	154	30	350.00	9 8	Do.
son	134	25	197.49	7	Do.
.....	166	29	345.46	9	Do.
.....	148	28	336.00	8 6	Do.
.....	147	25	178.00	6 4	Do.
Whington	170	31	449.00	7	Wood, stern-wheel.
.....	137	26	241.00	5 6	Do.
ard	177	29	466.00	7 5	Do.
.....	141	24	193.08	5 6	Do.
.....	120	24	135.99	9 7	Wood, propeller.
n	108	23 5	166.00	4 6	Wood, stern-wheel.
.....	117	22	100.00	6 6	Do.
.....	108	25	209.00	4 6	Do.
.....	143	28	278.00	5	Do.
.....	160	30	350.00	6	Do.
snag boat Skagit	130	31	(?)	26	Do.

depth of 250 feet at lower low water was fixed upon as the proper depth for the channel, as this depth will accommodate very nearly all vessels plying on the sound and more than accommodate any now plying on Olympia. It is fair to suppose, however, that, as Olympia and vessels of the largest class will visit it.

A width of channel of 250 feet was fixed upon, as this will permit vessels to pass each other and will allow those at present calling at Olympia to do so. It is, however, deemed wise in laying out the work to provide for a width of 400 feet in the future, which will be desirable if the commerce of Olympia shall grow as anticipated by some. This will enable vessels to lie at each side of the channel and yet permit others to pass in and out.

In width it is proposed to provide for by putting the bulkheads, which the dredged material will be deposited, 400 feet apart. It is now proposed to dredge out a basin at the upper or southern end of the channel, in plan as shown on the accompanying map. This basin is an extreme length of 1,600 feet and a maximum breadth of 400 feet and is designed with a view of interfering as little as possible with the existing wharves and docks.

The method of construction proposed is to build a pile and brush wall on the line shown, and to pump the material from the channel into a basin situated beyond the bulkhead.

It is deemed best to keep the Des Chutes River out of this channel as it is feared that, under favoring circumstances, it would tend to fill the channel with debris from above. This deflection will be accomplished by building the bulkhead completely around the upper end and side of the channel. Provision for passing small boats through the draw of the Fourth street bridge can be made by limiting the height of the bulkhead at the draw to just sufficient elevation to turn the water into the Des Chutes. This will enable anything that is likely to desire to do so to get above the bridge.

The work of constructing the 250-foot channel with basin, as shown, is estimated to cost \$275,000.

ESTIMATE FOR THE IMPROVEMENT OF OLYMPIA HARBOR, WASHINGTON.

For a dredged channel 250 feet wide (bottom width) and 12 feet deep at water, bulkheads 400 feet apart, and a basin of equal depth in plan, as shown accompanying map:

70,775 linear feet of piles, at 15 cents driven	\$10,61
16,705 feet, B. M., cross bracing for basin bulkhead, at \$20 in place	33
9,400 feet, B. M., longitudinal bracing, bulkhead, at \$20	18
3,000 cords brush, at \$2.50 in place	7,50
1,000 tons stone ballast at \$2, in place	2,00
1,141,575 cubic yards excavation, at 20 cents dredged	228,31
3,000 pounds iron spikes, at 10 cents in place	30
Add 10 per cent for superintendence and contingencies	24,9

274,1

In round numbers

275,0

The following commercial statistics of Olympia have been furnished by the Board of Trade of the city:

Population of Olympia, census—

1890	4,
1889	3,

Increase in one year

Gain (per cent)

Population of Thurston County, census—

1890	9,
1889	7,

Increase in one year

Gain (per cent)

MANUFACTURING INTERESTS OF OLYMPIA.

West Side Mill Company (lumber): Employés, 60; capacity, 50,000 feet daily; capacity, lath, 10,000 daily; capacity, pickets, 10,000 daily; pay roll, \$1,000 per month.

D. Allison & Co. (lumber): Employés, 20; capacity, 10,000 feet daily; pay roll, \$1,300 per month.

George S. Allen (lumber): Employés, 30; capacity, 25,000 feet daily; capacity, lath, 10,000 daily; pay roll, \$2,200 per month.

Springer & White (lumber): Employés, 50; capacity, work up 2,000,000 feet yearly; capital, \$50,000; pay roll, \$3,200 per month.

T. Z. Slater (shingles): Employés, 20; capacity, 100,000 daily; pay roll, \$1,200 per month.

Tumwater Shingle Company: Employés, 15; capacity, 70,000 shingles daily; pay roll, \$1,000 per month.

Puget Sound Pipe Company (pipe factory): Capital, \$50,000; capacity, six assorted pipes per month; employés, 50; pay roll, \$4,000 per month.

Olympia Foundry and Machinery Company: Employés, 15; capacity of foundry, 10 tons daily; pay roll, \$1,000 per month.

Sternberg's Furniture Factory: Employés, 15; pay roll, \$1,000 per month.

Miller & Springer (cigar factory): Employés, 7; pay roll, \$450 per month.

Electric plant: Electric Light and Power Company. Flour mill: capacity, 100 barrels daily.

50 barrels; 50 tons chopped feed; complete corn-meal plant; employes, 10; pay roll, \$100 per month. Lighting plant: Capacity, 3 arc dynamos, operating 130 arc lights; 2 railroad generators, 150 horse power each; 2 incandescent lights; 1 motor, 100 horse power; employes, 15; pay roll, \$300 per month. They have 28 miles arc wire and 5000 feet of incandescent wire strung.
 Olympia water works: Employes, 15; pay roll, \$1,000 per month; capacity, 2,000,000 gallons daily; pressure, 45 pounds, both fire and domestic; reservoir holds 2,700,000 gallons.

NAVIGATION.

There are ten companies operating boats on the sound at Olympia, which are as follows, giving names of boats and their tonnage per annum for 1890:

Companies.	Names of boats.	Tonnage.
Union Pacific Railway Co	Steamer Potter	} 7,000
	Steamer Hassallo	
	Steamer Emma Hay ward	
	Steamer Olympian	
	Steamer North Pacific	
Columbia River and Puget Sound Navigation Co.	Bailey Gatzert	} 1,200
	Fleetwood	
Port Townsend Wharf and Navigation Co	Clara Brown	6,000
Willey Steamship Co	Multnomah	} 4,100
	Willey	
Puget Sound and Alaska Steamship Co	Washington	} 2,500
	W. K. Merwin	
Pacific Navigation Co	State of Washington	} 1,100
	Cascade	
Port Blakely Mill Co	Steamer Nellie	2,000
A. Paul and Tacoma Lumber Co	Zephyr	} 500
	Messenger	
Walter Artell	Jessie	500
H. M. Perin, agent	Steamer The Doctor	1,000
Total tonnage of all boats for 1890		25,900

The increase of tonnage on boats for year 1890 over 1889 is 30 per cent; at this rate the tonnage for 1891 will be 33,670 tons. This is a low estimate, as the Union Pacific Railroad Company has just closed one contract for receiving the material for a new court-house which amounts to over 6,000 tons.

The tonnage for the year 1890 on the Port Townsend Railroad Company was 15,000 tons.

The Northern-Pacific Railroad has just completed its line to Olympia, and the Union Pacific will be completed in a few months, so a conservative estimate of the increase in railroad tonnage would be at least 200 per cent, or 45,000 tons.

Total valuation of all property, as equalized by the board of equalization.

	1889.	1890.
City of Olympia	\$1,144,621	\$3,296,388
County (outside of Olympia)	1,492,065	3,280,477
Thurston County	2,636,686	6,556,865
Increase in valuation in one year		3,920,179
Per cent of gain in one year		148.6

FINAL ASSESSMENT FIGURES.

The final figures for the city assessment, as equalized by the Board, show the real estate assessment to be \$5,597,455, which was reduced

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\$40,617, leaving a total of \$5,556,838. The personal assessment was \$150,607, and was reduced \$3,225, making a total for both real and personal of \$6,319,670, on which a 6-mill tax is levied.

There is submitted herewith the report of Mr. Bolton W. DeCourcy, who made the reëxamination and survey of Olympia Harbor.

Very respectfully, your obedient servant,

THOMAS W. SYMONS,
Captain, Corps of Engineers.

Brig. Gen. THOMAS L. CASEY,
Chief of Engineers, U. S. A.

(Through Col. G. H. Mendell, Corps of Engineers, Division Engineer, Pacific Division.)

[First indorsement.]

U. S. ENGINEER OFFICE,
San Francisco, Cal., September 30, 1891.

Respectfully forwarded to the Chief of Engineers, Washington, D. C., recommended.

G. H. MENDELL,
Colonel, Corps of Engineers, Division Engineer.

REPORT OF MR. BOLTON W. DECOURCY.

PORTLAND, OREGON, *March 17, 1891.*

CAPTAIN: I have the honor to report that, in obedience to your order given February 16, 1891, I proceeded to Olympia, Wash., and made the survey as directed. I held consultation with most of the principal citizens to ascertain their wishes and ideas of what was necessary for the improvement of the harbor; also with the city officials, the names of a few of whom I will mention:

The mayor of Olympia, Mr. Ellis, Mr. Sickles, and others of the city council; Governor Laughton, General McKenny, Hon. Thos. H. Cavanaugh, surveyor-general; Captain McMicken, Mr. Phillips, president of the First National Bank; Mr. Shannon, vice-president of the same bank; Mr. Chaplin, Mr. Govey, Mr. Robinson, proprietor of the Olympia Daily Tribune; and Mr. Phelps, manager of the Port Townsend Southern Railroad. These gentlemen are among the heaviest tax-payers and largest property owners in Olympia.

I had conversations also with Messrs. Pease, Willey, Parker, Hennessey, and Hatch. These gentlemen command the largest steamers running between Olympia and Sound ports.

All these persons were unanimous in recommending and requesting the consideration of a channel commencing at the Fourth Street Bridge, where the draw is situated, and between the wharves owned by Mayor Horr and Mr. Ellis, and running direct in a northerly course to deep water.

The steamer masters ask that the channel be made quite straight in alignment, claiming that this would confer one of the most material benefits to be derived from an improved channel, as the fogs are frequent about Olympia and it would be difficult for them to keep their steamers from grounding should it deviate from a straight line.

The citizens advocate a depth of 16 feet at lower low water, on the expectation of, in that case, having the ocean steamships plying between San Francisco and Portland and the ports of Puget Sound come direct to Olympia to discharge any portion of their cargoes destined for that city, instead of transferring at Seattle or Tacoma, as at present.

The steamer masters mentioned above, on the other hand, say that 12 feet depth at lower low water is ample for all purposes.

I was informed that the daily arrival of steamers at the present time is nine, of which four are large ones plying from Seattle; the remainder are small, running to insignificant places on the different islands and inlets of the sound.

The different masters of these large boats do not deem a basin necessary for their convenience in turning their craft. They say they prefer backing out, taking advantage of the deep water in a northwest direction from the place where the sight

post is situated, then steaming ahead into the channel and turning north. Proceeding thus, they say, saves time over making a complete turn in a basin. However, should the channel be improved to 12 feet depth it will be necessary to also excavate some in this channel.

The Bowers dredge had completed its work when I arrived, and had left for Tacoma. I ascertained, however, that the dredge had excavated and deposited inside the revetment or bulkhead for the Northern Pacific Railroad Company 250,000 cubic yards.

The dredge was brought there from Tacoma about the 10th of November. The borrow-pit is parallel with the bulkheads and distant therefrom 500 feet. It is 400 feet wide and 17 feet deep at low water. At one place, in consequence of neglecting to cut off some springs, the pile bulkhead gave way. I took advantage of this place to endeavor to obtain the natural slope of the material, which is composed of sand, silt, and small gravel. As nearly as it was possible to judge, it showed two horizontal to one perpendicular.

As to width of channel, the masters of the steamers recommend, and the citizens request the consideration of a bottom width of 250 feet. The space left between the piling of the Horr and Ellis wharves is about 95 feet; otherwise there are no improvements encroaching on the space called for by such a channel.

The Des Chutes River has not sufficient water to be useful for scows; at lower low water its channel through the mud flats is not over 20 feet wide and 2 to 3 feet deep. It will be necessary to deflect it and prevent its discharge into any artificial channel made. The river has a small channel at the west end of the Fourth Street Bridge, which will become the main channel by preventing the water from getting into that dredged. This can be done by a short bulkhead above the Fourth Street Bridge near where the channel divides.

The steam vessels plying to Olympia, with their depth of draft, are as follows:

The *T. J. Potter*, 7 feet (according to her mate and first officer) or 10 feet (as given by Mr. Jefferson); the *Bailey Gatsert*, 7 feet; the *Washington*, 5½ feet; the *Multnomah*, 8 feet; the *Willie*, 4 feet; the *Nellie*, 4 feet; the *Clara Brown*, 6 feet; the *Jessie*, 4 feet; the *Colby*, 4 feet; the *Emma Hayward*, 7½ feet; the *Hassallo*, 6 feet. The first nine of the above are regular arrivals; the last two occasional. There are tugs also that put in, and sometimes a schooner. These draw from 8 to 9 feet.

The citizens of Olympia with whom I conferred all stated that the long wharf in the event of harbor improvement would be abandoned. In that case the piling might be used for revetment purposes as far as the wharf reaches, saving a good deal of new piling.

The total length of excavation required to reach 12 feet depth at lower low water will be 8,400 feet.

The steamer *Potter* is 230 feet in length, beam 35 feet, as stated by Mr. Jefferson. She is the largest steamer at present calling at Olympia.

The masters of the several steamers with whom I conversed said that when the wind was blowing hard, in order to turn they hold on to the wharf with their stern lines and swing the head round. This requires for the *Potter* about the width requested for the bottom, *i. e.*, 250 feet. A high wind would blow them onto the shallows if they endeavored to turn by backing out.

Piling can be procured at 5½ cents per linear foot.

The narrow gauge from Tenino, since it came into the hands of the Oregon Improvement Company, has been made standard gauge and extended to the coal bunkers at Britblers Cove, and its depot established at the crossings of Fourth Street Bridge.

The Tacoma, Olympia, and Gray's Harbor Railroad, being built by the Northern Pacific, is in an advanced state, and it is expected will be finished in sixty days. The Portland and Puget Sound Railway Company has a good deal of grading done, but has suspended work for the present.

The above is the entire improvement, as far as roads in contemplation are concerned.

I have made a map and estimate, with tracings of the result of the survey.

Respectfully submitted.

BOLTON W. DECOURCY.

T T 13.

[Printed in House Ex. Doc. No. 35, Fifty-second Congress, first session.]

PRELIMINARY EXAMINATION OF TILLAMOOK BAY AND BAR, OREGON.

UNITED STATES ENGINEER OFFICE,
Portland, Oregon, December 2, 1890.

GENERAL: I have the honor to state that in accordance with the direction in your letter of September 20, 1890, a preliminary examination has been made of Tillamook Bay and Bar, Oregon, and the following report thereon is submitted:

Tillamook Bay and Bar were first examined in 1886 by Lieut. Edward Burr, under direction of Capt. C. F. Powell. In 1887 a survey was made of the interior waters under direction of Captain Powell, who in 1888 submitted a project and estimate for work to be done.

The river and harbor act of August 11, 1888, appropriated \$5,200 for Tillamook Bay and Bar, and this money was expended in making a survey of the bar entrance and building a dike and revetment work, for the improvement of Dry Stocking Bar, in Hoquarton Slough.

Lieutenant Burr in 1886 and Captain Powell in 1887 and 1888 recommended that all works of improvement be confined to the inner waters, reporting that "the entrance to the bay requires no attention."

In order to obtain the most recent data with regard to the development on the bay, Mr. A. J. McMillan, assistant engineer, was dispatched to Tillamook the first part of November, 1890, to make the preliminary examination required by your orders. Mr. McMillan's report is appended hereto.

After a full consideration of all available information in regard to Tillamook Bay and Bar, I am of the opinion that the present demands of commerce do not require any work looking to the improvement of the entrance to the bay. All accounts agree that this is one of the best small bar harbors on the coast. The bar channel is straight, shifts very little and has an available depth of 10 feet and more at low water.

Tillamook City is the most important place on the bay, and seems to be situated in the best place for a local distributing point. It is desirable that the boats which regularly trade with the bay should be able to ascend to this place, and to do this it will be necessary to make improvements at Junction and Dry Stocking bars.

I am of the opinion that Tillamook Bay is worthy of improvement to the extent of improving the waters from the entrance to Tillamook City, so that vessels which can safely cross the bar can ascend thereto.

make a proper survey and prepare plans and estimates for the will cost, it is estimated, about \$400.

Very respectfully, your obedient servant,

THOMAS W. SYMONS,
Captain, Corps of Engineers.

ig. Gen. THOMAS L. CASEY,
Chief of Engineers, U. S. A.

through Col. G. H. Mendell, Corps of Engineers, Division Engineer,
fic Division.)

[First indorsement.]

WASHINGTON, D. C., *December 12, 1890.*

spectfully forwarded to the Chief of Engineers.

r reasons herein stated Tillamook Bay is worthy of improvement
e extent herein recommended.

G. H. MENDELL,
Colonel, Corps of Engineers, Division Engineer.

REPORT OF MR. A. J. McMILLAN, ASSISTANT ENGINEER.

PORTLAND, OREGON, *November 11, 1890.*

TAINTAIN: I have the honor of submitting the following report of a preliminary
ination of Tillamook Bay, made in accordance with your instructions.

Entrance to Tillamook Bay is situated about 50 miles south of the mouth of the
mbia River. The entrance is straight and has a depth of about 10 feet of water
a bar at low water. It is one of the best small bar harbors on the coast, it not
subject to shifting to the north or south like many others. Steam schooners
aded to 12 feet draft with lumber at the Hobsonville Mill, and pass safely
t. The bay has a length of over 5 miles, and a breadth of over 3. It is very
being at extreme low water an immense mud flat with a few crooked channels
gh it. Any craft that can cross the bar has water enough to reach Hobson-
and Bay City, but not enough to reach the head of the bay on account of shoals.
rorst of those shoals is the one known as Junction Bar, and is situated a short
ice below where the waters of the Kilchis and Wilson rivers meet the waters
e Tillamook and Traak rivers at low water. This shoal has been formed by the
ent brought down by those streams. It is about 1,000 feet long, and has only
; 1 foot of water on it at extreme low water. I think that for the improvement
is place, it would be necessary to run a low stone dike from the south side of
ay to concentrate the water on this place and keep it scoured out. The next
is obstruction to navigation in going up Hoquarton Slough to the town Til-
ok, is Dry Stocking Bar, which is situated at the junction of the Tillamook
and Hoquarton Slough. This bar has at present only a few inches of water
at extreme low water. The dike that was carried away by the freshets of
vinter should be replaced as soon as possible. If those two places were im-
d, the schooners that now go up to the town of Tillamook, would have no dif-
y in reaching there. They draw 7 and 8 feet of water and have to cross those
s at high water.

o town of Tillamook, the principal one on the bay, is situated at the head of
ation on Hoquarton Slough, and is about 3 miles above Dry Stocking Bar.
e its location is a good one in regard to the adjacent agricultural lands, it is a
ne in regard to navigation. The slough at the town is less than 100 feet wide,
widens out toward the bar by the addition of the Daugherty, which is a part
e Wilson River that comes into it about 1 mile below town, and also by the
ion of the Traak River, which enters it about 1½ miles farther down. The town
population of about 300. There is a sawmill here of 15,000 feet capacity,
a cuts mostly for local demand but occasionally ships some spruce lumber to
and.

r City is situated at the place designated as Sandstone Point on the Coast Sur-
harts. It has a sawmill of 6,000 feet capacity, and cuts for local demand.
own has a population of about 50 persons.

obsonville is situated at Memaluct Point. There is a sawmill here of 50,000
apacity, which cuts spruce for the San Francisco market. There is also a can-
at this place, but it did not run this season.

Garibaldi is situated at Bailey Point near the entrance. There is a cannery at this place that put up about 20,000 cases of salmon this season.

The exports of the bay for this year are as follows:

	Value
Lumber, 12,000,000 feet.....	\$180,000
Butter, 40 tons.....	16,000
Apples, 3,500 boxes.....	1,750
Potatoes, 4,000 bushels.....	2,000
Hides, 600.....	1,200
Sheepskins, 300.....	300
Stock, 400 head.....	8,000
Total.....	209,150

The vessels at present entering the bay are the steam schooners *South Coast*, *Augusta*, *Truckee*, and *Louis Olsen*. The *South Coast* and *Truckee* load at Hobsville with lumber for San Francisco. The *Augusta* and *Louis Olsen* are coast schooners and go up to Tillamook.

After a careful consideration I have come to the conclusion that the entrance this bay is in good enough condition to suit all the present demands of commerce but I consider that the Government should replace the dike at Dry Stocking and do something toward the improvement of Junction Bar.

The present routes of travel to reach this bay are by daily stage from North Yamhill, by triweekly stage from Sheridan, by trail from Astoria, and by coast schooner from Astoria.

There is being built a wagon road from Forest Grove down Wilson River to Tillamook. It will be finished by the end of next June. It will be a better road than the one from North Yamhill and have 1,400 feet less elevation on the summit of Coast Range.

Very respectfully, your obedient servant,

Capt. THOS. W. SYMONS,
Corps of Engineers, U. S. A.

A. J. McMILLAN

SURVEY OF TILLAMOOK BAY AND BAR, OREGON.

UNITED STATES ENGINEER OFFICE,
Portland, Oregon, October 19, 1891.

GENERAL: I have the honor to submit the following report upon survey of Tillamook Bay and Bar.

The last river and harbor bill contained an item providing for the examination and survey, if found necessary, of "Tillamook Bay and Bar."

Being charged with making the preliminary examination, I submit my report thereon under date of December 2, 1890.

Under date of December 13, 1890, I was directed to make the necessary survey.

Mr. William H. Wood was engaged to make the survey of the inner harbor, and his report is herewith.

Tillamook Bay is situated about 50 miles south of the entrance to the Columbia River, and at high tide is about 7 miles long and $2\frac{1}{2}$ miles wide; at low tide it is a great sand and mud flat cut by three channels, the North, Middle, and South channels. These channels are shown on the accompanying map,* which embraces all of the bay within about three-fourths of a mile from the harbor throat.

The South Channel is unimportant, being shoal and not used for purposes of navigation.

The Middle Channel is the through channel by which access is had to the head of the bay and to Tillamook City, the principal town of the locality, which is situated on Hoquarton Slough.

The North Channel runs along the north shore of the bay from the entrance to Shell Point, about a mile above Bay City, where it terminates.

* Not reprinted; printed in House Ex. Doc. No. 35, Fifty-second Congress, first session.

The difficulties met with in the navigation of the bay are three in number and are as follows:

First. Boats using the North Channel can not proceed up the bay except by returning down the North Channel nearly to the entrance of the bay, and then turning and going up the Middle Channel. It is proposed to obviate this difficulty by making a connection between the Middle and North channels so that boats can go to points along the north shore and then keep right on up to the head of the bay.

Second. The second difficulty is met with at Junction Bar, a shoal bar in the Middle Channel just below where the South Channel diverges from the main or middle channel. It is reasonable to believe that this bar is due to the interference of currents, and particularly to the weakening of the ebb current in the Middle Channel due to the branching off of the South Channel.

The remedy proposed for this difficulty is to partially shut off the water and stop its running into the South Channel and to confine more effectually the currents of the Middle Channel at the bar.

Third. The third difficulty consists in a shoal at a point known as Dry Stocking Bar, at the junction of Hoquarton Slough with the Tillamook River.

This is due to the widening of Hoquarton Slough and the back-water from Tillamook River.

A marshy island divides Hoquarton Slough into two channels, and the obvious remedy for the difficulty is to close up one of these channels by a dike, compelling the channel into the other and thus causing a deeper scour to take place.

A dike was constructed at this point some years ago, but was carried away by the winter freshets.

In order to make the connection between the North and Middle channels it will be necessary to dredge a channel across the intervening sand flat; and in order to maintain this channel it will be necessary to close up the Middle Channel so as to direct the ebbing and flooding waters through the dredged channel. This it is proposed to do by a dike which should be extended across the sands separating the North and Middle channels, and for some distance upon the sands bordering the Middle Channel on the south, in order to prevent the Middle Channel from cutting in around the deflecting dike.

It is proposed to make the dredged channel 200 feet wide on the bottom and 6 feet deep at low water. This will require the dredging of 65,000 cubic yards of material. The required deflecting dike will be 2,000 feet long, and can only be put in entirely across the channel after the opening of the new channel.

This dike can be built of piles, brush, and stone, at an average cost of \$12 per foot. It is proposed to leave this dike 3 feet below high-water mark.

For the removal of Junction Bar a dike 5,000 feet long will be required. This will be built of piles, brush, and stone, and is estimated to cost \$5 per foot.

For the removal of Dry Stocking Bar a dike will be required 950 feet in length; the dike to be built of piles, brush, and stone, at an estimated cost of \$10 per foot.

The following is the estimated cost of the work proposed:

ESTIMATE.

Connection between North and Middle channels:

Dredging 65,000 cubic yards, at 50 cents.....	\$32,500
2,000 feet dike, at \$12	24,000

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Removal of Junction Bar:	
5,000 feet dike, at \$5	\$25,000
Removal of Dry Stocking Bar:	
950 feet dike, at \$10	9,500
	91,000
Contingencies, engineering, etc.	9,000
Total	100,000

COMMERCIAL STATISTICS.

There is given herewith the report of Mr. W. H. Wood, of date of June 16, upon the commercial statistics of Tillamook Bay.

This, together with my preliminary report and the report of Mr. A. J. McMillan accompanying it, furnish all the information available regard to the commerce of Tillamook Bay.

TILLAMOOK CITY, June 16, 1891.

DEAR SIR: I have the honor to submit the following report of the commercial statistics of Tillamook Bay for the year ending June 30, 1891, as per your instruction of June 2, 1891, as follows:

Following are the separate amounts, in tons, of the exports and imports of the tire bay; the same will be found in the tabulated statement attached:

EXPORTS.

11,600,000 feet B. M. lumber, at 4 pounds per foot	Tons 23, 2
21,000 cases canned salmon, at 72 pounds per case	7
130 barrels salt salmon, at 400 pounds per barrel	
Butter	
Wool	
Hides	
3,000 boxes apples, at 50 pounds per box	
100 cases eggs, at 50 pounds per case	
Hay	
General merchandise	

IMPORTS.

General merchandise	3, 3
Agricultural implements	1
Machinery	1
Flour	1
Feed	2
Shingles	1
Brick	4

Making a total of 24,188½ tons exported and 4,106 imported during the year.

Besides this there were exported 1,200 head of cattle which were driven out Astoria.

VESSELS.

The number of vessels of all descriptions crossing the bar during the year was in and 60 out. Their names, registered tonnage, number of trips each, are as follow

Steam schooner.	Tonnage.	Draft.	Trips
Truckee	350	13 6	
Scotia	200	11 4	
Laguna	200	11 6	
Augusta	87	7 9	
W. H. Harrison	70	6 9	
Louis Olsen	80	7 6	
Elmore	30	8 6	

two vessels *Truckee* and *Scotia* ply between Hobsonville and San Francisco, bringing both freight and passengers. They load out with lumber from the Truckee Lumber Company's mill at Hobsonville for San Francisco. The *Laguna* plies between Hobsonville and San Francisco, loading out with lumber, but carries no passengers. All three of the above vessels are operated solely by the Truckee Lumber Company.

The steam schooner *Augusta* carries both freight and passengers, making irregular trips between Tillamook City and all points on Tillamook Bay and Astoria and Clatsop. The steam schooner *W. H. Harrison* made one trip from Portland with general merchandise for Bay City. The steam schooner *Louis Olsen*, owned by Mr. Olsen, of Tillamook City, made six trips from Portland to Tillamook City, bringing general merchandise. She is now off the bay. The steam schooner *Elmore* made four trips from Astoria, bringing supplies to Mr. Elmore's cannery at Garibaldi.

There are no regular vessels plying to Tillamook City, and I am unable to learn the lines of transportation extending to points beyond San Francisco and Portland, except in the case of the *Truckee*, which is loading lumber for one trip to some point in Mexico.

There are now plying between points on Tillamook Bay the following boats: The tugboat, *General Garfield*, 10 tons register, drawing 5 feet of water, towing rafts and carrying freight. She ascends as far as tide water extends. Two small sailboats carry passengers and some freight. There is also one barge with a driver outfit.

The above-mentioned boats and vessels include all that are now or have been in operation upon the bay during the year.

The other lines of transportation to and from Tillamook City and Bay are: Tillamook City to Sheridan via Grand Ronde, 60 miles, triweekly stage. Tillamook City to North Yamhill, Oregon, 42 miles, daily stage. Tillamook City to Hobsonville, Oregon, via Bay City, 9 miles, daily stage. Garibaldi to Nehalem Bar, 9 miles, daily stage. All of these stage lines carry the United States mail except from Garibaldi to Nehalem.

INDUSTRIES AND RESOURCES.

The whole of Tillamook County, except a portion bordering on the Nehalem River, is tributary to Tillamook Bay. Its principal resources are now and will be in the raising of lumber, canned salmon, farming, dairying, and stock raising. The products, and their home consumption for the year 1890-'91, are estimated as follows:

Products.	Amount.	Tons.	Value.
Timber.....feet, B. M.	14,230,000	28,460	\$13.00 per M.
Canned salmon.....cases	21,000	756	4.60 per case.
Salmon.....pounds	52,000	26	.02½ per pound.
Flour.....do.	76,000	38	.25 per pound.
Wheat.....bushels	2,500	400	.55 per bushel.
.....	5,000	8.00 per ton.
.....pounds	5,000	2½	.15 per pound.
.....head	1,200	22.00 per head.
.....pounds	6,000	3	.06 per pound.
.....	600,000	60	2.00 per M.
.....	10,000	20	8.00 per M.
.....boxes	3,000	210	2.00 per box.
.....pounds	22,000	11	.10 per pound.

There are some other small productions, but as they are all consumed on the bay they are not estimated.

CITIES AND TOWNS.

There are four small cities and towns situated on and tributary to Tillamook Bay,

Garibaldi, situated three-quarters of a mile inside the bar, with a population of 112; it has two general stores, two hotels, and one salmon cannery. The principal industry is fishing and canning salmon, during which season the population is much increased.

Hobsonville, situated on the east shore of the bay, is 1½ miles south from Garibaldi. It has a population of 112; 1 general store, 1 hotel, post-office, and one salmon cannery. The Truckee Lumber Company's mill is located here, which mill has shipped all the export lumber to date. The principal industries are lumbering and canning salmon.

Bay City, situated on east shore of the bay, 5 miles south of bar; population, 374; in 1890, the population was 80. Bay City has a newspaper, 2 general stores,

post-office, 1 hotel, 1 school building, 1 church, 1 sawmill, capacity 10,000 M. per day; 1 sash and door factory, 1 hardware store, and storage ware and wharves. About \$40,000 has been expended in new buildings this year there are now being erected 2 stores, 2 hotels, 1 barrel and stave factory, 1 besides several dwelling houses. The available resource is lumber.

Tillamook City, county seat of Tillamook County, is situated on Hoop Slough, 12 miles south of the bar of Tillamook Bay. It is the oldest settlement has a population of 650 people.

The county court-house is located at Tillamook City. It has also 1 school building, 1 church, post-office, 1 sawmill with a capacity of 15,000 feet B. M. per day, 1 furniture factory, 6 general stores, 1 jewelry store, 2 fruit stores, 1 boot and shoe store, 1 agricultural implement store, 1 hardware store, 1 furniture store, 2 hotels and 2 newspapers; also 2 blacksmith shops, 2 wagon shops, 2 livery stable storage warehouses. The industries are lumbering, agriculture, dairying, and raising. All the agricultural land is tributary to Tillamook City.

Five rivers or streams empty into Tillamook Bay from the east, each has source in the Coast range of mountains, viz: Miami, Kilchis, Wilson, Tr Tillamook rivers. Large bodies of timber land border on the heads of these consisting of spruce, fir, larch, and cedar, a large part of which is tributary to the bay; it has been roughly estimated at 20,000,000,000 feet B. M.

The bottom lands are very fertile and although as yet covered with timber to a great extent, they are the prospective agricultural lands of the county. The timber land is as yet unsurveyed. It is being located as fast as surveys are made.

The estimated increase of population is 20 per cent for the county since 1890.

No mines or minerals have yet been discovered tributary to the bay, although it is claimed that indications of coal do exist. Coal in some quantities has been discovered on Coal Creek, a tributary of the Nehalem River, about 30 miles north of Tillamook City, but no mining has been done.

This information has been derived from various sources; from the county records of the various vessels and merchants' books, and by general inquiry and observation.

I have been unable to complete the table through preceding years to any extent as no records were kept, and the older merchants did not keep books. Many have taken place.

No records of the vessels or their cargoes have been kept, not even their arrivals and departures, and my chief source of information was from the freight books of the vessels, which are incomplete and do not extend backward more than two or three years.

Hoping the above may be satisfactory, I remain,

Your obedient servant,

Capt. T. W. SYMONS,
Corps of Engineers.

W. H. WOOD

There is also given herewith the report of Mr. W. H. Wood of his survey of the inner bay:

TILLAMOOK, June 30,

SIR: I have the honor to submit herewith the following report of my survey of the inner harbor of Tillamook Bay, Oregon, made during June, 1891:

Tillamook Bay is situated about 50 miles south of the bar of Columbia River, latitude about 45° 33' north, longitude 123° 56' west from Greenwich.

At high tide this bay is a body of water about 7 miles long and from 2 to 3 miles wide, while at low tide it consists of a great sand mud flat, cut by three channels, known as the North, Middle, and South channels. The entrance to the bay is about one-half mile wide.

The South Channel diverges from the Middle Channel near Dicks Point. It is used in many places and is not used in navigation.

The North Channel forms near the bar and extends up past Hobsonville to Bay City. It runs out near Shell Point.

The Hobsonville and Bay City vessels use this channel. No fresh-water of any extent flows into it above Hobsonville, and from report it is filling up gradually.

The Middle Channel is the one used by vessels that ply to Tillamook City. The situation now is, any vessel going to Hobsonville or Bay City, in order to reach Tillamook City, must back out to near the bar and take the Middle Channel at any stage of the water.

ring into the bay from the north and east are six rivers or streams, the Miami, Wilson, Tillamook, and Trask rivers, and the Hoquarton Slough. The steam tug *General Garfield*, drawing 5 feet of water, ascends the Tillamook 3 or 4 miles, while it and the steam schooner *Augusta*, the latter drawing 7 inches, both ascend Hoquarton Slough to Tillamook City at high tide. None other streams are navigable.

Obstructions to navigation in reaching Tillamook City by way of the Middle Channel are Junction and Dry Stocking bars, separately described as follows:

Junction Bar, as shown on the accompanying map, is situated at a point where the Wilson River joins those of the middle channel of the bay. It is caused by the debris from Wilson River lodging at this point.

To improve same it is proposed to build a dike from the shore near Dicks Point, a distance of 5,000 feet. This will prevent the water from going through the south channel and force the ordinary currents of both channels to flow over this bar, thus scouring it out. The material composing this bar is sand and gravel, and is very loose and easy to scour.

Stocking Bar is situated where Tillamook River joins Hoquarton Slough, and is situated below where the Trask River joins the same slough. Tillamook River is situated on Hoquarton Slough about 3 miles above this point, all vessels going there have to cross this bar.

I would recommend that a high-water dike be built at this point as shown on the accompanying map, of length 950 feet. Sufficient strength will be required to withstand the winter storms, which are very severe, as shown by driftwood and other water marks.

The dike will occupy about the same position as did a pile dike that was previously constructed at this place, but which was too weak to withstand the floods, and is now almost entirely destroyed. Of the improvements previously made here, the spur dikes and revetment on south side of channel still stand and are all right for the intended purpose. This bar is sand and mud, and by confining the channel by this proposed dike, I believe it will scour out 4 or 5 feet deep. At present it is only 6 inches of water on it at low tide.

Accompanying this report are designs for dike and excavation to change channel, joining the Middle and North channels together, providing such a change is necessary.

Building this dike and excavating through this sand bank it will force the water flowing in the Middle Channel to flow out through the North Channel, thereby securing one direct navigable channel from the entrance to Tillamook City, allowing the vessels that come to Bay City to proceed direct to Tillamook City. I recommend the dike to be left 3 feet below high water.

The bar of Tillamook Bay, from all the information I could gather, is not subject to much change, and is in about the same position and condition as previous reports show.

Four steam schooners that now irregularly cross the bar are the *Augusta*, *Scotia*, *Truckee*, and *Truckee*. Depths in water are respectively 7 feet 10 inches, 11 feet 4 inches, 11 feet 6 inches, and 13 feet 6 inches. They always await favorable winds at high tides. Soundings taken by the captains of each are reported and give depths on the bar. These soundings as reported will, reduced to lower low-level, give a mean of 12 feet. The steamer *Truckee* claimed to be bar bound nine days during April and May, 1891, but all four are going in and out at low tide without trouble. The bar has only one buoy to mark the channel, and the only way seems to be in keeping the channel.

COMMERCE.

The commerce of Tillamook Bay is all confined to the north shore and to Tillamook City on Hoquarton Slough. It consists in canned salmon, lumber, and agricultural products, in amounts and distribution as per my report to you of date June 1, of "commercial statistics," of Tillamook Bay.

The commerce is capable of much increase, especially the lumber interests, large quantities of timber being found on all the streams, and tributary to the bay are the sawmills, one at Galibaldi and one at Hobsonville, only one of which was closed during the past year.

The *Truckee Lumber Company*, located at Hobsonville, are the only exporters of lumber. Their lumber is shipped to San Francisco. Two other mills cut lumber for the local market.

The materials that will be needed in the construction of these several dikes can be produced here, except iron bolts. Piling in any quantity can be taken out at any point, and the sawmills can furnish lumber, while brush, such as alder, oak, and spruce boughs, is along all the streams and can be boated to places remote. Clay is found in the underlying strata of all the tide lands and can be conveniently placed in works.

Rock extends along the shore from Memaloose Po 200 feet high. Good and convenient quarries of ss loaded on barges at any stage of tide.

I have examined old piling for teredos and find the sonville.

SURVEY.

The field work of this survey was done during the of June, being finished June 12, 1891.

The Coast Survey signals were re-located on Men Shell points, and intermediate signals located by sea

Soundings were taken from a Columbia River fish men doing the pulling. The position of the sound readings from boat to signals on shore.

In platting the map the points first given above Coast Survey triangulation and the intermediates fr

A tide gauge was set on wharf near Easterbrook I ing Bar. The readings of high and low water were c tion for zero obtained by comparing it with Coast S sponding time, and from this all the soundings w level.

The mean rise of tide at Easterbrook Point is 7.3 6.6 feet, as so compared and calculated.

It is believed that by placing dikes and excavation accompanying sketches, a minimum depth at low v maintained in one continuous channel from bar of baldi, Hobsonville, and Bay City.

Very truly, yours,

To Capt. T. W. SYMONS,
Corps of Engineers, U. S. A.

SURVEY OF TILLAMOOK

In my report on the preliminary examina Bar there is the following in reference to the

After a full consideration of all available informat and Bar I am of the opinion that the present demar any work looking to the improvement of the entr agree that this is one of the best small bar harbors o straight, shifts very little, and has an available de water.

I am of the opinion that Tillamook Bay is worthy i improving the waters from the entrance to Tillamoo safely cross the bar can ascend thereto.

In my project for the expenditure of the proving harbor at Tillamook Bay" it is re things that a survey of the bar be made.

This survey was intended to procure the i regard to the entrance, and to determin importance had taken place about the entra vey of June, 1889.

The last survey was made July 6, 1891, the steamer *Gen. H. G. Wright*. A plot c soundings reduced to the plane of the mean sent with this report.

This survey and plot shows an excellent 11 feet at mean low water or 17½ feet at mea

At the harbor throat, between Green Hill distance between high-water lines is 1,820 fe curves is 500 to 600 feet. The greatest dept

The bar is about 9,000 feet from the harbo gable passage across it of 11 feet at the mea

* Not reprinted; printed in House Ex. Doc. No. 35, Fift

The least distance across the bar between 12-foot curves is 1,100 feet, and the distance between 18-foot curves is 3,500 feet.

The survey was made in the summer and shows the location of the bar and channels during the summer, when the northerly winds prevail. As the winter season approaches and the prevailing winds veer to southerly, the bar and entrance channels move to the northward until they occupy about the position marked on the plot. As winter recedes they return to the position shown.

A comparative chart has been made which shows the surveys of 1867, 1890, and 1891. From this it is seen that the last two surveys give practically identical results.

The survey of 1867 shows the bar and entrance channels somewhat to the north of the location existing at the time of the later surveys, and with a least depth of 15 feet at low water. This would indicate one of two things, either that the bar has shoaled in the last twenty-year years or that in shifting from south to north or north to south a deeper channel than ordinary is sometimes developed.

It is altogether probable that the latter is the case, although the difference may be due to a combination of the two.

Another fact is observed, which is that the bar occupies practically the same position in regard to the sea that it did in 1867, it having either advanced nor receded.

Mr. John R. Savage, who made the survey of the bar and entrance, reports as follows:

* * * * *

I have the honor to submit the following report on the survey of the entrance to Tillamook Bay, Oregon:

Arrived off Tillamook Bay on the steamer *Gen. H. G. Wright* on the morning of Monday, the 6th of July, 1891, but found upon entering only one or two of the former signals for sounding standing, so we started in and ran a line 3,200 feet in length approximately east and west across the northern end of the sand spit, and with that for a base extended a series of triangles which sufficed to locate most of the signals which were being erected as the triangulation was done, but with two signals, viz, "South" and "Beach," it was necessary to run lines from the nearest triangulation points to locate them. About four and a half days were spent in locating and erecting nine or ten signals, most of them temporary, with usually six or seven available men. I took the precaution to erect the signals for the bar soundings first, so that we could take advantage of the prevailing favorable condition of the weather to work on the bar, which we were able to do on two days before the signals were completed.

In all the equivalent of four days were spent in working on the bar, where there were four different sets of intersecting lines run so as to furnish a reliable check on the work, as well as to develop in an expeditious manner all portions of the bar. While sounding on the bar about 780 sextant observations were taken and 3,160 asts of the lead made.

One complete day sufficed to work up the inside of the bay as far as I considered necessary, and about two days were required to locate the high and low water lines, and in each case the mean was taken.

The bar covers a very considerable area, being quite a little larger than the bar at the entrance to Yaquina Bay, and it also shows a very fair depth of water for an unapproved entrance; in the channel, at low water, the least depth being about 11 feet. At the time of writing I have not been able to put the soundings on the map, but this is only an estimate, but it is a conservative one, I think.

The men who are acquainted with this bar, mainly the captains of the vessels passing over it, tell me that during the winter months the channel is very much to the north of its present position; in fact during last winter the two beacons "West Pass" and "Spit" were used as a range for the channel by the masters of the vessels crossing the bar, but during the spring the channel gradually shifted to the south until it took its present position, which it has held for about the last two or three months, and is likely, I think, to continue in the same place till fall.

As soon as we tied up in Tillamook Bay I established a tide gauge, which I had set night and day for nearly two weeks, and by a comparison with the time and eight of tides as computed from the tide tables, I determined the datum of the

mean of lower low waters to come on the 2-foot mark of the gauge, which was nailed facing west to a pile on the northeast corner of the Cannery Wharf at Garibaldi. The total time consumed in making this survey was two weeks.

Very respectfully, your obedient servant,

THOMAS W. SYMONS,
Captain, Corps of Engineers.

Brig. Gen. THOMAS L. CASEY,
Chief of Engineers, U. S. A.

(Through Col. G. H. Mendell, Corps of Engineers, Division Engineer, Pacific Division.)

[First indorsement.]

U. S. ENGINEER OFFICE,
San Francisco, Cal., November 11, 1891.

Respectfully forwarded, recommended.

G. H. MENDELL,
Colonel, Corps of Engineers, Division Engineer.

T T 14.

[Printed in House Ex. Doc. No. 21, Fifty-second Congress, first session.]

PRELIMINARY EXAMINATION OF SWINOMISH SLOUGH, WASHINGTON, WITH THE VIEW OF CONSTRUCTING A SHIP CHANNEL THROUGH THE SAME, CONNECTING SARATOGA PASSAGE AND SKAGIT BAY WITH PADILLA BAY.

UNITED STATES ENGINEER OFFICE,
Portland, Oregon, November 4, 1890.

GENERAL: In compliance with orders contained in your letter of September 20, 1890, I have the honor to make the following report of the preliminary examination of "Swinomish Slough, Washington, with the view of constructing a ship channel through the same, connecting Saratoga Passage Skagit Bay with Padilla Bay, and to report the most suitable and feasible plan for making such improvement, with the cost of the same."

I have lately made an examination of Swinomish Slough in connection with the subject of the bridge of the Seattle and Northern Railroad Company over this slough, and deem no further preliminary examination necessary, as this slough is in my opinion worthy of improvement.

There are three routes for boats passing from the northern portion of the great body of water known as a whole as Puget Sound, to the southern portion thereof.

One way is by keeping through Rosario Straits, through the main sound to the west of Whidby Island and through Admiralty Inlet. The second is by passing through Rosario Straits and Deception Pass into the waters of Skagit Bay, Saratoga Passage, etc.; and the third way is by taking advantage of the navigable waters of Padilla Bay, Swinomish Slough, and Skagit Bay. The first two routes are dangerous for smaller boats, owing to the heavy seas which prevail at times in the main sound to the west of Whidby Island, and to the dangerous passage through Deception Pass.

The route by Swinomish Slough is the most easterly and the safest for the smaller boats which ply on the sound and the many rivers and inlets thereof, as well as the shortest. It is, however, shallow and crooked.

Swinomish Slough is especially valuable for the passage of rafts of logs and lumber from one portion of the sound to the other. It has always been an important highway of commerce, but its relative importance has been changed somewhat of late by the introduction of ~~larger~~ boats on the sound, the building of railroads, and the

l by the bridge constructed by the Seattle and Northern Railroad any across its northern end.

ides the importance of this slough as a highway for general com- it passes through an exceedingly fertile country, the Skagit the greater part of which has been reclaimed by dikes and ht under cultivation. This Skagit Delta is in many respects the important agricultural area west of the Cascade Mountains in ington, and its means of water communication should be carefully ed and as far as practicable, added to.

survey for a ship channel will include a length of about 13 miles the shallow flats of Skagit and Padilla bays, and through Swi- h Slough.

estimated that the survey for a ship channel, the preparation of and estimates, will cost about \$3,000.

Very respectfully, your obedient servant,

THOMAS W. SYMONS,
Captain, Corps of Engineers.

Gen. THOMAS L. CASEY,
Chief of Engineers, U. S. A.

rough Col. G. H. Mendell, Corps of Engineers, Division Engineer, e Division.)

[First indorsement.]

U. S. ENGINEER OFFICE,
San Francisco, Cal., November 14, 1890.

pectfully forwarded.

reasons herein stated, Swinomish Slough is worthy of improve-

G. H. MENDELL,
Colonel, Corps of Engineers, Division Engineer.

Y OF SWINOMISH SLOUGH, WASHINGTON, WITH THE VIEW OF STRUCTING A SHIP CHANNEL THROUGH THE SAME, CONNECTING TOGA PASSAGE AND SKAGIT BAY WITH PADILLA BAY.

UNITED STATES ENGINEER OFFICE,
Portland, Oregon, December 14, 1891.

GENERAL: The last river and harbor bill contained an item for the nation and, if deemed worthy, survey of "Swinomish Slough, he view of constructing a ship channel through the same, con- g Saratoga Passage Skagit Bay with Padilla Bay, and to report ost suitable and feasible plan for making such improvement, with et of the same."

ler date of September 19, 1890, I was directed to make the re- l examination, and under date of November 4, 1890, I submitted port thereon.

Department letter dated November 21, 1890, I was directed to make quisite survey.

ow have the honor to submit the following report, with maps,* and estimates.

o No. 1 is a vicinity map, showing the relations of Swinomish h to the principal cities, navigable channels and passes of Puget l.

reprinted; printed in House Ex. Doc. No. 31, Fifty-second Congress, first

As shown by this map, there are, in this general portion of Puget Sound, three routes for boats passing to the southern portion, or the reverse.

One way is by keeping in the main sound between the Fidalgo islands and through Admiralty Inlet to the west of Fidalgo Island and then into the waters of Skagit Bay, Saratoga Passage, and Swinomish Slough. Another way is by taking advantage of the navigable channels between Swinomish Slough, and Skagit Bay. The latter is not so good for smaller boats owing to the heavy sea in the main sound to the west of Whidby Island, and the passage through Deception Pass.

The route by Swinomish Slough is the most direct for the smaller boats which ply on the sound and its inlets thereof, as well as the shortest. It is crooked in some portions, and the approach to the low flats of Padilla Bay on the north, and Swinomish Slough on the south.

Swinomish Slough is the ordinary meeting place of the tides at times the tides run in one direction entirely.

The ordinary rise and fall of the tides in this slough is about 10 feet.

As a general thing throughout the slough there is a depth of water; opposite La Conner there is 10 feet, and south of La Conner this increases to 34 feet, and at the "Hole in the Wall" there is a depth of 100 feet.

Where the slough widens out, and especially where it has been in the habit of putting fish traps, this depth of water is not over 2 feet at low water in any practical portion there is, is very crooked.

At the northern end the slough opens out into the southern portion of Padilla Bay in the midst of the Fidalgo islands.

At the southern end of the slough there are the waters of Skagit Bay. The slough is bounded on the east by a rocky island called McGlinns Island.

The east passage is wide and shallow, having a depth of water at low water. The west passage between the Fidalgo islands is at the narrowest place 22 feet deep of water for all purposes.

This short and crooked passage is so situated on either side, that one can not see through it. On account of its character it has received the descriptive title of the "Hole in the Wall."

This is the passage used almost exclusively.

EXTENT OF IMPROVEMENT

Situated as it is, Swinomish Slough is a natural channel for local steamboats plying on the sound, for small sailing craft going up the slough and in to the Sound, and taking out the products of the Sound.

There is not at present, and I do not believe any necessity for large sea-going vessels to pass through Swinomish Slough. These vessels, in passing from the Sound to the other, would naturally take the wide and direct route of the La Conner Inlet or Saratoga Passage.

In fixing upon the extent of improvement proposed

ation has chiefly been given to the class of vessels before named, *i. e.*, local steamboats, small sailing craft, and rafts of logs and lumber.

The principal use which would be made of the slough would be by the smaller steamboats running to the Nooksack River and the Bellingham Bay towns, Whatcom, Fairhaven, and Sehome, to points in the Skagit River country, the Stillaguamish and Snohomish countries, and to Seattle and Tacoma. These boats draw ordinarily about $3\frac{1}{2}$ to 4 feet.

The sailboats which might use the slough would draw probably not more than 8 to 10 feet.

In deciding upon a depth to be given the navigable pass, this has been fixed at 4 feet below mean low water.

This will give a continuously open passageway for the smaller steamboats, while boats drawing more than 4 feet could always pass through by waiting for the tide.

If in the future, commerce should justify it, this navigable depth could be increased.

The width of the dredged channel has been fixed at 100 feet in order to allow boats or a boat and a raft to pass each other.

Throughout the slough there are places where a greater depth than 4 feet at low water exists naturally, and with much greater widths than 100 feet.

In the long stretch of dredged channel from the southern entrance of the slough to the deep water of Skagit Bay, 2 miles in length, it has been deemed proper to provide a basin where boats could accumulate or lie without interfering with the passage of others. This has been made where the channel passes nearest to Goat Island, and is designed to be widened to 200 feet for 750 feet in length.

As nearly all the traffic into or through the slough will be from north to south, or the reverse, the channel designed to be dredged from the entrance of the slough to the deep water of Skagit Bay is given as much of a southerly trend as practicable. In fixing upon its position, advantage has been taken of a natural channel. The same has been done at the northern end of the slough in locating the channel across the Padilla Flats.

It is feared that if channels are dredged across the Skagit and Padilla Flats they will fill up again unless some means are taken to prevent. With the object of preventing this, a series of dikes shown on the map is proposed. These dikes consist of piles driven about 8 feet apart, and wattled with brush from the bottom to about 2 feet above low water. With the aid of the dikes as located, it is believed that currents sufficient to prevent the deposition of sediment, will be compelled through the dredged channels.

ESTIMATES.

For a channel 4 feet in depth at the mean of the lower low waters, the amount of dredging which will be required for this work in Padilla Bay is 90,000 cubic yards; in Swinomish Slough, 143,000 cubic yards, and in Skagit Bay, 180,000 cubic yards.

The material throughout is sand and mud, all of which can be readily removed by the sand-pump dredges.

The material in Skagit and Padilla bays can be disposed of by conveying it to a distance of 1,000 feet or more through pipes and depositing it upon the mud flats. Most of the material in Swinomish Slough can be disposed of by depositing it in neighboring sloughs or behind

dikes. A small amount will probably have to be dumped. For this an incalculable distance and dumped. For this an incalculable

ESTIMATE.

Dredging: •	
Padilla Bay, 90,000 cubic yards, at 20 cents	
Skagit Bay, 180,000 cubic yards, at 20 cents	
Swinomish Slough, 125,000 cubic yards, at 20 cents	
Swinomish Slough, 18,000 cubic yards, at 30 cents	
Diking:	
Padilla Bay, 6,000 feet wattled pile dike, at \$1 ...	
Skagit Bay, 14,500 feet wattled pile dike, at \$1 ...	
Swinomish Slough, to retain wasted material, 6,000 feet dike, at \$1	
Total	
Add 10 per cent for contingencies, superintendence, etc	
Total	
In round numbers	

SHIP CHANNEL.

Although Swinomish Slough is not considered to the extent of making through it and Padilla and Skagit bays a channel for deep sea has been made of the cost of such a channel follows approximately the course of the small large map sent herewith. It varies from it in and in passing to the east and around Me through the passage known as the "Hole in the Rock" would be too tortuous and dangerous for large

While the smaller channel could probably be directed the tidal currents, it is more than probable a channel would require constant dredging to great a benefit as possible from the tidal currents channel and to guard against sediment, it would be stronger, higher, and closer dikes.

The ship channel estimated for is planned to be at low water, to be 80 feet wide in Swinomish and Padilla and Skagit bays.

The estimated cost is as follows:

Dredging:	
Padilla Bay, 2,800,000 cubic yards, at 20 cents	
Skagit Bay, 2,300,000 cubic yards, at 20 cents	
Swinomish Slough, including disposal of material, 1,000,000 cubic yards, at 25 cents	
Diking:	
In Padilla and Skagit bays, 30,000 feet, at \$4 per foot	
Add 10 per cent for contingencies, engineering, etc ...	
Total	

COMMERCIAL STATISTICS.

Swinomish Slough has been an important factor in the development of the Puget Sound country and it is asserted without hesitation, the details regarding its meager. The use of the slough has been serious

ing the past two years by the bridge of the Seattle and Northern Railroad Company across its northern end. This bridge was built at an angle of about 45 degrees with the current, with a narrow draw span and a draw rest extending almost entirely across the channel. The bridge was complained of, and by direction of the Secretary of War a new drawbridge with a span of 158 feet, giving a clear passage in the direction of the current 100 feet wide, is being built. This it is expected will be completed by January 1, 1892.

The slough has in a few places during recent years also become very shoal, rendering it necessary at all times for boats to wait for high water in order to pass through.

With the removal of the obstruction to navigation caused by the railroad bridge and the deepening of the channel as proposed in this report, there can be no doubt of the greatly-increased use of this short and safe route between the upper and lower portions of Puget Sound.

Aside from the through travel, the improvement of Swinomish Slough will be of immense benefit to the commerce of the Skagit Delta through which it passes. This Skagit Delta is mostly composed of tidal marshes which have been reclaimed by diking and which have an almost fabulous fertility. They are penetrated by several tidal sloughs opening into Swinomish Slough or into Skagit and Padilla bays, which are the principal highways for the removal of their products and the shipping in of merchandise, building material, etc.

The products of the country are chiefly hay, oats, fruits, etc., which find ready and good markets in the larger cities and the lumber camps.

La Conner, the county seat of Skagit County, is situated on Swinomish Slough and is a beautiful little town of about 1,000 inhabitants. The town is old and conservative, having been established in 1869.

The large plat sent herewith shows the slough and the work as projected.

The following statement has been furnished this office by Dr. G. V. Jathoun, ex-mayor of La Conner:

Of the practicability of dredging the Swinomish, nothing need to be said herein, as that point will be fully covered by the report of the engineering corps that made this survey last spring. The next thing to be determined is the necessity for the proposed improvement, which question involves the consideration of safety, distance, and convenience.

All traffic passing from the waters upon which are situated the towns of Padilla, Bay View, Edison, Samish, Anacortes, Guernes, and all points in Whatcom County, to Seattle, Tacoma, Olympia, Coupeville, Oak Harbor, Snohomish, Stanwood, and all other points south of Skagit Bay, is directly affected by the condition of this inside route.

All this traffic must either pass through Swinomish Channel or take the tedious and dangerous outside route west of Fidalgo Island, where it encounters, between Deception Pass and Burrows Bay, the roughest water on Puget Sound or the Straits of Fuca. There the strong tides emerging from Saratoga Passage meet opposing southwest winds, and produce seas against which even the most seaworthy craft can not contend with any degree of safety, and the craft ordinarily plying these waters are utterly unfitted for such an ordeal.

Then at Deception Pass even greater dangers are encountered. The tide, suddenly confined between walls of solid rocks within a very narrow passage, rushes through at a velocity of from 8 to 15 miles per hour, creating great whirlpools and eddies. Very few steamers can make headway against the tide when at its worst, and none can do so with safety.

In attempting the passage against the tide, all steam is necessarily pressed on, and at such a time if the vessel strikes a whirlpool and takes a sheer, only the promptest action can prevent a catastrophe. Thus far, though numerous minor accidents have occurred, the caution of pilots has prevented any very serious mishaps, though hair-breadth escapes have often been reported, and it is generally recognized as an indisputable fact that the sinking of a steamer there would result in a loss of all on board. The steamboat inspectors, comprehending this, have forbidden vessels from attempting the passage except at slack water.

The inside route, through Swinomish Slough, possesses every advantage over the outside route. It shortens the distance from Skagit Bay and Saratoga Passage Padilla Bay and the Gulf of Georgia by some 15 miles, and provides a safe shelter route of sufficient width, free from heavy seas or dangerous tides and rocks, the obstacles being sand bars, which impede progress.

As before stated, the entire commerce between the northern and southern ports of Puget Sound, dividing on Fidalgo Island, is affected by the condition of the inside route. Such traffic as does not now take this route would do so if the channel was improved to admit of low-tide navigation.

It would be impossible, without a pretty thorough investigation, to form a reliable estimate of the amount of annual tonnage of receipts and shipments.

Owing to the numerous bays, inlets, rivers, and so-called "sloughs" intersecting every portion of the country, all lined with warehouses and granaries, near every farmer, logger, and manufacturer, as well as every merchant, is a shipper.

Then, too, we must remember that for every ton of grain or produce shipped a proportionate amount of provisions, machinery, and merchandise is imported.

For the country situated in the immediate vicinity of this channel and direct tributary thereto, it is estimated that there are shipped annually 20,000 tons of wheat (1,250,000 bushels), 20,000 tons of hay, and other products. Over 5,000 tons of merchandise, etc., is imported.

No data is available bearing on the log and lumber shipments, but as logging is even a more important industry than agriculture, some idea may be formed of the quantity of logs and lumber seeking shipment by this route.

As these figures take into consideration only a very small scope of country, perhaps a better idea of the importance of this channel and the necessity of its improvement may be gained by an enumeration of the list of steamers running here regularly or occasionally, as follows:

Steamers making regular trips: Hassalo and Idaho, daily; Mary F. Perly, twice weekly.

Steamers regularly engaged in the grain trade: Fanny Lake, E. W. Purdy, L. Perry (steam schooner), Utsalady (steam schooner), Maid of Oregon (steam schooner)

Making frequent or occasional trips:

Wasco.	Eliza Anderson.	Glide.
Alki.	Emma Hayward.	J. B. Libby.
Cascade.	Enterprise.	James McNaught.
City of Quincy.	Gem.	Josephine.
Clara Brown.	Geo. E. Starr.	Lilly.
Daisy.	Gleaner.	Brick.
Edison.	Biz.	W. K. Merwin.
Washington.	W. F. Munroe.	Yakima.
Yukon.	Zephyr.	Fairhaven.
J. C. Brittain.	Henry Bailey.	Sehome.
Isabelle.	Margy.	Mame.
May Queen.	Messenger.	Michigan.
Nellie.	Otter.	Saranac.
Seattle.	Skagit Chief.	Snsie.

The report of Mr. John R. Savage, assistant engineer, who made the survey, is herewith.

Very respectfully, your obedient servant,

THOMAS W. SYMONS,
Captain, Corps of Engineers.

Brig. Gen. THOMAS L. CASEY,
Chief of Engineers, U. S. A.

(Through Col. G. H. Mendell, Corps of Engineers, Division Engineer Pacific Division.)

[First indorsement.]

U. S. ENGINEER OFFICE,
San Francisco, Cal., December 18, 1891.

Respectfully forwarded to the Chief of Engineers, U. S. Army, recommended.

G. H. MENDELL,
Colonel, Corps of Engineers, Division Engineer.

REPORT OF MR JOHN R. SAVAGE, ASSISTANT ENGINEER.

CAPTAIN: I have the honor to submit the following report of the survey of Swinosh Slough with a view of constructing a ship channel through the same, connecting Saratoga Passage Skagit Bay with Padilla Bay:

Swinomish Slough is the channel separating Fidalgo Island from the Swinomish Is., and joining Skagit Bay, Saratoga Passage, with Padilla Bay. This slough extends almost due north and south for about 6 miles in longitude $122^{\circ} 30'$ west of Greenwich and in about latitude $48^{\circ} 25'$ north, and affords a direct inside passage for steamers from the upper end of Puget Sound through Saratoga Passage to Bellingham Bay, thus avoiding rough water and the dangers of Deception Pass.

On the north, through Padilla Bay, Swinomish Slough has connection with the Strait of Fuca by way of Ship Harbor and Rosario Strait; and on the south, by Skagit Bay and Saratoga Passage through Deception Pass, there is also connection with the straits; but the main commercial value of this slough is owing to the fact of its being a connecting link for direct water communication between the Bellingham Bay entry and all the upper Sound points.

Fidalgo Island, which forms the west shore of Swinomish Slough, is heavily wooded generally, and in the main is more or less broken up and hilly, although towards the north end of the slough there is quite an extent of tide-flat land, from half to 1 mile wide, and about $2\frac{1}{2}$ miles long, of which a part has been reclaimed and is now cultivated by the Swinomish Indians, who own all the portion of Fidalgo Island bordering on the slough. To the east of Swinomish Slough there is a magnificent stretch of farming country, perfectly level, that was originally salt marsh, which has been reclaimed by means of dikes and is now an extremely fertile section, the special products of which are hay and oats, of which very large crops are raised.

These flats vary in width from 4 to 6 miles and extend for many miles to the north and south.

Skagit and Padilla bays are in the main mud flats bare at low water, with a number of small narrow channels running through them, but in a large part of Swinosh Slough there is a very good depth of water. There are a number of shoals or bars, however, nearly bare at low water, which were formed in all probability by the fish traps or dams built by the neighboring Indians some years ago, but these can be easily improved by dredging.

The traffic through the slough has been very light in the last two or three years on account of the Seattle and Northern Railway bridge, which has been a serious obstruction to navigation. It is now, however, being replaced by another bridge with a clear span of 100 feet.

FIELD WORK.

The town of La Conner was the center of operations for this survey, which was started April 23, 1891, with the initial point of the meander line near the lamp designated on the map as "Lamp 100." This line, known as the A line, followed the west shore of Swinomish Slough pretty closely for a distance of about $6\frac{1}{2}$ miles to the edge and trestle of the Seattle and Northern Railway, from which place another line (the B line) was run south on the west shore of the slough a distance of nearly 5 miles to the north end of the channel known as the Hole in the Wall. These two lines were connected at various points by triangulations and the field work was thoroughly checked as the latitudes and departures of the lines were calculated. The location of the shore lines of the slough and the erection and location of the necessary signals for sounding were very quickly done, once the meander lines were run.

A series of triangles was extended through the Hole in the Wall, as chaining is impossible on account of the rocky and precipitous nature of its shores. Here various signals for sounding were located with reference to the triangulation points or by cuts from two or more of these points; the prominent points of the shore line through this channel were located by sextant observations and the shore line sketched in through these points.

The A line was run 3,600 feet south of its initial point and there was also a short line run on the east shore of McGlinns Island to locate the shore line and the necessary signals for sounding.

The soundings were taken from a small rowboat at equal intervals on diagonal lines across the slough, the points on and the ends of which were located by simultaneous observations with two sextants to the signals established and located on each shore. The soundings were taken in feet and reduced to the mean of the lower water. The soundings shown in Skagit and Padilla bays are taken from the work of the U. S. Coast and Geodetic Survey from the survey of 1890.

TIDE GAUGES, ETC.

Tide gauges were established near each end of the slough and read continuously for a period of about a month; and experienced in reconciling the daily readings with the values computed from the tide tables for the Pacific Coast of the nearest points named in the tide tables from stated above, the datum to which the soundings were observed lower low waters, which is the plane of reference and Geodetic Survey.

The tides in Skagit Bay run rather higher than those in and heights of high and low water are as a rule from one hour later than those in the latter bay, and in consequence at the Padilla end of the slough begins to ebb before the end, which causes the flow of water in the slough at about any direction, even after the tide has really begun to fall. The tidal currents in Swinomish Slough run with considerable

BORINGS.

Borings were made with a steel rod at all the shoals in every place where excavation is necessary. In nearly every instance met with in working the rod down to more than the plane of the lower low waters, and in every case the stratum which I should judge extended as far as the rod penetrated instances when the indications were that gravel was present. Borings however were taken at the cut near Whitney's bar, an alluvial nature, and, while of course it is not so easy to cause no particular difficulty in excavating. The material is of about the same nature as the above, only probably a

PROJECT FOR IMPROVEMENT.

The proposed project of improvement consists of a channel bottom, dredged to a depth of 4 feet below the mean of deep water on the south in Saratoga Passage through closely the eastern shore of Fidalgo Island through the side hole in the Wall, into Swinomish Slough, thence by the usual route practicable to and through Padilla Bay to deep

The channel through Skagit Bay leads directly from the hole to the east of Goat Island, where it makes a small inlet to the small shallow channel across the flats to deep water in

The channel skirting the southern shore of Fidalgo Island near Yacuzzi just north of Seal Rocks could be improved above named, but on account of its being more direct and through the slough the channel to the east of Goat Island is preferred.

In Padilla Bay, as shown on the map accompanying this report, the channel will follow closely the present channel leading directly to deep water, although on the score of economy if this channel were tortuous than is desirable.

Through Swinomish Slough the places where dredging is the most necessary by the hatched channel.

In the slough proper, I think there will be nothing to be feared from filling up again, at least not for several years, and then, but the disposition of the material dredged from the

In the outside bays, however, the problem of retaining the channel is proposed to be dredged is one of considerable magnitude, and will be easily disposed of by pumping behind the dikes and allowing the same to settle there.

It is believed that some sort of a training wall or wall of rock shown on the map, will be necessary in order to retain the channels in Skagit and Padilla bays.

At the north end of the slough the material to be dredged out, and bar south of the Seattle and Northern Railroad, may be disposed of by pumping it into one of the numerous sloughs west of the main channel; at the extreme southern end of

excavated could be conveniently wasted behind a brush dike on the tide flat to the west and probably made use of in reclaiming this land for agricultural purposes.

In the 2 miles of the slough from La Conner north there are about 50,000 cubic yards of excavated material to be disposed of, either behind brush dikes above or below high water, or by filling up small sloughs, or by removal on scows and wasting at the nearest available place. There is a stretch of excavation about one-fourth of a mile north of La Conner and about 1,000 feet in length, amounting to nearly 6,400 cubic yards, the excavated material from which can be conveniently wasted between the small island near by, and the mainland, by joining each end of the island with the mainland by a dike to retain the excavated material.

The excavated material in the long stretches near the mouth of White Slough and north of the same could be probably best disposed of by wasting behind a brush dike, constructed along or near the low-water line on the west shore of Swinomish Slough for a distance of about 3,900 feet opposite these shoal parts of the slough.

The disposal of any excavated material in the slough could be safely counted on, not to cost more than 10 cents per cubic yard. This includes disposal by removal on scows, or wasting behind dikes, or by any of the means mentioned, and the disposal of the 18,000 cubic yards remaining out of the above 50,000 cubic yards has been estimated at this, 10 cents per yard cost, without any specified place of deposit.

This channel proposed, from Saratoga Passage to deep water in Padilla Bay, has a total length of about 10.8 miles, with a total cost of \$122,000, or a cost per mile of about \$11,300.

SHIP-CHANNEL PROJECT.

According to the terms of the law authorizing this survey, an estimate is to be made of the cost of a ship channel through Swinomish Slough to connect Saratoga Passage and Padilla Bay; and a cross section with a depth of 25 feet below the mean of the lower low waters, and with a bottom width of 80 feet, and side slopes of one on one and a half, has been adopted for the project through the slough.

In Padilla and Skagit bays the same depth is used, but the bottom width is enlarged to 150 feet, with the same side slopes as above.

The route chosen for the ship channel is practically the same as that for the 4-foot channel, with the exception that the former passes to the east of McGlinus Island instead of through the "Hole in the Wall;" the ship channel is more direct however, as 2,000 feet has been adopted as the smallest radius.

Through Skagit Bay the route is, as in the 4-foot channel, to the east of Goat Island, where it swings off to the east of McGlinus Island, around which it passes on a curve of 2,000 feet radius and parallels the shore on a tangent for about 2,000 feet, when it reverses on another curve of 2,000 feet radius to the rocky point southwest of La Conner, where there is another reversed curve, this time of 3,000 feet radius and swinging to the west or left, which carries it past La Conner, whence the line is nearly straight or with long tangents, connected by curves of 3,000 feet radius to Padilla Bay, where the natural channels are followed as much as possible to deep water, a distance of about 4 miles from the slough.

The nature of the traffic through Swinomish Slough and from Saratoga Passage to points north thereof will never in my opinion demand a channel of the above cross section, as this class of trade is fully satisfied by steamers of 4 and 5 feet draft, and in consequence of this the estimates for this 25-foot channel have not been prepared with the same care as those for the 4-foot channel, but still they are a close approximation of the cost of such a channel. The price of dredging has been taken at 20 cents per cubic yard, and an allowance of 5 cents per yard has been made in addition to this, to cover the disposal of the material excavated in Swinomish Slough.

The total length of the ship channel from deep water in Saratoga Passage to deep water in Padilla Bay is about 12.5 miles, and the total cost for improving the same would be about \$2,134,000, or an average cost of \$170,320 per mile.

ESTIMATES.

Channel of 4 feet depth at low water:

Dredging.

Skagit Bay, 180,000 cubic yards, at 20 cents	\$36,000
Swinomish Slough, 125,000 cubic yards, at 20 cents	25,000
18,000 cubic yards, at 30 cents, including disposal	5,400
Padilla Bay, 90,000 cubic yards, at 20 cents	18,000

Total dredging..... 84,400

Diking.

Skagit Bay, wattled dike
 Swinomish Slough, brush dike
 Padilla Bay, wattled dike

Add 10 per cent for contingencies

Total

Ship channel:

Dredging.

Skagit Bay, 2,300,000 cubic yards, at 20 cents..
 Swinomish Slough, including disposal of mate
 yards, at 25 cents

Padilla Bay, 2,800,000 cubic yards, at 20 cents..

Total dredging.....

Diking.

In all, 30,000 linear feet, at \$4 per foot.....

Add 10 per cent for contingencies.....

Total

Very respectfully, your obedient servant,

 T T 15.

REPORT OF BOARD OF ENGINEERS ON PROPOS
 NECT LAKES UNION, WASHINGTON, AND S
 SOUND, WASHINGTON.

[Printed in House Ex. Doc. No. 40, Fifty-second Con

UNITED STATES E
Portland, Oreg

GENERAL: The Board appointed under the
 tember 19, 1890, to select and survey the most
 estimate the expense of construction of a ship
 Union, Washington, and Samamish with the
 having completed that duty, has the honor
 report:

The Board visited Seattle and carefully exa
 the feasible routes for the prescribed canal, a
 acter and extent of surveys and investigations
 for a complete understanding and discussion o

public meeting was held at the rooms of the Seattle Chamber of Commerce, at which citizens and other interested parties expressed their views upon the canal and its various features.

Written communications received by the Board appear as appendixes to the report.

The Board designated Capt. Thomas W. Symons as disbursing officer and as executive in charge of the surveys and preparation of the plans. The services of Mr. Philip G. Eastwick, civil engineer, were secured, and he was placed in charge of the surveys, with headquarters at Fremont, a suburb of Seattle. Mr. Eastwick's report is herewith.

Accompanying this report is a general map upon a scale of 1 inch to 14 miles, showing the vicinity of Seattle with Lakes Union, Washington, and Samamish, in their relation to Puget Sound. Upon the same sheet is a map, illustrating in greater detail the canal survey from Puget Sound to Lake Washington, upon a scale of 1 inch to 400 feet, with longitudinal profiles and cross sections along the selected canal routes.

GENERAL DESCRIPTION.

The city of Seattle, the largest city of the State of Washington, is situated upon Duwamish or Elliott Bay, an indentation of the eastern shore of Puget Sound, about half way between the upper or northern and its junction with the Strait of Juan de Fuca.

In the immediate vicinity of Seattle are the three lakes, Union, Washington, and Samamish, and also Salmon Bay and Smiths Cove, two arms of Puget Sound situated on the line of the projected canal.

LAKE UNION.

Lake Union, which is nearest to the heart of Seattle and to the Sound, is the smallest of these lakes. It has an area of 905 acres, of which 499 acres cover a depth of 25 feet, and with a maximum observed depth of 60 feet. The area of the drainage basin of this lake is 6 square miles.

This lake receives, in addition to the supply of water from its drainage basin, a considerable supply from Lake Washington, through a canal which has been cut through the divide separating the two lakes, and which is used for the passage of saw logs and small vessels.

The outlet of Lake Union is a small stream running from its extreme northern end into Salmon Bay. The distance between the lake and the Sound is 5,700 feet.

The general and average elevation of the surface of the water in Lake Union is 25.5 feet above extreme low water in Puget Sound, or 7.8 feet above extreme high water.

LAKE WASHINGTON.

Lake Washington, the largest of the three lakes, lies directly east of Lake Union and of Seattle. It is 19 miles long, averages about 2 miles in width, and has an area of 38.9 square miles, or 24,896 acres, of which probably 22,000 acres cover a depth of 25 feet or more.

The depth of this lake is very great. Soundings were not made over the entire area of the lake, but it is stated on apparently creditable

authority that depths of 600 feet have been observed. The deepest water observed was 150 feet, the length of the sounding line used.

The area of the drainage basin of the lake is 182 square miles. It receives, in addition, the drainage of the basin of Samamish Lake and River, the areas of which amount to 211 square miles.

The outlet of the lake is Black River, which unites with White River, 2.5 miles below the lake, forming the Duwamish River. The Duwamish River follows a tortuous course for about 14 miles and empties into Duwamish Bay. A short distance below the outlet of the lake the Cedar River joins the Black River and flows with it to the Duwamish, except in times of flood in Cedar River, when the waters of Cedar River run partly into Lake Washington, which thus acts as a safety valve to prevent excessive flooding of the Duwamish River valley.

The general elevation of the surface of the lake is 33 feet above extreme low water in Puget Sound, or 15.3 feet above the highest tides. It is 7.5 feet above that of Lake Union.

The most extensive shoals of Lake Washington, where the water has a depth of less than 25 feet, are at the head and at the foot of the lake and in Union Bay. That at the head of the lake, formed by the deposition of sediment brought down by the Samamish River, covers an area of about 300 acres. That at the foot of the lake is evidently formed by the deposition of sediment brought down Cedar River during floods, when a large volume of the waters of that river is emptied into the lake. The area of this shoal is about 300 acres.

The shoal in Union Bay covers an area of 610 acres, almost the entire area of that bay.

The localities of the remaining shoals and their areas, are as follows:

	Acres.
Juanita Bay, north of Kirkland, about.....	150
Three indentations, south of Houghton, about.....	200
Meydenbauer Bay, about.....	75
Mercer Slough Bay, about.....	200
Island Shoal, about.....	25
Waterworks Bay, about.....	25

LAKE SAMAMISH.

Lake Samamish lies to the east of Lake Washington and is separated from it by high ridges. It is about $7\frac{1}{4}$ miles long, with an average width of a little over 1 mile. The area of the lake is 8 square miles, and that of its drainage basin 102 square miles. Its outlet is through the Samamish River, which, 17 miles in length, flows through a swampy valley into Lake Washington. The observed elevation of the surface of this lake is 41.2 feet above low water in Puget Sound, or 9.6 feet above that of Lake Washington.

The fluctuations of the surfaces of these lakes are moderate and are elsewhere described.

SALMON BAY.

Salmon Bay is an estuary connecting through Shilshole Bay with Puget Sound.

In these bays the tide has a mean range of about 11 feet and an extreme range of nearly 18 feet.

At extreme high tide the level of Salmon Bay is 7.8 feet below the usual level of Lake Union.

This bay has not sufficient depth, even at high stage of tide, for the passage of vessels of considerable draft.

It will be seen hereafter that the recommended projects provide that the level of the water of Salmon Bay be raised to and maintained at the level of Lake Union, 7.8 feet above extreme high tide. This will necessarily cause the shores of Salmon Bay to be permanently submerged. Part of the submerged land, particularly in the town of Ballard, is occupied by buildings and wharves.

While the raising of the level of Salmon Bay must, to a very large degree, increase values of riparian lands by making a deep-water harbor, yet the submergence of certain lands gives rise to damages and losses, the particular dimensions of which have not been ascertained by the Board. These constitute a liability which is not embraced in the estimates.

The proper disposition of material excavated from the canal prism and from the bays and lakes by dredging, in raising these submerged lands, will be a factor in reducing considerably the measure of these damages.

CANAL TO LAKE SAMAMISH.

By its orders, the Board was directed to include in its survey and estimates, a section of canal connecting Lake Washington with Lake Samamish. It early became evident, however, that there was no real demand for this canal or expectation that it would be given serious consideration. Its cost has been ascertained by survey to be much greater than the cost of the portion connecting Puget Sound with Lakes Union and Washington, and the advantages to be derived from it are very small.

In the general consideration of the subject, therefore, the attention of the Board was, for the reason above stated, more particularly directed to the section connecting Lakes Union and Washington with Puget Sound.

ROUTES.

There are five possible routes for a canal connecting Lakes Union and Washington with Puget Sound.

First. By way of Duwamish Bay and the valley of the Duwamish and Black rivers to Lake Washington and across the Portage from Lake Washington to Lake Union.

Second and third. By way of depressions between the southern part of Lake Union and Duwamish Bay. (These routes were considered twenty years ago by Gen. Barton S. Alexander, and are known in his report as the "Mercer farm route" and the "Tramway route.") Thence from Lake Union to Lake Washington by a canal through the Portage.

Fourth. By way of Shilshole Bay, Salmon Bay, the valley of the outlet of Lake Union to Lake Union, and thence between Lakes Union and Washington, by a canal through the Portage.

Fifth. By way of Smiths Cove to the upper end of Salmon Bay; thence as in the preceding route.

The first route, by the valley of the Duwamish, was soon eliminated on serious consideration by its great cost and other disadvantages.

The second and third routes were practicable twenty years ago, when Seattle was a straggling village, but the land traversed by these routes is now built up with business blocks and residences and the cost of the right of way is prohibitory.

The fourth and fifth routes are entirely feasible. They have received full consideration, and estimates of cost by both routes are submitted. These two routes coincide in alignment from Lake Washington to

Salmon Bay; they differ in that one makes the connection with Puget Sound from the head of Salmon Bay, by the lower end of Salmon Bay and Shilshole Bay, and the other through a low gap to Smiths Cove and Duwamish Bay.

In each of the projects, by Shilshole Bay or by Smiths Cove, vessels to pass from the waters of Puget Sound to Salmon Bay through a masonry lock placed close to the Sound, having a lift varying according to the stage of the tide.

The dimensions of the lock are as follows: Length, 400 feet; width, 50 feet; depth on the sill at extreme low stage of tide, 16.6

These dimensions have been the subject of careful consideration the way of adapting them, at a minimum cost, to present requirements of navigation and to those of the next few years.

The depth on the sill at the lowest high water is 26 feet, so that in the lowest class of tide there will be two occasions in each two four hours when vessels of 26 feet draft may pass through the lock.

As lower high waters are generally followed by higher low waters which are but little lower than the preceding high water, there generally be two or three hours at each high water during which vessels of this draft can pass the locks. The periods for passage of sea vessels of ordinary draft (22 or 23 feet) will last from about tide on the flood to half tide on the ebb, an interval, including tides of the day, equal to twelve to fifteen hours. During the remaining portion of the day the lock will pass only vessels of less draft than 22 feet, but at all times there will be an opportunity to lock vessels of 16 feet draft.

This arrangement, which now appears to the Board to afford all the facility with a minimum of expense, may require to be modified by further consideration of increasing commerce. The sill can be lowered at any time preceding construction, and any desired degree of accommodation can be secured by an increase of cost, which, being confined mainly to the cost of the lock, need not be very great.

As before stated, both projects provide that Salmon Bay shall be raised and maintained to the level of Lake Union.

Both projects include channels excavated through the shoals out of the tidal locks, 300 feet wide and 26 feet deep at the lowest high tide that through Shilshole Bay one-half mile, and that at Smiths Cove one-third mile long, and basins at the outlets of the locks 500 feet wide and 26 feet deep at extreme low tide, in which vessels may lie to avoid lockage.

The Shilshole Bay project provides for dredging a channel through Salmon Bay to a point common with the Smiths Cove route.

The Smiths Cove project embraces a canal 80 feet wide on the bottom, 158 feet wide at the water line, and 26 feet deep through the shoals between the outer lock and Salmon Bay, and dredging through Salmon Bay to the same common point at its head.

The following features are common to both projects, viz:

A canal 6,700 feet long, 80 feet wide on the bottom, 158 feet wide at the water level, and 26 feet deep, connects the head of Salmon Bay with Lake Union.

This channel is extended through the shoal portions of Lake Union 200 feet wide, to the portage which intervenes between Lakes Union and Washington. Through this portage a canal 2,600 feet long, 80 feet wide on the bottom, 158 feet wide at the water line, and 26 feet deep connects Lake Union with Union Bay, a shoal bight in Lake Washington. A lock 400 feet by 50 feet, with a lift of about $7\frac{1}{2}$ feet and depth of 26 feet, overcomes the difference of level between the two lakes

l channel 4,600 feet long through Union Bay completes the n with deep water in Lake Washington.

tion the Smiths Cove project requires a dam at the narrows i Bay which shall serve to raise its level to the required height. imated cost of the Shilshole Bay system with masonry locks 000, and of the Smiths Cove route \$3,500,000.

ilshole Bay system costs \$600,000 less than that by Smiths n the other hand, the latter route possesses advantages in that ce is in the harbor of Seattle, whereas the entrance to the other s distant, and, secondly, the Smiths Cove entrance and lock posed to bombardment by an enemy's fleet. For these reasons is Cove route is to be preferred.

er of these routes there will be added to the commercial fa- Seattle three fresh-water areas where timber wharves can be ed and cheaply maintained and where vessels can lie in per- et water of a constant level.

Sound at Seattle the tide has an average range of more than nd an extreme range of nearly 18 feet. These oscillations of all embarrassing to commerce, and wooden wharves are liable troyed by the teredo in a few months. Marine insects are ructive to piling and other timber placed in the waters of und.

Bay, maintained at the level of Lake Union, will contain 35 ater area more than 25 feet deep, and an additional area of can be brought to the same depth by a moderate amount of

nion has an area of 499 acres of a depth of 25 feet, which can, le, be increased by dredging. The total area of the lake is 905

wo areas combined about equal the total area of the docks of nd they can readily be made equal by dredging to the area of pool docks.

ashington area contains 38.9 square miles, or about 25,000 acres, about 22,000 acres are covered with a greater depth of water et.

essential respects the extension from Lake Washington to Lake 1 conforms to the dimensions of the canal and locks below, g Lakes Washington and Union with Puget Sound.

g upon the dimensions of the canal and locks the Board has d to provide accommodation for the present and immediately e demands of commerce, and to so leave matters that, if in e greater accommodations should be required, they can be hout interfering with the usefulness of the present proposed he locks are designed to pass the largest type of ships which et Sound. A list of this shipping, as far as could be ascer- here given.

STEAM VESSELS.

Name.	Length of keel.		Beam.		Draft.	Tonnage.
	Ft.	In.	Ft.	In.	Feet.	
.....	331	3	42	2	20	3,300
.....	310		40	6	21	2,800
.....	310		40	6	21	2,300
.....	315		39	2	21	2,300
Pacific.....	331		38	5	22	1,800
.....	320	6	38	6	21	3,000
.....	198		35	3	17	1,000
.....	191	5	36	1	16	875
.....	200	1	38	2	17	1,300

For extreme length, from 10 to 30 feet must be added to the length of keel noted, and for extreme width 1 foot must be added to the beam. Heights from the water line to tops of masts will range from 120 to 150 feet.

SAILING VESSELS.

Name.	Length of keel.		Beam.		Draft.	Tonnage.
	Fl.	In.	Fl.	In.		
Spartan.....	207	6	40	5		22
Ivanhoe.....	202	3	39	3		23
Germania.....	170	2	36			19
Templar.....	155		35	2		19
Blue Jacket.....	193	8	45			21
Detroit.....	197		38	5		22
Levi G. Burgess.....	217	5	41	2		23
Alexander McNeil.....	174	4	36	6		20
Margaret.....	201		39	1		23
Highland Light.....	194	9	38	1		23
Commodore.....	226	9	41	8		24
Abner Coburn.....	225		43	2		26
Elizabeth.....	231	5	41	8		26
Alexander Gibson.....	247	5	42	6		27
J. B. Walker.....	247	1	42	2		27
Daniel I. Tenny.....	212	4	40	4		24
Undaunted.....	207	3	41			23
Edward O'Brien.....	259	1	42	2		27
T. F. Oakes.....	264	6	42	2		24
Genl Fairchild.....	203	4	38	8		23

For length "over all" from 30 to 45 feet must be added to the length of keel noted, and for extreme width 1 foot must be added to the beam. Heights from the water line to top of masts will range from 150 to 180 feet. Top masts can be so lowered as to reduce this height 30 feet.

From these lists it is seen that all the steamships are readily accommodated, and that all the sailing ships can be accommodated as to length and width are concerned, and generally with a tug in the lockage. There are a few ships noted that when fully loaded draw 20 feet.

As the tendency in modern ship building is toward steamship sailing vessels of smaller draft, it was not deemed necessary to provide at present for the few wooden sailing ships of excessive draft.

There are two classes of ships which can not be passed through the proposed lock, "ocean greyhounds," similar to those plying on the Atlantic, which would be debarred on account of their great length, a large first class battle-ships and protected cruisers of the Navy, would be debarred on account of their great breadth of beam these may be added the large side-wheel steamers engaged in the traffic on Puget Sound, of the type of the *Olympian*, which is 100 feet wide, guard to guard.

If future developments shall show the desirability of providing for the passage of such shipping, other locks can be built of size sufficient to afford the required accommodation.

The ideal arrangement for this canal would be that laid down for the Manchester Ship Canal, where three locks are planned side by side, one 80 by 600 feet, one 50 by 350 feet, and one 30 by 175 feet.

WATER SUPPLY.

Lake Washington furnishes an ample water supply for the service of the canal.

As it is probable that the lower lock will be used much more

tly than the upper lock, and as the latter at each lockage passes water than the former, it is necessary to provide for the passage

Lake Washington to Lake Union of the additional amount of x required for the lower lock. Proper weirs for this purpose are added at the upper or Portage Lock. If future developments should show the necessity for more water than can well be taken from Lake Washington, an additional supply can be obtained by turning the x of Cedar River into the lake. This can be done at a comparatively small expense.

MODIFICATIONS.

Various modifications of the plan finally fixed upon were considered by the Board of Engineers.

Principal among these was the project of bringing Lakes Union and Washington to the same level, and avoiding thereby a lock at the lake.

Three ways of doing this are available: By lowering Lake Washington to the level of Lake Union; by raising Lake Union to the level of Lake Washington, or by establishing a common intermediate level by raising one and lowering the other.

Upon a full consideration of the advantages and disadvantages which these modifications presented, it was concluded that, owing to the disturbance they would make in values along the shores of the lakes, and the consequent damages, the best plan would be to leave the lakes at their present levels and connect them by a lock.

Another modification consisted in putting in the outer lock at the head of Salmon Bay and dredging thence a channel through Salmon Bay and Shilshole bays to the deep water of the Sound; but this would greatly increase the excavation required, and the project would be deprived of the great benefit of the fresh-water basin of Salmon Bay. This is a modification which might well be considered, however, if the question of damages at Ballard and elsewhere along Salmon Bay can be satisfactorily solved.

If, in the Smiths Cove route, the question of damages arising from raising and maintaining permanently the waters of Salmon Bay at the level of Lake Union be not capable of satisfactory adjustment, the outer part of that bay, where the greater amount of damages would accrue, can be left in its present condition, and the canal be carried straight through from the outer lock to the Fremont Cut and Lake Union, and separated from the lower part of Salmon Bay, in its passage through the head of that bay, by an embankment.

A modification was also considered of substituting, for masonry locks, locks composed partly of masonry and partly of timber. If this were done, there would be a saving of \$276,000 in the cost of the outer lock, if situated at Smiths Cove or Shilshole Bay, and a saving of \$3,000 in the cost of the lock at the portage between Lakes Union and Washington.

In the project for the section of canal connecting Lakes Washington and Samamish, it is designed to carry the level of Lake Samamish through the canal to a lock placed near Lake Washington, to overcome a difference of $10\frac{1}{2}$ feet, and to make the canal partly in cut and partly in embankment. This project would have to provide for the drainage of Lake Samamish through the canal. The valley of the Samamish River is occupied by the Seattle, Lake Shore and Eastern Railway, and contains 2,180 acres of valuable land, the surface of which

Lake Washington.

The project of the Board contemplates the raising of Salmon Bay to the level of Lake Union, $7\frac{1}{2}$ feet at tide level, and permanently maintaining them at that level, which will flood an area of 187 acres above high tide, by which it is about the head of Salmon Bay, above the town.

There are some improvements about the town which will be interfered with by this raising of the water, and the necessary work required to raise the buildings and other structures affected will be considerable.

It is proposed to utilize to as great an extent as possible the material excavated from canal and channels in filling up the lands overflowed.

No other provision is made for these lands, as the great additional value given to them by the construction of basins, etc., would more than compensate the own expense which they might be put to in raising the water.

It is proposed to make, as previously stated, a cut through the outer lock to deep water in the Sound, 300 feet long, at the lowest high water.

No provision is made for maintaining this channel, but it is definitely ascertained by experience whether any other work will be required.

If it be found, after the canal is put in operation, that it shoals up, it can be maintained either by dredging to exclude drift, or by both.

ESTIMATES.

The projects contemplate the construction of a canal provided with an upper and a lower lock-gate constructed of steel, with all necessary capstans and valves, with hydraulic engines to operate them.

First, by building, instead of a double-track canal in which ships can pass each other, a single-track canal in the three portions of the canal excavated in the uplands, namely, from the outer lock to Salmon Bay, from Salmon Bay to Lake Union, and from Lake Union to Lake Washington. The canal designed and for which the estimates are made is 80 feet wide on the bottom and 158 feet at the water line. By reducing the width at bottom to 50 feet and at the water line to 128 feet, ample provision would be made for the passage of ships from one basin to another and out to the Sound, and the additional width can be given when demanded by the increase of commerce.

In the same manner the excavated channels in Salmon Bay, Lake Union, and Union Bay could be reduced from 200 feet in bottom width to 150 feet without any immediate disadvantage and the greater width is provided for when demanded by the increase of commerce.

By thus reducing the width of the canal and basin channels, a saving in excavation of 800,000 cubic yards can be made, equivalent to \$400,000, reducing the estimated cost of the work to \$3,100,000.

A second way of reducing the estimated cost would be by the substitution of composite locks of masonry and timber for masonry locks. In this way a saving of \$529,000 could be effected, still further reducing the cost to \$2,571,000.

Shilshole Bay route.—The total estimated cost of the complete project for the ship canal from deep water in Puget Sound via Shilshole and Almon bays and Lake Union to Lake Washington is \$2,902,859.23, or rounded numbers \$2,900,000.

As in the other case, the estimated cost of \$2,900,000 can be materially reduced in two ways.

First, by reducing the upland cuts from a bottom width of 80 feet to bottom width of 50 feet, and the dredged channel through Salmon Bay, Lake Union, and Union Bay from 200 feet to 150 feet width at bottom. This would reduce the amount of excavation 670,000 cubic yards, equivalent to \$335,000, and the total estimated cost to \$2,565,000.

The second way of reducing the cost would be by substituting composite locks of masonry and timber for masonry locks. In this way a saving of \$529,000 could be effected, reducing the total estimated cost still further to \$2,036,000.

The estimates for both routes include the securing of sufficient land for the full width of canal.

The following is a summary of the estimates:

<i>Smiths Cove route:</i>	
Double-track canal with masonry locks	\$3,500,000
Double-track canal with composite locks	2,971,000
Single-track canal with masonry locks	3,100,000
Single-track canal with composite locks	2,571,000
<i>Shilshole Bay route:</i>	
Double-track canal with masonry locks	2,900,000
Double-track canal with composite locks	2,371,000
Single-track canal with masonry locks	2,565,000
Single-track canal with composite locks	2,036,000

The estimated cost of the canal to connect Lakes Washington and Sammamish is \$4,927,230.

PHYSICAL DATA.

The following table gives in a convenient form the principal physical data connected with the project:

Distances:		
Extreme low tide, Puget Sound feet..	0
Mean low tide, Puget Sound do...	3.1
Lowest high tide, Puget Sound do...	5.4
Mean high tide, Puget Sound do...	11.1
Extreme high tide, Puget Sound do...	17.7
Lake Union, lowest water observed above extreme low tide of Puget Sound feet..	25
Lake Union, highest water observed above extreme low tide of Puget Sound feet..	36.1
Lake Washington, lowest water observed above extreme low tide of Puget Sound feet..	31.4
Lake Washington, highest water observed above extreme low tide of Puget Sound feet..	31.1
Lake Washington, highest water flood marks above extreme low tide of Puget Sound feet..	34.1
Lake Sammamish, lowest water observed above extreme low tide of Puget Sound feet..	41.1
Lake Sammamish, highest water observed above extreme low tide of Puget Sound feet..	44.1
Divide between Smiths Cove and Salmon Bay, above extreme low tide, Puget Sound feet..	49
Divide between Lakes Union and Washington, above low water of Lake Washington feet..	30
Divide on "Homer Farm Route," above Lake Union do...	108
Divide on "Thruway Route," above Lake Union do...	105
Areas, Acreage, etc.:		
Salmon Bay:		
Total water area acres..	311
5 feet and over in depth do...	3
Less than 5 feet in depth do...	28
Lake Union:		
Total water area do...	91
5 feet and over in depth do...	49
Less than 5 feet in depth do...	40
Greatest depth feet..	6
Lake Washington:		
Total water area, about acres..	21,89
5 feet and over in depth, (about) do...	22,00
Greatest depth, probably feet..	60
Lake Sammamish:		
Total water area acres..	5,12
5 feet and over in depth, (about) do...	4,00
Less than 5 feet in depth do...	1,12
Greatest depth, (about) feet..	10
Lake Union Drainage Basin sq. miles..	4
Lake Washington Drainage Basin do...	182
Sammamish River Drainage Basin do...	109
Lake Sammamish Drainage Basin do...	102
Distances:		
Deep water to outer lock, Shilshole Bay (from 30-foot curves) feet..	2,400
Deep water to outer lock, Smiths Cove (from 30-foot curves) do...	2,000
Outer lock, Shilshole Bay to head Salmon Bay do...	12,900
Outer lock, Smiths Cove to head Salmon Bay do...	12,000
Head Salmon Bay to Lake Union, "Fremont Cut" do...	4,800
Through Lake Union to upper lock do...	13,300
Upper lock through Portage Canal to Union Bay and Lake Washington feet..	3,100
Through Lake Union to deep water in Lake Washington do...	4,000
From Portage Canal to Lake Sammamish miles..	13
From Lake Union to Cedar River do...	17
Lake Washington, down Slack to Cedar River do...	2
Lake Washington, down Slack to White River do...	2
Duwamish River from junction of Black and White rivers to Duwamish Bay, length of valley miles..	8
Duwamish River, length of river miles..	14

mces—Continued.

Outlet Duwamish River across tide flats to deep water	do...	14
Diameter Union Bay	do...	14
Duwamish Bay to Lake Union by "Mercer Farm Route"	feet..	5,100
Duwamish Bay to Lake Union by "Tramway Route"	do...	6,310
Dimensions:		
Approach to outer lock, depth below lowest high water	feet ..	26
Approach to outer lock, width at bottom	do...	300
Outer lock:		
Width of gates, clear width	do...	50
Length of lock chamber, clear length	do...	400
Depth of lock, clear depth	do...	26
Lift of lock	do...	7.3-26.1
Lift of lock, average	do...	14.4
Channel in Salmon Bay, Lake Union, and Union Bay, depth	do...	26
Channel in Salmon Bay, Lake Union, and Union Bay, width at bottom	feet..	200
Canal excavated in uplands, Smiths Cove Cut, Fremont Cut, and Portage Cut, width at bottom	feet..	80
Canal excavated in uplands, Smiths Cove Cut, Fremont Cut, and Portage Cut, width at water line	feet..	158
Canal excavated in uplands, Smiths Cove Cut, Fremont Cut, and Portage Cut, depth	feet..	26
Upper lock, same dimensions as outer lock.		
Lift of upper lock, maximum	do...	8.1
Lift of upper lock, average	do...	6.6

Very respectfully, your obedient servants,

G. H. MENDELL,
Colonel, Corps of Engineers.

THOS. H. HANDBURY,
Major, Corps of Engineers.

THOMAS W. SYMONS,
Captain, Corps of Engineers.

Brig. Gen. THOMAS L. CASEY,
Chief of Engineers, U. S. A.

REPORT OF MR. PHILIP G. EASTWICK.

UNITED STATES ENGINEER OFFICE,
Portland, Oregon, September 14, 1891.

Sir: Under your instructions I have made surveys upon which to base a project, estimates, for the construction of a canal to connect Lakes Union, Washington, Samamish with Puget Sound, as authorized by the river and harbor act of September 19, 1890, and respectfully submit the following report:

Accompanying this report is also submitted a sheet upon which is drawn to a scale one-third of an inch to 1 mile, a general map showing the position of the lakes and their surroundings with reference to Puget Sound; and a map to a larger scale showing more in detail that part of the selected canal route between Lake Washington and Puget Sound.

On the general map are shown the respective drainage basins of the three lakes of the Samamish River, the link connecting Samamish Lake with Lake Washington.

The following table shows the relative elevations of the surface of the water in the lakes and of the high and low water planes of Puget Sound, the plane of extreme water, as deduced from observations extending over six weeks, being taken as a plane of reference.

		Feet.
Samamish.....	Highest observed	43.1
	Lowest observed	41.2
Washington.....	Highest flood marks	34.4
	Highest observed	33.1
Union.....	Lowest observed	31.6
	Highest observed	26.1
Whole Bay	Lowest observed	25.0
	Extreme high tide	17.7
	Mean high tide	13.6
	Lowest high tide	9.4
	Mean low tide	3.3
	Extreme low tide	0.0

...at the head of Lake Washington. Through this ridge no available route for a canal to connect these two of the Samamish River.

This valley is nearly thirteen miles long as measured on a straight line, however, to the sinuosities of the river, its length is about 15 miles.

The upper part of the valley is broad and flat and much of it is marshy and frequently flooded. Descending the valley narrows, and at three places, where the flanking hills meet, the thread of the valley is very abruptly defined.

An extension should at the head of Lake Washington, through the hills, will increase the total length of the canal and its advantages.

The Samamish River at low water is a stream of small size, 30 feet in width with a depth generally of from 6 to 8 feet. It reaches of rapid water passing over gravel bars where it flows in a narrow channel.

Black River, the natural outlet to Lake Washington, is about 2½ miles from the lake, the confluence of the Black River and the Duwamish River. Black River passes through a depression in the edge of a broad valley, receiving the water of a half-mile below the outlet of Lake Washington. It carries a current of about 5 miles per hour over a shoal and rocky bottom for a distance of over half a mile. A strong current continues to the head of the Duwamish, where it is about 100 feet wide.

Duwamish River flows in a very sinuous course through the head of Duwamish or Elliott Bay. On an air line between the head of Duwamish Bay the distance is about 8 miles. By the sinuosities of the river, the distance is increased to 14 miles. The floor of the bay is low and subject in most parts to overflow during freshets. This is due to the deposit of sediment, raised the surface of the valley to a higher elevation than that of the more distant part near the hills, a slightly elevated river ridge having been formed.

From the outlet of Duwamish River, at the high-water mark of the bay, there is a distance of 1½ miles over a shoal bottom which is bare at low water.

It will thus be seen that the length of a canal from Lake Washington to Puget Sound will be at least 12 miles, with such inevitable obstructions to direct alignment that will upon definite

With locks so located upon the line of the canal that it

nal prism, making the total excavation for a canal by this route exceed 14,000 cubic yards.

Most of the excavation through the valley of the Black River (which, however, a small part of the entire excavation) would be of rock. The large mass, however, of the entire excavation will probably be of easy-cutting sedimentary material. The Black and Cedar rivers at the time of freshets carry down large amounts of sand and mud, which, being deposited in the lower channel, will require to be periodically removed.

Without attempting to make a close estimate of the cost of a canal over this route, taking not only the excavation, but also the construction of water-tight embankments throughout nearly the entire length of the canal, of one or more locks with a lift of 34 feet, and of waste weirs, the compensation for land occupied, and expenses incidental to the work, it may safely be assumed that the cost of excavation and of maintenance will be so far in excess of that of a canal via Lake Union and Salmon Bay, and the benefits to commerce less advantageous, that the further consideration of this project is not warranted.

The western shore of Lake Washington, nearly midway of its length, is an opening about 2,000 feet wide between low headlands leading into Union Bay. This bay is nearly circular in shape and about 1½ miles in diameter. Its extreme western margin approaches the shore line of the eastern arm of Lake Union at a place heretofore referred to as "The Portage," where the divide is narrow and low. This place is the only remaining possible canal outlet from Lake Washington in the direction of Puget Sound.

Union Bay is very shoal throughout nearly its entire extent, and has a bottom of considerable depth. A canal line through the bay would cut into this material for a length of 4,600 feet.

A canal cut through "The Portage" will be 2,600 feet long, and the material, as noted by the walls of the existing canal, will be cement gravel and hardpan.

Lake Union, from the western end of "The Portage" cut, to a depth of 26 feet, distance is 5,600 feet. Near "The Portage" on this line the descent is abrupt to a depth of 18 feet at a distance of 700 feet. Over the remaining distance to 26 feet the bottom falls gradually with but little undulation.

The low-water level of Lake Washington is 6.6 feet higher than that of Lake Union. Flood marks of Lake Washington indicate a level 2.8 feet higher than the low-water level.

Between Lake Union and Puget Sound is a high ridge cut by a low and wide gully at the western end of the extreme western arm of the lake. Through this gully the natural drainage of the lake finds its outlet to the head of Salmon Bay, in the direction of Puget Sound.

At the extreme south end of the lake are two depressions in the ridge separating Lake Union from Duwamish Bay. These depressions are referred to in the report of General Alexander, dated December 15, 1871, where they are designated as possible canal routes. They are in that report designated, respectively, as the "Mercer Farm route" and the "Tramway Route." The distance between the lake and bay by the former route is 5,100 feet, and by the latter route, 6,310 feet, and the maximum elevations are 108 and 105 feet respectively.

At the time of this first examination of these routes (nearly twenty years ago) the land traversed by them was for the most part wild and unimproved, with an occasional clearing which added but little to its value. Much of the land in the city was then, and remained for many years later, public land subject to entry under the homestead and preemption laws, not having sufficient value to attract the attention of settlers. The shores of the lake and bay at the termini of these routes were unimproved and unoccupied except by the cheaply constructed and temporary wharves of the Seattle Coal and Transportation Company, who at that time operated a tramway over one of the routes in the transportation of coal, the need of which would have been obviated upon the construction of a canal. Seattle at that time was a village covering a very small area and having a population of about 1,200. It was surrounded by dense forests traversed by few wagon roads and trails.

According to the recent census, the city of Seattle has a population of 45,953. The boundaries of the city of Seattle now extend to, and far beyond, the location of the canal routes. Streets have been laid out, opened, and improved; costly manufacturing establishments, stores, warehouses, and dwellings have been erected, the district changed to busy industrial adjuncts to Seattle, or to residential suburbs. The water fronts on Lake Union and on Duwamish Bay are occupied by industrial establishments, warehouses, and shipping wharves.

As a result, the value of the land traversed by these routes has during the past twenty years increased to figures of large proportions, and the conditions which warranted the consideration of a project for the construction of a canal over either of the routes have now so radically changed that the consideration of such a project may with

reason be abandoned in favor of the route via the valley of lake to the head of Salmon Bay.

The last named valley extends from Lake Union to the distance of 6,700 feet. The floor of the valley is broad and level per end being but a few feet higher than the low-water level below that level at, and near Salmon Bay. The entire area has been laid off into town lots upon which a few cheap and a few sawmills have been erected, the removal of some upon the construction of a canal.

To the north of this valley and of Salmon Bay the hills rise undulating plateau. To the south lies Duwamish or Ell high and broad plateau through which is a low, narrow, and the head of Salmon Bay with Smiths Cove, an indent Duwamish Bay. Through this gap the distance is 5,000 lines and the maximum elevation, 49 feet above extreme low in both Smiths Cove and Salmon Bay are extensive mud Through this gap the lines of the Seattle, Lake Shore and company, and of the Seattle and Montana Railway Company, Seattle to the north and east.

From its head Salmon Bay extends in a westerly direction Puget Sound, at the inner edge of Shilshole Bay, from where water is nearly one-half mile over a sand beach, much of of the low-tide plane.

Estimates of the cost of construction have been prepared the lakes with Puget Sound, both on the routes from Shilshole Cove to the head of Salmon Bay, and thence by a common "The Portage" to Lake Washington, and also from Lake Samamish via the valley of Samamish River. These estimates in the Appendix and are stated in their totals, as follows:

TOTALS OF ESTIMATES.

A.	From Puget Sound to head of Salmon Bay:	
	1. Via Shilshole Bay	
	2. Via Smiths Cove	
B.	From head of Salmon Bay to Lake Union:	
	Via the valley of the outlet to the lake	
C.	From Lake Union to Lake Washington:	
	Through "The Portage"	
D.	From Lake Washington to Lake Samamish:	
	Via the valley of Samamish River	
	To these estimates of the cost of construction between Puget Sound and Lake Union, either via Shilshole Bay or Smiths Cove, is added the estimated cost of changing the location of the line of the Lake Shore and Eastern and of the Seattle and Montana Railway. And of raising mills, dwellings, stores, etc., in the town of Ballard. Detailed estimates of these are given in the Appendix.	
	In case of the construction of the canals by both the Lake Union and Lake Union routes, \$27,500 should be deducted from the summary of the cost of the dam at the narrows in Salmon Bay, which in that case would be the same.	
	The foregoing estimates are summed up for different companies as follows:	

I.—Puget Sound to Lake Union.

a.	Via Shilshole Bay:	
	Puget Sound to head of Salmon Bay	
	Head of Salmon Bay to Lake Union	
	Change of railway location	
	Raising mills, etc., in Ballard	
	Total	
b.	Via Smiths Cove:	
	Puget Sound to head of Salmon Bay	
	Head of Salmon Bay to Lake Union	
	Change of railway location	
	Raising mills, etc., in Ballard	
	Total	

Via Shilshole Bay and Smiths Cove combined:	
Puget Sound to head of Salmon Bay, via Shilshole Bay.....	\$1,019,319.51
Via Smiths Cove.....	\$1,564,721.51
Less cost of dam.....	27,500.00
	1,537,221.51
Head of Salmon Bay to Lake Union	506,000.00
Change of railway location	89,832.97
Raising mills, etc., in Ballard.....	165,000.00
	3,317,373.99

Adding to each of these estimates the estimated cost of construction between Lakes Union and Washington, the following estimates are obtained:

II.—Puget Sound to Lake Washington.

Via Shilshole Bay.....	\$2,902,859.23
Via Smiths Cove	3,448,261.23
Via Shilshole Bay and Smiths Cove combined	4,440,080.74

Added to these last estimates the estimated cost of construction between Lakes Washington and Samamish be added, the entire cost of the canal connecting Lakes Samamish, Washington, and Union with Puget Sound under the different projects obtained as follows:

III.—Puget Sound to Lake Samamish.

Via Shilshole Bay.....	\$7,830,089.23
Via Smiths Cove	8,375,491.23
Via Shilshole Bay and Smiths Cove combined	9,367,310.74

All estimates through the higher lands are based upon excavation of a canal having a cross section as shown on the accompanying map; the bottom of the canal to be 80 feet wide and side slopes $1\frac{1}{2}$ horizontal to 1 vertical up to the tow path or lower berme, 32 feet above the bottom; the depth of water in the canal to be 26 feet. In the deeper excavation the slopes above the towpath or lower berme to be $1\frac{1}{2}$ horizontal to 1 vertical, with narrow bermes at each 20 feet of excavation.

Through Salmon Bay, interior to the outer locks and through the shoals in Lakes Union, Washington, and Samamish, and in Union Bay, the estimates contemplate excavation of a channel 200 feet wide carrying 26 feet depth of water. Exterior to the locks of Salmon Bay and Smiths Cove the width of channel is increased to 300 feet, with a depth of 26 feet of water at the lowest high tide.

The locks at the outlet of the canal systems both at Salmon Bay and Smiths Cove are to be placed near to Puget Sound, raising the water in the canal between them to Lake Union to the level of the lake. Each of the locks will have a lift of 16.7 feet at low high water and 26.1 feet at extreme low water.

The lock at the Portage provides for a lift of 8.1 feet, the difference of level between the low-water level of Lake Union and the extreme flood level of Lake Washington. This lock is to be placed at the head of Lake Union and adjoining the excavation at the Portage.

The lock between the waters of Lakes Washington and Samamish provides for a lift of 10 $\frac{1}{2}$ feet, the difference of level between the low-water level of Lake Washington and the high-water level of Lake Samamish. Upon the excavation of the canal through the Samamish River Valley, a freer outlet of the waters of Lake Samamish will be offered, and the rise in that lake, which is caused by the present restricted channel, will be reduced, thereby lessening the lift required in the lock. This lock is to be placed at the outlet of the river at the head of Lake Washington.

The projects contemplate the construction of masonry locks provided with an upper and a lower lock gate and a guard gate constructed of steel, all necessary machinery, inlet and outlet valves, and hydraulic engines to operate them, the power to be supplied by turbine wheels. The locks are to have a clear width of 50 feet at the gates and in the main lock prism, and a length to accommodate vessels 400 feet long.

Independent estimates have been made for all locks wherein the masonry has been, far as practicable, replaced by structures of timber.

Substituting such structures, the cost of the respective locks will be reduced as follows:

	Difference in cost.
Lock at outlet at Salmon Bay.....	\$276,000
Lock at outlet at Smiths Cove	276,000
Lock at the Portage	253,000
Lock at head of Lake Washington	271,000

In the following table are given the lengths of the several reaches of canal on the route via Shilshole Bay, including the lengths of the intermediate reaches in Lake Union and Washington, where no improvements are required.

	Miles.
From deep water of Puget Sound to head of Salmon Bay and point common with Smiths Cove Canal	2.84
Fremont Canal and approaches	1.02
Lake Union (no improvement required)	1.50
Portage Canal and approaches	2.42
Lake Washington (no improvement required)	8.25
Samamish Valley Canal and approaches	14.07

From these data the following table of lengths is deduced:

	Miles.
From Puget Sound to Lake Union	3.86
From Puget Sound to Lake Washington	7.78
From Puget Sound to Lake Samamish	30.10

The length of the reach from deep water of Puget Sound at Smiths Cove to a connection with the route via Shilshole Bay, at the head of Salmon Bay, is 2.59 miles.

No data concerning the rainfall in the immediate vicinity of the area of the drainage basins of the lakes are to be obtained. The nearest approach to such information is furnished by the continuous observations of rainfall since the beginning of 1878, made on Bainbridge Island, which lies at the west side of Puget Sound, about 12 miles due west of the middle of Lake Washington.

The following table gives the total amount of rainfall for each year at the locality named:

Year.	Rainfall.	Year.	Rainfall.
	Inches.		Inches.
1878.....	46.35	1885.....	42.58
1879.....	64.87	1886.....	34.32
1880.....	49.35	1887.....	49.03
1881.....	53.82	1888.....	33.97
1882.....	42.53	1889.....	27.58
1883.....	34.85	1890.....	30.25
1884.....	34.03		

The record for the year 1890, by months, is as follows:

Year.	Rainfall.	Year.	Rainfall.
	Inches.		Inches.
January.....	7.71	July.....	0.50
February.....	4.88	August.....	0.26
March.....	2.90	September.....	0.07
April.....	1.54	October.....	3.00
May.....	1.03	November.....	0.74
June.....	1.65	December.....	5.00

From the first table it will be seen that the average rainfall per annum for the thirteen years is 41.74 inches, and for the last five years, 34.62 inches.

The second table shows that the greatest precipitation occurs in December and January, when nearly 45 per cent of the whole annual rainfall is precipitated, and that the months of July, August, and September cover a dry period when the precipitation is less than 0.3 per cent of the total.

The data upon which to estimate the amount of the evaporation from the surface of the drainage basins and lakes are not obtainable, but from the fact that at all times, except when checked by a rise in the waters of Cedar and White rivers, there is an outflow from Lake Washington through Black River, it is evident that the supply from the drainage basins exceeds at all times the evaporation. The amount of this excess during the dry seasons it is impossible to determine upon any data at present available.

An examination of the tide tables for the Pacific coast, for 1891, shows that the average height of all the tides during the months of July, August, and September, referred to the plane of extreme low water, is 10.6 feet, and, as the low-water level of Lake Union, after the construction of the canal, will be 25 feet above the same plane, the average lift of the lock, either at Salmon Bay or Smiths Cove, will be 14.4 feet.

The lift of the lock at the Portage at the low-water stage in both lakes is 6.6 feet, and as the length and width of the prism of this lock are the same as those of the locks at Puget Sound, the relative amounts of water discharged in one lockage at each lock will be in proportion to the lift. The lock at Puget Sound will consequently require for the lockage of a vessel an average of nearly 2.2 the amount of water that is required for the same work at the Portage lock.

As the supply of water from the basin of Lake Union is, during the summer months, practically nothing, all the water required for the lockage at the outer lock during that period must be drawn from Lake Washington.

With a number of lockages at the Portage equal to that at the outlet, the volume of water supplied by the former lock will be less than one-half that required for the latter, and the deficiency must be supplied from Lake Washington by a channel other than that through the Portage lock. In the projects upon which the estimates have been based this is provided for, for the dry season of the year by an independent culvert or channel.

During the portion of the year when the water supply to Lake Washington is such to keep the water of that lake at a higher elevation, this deficiency is provided for by the overflow of a waste weir at the Portage lock, any excess not required at Puget Sound being disposed of by a similar waste weir at the outer lock.

It is estimated that the average time required to fill or to supply the prism of either of the locks at Puget Sound will be ten minutes, and that ten minutes more will be consumed in moving a vessel in and out of the locks and in opening and closing the gates, or that, for one complete lockage, either up or down, twenty minutes will be required.

The greatest number of vessels can be passed through either of the locks in a given day by passing them alternately up and down, and under such conditions, one lockage of water only will be used in passing one vessel each way.

Under the assumption as to the time of operating the outer lock, one vessel can be passed each way through the lock in forty minutes, or thirty-six vessels each way in twenty-four hours, and thirty-six lockfuls of water will be required for the work. The following are the areas of the respective drainage basins, and of the water surfaces which they contain:

	Area of drainage basin.	Area of water surface.
	Sq. miles.	Sq. miles.
to Samamish	102	8.0
Samamish River	109	0.3
to Washington	182	38.9
to Union	6	1.5
Total	399	48.7

The cubic capacity, between gates, of the prism of either of the locks at Puget Sound, with an average lift of 14.4 feet, is 302,400 cubic feet. As the inflow to Lake Washington during the dry season of the year is in excess of the evaporation from the surface, any draft upon the waters of that lake for lockage at Puget Sound will in part be supplied by the excess, which will be diverted from the outlet at Black River.

Assuming, however, that there is no excess to supply this draft in part, but that the lockage water required tends to the lowering of Lake Washington, it is evident that the level of that lake will be reduced by continuous lockages during the three dry months to the extent of a little over nine-tenths of a foot. Any leakage of water at the outer lock in excess of that required for lockage and the water necessary to operate the hydraulic motors at the lock, will increase the abatement. The amount of the leakage will depend upon the construction of the gates and valves, and by care in construction can be reduced to a small amount. The water required for power will be but a small fraction of the whole, and, until data for more accurate computation are afforded, may be ignored as immaterial in these approximations.

Assuming that the supply during the dry season will be furnished by the entire water area of Lakes Samamish and Washington and Samamish River, covering an area of 47.2 square miles as against 38.9 square miles of Lake Washington alone, the abatement due to the lockages during the three dry months will be three-fourths of a foot.

In the computations upon which the above statements are based, no account is taken of the small amount of rainfall during the three dry months, which if considered would decrease the computed abatement of the lakes.

Continuous and uninterrupted operation of one of the locks on Puget Sound during an entire year will admit of 13,140 double lockages. A rainfall of 4.35 inches during the entire year drained from the basins of Lakes Washington and Samamish and of Samamish River, having a combined area of 393 square miles, will supply all the water required for the total annual lockage.

The average current through the canal due to the water required for lockage will be less than three one-hundredths of a mile per hour.

Cedar River, entering Black River about a half a mile below the outlet of Lake Washington, is subject to annual floods, due chiefly to the melting of the snow at the head waters of that river in the Cascade Mountains. At infrequent intervals in the past, this flood has reached large proportions, overflowing for a short time the land adjacent to the outlet. The ordinary annual floods, however, do not overflow the banks of the river.

During the height of these floods the direction of the current in Black River between Lake Washington and the mouth of Cedar River is reversed, and a large volume of water is then emptied into the lake, causing a very marked elevation of its level.

In former years the Duwamish River was more obstructed by bars and drift than at present, and the low bottom lands in that valley were covered with a dense growth of brush, which impeded the flow of water when those lands were submerged. The subsequent removal of obstructions in the river and the clearing of the low lands, permitting a much freer discharge toward Puget Sound, has so materially checked this reflux into the lake that the fluctuations of its level have been reduced from about 7 feet, reported as the maximum of former years, to less than 3 feet in recent years. It is probable that the last noted rise in the level of the water may be somewhat exceeded, as there has not been in recent years any extraordinary flood corresponding to that causing this former maximum rise.

The construction of the canal will open an outlet that will carry off to Puget Sound, through Lake Union and Salmon Bay, such a volume of water that will tend to further reduce the range of fluctuation in Lake Washington, without inducing a current that will interfere with the traffic on the canal or endanger its banks. The effect in that direction of the relatively small discharge through the existing small canal at the Portage has been manifest during the past few years.

It appears probable, therefore, that upon the recurrence of one of the greater floods after the construction of the canal, the rise, by reason of the freer outlet offered to the water, will not be so excessive as to interfere with the navigation of the canal.

The fall in Black River is such as to admit of the enlargement of its channel by deepening the rock bottom so as to permit a larger outflow from Cedar River to the Duwamish River.

The following table shows the fall of the surface of the water in Black River, at the time of survey, over a distance of 8,300 feet, from the lake:

	Distance from Lake Washing- ton.	Interval.	Elevation above datum.	Fall.	Rate of fall: foot per hundred.
	Feet.	Feet.	Feet.	Feet.	
Lake Washington	0		33.1		
Head of Cedar Rapid	3,100	3,100	33.1	0.0	0.00
Mouth of Cedar Rapid	3,600	500	32.5	0.6	0.12
Foot of Cedar Rapid	4,500	900	30.8	1.7	0.19
Station 45	5,700	1,200	29.5	1.3	0.11
County road bridge	6,600	900	28.5	1.0	0.11
Railroad bridge	8,300	1,700	26.7	1.8	0.11

In the early consideration of the canal project the equalization of the levels of Lakes Washington and Union was considered: First, by lowering the level of Lake Washington to that of Lake Union; second, by raising the level of Lake Union to that of Lake Washington; and third, by bringing the levels of both lakes to one intermediate between the two.

All of these schemes dispense with the need of a lock at the Portage.

In those wherein the water of Lake Union is raised, the amount of excavation in the canal prism between that lake and the outer locks will be decreased, but the cost of the locks will be increased by reason of the greater lift required to be given to them.

The structures on the lake front are built to the present water level, and should the water materially be raised, extensive and costly changes would be required in such structures.

The raised lake level being carried through to the outer locks at Puget Sound, the lower parts of the town of Ballard and other low land adjacent to Salmon Bay,

which, under the projects adopted in the estimates, are submerged, would be flooded to a greater depth, and the area of submerged land largely increased; to such an extent, indeed, as to increase the measure of damages largely in excess of that of any advantages that might be derived from such a scheme.

The lowering of the level of Lake Washington to any considerable extent would check the outflow through Black River, or stop it altogether, and draw from Cedar River, especially during its flood, a larger part of its waters. In that case an outlet for the entire water of the drainage basins of the lakes, together with a part of the waters of Cedar River largely in excess of that now emptying into the lake, would have to be provided for through the canal. This would materially interfere with the navigation of the canal, not only during the time of floods, but for some time thereafter, until the accumulated waters had passed off.

Under such a scheme, the amount of the excavation at the Portage would be very materially increased.

It is very doubtful whether it is practicable, to lower, through a canal of the standard cross-section to be cut through at the Portage, the surface of the water in Lake Washington, containing an area of 38.9 square miles, to the level of Lake Union, and that of Lake Samamish and Samamish River, containing an area of 8.3 square miles attendant upon it.

The want of reliable data of the amount of rainfall and evaporation in this district; of the duration and rate of fall during the heaviest precipitation; of the retardation of such precipitation on its way to the lakes, and of the volume of discharge through the channels of Samamish and Black rivers, under various conditions of elevation of water surface in the lakes, precludes any estimate of the effect of the inflow and outflow upon the lakes.

The ratio of the area of the larger drainage basin, containing Lakes Samamish, Washington, and the Samamish River, to the entire water area of the basin is 8.33, and to the area of Lake Washington alone 10.10. These figures represent also the ratio of rise in the lakes to depth of rainfall, if such rainfall were entirely to find its way without impediment to the lake and the outflow were at the same time prevented. As, however, the rainwater is retarded on its way to the lake and the discharge is increased as the lake rises, a rise to the extent denoted by these figures will not occur. Only in the event of a heavy rainfall continuous for a long time can there be any material rise in the surface of the water of the lakes, and upon the abatement of such excessive rainfall the lowering of the water surface will speedily follow by reason of the uninterrupted outflow. Heavy rainfalls are of rare occurrence in this country. The rainfall during the rainy season is such as to make the supply to the lakes nearly uniform.

The excavations along the lines of railroads and streets, in the small canal through the Portage and in the ditch connecting Lake Union with Salmon Bay, show that the material of the upland in the vicinity of the proposed canal is largely composed of cement gravel, and hardpan, with occasional beds of loose gravel and sand. Nowhere in the vicinity is rock exhibited except in the case of infrequent granite boulders. The nearest rock in place is a soft sandstone ledge which crosses Lake Washington in nearly east and west direction at the south end of Mercer Island and crops out in the Duwamish Valley a short distance above the mouth of the Duwamish River. This ledge is nowhere nearer than 6 miles to the proposed canal.

In the gap between Salmon Bay and Smiths Cove the railroad excavation which has been made, has been through loose sand and gravel easily moved. It is probable that the deeper excavation required for the canal will be largely in harder material.

In Smiths Cove and Salmon Bay the bottom is of soft mud, with a substratum of hard material. The notes of pile-driving on the line of the Seattle and Montana Railroad show the depth of soft material in Smiths Cove, except near the high-water mark, to be from 30 to 50 feet, and throughout a greater part of their line across Salmon Bay to be from 20 to 30 feet deep, the hard material being below the limit of a canal excavation.

The area of deep soft mud in Salmon Bay does not probably extend beyond the wider parts of the bay. At and below the narrows, the depth of soft mud is probably less, but as the depth of excavation for the canal is much less there, than where the deeper mud occurs, the canal excavation will probably not to any extent encroach upon the harder substratum.

The bottom of Lake Union, wherever excavations will be required, is of stiff clay. In Union Bay the ooze forming the bottom is of considerable depth. The character of the substratum is not known, but it is probably of hard material similar to that exposed in the adjacent cut through the Portage.

Elsewhere along the line of the canal between Puget Sound and Lake Washington the material, with but little exception, will probably be found to be cement gravel and hardpan.

The material of the surface in the Samamish Valley is an alluvium, the depth of

which is unknown except at a few places in the upper part of the valley an abrupt bends near Lake Washington, where gravel and hard material are sh

Lake Union has an area of $1\frac{1}{2}$ square miles. The length along its axis is and the length of shore line following its principal indentations is $8\frac{1}{2}$ miles. The greatest depth of water is 60 feet, averaging 50 feet over a large extent, except near the head of the lake, at the Portage, where the water shoals. The shore is abrupt. This lake affords good anchorage ground.

Lake Washington has an area of 38.9 square miles, and a length, measured north to south, of $18\frac{1}{2}$ miles. The length of the main shore line is 50 miles, of Mercer Island $11\frac{1}{2}$ miles. The greatest width is $4\frac{1}{2}$ miles at the head of the island, and the average width a little more than 2 miles. Excepting at the upper and lower ends of the lake, and in the few small bays along the shore, no shoals have been discovered. The shores, except at the shoals, are abrupt and fall rapidly to a depth exceeding 20 fathoms. It is reported on credible authority that the greatest depth exceeds 100 fathoms.

Lake Samanish has an area of 8 square miles and a length of 7 miles. The length of shore line of 18 miles. The greatest width is $1\frac{1}{2}$ miles and the depth is little over 1 mile. The shores are generally abrupt. A depth of from 50 to 100 fathoms is quite uniformly maintained. The shoals at the head and foot of the lake are of small extent.

Very respectfully, your obedient servant,

Capt. THOMAS W. SYMONS,
Corps of Engineers, U. S. A.

PHILIP G. EASTMAN

APPENDIX—ESTIMATES OF COST.

A.—FROM PUGET SOUND TO HEAD OF SALMON BAY.

1.—Via Skillehole Bay.

Excavation:	Cubic yards.		
Skillehole Bay.....	302,000		
Pool below lock.....	161,000		
Lock pit.....	84,000		
Salmon Bay.....	324,000		
Total	871,000	at 50 cents	
Masonry lock at Salmon Bay:			
Inclosure for lock pit:			
Piles, 140, 28 feet long, 3,920 linear feet, at 8 cents.....			\$313.60
Driving piles, 140, at \$3.....			420.00
Lumber:		Feet, B. M.	
Wales, 3,400 linear feet, 8 by 8 inches.....	18,000		
Plank, 15,000 square feet, 3 inches.....	45,000		
Total	63,000	at \$10	630.00
Iron and spikes:		Pounds.	
Drift bolts, 420, $\frac{3}{4}$ by 20 inches.....	700		
Cut spikes, 6 inches.....	1,000		
Total	1,700	at 4 cents	68.00
Erecting, 63,000 feet, B. M., at \$10.....			630.00
Embankment, spreading and packing spoil from excavation, 16,000 cubic yards, at 10 cents.....			1,600.00
			\$3,661.60
Masonry:	Cubic yards.		
Main lock walls.....	32,000		
Guard-gate recess walls.....	2,000		
Upper lock-gate recess walls.....	1,300		
Lift wall.....	220		
Invert under main prism.....	3,700		
Invert under head bay.....	360		
Invert under tail bay.....	250		
Upper miter-sill wall.....	60		
Double lower miter-sill wall.....	140		
Stepdown.....	25		
Counterparts.....	570		
Culverts.....	1,600		
Total	42,225		
Classified as follows:			
Granite coping, quoins, sills, etc., 300 cubic yards, at \$75.....			22,500.00
Wall and culvert facing, 1,700 cubic yards, at \$15.....			25,500.00
Concrete, 40,225 cubic yards, at \$8.50.....			341,912.50
			389,912.50

Steel lock gates:		
Guard gate, 26 tons, at \$200.....	\$5,200.00	
Lower lock gate, 34 tons, at \$200.....	6,800.00	
Upper lock gate, 16 tons, at \$200.....	3,200.00	
Gate bearings, 6 tons, at \$500.....	3,000.00	
Gate anchorages, 6 tons, at \$500.....	3,000.00	
Gatepost anchor straps, 6 tons, at \$500.....	3,000.00	
		\$24,200.00
Power house (including foundations and turbine wells).....		10,000.00
Machinery for operating locks and erecting same:		
Turbines, 56 inches diameter, 2, at \$1,500.....	3,000.00	
Hydraulic gate engines, 6, at \$500.....	3,000.00	
Hydraulic sluice gates and engines, 4, at \$1,000.....	4,000.00	
Hydraulic capstans and engines, 6, at \$1,000.....	6,000.00	
Hydraulic accumulator pumps, 2, at \$1,500.....	3,000.00	
Accumulator reservoir.....	2,000.00	
Smoothing posts, 6, at \$200.....	1,200.00	
Pipes and valves to connect engines with accumulator reservoir.....	3,000.00	
Supply and delivery pipes for turbines.....	5,000.00	
		30,200.00
Dwelling house for gate-tenders.....		5,000.00
Electric-light plant for lighting locks.....		4,000.00
Removal of bulkheads.....		2,000.00
		\$468,974.10
Dam and embankment at Salmon Bay:		
Excavation for foundation, 3,000 cubic yards, at 50 cents.....	1,500.00	
Lumber, 270,000 feet B. M., at \$10.....	2,700.00	
Iron and spikes, 26,000 pounds, at 4 cents.....	1,040.00	
Erecting timber structure, 270,000 feet B. M., at \$10.....	2,700.00	
Earth and puddle filling of dam:		
Puddle, 1,600 cubic yards, at 75 cents.....	1,200.00	
Earth (spoil from excavation), spreading and ramming 3,200 cubic yards, at 20 cents.....	640.00	
		1,840.00
Earth embankment: To connect dam with lock walls and shore (spoil from excavation), spreading and ramming 40,000 cubic yards, at 15c.....		
		6,000.00
Stone revetment: To protect outer slope of dam, 3,200 cubic yards, at \$2.....		
		6,400.00
		22,180.00
		926,654.10
Engineering and contingencies, 10 per cent.....		92,665.41
Total.....		1,019,319.51

S. Via Smiths Cove.

Excavation:		Cubic yards.	
Smiths Cove.....	223,000		
Pool below lock.....	178,000		
Lock pit.....	84,000		
Boulevard cut.....	963,000		
Salmon Bay.....	249,000		
Total.....	1,697,000, at 50 cents.....		\$848,500.00
Masonry lock at Smiths Cove: Estimated to cost the same as that at Salmon Bay.....			468,974.10
Dam and embankment at the narrows in Salmon Bay.....			25,000.00
Wagon-road bridge, pier, and abutments, including electric motor to operate same.....			8,000.00
Land damages and right of way:			
21 acres at \$1,000.....	\$21,000.00		
17 acres at \$3,000.....	51,000.00		
			72,000.00
			1,422,474.10
Engineering and contingencies, 10 per cent.....			142,247.41
Total.....			1,564,721.51

B.—FROM HEAD OF SALMON BAY TO LAKE UNION.

Excavation:		Cubic yards.	
Fremont Cut.....	701,000		
Shoal, foot of Lake Union.....	13,000		
Total.....	714,000, at 50 cents.....		\$357,000
Wagon-road bridges, piers, and abutments, including electric motors to operate same:			
At Ross, 1 bridge.....		\$8,000	
At Fremont, 1 bridge.....		8,000	
Electric wire line to connect motors at bridges with dynamo at lock, 4 miles, at \$500.....		2,000	
			18,000

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Land damages and right of way, 30 acres at \$3,000.....	
Removal of sawmill, dwellings, and stores at Fremont.....	
Engineering and contingencies, 10 per cent.....	
Total.....	

C.—FROM LAKE UNION TO LAKE WASHINGTON.

Excavation:	Cubic yards.
Shoal, head of Lake Union.....	135,000
Lock pit.....	67,000
Portage cut.....	537,000
Shoal, Union Bay.....	280,000
Total.....	1,194,000, at 50 cents.....

Masonry lock at Portage: -
 Inclosure for lock pit, embankment (spoil from excavation), spreading, and packing, 14,000 cubic yards, at 10 cents..... \$1,400.00

Masonry:	Cubic yards.
Guard-gate recess walls.....	1,400
Main lock walls.....	24,000
Upper lock-gate recess walls.....	1,400
Lift wall.....	115
Invert under main prism.....	3,700
Invert under head bay.....	360
Invert under tail bay.....	250
Upper miter-sill wall.....	60
Double lower miter-sill wall.....	140
Stepdown.....	30
Counterparts.....	340
Culverts.....	1,600
Total.....	33,385

Classified as follows:

Granite coping, quoins, sills, etc., 300 cubic yards, at \$75.....	\$22,500.00	
Wall and culvert facing, 1,500 cubic yards, at \$15.....	22,500.00	
Concrete, 31,385 cubic yards, at \$8.50.....	268,472.50	313,472.50

Steel lock gates:

Guard gate, 16 tons, at \$200.....	3,200.00	
Lower lock gate, 13 tons, at \$200.....	2,600.00	
Upper lock gate, 18 tons, at \$200.....	3,600.00	
Gate bearings, 6 tons, at \$500.....	3,000.00	
Gate anchorages, 6 tons, at \$500.....	3,000.00	
Gatepost anchor straps, 6 tons, at \$500.....	3,000.00	
		18,400.00

Power house, including foundations and turbine wells.....

Machinery for operating locks and erecting same:		10,000.00
Turbines, 56 inches diameter, 2, at \$1,500.....	3,000.00	
Hydraulic gate engines, 6, at \$500.....	3,000.00	
Hydraulic sluice gates and engines, 4, at \$1,000.....	4,000.00	
Hydraulic capstans and engines, 6, at \$1,000.....	6,000.00	
Hydraulic accumulator pumps, 2, at \$1,500.....	3,000.00	
Accumulating reservoir.....	2,000.00	
Snubbing posts, 6, at \$200.....	1,200.00	
Pipes and valves to connect engines with accumulator reservoir.....	3,000.00	
Supply and delivery pipes for turbines.....	5,000.00	
		30,200.00
Dwelling house for gate-tenders.....		5,000.00
Electric-light plant for lighting locks.....		4,000.00
Removal of bulkheads.....		2,000.00

Dam and embankment at Portage:

Excavation for foundation, 3,000 cubic yards, at 50 cents.....	1,500.00
Lumber, 170,000 feet B. M., at \$10.....	1,700.00
Iron and spikes, 15,000 pounds, at 4 cents.....	600.00
Erecting timber structure, 170,000 feet, B. M., at \$10.....	1,700.00
Earth and puddle filling of dam:	
Puddle, 800 cubic yards, at 75c.....	600.00
Earth (spoil from excavation) spreading and ramming 1,600 cubic yards at 20c.....	320.00
	920.00
Earth embankment to connect dam with lock walls and shores (spoil from excavation) spreading and ramming 10,000 cubic yards at 15c.....	1,500.00
Stone revetment to protect outer slope of dam, 2,000 cubic yards at \$2.....	4,000.00

ortage to supply water for outer lock during the		
idges, piers, and abutments, including electric mo-		\$5,000.00
le same:		
one bridge	\$8,000.00	
rtage, one bridge	12,500.00	
re line to connect motors at bridges with dynamo		
1/4 miles, at \$500	750.00	
		21,250.00
and right of way, 12 acres, at \$500		6,000.00
it of existing canal rights		30,000.00
		<hr/>
		1,020,642.50
nd contingencies, 10 per cent		102,064.25
		<hr/>
		1,122,706.75

D.—FROM LAKE WASHINGTON TO LAKE SAMAMISH.

canal prism:	Cubic yards.	
of Lake Washington	455,000	
	50,000	
Valley cut	6,770,000	
	<hr/>	
	7,275,000 at 50 cents	\$3,637,500.00
rainage ditches at sides of canal, 504,000 cubic yards at 50 cents		252,000.00
at Lake Washington, complete		410,000.00
nkment at Lake Washington		15,000.00
idges, piers, abutments and approaches, 9 bridges at \$8,000		72,000.00
ea, piers and abutments, 4 bridges at \$10,000		40,000.00
adjustment of Seattle, Lake Shore and Eastern Railway		10,000.00
and right of way, 423 acres at \$100		42,800.00
		<hr/>
		4,479,300.00
nd contingencies, 10 per cent		447,930.00
		<hr/>
		4,927,230.00

Change of railway locations at and near Salmon Bay.

ESTIMATE OF COST.

000 cubic yards, at 30 cents		\$9,600.00
(spoil from excavation) spreading 104,000 cubic yards, at 15 cents		15,600.00
restle, 1,300 linear feet, at \$9		11,700.00
astle, 1,100 linear feet, at \$6		6,600.00
combined rail and wagon roads, including piers, abutments, and electric		
ate same		15,000.00
adjusting old track two-thirds of a mile, at \$400		266.67
odoned track 3 1/2 miles, at \$400		1,250.00
or excess of new location over old location, new rails, 60 pounds per yard,		
).....	\$760.00	
00 pounds, at 4 cents		88.00
s, 300 pounds, at 5 cents		15.00
l, at 25 cents		2,250.00
s, 18,000 pounds, at 4 cents		720.00
		<hr/>
ck, 3 1/2 miles at \$800		3,833.33
0 cubic yards, at 75 cents		2,666.67
acres, at \$1,500		6,150.00
		<hr/>
		9,000.00
		<hr/>
nd contingencies, 10 per cent		81,666.34
		8,166.63
		<hr/>
		89,832.97

TION OF MR. WESLEY WILSON, MANAGER SEATTLE COAL AND IRON COMPANY.

SEATTLE, WASH., March 24, 1891.

N: Having been advised of your being in this city for the purpose of the ground of the proposed route of said canal, and obtaining such as to the necessity of, and needs to, commerce and the probable advantages therefrom, I would, on behalf of the Seattle Coal and Iron Company, submit the following for your information:
are located at Gillman, King County, Wash., about 3 miles distant from Samamish Lake.

Sound with Lake Washington. After reading, we would be pleased to have forward it, with other matter which you are having prepared bearing upon the same, to the War Department.

Your statement has been compiled as suggested by you when here.

Yours respectfully,

J. FURTH, *President.*

G. H. MENDELL.

REPORT OF COMMITTEE OF CHAMBER OF COMMERCE, SEATTLE, WASHINGTON.

A survey authorized by Congress of the proposed ship canal to connect Lake Washington with Puget Sound at Seattle having been completed, the attention of the honorable Secretary of War is respectfully invited to the following, presenting the reasons why this canal should be promptly constructed: The city of Seattle is situated almost in the center of the Puget Sound region and has a frontage on the water of more than 6 miles. Lake Washington lies abreast of the city to the westward parallel with the Sound, and stretches north and south 25 miles. Deeply fringed by a cordon of mountains, its surface is always smooth and safe; its shores are old but not rocky; its waters of ample depth, soundings showing in places 100 fathoms. Its width near its central section is 4 miles and the average nearly 3 miles, affording ample room for handling vessels of the largest size. The absence of tidal currents renders its waters always tranquil and of even stage, and preserves its purity without washing. Its outlet by an easy channel, enters the sound almost within the present city limits.

Surrounded with fertile lands, backed with inexhaustible forests of finest timber, and rich in fields of coal, iron, and the precious minerals, the situation of this lake has long been calculated to attract the attention of every one versed in the ways of business or commerce who has visited the region.

Other than such body of fresh water lying within 2 miles of a noble ocean road, with which it may easily be joined, capable of being converted into anchorage for the fleets of the world, does not exist elsewhere; it would seem to have been provided by nature as a complement to our harbor system which it will make complete in its every feature.

Puget Sound having been for many years marked as the seat of coming great commercial expansion has become within the last decade the focus of enterprise looking toward the realization of its future—four great transcontinental railway lines for the benefit of their systems. Already two of them are establishing ocean steamship lines to the coast and the oriental islands.

From all parts of the world come fleets of vessels to this inland sea to compete for cargoes which lie close at hand or arrive as freights of railways spanning the continent. The tide of progress in trade and commerce is so rapid as to run ahead of the facilities provided, and such is likely to be the case for years to come. The future of the industrial world are set this way.

A cursory glance at the resources of the State shows good reasons for faith in its future.

The wheat yield of 1890 was about 20,000,000 bushels; that for the current year is estimated at over 25,000,000 bushels. The capacity of the natural wheat fields of the State is conservatively placed at 200,000,000 bushels per year. Oats, barley, hay, and all kinds, and every farm crop reward the husbandman with prodigious returns.

The lumber output of the State for 1890 was 1,222,830,042 feet, an increase of 538,647,042 feet over the preceding year; nearly 100 per cent. This does not include the lumber used for laths, pickets, etc., estimated at 174,186,800 feet.

The coal output of the State for 1890 was 1,349,773 tons, an increase of 438,246 tons, or 50 per cent for the year.

Coal measures are practically limitless in extent, and the business of shipping in its infancy. The amount of coal mined hereafter, will be limited only by demand for it.

Ores of the best quality, fit for steel making, cover an immense field. Works of manufacture are in course of construction at several points.

Precious metals are distributed over a wide area and promise enormous development of wealth. Gold, silver, lead, copper, tin, asbestos, graphite, limestone, granite, and sandstone are to be added to iron and coal. All these combined with an opulence of mineral resources rarely encountered in a single field.

The State possessing unrivaled agricultural advantages, and having superadded to such incomparable wealth in timber and minerals, can not fail to develop manufacturing interests on a gigantic scale. Such development is already entered upon. With the completion of the railroads now speeding to their terminals at this point, the State will have an enjoyment of transportation lines greater and more perfect

REPORT OF THE CHIEF OF ENGINEERS, U. S. ARMY.

than any one State has hitherto equipped. She will be prepared to hand almost dispatch the traffic which throngs at her gates.

Great as the outlook for domestic trade may appear, it does not exceed in scope far foreign commerce.

The trade of China alone is conservatively estimated at \$150,000,000 annually. Every year she adds to her demands for timber, flour, and manufactured goods the special field for expansion of trade is in manufactures of iron and steel.

A country without railways, her manifest destiny is to develop on a scale of enormous consumption of iron and steel, these modern means of improvement. Asiatic Russia has already entered upon this work in her possession of China.

Australia is opening wide doors to American trade, and cargoes to her ports fly yearly in increasing ratio.

Japan and the coast countries are adding steadily to the volume of our business with our people. In South American countries, several of them possess immense timberless resources, the lumber of Puget Sound finds a profit growing market. This trade is apparently only in its incipiency, and is but mental continuity.

Machinery, domestic products, manufactures of iron and wood, every product our workshops, whether made on the Pacific or coming by rail from eastern States, are now to be added to the list of articles for trade with the South. America, a people rich in resources but possessing meager manufactures, cargoes of sugar, coffee, spices, precious woods, dyestuffs, gums, tropical fruits will seek these waters for their distribution by the railways to ports.

The Nicaragua Canal, a necessity of the times and certain now of speedy completion, will open the door to a vast tide of commerce from the Atlantic seaboard ports both of the United States and South America. It will reduce by one distance to European ports and bring a mighty current of traffic from that

They from Asia, the greater island of the southern ocean, the rich East Indies the immense stretch of the Atlantic seaboard, the fruitful lands of western America a rising volume of commercial business is sweeping this way and must be and cared for in Puget Sound or else surrendered to our active and determined competitors to the north of us.

It is not too much to assert that a commercial expansion equal to that now going at the port of New York is impending for these waters. A far greater accommodation of the domestic trade will be essential for the proper accommodation of the domestic trade. The present situation is such that nearly the entire shipping of the coast is concentrated at a few ports embracing Puget Sound, Seattle, Tacoma, and the mouth of the Columbia River, whereas many good harbors are scattered all over the coast north of New York.

It is not too much to say that the water to Puget Sound than to England, and the water to the West Indies makes the halfway point. Japan and all the coast countries of the East are nearer to us than to London. Our commerce with the coast countries of the East of China. Statistics show that New York is nearer to Canton than is Liverpool, and that the distance from New York to Canton is less than from Liverpool to Canton.

Statistics also show that the distance to Asiatic points over the Pacific is less than the distance over the Atlantic. It is called from Prof. Ruffer's interesting work on the subject, and is a significant data upon the nearness of Puget Sound to the coast of the continent.

Seattle to—	
Mouth of Amoor River
Vladivostok
Shanghai
Canton
Singapore
Southwest point of Australia
Calcutta
San Francisco to—	
Mouth of Amoor River
Vladivostok
Shanghai
Canton
Singapore
Southwest point of Australia
Calcutta
Liverpool to—	
Mouth of Amoor River
Vladivostok
Shanghai

	Miles.
London to—	
Canton	10,900
Southwest point of Australia.....	10,750
Singapore.....	9,300
Calcutta.....	8,700
New York to—	
Canton via Puget Sound	9,500
Shanghai via Puget Sound.....	8,000

It is well known that the carrying trade of the world is being transferred by land stages from wooden bottoms to hulls of iron and steel. So fast is the transfer being made that wooden-ship yards are constantly going out of commission. Vessels of war are no longer thought of as made of wood. Iron and steel rule the shipping of all seas.

It is also well known that iron and steel bottoms are subjected in salt water to marine growths which adhere to them and greatly impair the efficiency of the vessel. The speed of an iron ship is often reduced one-half by the presence of grass and barnacles adhering to the bottom, and the cost of maintaining such vessels is greatly increased by the necessity of frequent docking to remove these accretions. Fresh water destroys marine growths and prevents their formation. Barnacles and grass, which cover a vessel's bottom in salt water to the weight of many tons, are killed by a few days in fresh water and will drop off, or may be removed easily with scrapers. An advantage to iron shipping is an accessible body of fresh water that masters of vessels regard it as of importance next to cheap fuel at ports of arrival from sea voyages. One of the material advantages named in favor of building the Nicaragua canal is the fact that a fresh-water lake lies midway of the canal route, and that ships will pass through it in their progress from ocean to ocean, and may avail themselves of its waters for freeing their bottoms from marine accretions. Puget Sound possesses cheap fuel for coaling vessels; it has abundant cargoes; a canal to Lake Washington will add immensely to its present advantages.

The most ample and perfect facilities are of paramount importance to the great transcontinental railway lines which converge here. If these railways are important and necessary from a military point of view, if they tend to bind together and unify the distant sections of the Republic, and hence possess national importance beyond their value as transportation lines, then certainly they should have complete terminals to enable them the more efficiently to aid in the work of developing and building up the material resources of the country. In this connection the fact is not to be lost sight of that our neighbor, the Dominion of Canada, spares no expense and loses no opportunity to make perfect her transcontinental railway facilities, and that she is preparing to contest with us most strenuously for the ocean carrying trade.

With a ship canal joining the waters of Lake Washington with Puget Sound our advantages for shipping would be ideal in character and superior to those of all other ports on the Pacific coast. Indeed this unique advantage would make Seattle preëminent in facilities for shipping amongst the ports of the world. A vessel could come to her dock in fresh water undisturbed by tidal flow, and while discharging cargo her bottom would be cleared of its burden of barnacles and grass without the delay and expense of docking. She would thus save two or three days' time as well as a considerable expense. The demurrage alone thus avoided would aggregate for the tonnage entering the port a large sum each year.

But a desideratum of the most practical importance is the increase of shore line available for wharves and docks which this improvement involves. Lake Washington would add more than 50 miles to the dock front of Seattle, increasing her harbor capacity tenfold and giving here transcendent advantages in this regard.

The maintenance of wharves and docks in the salt water of the sound is now very expensive by reason of the ravages of the teredo, which often destroys piling within a single year after driving, so that the wharf goes down with loss of its entire stores, and sometimes with loss of life. In the fresh water of the lake, piling will last practically without limit. The saving in the cost of maintaining docks and wharves alone would far more than repay the cost of constructing the canal.

Manufactures already seek Lake Washington; its fresh water for making steam, its nearness to the coal and iron mines, its cheap wharf facilities, and the ample room which it affords, counterbalancing the expense of rehandling the manufactured product when shipped foreign. Three railway lines, the Northern Pacific, the Great Northern, and the Seattle, Lake Shore and Eastern, have already established themselves on the lake and its outlet, recognizing its great commercial importance. The Northern Pacific Company has built a road along the whole eastern front of the lake. But notwithstanding the railway facilities, many industrial enterprises are precluded from locating here while the canal is not built, such as flouring mills,

grain elevators and warehouses, lumber mills, etc. largely by water.

Every foot of the shore line of this lake would be commodated in shipping and manufactures, were

The timber of the coast and bordering upon the nearly level up, so that the vast forests of the interior would be cleared. Twenty million acres of timber. An immense timber product will come to La Crosse and other towns, when the proposed canal

When timbermen, shiners, and flouring mills, the immense yield of grain which the wheat fields around La Crosse are producing every year, await only time to be sent to the South is completed.

A comprehensive plan for the reduction of iron ore is now in construction at Kirkland, a massive and rich in Lake Washington. This black furnace the heavy iron mines situated about 20 miles east to another place, connects favorably with the railroad running to the west upon the continent. This ore will also bring superior making coal from the mountains of Kirkland. This will be culled and loaded at the lake.

Working with the use of the precious metals which will be abundant, the iron coming by

A great city placed at the focal point where the waterway will begin an infinite number and with the wonderful progress of the age will in it be a great town in the country. The unmistakable a great manufacturing and industrial center.

Congress is securing the Navy of the United States. The position of Puget Sound demands here that the best possible be already, to be followed

A ship canal into Lake Washington would add great advantages for naval vessels of any in the United States for entering the Pacific railways and there is a still stronger military reason for having and securing it from the control of private persons. It would be the link connecting the military and naval strength available for naval protection.

With commercial development would be greatly increased, associated by reason of the great shipping and sailing, the handling of cargoes, the transit of not one cent of all the various charges, and others, would be materially lessened, as a rule and all advantages provided for the city of Seattle.

Not only the commercial reasons, but for reasons of public safety, the Government should provide, control, and be of great national importance.

The end of the work is but a bagatelle in view of the possible extent of attainment.

ASSISTANT CHIEF OF ENGINEERS, SECRETARY OF THE NAVY, WASHINGTON.

THE
S.

DEAR SIR: Since reading and forwarding to you a copy of the Chamber bearing upon the Lake I was struck by the fact that some points of importance in character of locks, etc., were not mentioned. I then point attention to a few points that present themselves probably amount of coal tonnage. At the present time there is no canal in operation; one develop

are in process of development. The names and daily capacity of the mines in question, also the prospective capacity of each in the event of proper shipping facilities being afforded, are below given:

Name	Present daily capacity.	Prospective daily capacity.
.....	600	2,000
.....	1,000	2,000
.....	200	500
.....	1,000	2,000
.....	500	1,000
Total.....	3,300	7,500

The next feature that occurs is the amount of lumber that would be shipped from Lake Washington were facilities afforded.

There has been during the past year cut and floated on Lake Washington, for the use of mills located on Lakes Union and Washington, about 9,000,000 feet of logs, and there is being handled at the present time about 300,000 per working day. This would be but a small portion, comparatively speaking, of the amount that would be handled were there direct shipping facilities from the lake. It is estimated that there are 16,000,000,000 feet of timber now standing in King County. Over one-half of this, and an equal amount in Snohomish County, is directly tributary to Lake Washington. Western Washington is destined to be a heavy producer of metals, both base and precious, and while it would be impossible to estimate the extent of shipments of the various classes it is but proper that attention should be called to the fact that extensive iron and steel works are now under construction; that a company has recently been formed for the purpose of smelting and refining precious metals, the extent and value of which are each day becoming more apparent.

The size and character of vessels which would frequent these waters in case the canal was constructed would also be a matter of importance to you in determining the size of locks and depth and width of basins. From the best information I am able to gather from vessel owners, it is apparent that there is a decided tendency to increase the tonnage capacity. Bearing in mind that the freight would consist chiefly of coal, coke, grain, lumber, furniture, machinery, iron products and other metals, the canal and locks should be capable of handling vessels of large capacity. Vessel owners here are of the opinion that the prevailing type will be of about the following average: 2,000 tons register, 350 feet in length, drawing 26½ feet water, 43 feet beam. Grain merchants seem to think that vessels similar to the *John R. Kelly*, which has net tonnage of 2254, and length of 256.9 feet, breadth of 45 feet and depth 27.8 feet, will represent the style of grain carrier. The fact that the new steel barges, which are now in use to a limited extent on the Great Lakes, will also soon be plying these waters, should also be borne in mind. No estimate has been made of the probable amount of grain shipments, for the reason that our experience in this line has been for so limited a period (the first elevator shipments from this port being made in August, last year) that we can not with any degree of accuracy estimate it. The elevator, which was completed and commenced business last August, has handled under adverse circumstances about 1,000,000 bushels during the past ten months, all received from one line of road. With the completion of the great Northern and Union Pacific and the extension of other roads now in prospect, which will pass through large areas of grain-producing land, the amount of tonnage that can and will be handled under favorable conditions is beyond estimate. Bulletins issued by the Agricultural Department show that Washington outranks all States of the Union in the yield per acre of wheat, oats, barley, and potatoes. The yield of wheat in the State for the past year has been estimated at 20,000,000 bushels. Bearing in mind the immense area of as yet uncultivated lands adapted to the production of this and kindred grains, the future of that branch of business can not be estimated.

Yours respectfully,

J. W. DODGE, *Secretary.*

Col. G. H. MENDRELL.

REASONS WHY CONGRESS SHOULD CONSTRUCT A SHIP CANAL INTO LAKES UNION AND WASHINGTON, IN AID OF COMMERCE.

The advantages of a fresh-water harbor are so well known and have been so voluminously set forth by engineer officers of the Government at various times heretofore, that it is hardly necessary to give any further reasons why a fresh-water harbor should be preferred to salt water. If, therefore, it can be shown that the commerce

of the State of Washington is so extensive that the salt wharves, in deterioration of ships and vessels of all kinds, are reduced, or reduced to such an extent that the saving is not only to ship owners, but in lessening the cost of transportation upon the handling of the grain and other products hardly necessary to say that Congress will be justified in legislation to build a canal into these waters. This would save would be sufficient to pay a large interest upon the canal. It is a well-known fact that the ravages of the tides in Puget Sound, and, in fact, in all the waters of the Pacific Ocean, are a tremendous tax upon commerce. All the wharves in the Sound are on an average of less than two years. It is safe to say that a canal could be constructed in Lake Washington for the accommodation of the State at one-fourth the first cost that would be required for such wharves in the waters of Puget Sound or the Pacific Ocean, and warehouses would last at least ten times as long, and in the fresh waters of the lake would be an infinitesimal part of what they are in the salt water.

Again, the cost of handling ships at the wharves and the cost of shipping would be but a small fraction of what it is upon the coast, because of the level in the lakes being the same at all points, and once in the lake they would be absolutely safe from any danger.

If a canal were constructed into Lake Washington it would carry nearly all of the wheat and coal shipments of the State. This would be so because of the very small cost of handling the products above stated, and because it is easier for all of the waters of the lake than it is to reach the waters of the Sound.

As a matter of fact all of the existing railroads in the State have direct contact with the shores of the lake, and coal shipped now from the Sound, mainly from King and Tacoma, in a neighborhood of 2,000,000 tons per annum. These shipments are not unlikely to be more than doubled. The wheat product of the State in 1880 was nearly 30,000,000 bushels and it is safe to say that a canal would carry nearly 100,000,000 bushels. The eastern part of the State, all the wheat lands are cultivated, is capable of producing 100,000,000 bushels per annum, or say one-fourth of the present entire wheat product of the State. The saving in cost to the farmer and the merchant would be very large.

In addition to the coal and wheat products the amount of other products from the lake would be very large.

There are also within 30 miles of the lake some of the best iron mines in the world. Already transportation has been nearly provided for these deposits, and extensive works for the reduction of iron ore being built upon the shores of the lake, and it is expected that the mines will be in operation within one year from this time.

Summing up, it may be said that there is not a coal mine in the State of Washington that can not more readily ship its product into the Sound. Neither is there any iron deposit that can not be more readily shipped from the lake. As a site for very extensive manufactures that will be required on the coast, the point that begins to possess the advantages of Lake Washington is by superior natural location but because of the inexhaustible supply of iron. If the construction of a canal were begun at once, it could be the development of iron mining and manufacturing and wheat product, would more than justify the small expenditure.

THE CHAMBER OF COMMERCE
By E. O. GRAVES, Vice-President
J. W. DODGE, Secretary

COMMUNICATION OF MR. C. J. SMITH, GENERAL MANAGER

PORTLAND,

DEAR SIR: In relation to the business which this company is transacting upon the shores of Lake Washington in case a canal is built into the waters of Puget Sound to Lake Washington, I would say that this company is the owner of the Columbia and Puget Sound gauge road extending south and east of Seattle to the New

miles, and from the junction at Renton to Franklin Mine, a distance of 34 miles at Seattle.

There is located upon this road, coal mines belonging to this company, known as Newcastle Mine and Franklin Mine. In addition to these there is located upon the line of this railroad the Black Diamond and Cedar Mountain Mine. The entire output of these mines would find a shipping point upon Lake Washington in case a canal was cut through. The output of the mines is as follows:

	Tons.
1888, Newcastle Mine.....	84,000
Franklin Mine.....	177,000
1889, Newcastle Mine.....	67,000
Franklin Mine.....	90,000
1890, Newcastle Mine.....	130,000
Franklin Mine.....	74,000

Various causes have contributed to keeping the output of these mines very considerably below their average capacity. In 1888 there was a fire in both the Newcastle and Franklin mines, and in 1889 and 1890 the Newcastle Mine, owing to a keel, was shut down two months and the Franklin Mine four months. The estimated output of these mines, barring strikes, fires, or accidents, would be as follows:

	Tons.
Newcastle Mine.....	200,000
Franklin Mine.....	150,000
Black Diamond Mine.....	200,000
Cedar Mountain Mine.....	60,000
Total.....	610,000

All the output of these mines is transported over the railroad of this company and shipped from the coal bunkers of this company at Seattle, with the exception of the amount used for domestic purposes at Seattle. The amount of coal so used at Seattle would be about 75,000 tons per annum, leaving 535,000 tons for shipment. Owing to the difficulty with the teredo at Seattle, it is necessary to build these bunkers on copper-covered piles, and the shipments of the above mines are now made from these bunkers which cost this company \$168,000. Bunkers sufficient to enable this company to handle this product from fresh water at Lake Washington could be built for \$50,000, and could be maintained much more cheaply than the bunkers at Seattle.

The class of vessels transporting this coal are steamers ranging from 1,000 to 2,500 tons capacity, with an average draft, loaded, of 19 feet. In addition to the coal shipped in steamers, it has been the custom of both this company and the Black Diamond Coal Company to charter sailing vessels ranging in capacity from 1,000 to 2,000 tons, with draft, loaded, of about 21 feet. This company owns or controls a fleet on the Pacific coast of about twenty steamers, ranging in capacity from 1,000 to 2,500 tons, and are now engaged in the coastwise business, with routes from Portland to Puget Sound points, and from Puget Sound points to Alaska. They have a capture of vessels averaging every four days, and in addition to the coal carried, a very large merchandise and passenger business. It is probable that a considerable portion of this business would be done at the wharves on Lake Washington should this canal be cut through.

The Newcastle Mine of this company is located only a mile from the shores of Lake Washington, the Cedar Mountain Mine about 8 miles, the Black Diamond Mine about 18 miles, and the Franklin Mine about 18 miles. There would be a saving in distance in the carriage of coal to Lake Washington from Newcastle Mine of nearly 20 miles; from Cedar Mountain Mine of about 10 miles; from Black Diamond Mine of about 18 miles, and from Franklin Mine of about 14 over the carriage of coal to Seattle.

In addition to the mines already opened and developed, there are a number of prospects in this same region that would be opened and developed to a working output if the cost of transportation and wharfage charges were cheapened to the extent expected by the opening of service on the shores of Lake Washington.

Trusting the above information may be of use and benefit to you, I am

Yours, truly,

C. J. SMITH,
General Manager.

THOMAS W. SYMONS,
Captain, U. S. Corps of Engineers.

T T 16.

ESTABLISHMENT OF HARBOR LINES IN OLYMPIA HARBOR, VANCOUVER HARBOR (COLUMBIA RIVER AT VANCOUVER), AND BELLINGHAM (AT NEW WHATCOM AND FAIRHAVEN), WASHINGTON.

WASHINGTON, D. C., *March 1, 18*

SIR: On behalf of the harbor line commission of the State of Washington I have the honor very respectfully to submit herewith projects or preliminary plans for the improvement of the harbors of Olympia, Vancouver, and Bellingham Bay, in that State, and to seek your approval of the same.

Before any further explanation is made of these plans—here presented upon the accompanying maps*—and in order that the attention of the public may be directed to the fact that the preference which has heretofore been taken by the State of Washington with reference to its harbors and navigable waters may be more fully understood, I beg leave to present the following statement in relation thereto. The magnificence and unsurpassed advantages of these harbors have long been recognized by the entire civilized world. For beauty of scenery, variety, for safety, extent, convenience of access, and unlimited capacity for usefulness in the interests of commerce and navigation, they are not only far superior to any other harbors or waters of the kind on the Pacific coast between the Arctic Circle and Cape Horn, but they are not surpassed on the surface of the globe. They are surrounded by a country rich in every natural production required for the building up of great centers of wealth and population, and more particularly those things which contribute to the growth of commerce, manufactures, and the industrial arts. A description of its resources of timber, coal, iron, mineral, agricultural, and other products would fill a volume, while its climate is of that temperate and salubrious character which conduces in the highest degree to the physical development and the industrial activity of the human family. Every transcontinental railroad across the North American continent has already secured or is endeavoring to obtain terminal facilities on the shores of these harbors. The Canadian Pacific, the Great Northern, the Northern Pacific, and the Union Pacific Railroad companies all have terminal facilities of greater or less extent already in their possession, while every railroad company which is extending its track in the direction of the Pacific coast is hoping sooner or later to reach the waters of Puget Sound. The extraordinary growth of the commerce of Washington Territory is well known to require at this time anything more than passing reference. The figures are to be found in the commercial statistics of the country. The people of the State of Washington, upon their admission into the Union, realized these facts and determined to utilize to the greatest possible advantage the grand commercial facilities with which nature had provided them. For this purpose they adopted a line of policy which would enable them not only to preserve, maintain, and improve these commercial facilities, but also determined to retain the possession and control of the water front in its numerous harbors, in order that they might regulate tolls and charges of all descriptions thereon, they might make these charges reliably and permanently cheap, and they might keep these commercial facilities convenient and accessible to the public for all time to come. Therefore when their constitution was framed and adopted it contained among others the following provis-

* Not printed.

ARTICLE XV.—HARBORS AND TIDE WATERS.

SECTION 1. The legislature shall provide for the appointment of a commission whose duty it shall be to locate and establish harbor lines in the navigable waters of all harbors, estuaries, bays, and inlets of this State, wherever such navigable waters lie within or in front of the corporate limits of any city or within one mile thereof upon either side. The State shall never give, sell, or lease to any private person, corporation, or association, any rights whatever in the waters beyond such harbor lines, nor shall any of the area lying between any harbor line and the line of ordinary high tide, and within not less than 50 feet nor more than 600 feet of such harbor line (as the commissioners shall determine), be sold or granted by the State, its right to control the same relinquished, but such area shall be forever reserved for landings, wharves, streets, and other conveniences of navigation and commerce.

c. 2. The legislature shall provide general laws for the leasing of the right to build and maintain wharves, docks, and other structures upon the areas mentioned in section one of this article, but no lease shall be made for any term longer than ten years, or the legislature may provide by general laws for the building and maintaining upon such areas, wharves, docks, and other structures.

c. 3. Municipal corporations shall have the right to extend their streets over adjoining tide lands to and across the area reserved as herein provided.

In order that these provisions of the constitution might be carried into effect, and in further pursuance of the same line of policy, the following laws were subsequently enacted by the State legislature:

ARTICLE XVII (OF THE CONSTITUTION).—TIDE LANDS.

SECTION 1. The State of Washington asserts its ownership to the beds and shores of all navigable waters in the State up to and including the line of ordinary high tide in waters where the tide ebbs and flows, and up to and including the line of ordinary high water within the banks of all navigable rivers and lakes: *Provided*, that this section shall not be construed so as to debar any person from asserting his claim to a vested right in the courts of the State.

c. 2. The State of Washington disclaims all title in and claim to all tide, swamp, and overflowed lands patented by the United States: *Provided*, The same is not intended for fraud.

HARBOR LINE COMMISSIONERS, STATE OF WASHINGTON.

[Laws of 1889-1890, page 239.]

ACT to create a board of harbor line commissioners, prescribing their duties and compensation.

As enacted by the legislature of the State of Washington: **SECTION 1.** There is hereby created a board of harbor line commissioners, to consist of five disinterested persons to be appointed by the governor.

c. 2. The said commissioners shall hold office until the 15th day of January, next. Should a vacancy or vacancies occur in said board, by resignation or otherwise, the same shall be filled by appointment by the governor.

c. 3. The duties of the said harbor line commissioners shall be to locate and establish harbor lines in the navigable waters of all harbors, estuaries, bays, and inlets of this State, wherever such navigable waters lie within or in front of the corporate limits of any city or within one mile thereof upon either side, and to perform all other duties provided and prescribed in article 15 of the constitution of the State of Washington, and all such other duties as the law may prescribe, and wherever and whenever said board of harbor line commissioners shall have established the lines herein provided, in any of the navigable waters of the harbors, estuaries, bays, and inlets of this State, they shall file the plat thereof in the office of the secretary of State, and a duplicate thereof in the office of the clerk of the city or town where the harbor lines shall have been located, and from and after the filing of said plat, the harbor lines established as herein designated and displayed shall be, and the same are intended to be, the harbor line of that portion of the navigable waters of this State.

c. 4. The board of harbor line commissioners hereby created shall begin operations as soon as may be practicable, and are hereby authorized to employ a clerk, a competent surveyor, and such assistance as may be necessary, and to purchase such material and supplies as may be necessary to carry out the full intent and purpose

of this act, at such rates of compensation as into effect the provisions of this act the sum thereof as may be necessary, is hereby appropriated from the State treasury not otherwise appropriated.

SEC. 5. Each of the board of harbor line commission is hereby authorized to employ actual traveling expenses.

SEC. 6. The board of harbor line commission in the discharge of the duties provided for in this act shall be allowed the same as is provided for the proper vouchers so allowed the State treasurer on the State treasury for the same. The warrant on the State treasury for the same is hereby authorized to pay said wages. *Provided*, That no appropriation shall be made for this purpose: *Provided*, That the payment of which no appropriation shall have been made.

SEC. 7. The board of harbor line commission is hereby authorized to pay for all expenses incurred, and report the same to the Governor.

Approved March 28, 1890.

PUBLIC WAYS ACROSS TIDE

[Laws, 1889-1890,

AN ACT to establish and define public ways for water and for a mile either way from, all incorporated cities and towns.

Be it enacted by the legislature of the State, that there shall be established one or more public ways across the tide flats within or in front of any incorporated city or town within the State from any incorporated city or town within the State.

SEC. 2. The public ways provided for in this act shall be not more than fifty nor more than one thousand feet from the deep-water end, in not less than twenty feet inland across the State's tide lands.

SEC. 3. The public ways above provided for shall be located near as is practicable, within their bounds, and the tide flats in which they are located, and shall be necessary for the present or future convenience of the public.

SEC. 4. All public ways established under this act shall be correctly surveyed and of sufficient length so that they will pass over a post not less than four feet, and such posts shall not be less than eight inches in diameter. Such public ways shall be correctly surveyed with the Government surveys, or such other surveys as may be made, and a correct plat of all such public ways shall be filed with the commissioner of public lands of the State, one copy of which shall be filed with the secretary of the board of harbor line commission, and such plats shall be filed as city or town records.

SEC. 5. All the public ways that may be established under this act are, and shall forever be, reserved from private claims.

SEC. 6. Where the words "tide flat or tide land" are used in this act, they shall be construed to mean all lands over which the tide runs at low tide; and where the words "water" are used, they shall be construed to mean boats, barges, and other watercraft, and over of water.

SEC. 7. The board of harbor line commission is authorized to carry out the provisions of this act, and to employ such assistance and power as may be necessary to carry out the full intent and purpose of this act, and the amount of such assistance shall be such reasonable amount as shall be determined by the board.

SEC. 8. All bills incurred in carrying out this act shall be paid in the same manner as is provided for in the act, and the payment of bills incurred by the board of harbor line commission, for the payment of bills incurred by the board.

Approved March 28, 1890.

IMPROVEMENT OF HARBORS AND WATERWAYS.

[Laws 1891, page 405.]

ACT relating to the improvement of harbors and waterways in the State of Washington, and providing funds therefor.

As it enacted by the legislature of the State of Washington: SECTION 1. That seventy-five (75) per cent of the proceeds derived by the State from the sale of tide lands within the limits of any incorporated city or town in the State of Washington, or within one mile on either side thereof, shall be, and the same is hereby, appropriated, and the same shall be set apart as a special fund by the State treasurer for the construction and maintenance of a system of permanent and substantial improvements for aid of commerce and navigation in and for the harbor of such city or town wherein such tide lands may be sold, and the remaining twenty-five (25) per cent shall be paid into the general tide-land fund of the State.

SEC. 2. Wherever harbor lines are now, or may hereafter be, located and established within or in front of any incorporated city or town in the State of Washington, the harbor line commission is hereby authorized and directed to approve and adopt, subject to the approval of the Secretary of War, or an advisory board created by the Secretary of War, as soon as may be practicable, such a system of harbor improvements, within the harbor line strip of each harbor wherein such lines may be located and established, as will conform as nearly as practicable to any permanent improvements constructed or under construction therein as in their judgment, and upon the available advice of competent engineers, shall be best calculated to promote the permanent commercial welfare thereof; and all funds appropriated for the purpose of harbor improvements under the foregoing section of this act, or otherwise, shall be expended under the direction, supervision, and control of the harbor line commission of the State of Washington, and the State auditor is hereby directed, upon the filing of vouchers approved by the harbor line commission, to draw warrants on the State treasurer, payable out of the several harbor-improvement funds, and the State treasurer shall pay all said warrants out of said funds.

SEC. 3. In keeping the account of this fund the State treasurer is directed to credit each harbor with the amount received for sale of tide lands, as provided in section one of this act, and debit each harbor for warrants drawn for improvements therein.

SEC. 4. All acts or parts of acts inconsistent with the provisions of this act are hereby repealed.

Approved March 10, 1891.

The declaration in the constitution that the State of Washington asserts its ownership to the "beds and shores of all navigable waters in the State up to and including the line of ordinary high tide in waters where the tide ebbs and flows, and up to and including the line of ordinary high water within the banks of all navigable rivers and lakes," is simply the statement of a fact, and in no wise adds to the actual validity of the title by which the State owns this property. That the State of Washington, by virtue of its sovereignty, and not by virtue of any act of the General Government, is the absolute owner of these tide lands and lands under the navigable waters of the State, is a principle of law now so thoroughly established by repeated decisions of the Supreme Court of the United States, that it is no longer questioned by any lawyer or layman, court or judge, who has investigated the subject. As the original thirteen colonies came into the Union the proprietors of their tide lands by virtue of the sovereignty which they had inherited from the British Crown, and which they never relinquished, and as all the States in the Union which were subsequently admitted, were admitted upon precisely the same footing as the original thirteen States, so all of the States of the Union, Washington included, are the absolute owners of the tide lands within their limits, and can make any disposition of them which may be deemed advisable or expedient, subject only to the jurisdiction of the United States over the navigable waters of these States in the interests of commerce. Each State has a perfect right to decide for itself what disposition it will make of these lands, and the

may be sold and made available for business purposes. The growth of most of the larger cities in Washington is now hampered and retarded because of the delay which has unavoidably been incurred in the location of these harbor lines. Beyond question the State of Washington has a perfect and absolute right to locate these harbor lines and waterways and to construct within these lines such improvements as may be deemed advisable in the interests of commerce and navigation. The object of the location of these lines is to mark out such areas to be reserved from sale as shall be considered necessary for streets, landings, wharves, and other conveniences of commerce and navigation. The areas to be reserved within these harbor lines for these purposes are from 50 to 600 feet in width, in the discretion of the harbor-line commission. They are now and are to be forever reserved as the property of the State and retained under its control, to the end that the commercial facilities thereon provided shall be made cheap and free of access to the general public, and may not fall into the hands of private persons or corporations whose exactions might prove a heavy burden upon the commerce of the State and nation.

The State has already appropriated 75 per cent of the proceeds of the sale of tide lands within its harbors for their improvement, and will probably expend many millions of dollars in the construction of sea walls, bulkhead lines, wharves, streets, and other conveniences of commerce upon the water front of its harbors within the near future. The jurisdiction of the United States over the navigable waters of the State of Washington is for the promotion of the commercial interests of the country. The legislation of that State is actively engaged in the same direction and for the same purpose. Whilst the commerce of that State is rapidly assuming large proportions, and is not only of a national but of an international character, yet the State is more deeply interested, more vitally concerned, in the increase, in the encouragement, and in the good management of that commerce than anyone else can be, because its interest is direct, and every other interest is of an indirect or secondary character. Therefore it seeks and desires to act in harmony with the General Government in all plans and measures which have for their object the care, protection, and improvement of these great and growing commercial interests. It desires to cooperate with all agencies, whether local and municipal or State and national, in the work of securing, constructing, and maintaining a comprehensive system of harbor improvements commensurate with the enormous national and international commerce which will in the years to come be carried on in the ports of Washington.

In addition to the work of locating harbor lines and waterways, with which the Harbor Line Commission of the State of Washington is charged, we are also required by law to prepare and adopt plans for the permanent and substantial improvement of these harbors, subject to the approval of the Secretary of War. We are well aware of the fact that no action of the legislature of the State of Washington would be binding upon the Secretary of War, yet we have thought it not unreasonable to suppose that men who are charged with the care of great commercial interests, whether representing the General Government or the State of Washington, might agree upon some general plan or plans for harbor improvement, in accordance with which the sums appropriated or expended by each should be disbursed, either jointly or severally, but in such a manner that the greatest amount of benefit might be derived therefrom in carrying out the plans which might be found acceptable to both the State and the General Government. We hope

a slough or channel at the head of Hayden Island, to which we have added a project for dredging a channel 400 feet in width on the north side of the Columbia River, from the harbor of Vancouver to the deep water near the mouth of the Willamette River. It was the opinion of Maj. Handbury that the closing of the channel at the head of Hayden Island, thereby increasing the force and action of the current below the harbor of Vancouver, would open a channel through the bar which has formed below that harbor, but we are anxious to open a channel across this bar as soon as practicable, without waiting for the slow process of the river currents. With the exception of this channel the plan for which we ask your approval is identically that suggested by Maj. Handbury after a thorough and careful survey of the Columbia River in front of the city of Vancouver, and between that place and the mouth of the Willamette River, from which point to the ocean there is usually an ample supply of water for at least ships of medium size or capacity.

(3) For Bellingham Bay. This project consists simply of the location of harbor lines, including bulkhead and pierhead lines, which have been established after a careful survey and with special reference to the interests to the navigation in front of the city of New Whatcom. In connection therewith we desire your approval for the restoration of the waters of the Nooksack River to their original channel through the Lummi Indian Reservation, by which they formerly flowed into the Gulf of Georgia. A collection of driftwood and debris of all kinds at the head of the old channel, as exhibited on the accompanying map, caused a deflection of the channel into Bellingham Bay, and enormous masses of sediment, sand, and alluvial deposit are being carried into that bay, and, unless diverted elsewhere, will before many years entirely ruin the navigation of the harbor of New Whatcom, which includes the upper part of Bellingham Bay. The people of that locality are willing to do this work at their own expense and return the river to its old channel if you will approve of the project.

The people of Olympia and Vancouver are also anxious, at their own expense and without waiting for the action of Congress, to begin the work of improving their harbors, as outlined in this letter, if the plans herein suggested shall meet with your approval. The rapidly growing needs of commerce in these localities make it important that the more urgent needs of better commercial facilities shall be provided at the earliest practicable moment.

Hoping that the importance of the subject may be a sufficient apology for the length of this communication, and that you may favor us with an early, and if possible with a favorable, consideration of the requests therein made,

I have the honor to be, very respectfully,

WM. F. PROSSER,

*Chairman, Harbor Line Commission,
State of Washington.*

Gen. STEPHEN B. ELKINS,
Secretary of War.

[Second indorsement.]

OFFICE CHIEF OF ENGINEERS,

U. S. ARMY,

March 5, 1892.

Respectfully returned to the Secretary of War.

ENG 92—176

... of the harbor lines...
... of the act of September 19, 1916...
... War authority to cause the...
... made... that such...
... and... of...
... recommended that a Board...
... of Engineers, be constituted...
... harbor lines for the harbor...
... Bay, and that the maps and...
... be referred to such...
... of the Secretary the...
... from this office.

**The
Brigadier-General**

[Third movement]

WAR DEPARTMENT

Approved as recommended by the Chief of
Bureau of the Secretary of War.

[Fourth movement]

OFFICE CHIEF

Respectfully referred to Col. G. H. Mendonça,
Commandant of the Board of Engineers,
United States Army, Corps of Engineers,
Washington, D. C., for his consideration.

[Fifth indorsement.]

U. S. ENGINEER OFFICE,
San Francisco, Cal., May 23, 1892.

ectfully returned to the Chief of Engineers.
report of the Board is found in three tracings* containing both
l and graphic description of harbor lines recommended by the
for the respective harbors of Vancouver, Olympia, and Belling-
ay, sent in a separate package.

G. H. MENDALL,
Colonel, Corps of Engineers, Senior Member.

[Sixth indorsement.]

OFFICE CHIEF OF ENGINEERS,
U. S. ARMY,
May 31, 1892.

ectfully returned to the Secretary of War.
Board of Engineers appointed under authority of the Secretary
: (see second and third indorsements hereon), by Special Orders
Headquarters Corps of Engineers, March 12, 1892, to consider
ort upon the subject of harbor lines for the harbors of Olympia,
ver, and Bellingham-Bay, Washington, under the provisions of
12 of the river and harbor act of September 19, 1890, recom-
for the approval of the Secretary of War the harbor lines de-
l and described upon the accompanying charts.
recommended that the line selected be approved, and that the
ry place his approval upon the tracings submitted.

THOS. LINCOLN CASEY,
Brig. Gen., Chief of Engineers.

WAR DEPARTMENT, June 3, 1892.

harbor lines selected by the Board are approved.

S. B. ELKINS,
Secretary of War.

PTIONS OF THE HARBOR LINES (TAKEN FROM THE MAPS
SUBMITTED).

HARBOR LINES IN OLYMPIA HARBOR.

e harbor line: Beginning at a point on the line between townships 18 and 19
nge 2 west, 328.1 feet west from the meander corner to sections 2 and 35,
oint is in latitude $47^{\circ} 5' 00''$.44, and in longitude $122^{\circ} 54' 4''$.669; thence west
t; thence south $14^{\circ} 46'$ west, 2,156.3 feet; thence south $16^{\circ} 56'$ east, 11,250
2,996.9 feet on this line is the northwest and at 3,046.9 feet the southwest
the line bounding the east side waterway); thence south 42° west, 350.2 feet;
orth $16^{\circ} 56'$ west, 5,176.4 feet; thence south $84^{\circ} 6'$ west, 1,401.9 feet (at 725
line just touches the northeast corner of the Main Street Wharf); thence
 $54'$ east, 5,240 feet; thence south $84^{\circ} 6'$ west, 1,000 feet (at 19.8 feet on this
e northeast, and at 119.8 feet the northwest angle of the line bounding the
waterway, also the last 245.2 feet are coincident with the line bounding the
tes waterway); thence north $5^{\circ} 54'$ west, 5,240 feet (the first 188.5 feet of this
coincident with the line bounding the Des Chutes waterway); thence north
, 4,060 feet (the first 52.2 feet of this line are coincident with the line bound-
west side waterway); thence north $30^{\circ} 52'$ west, 4,000 feet; thence north

* Not printed.



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... THE ... AT NEW YORK

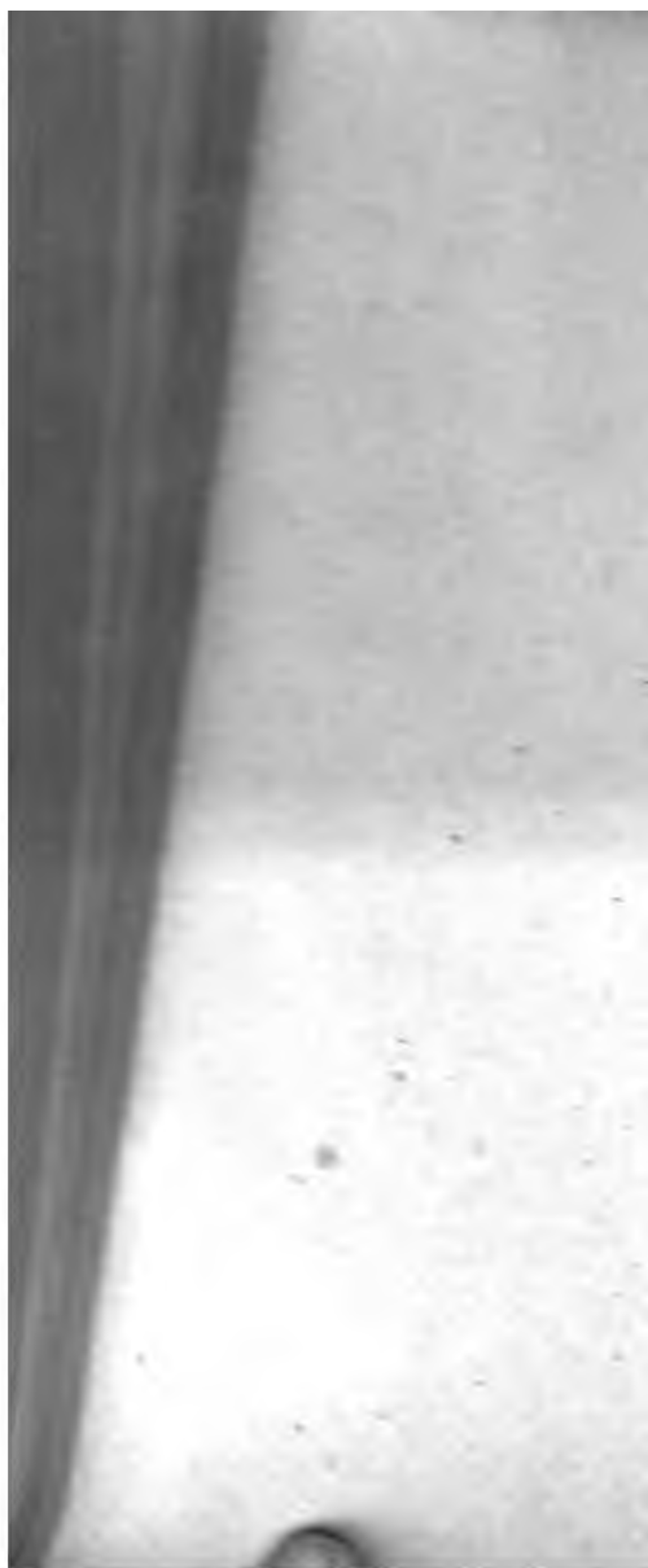
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surveys of the Pattle and Morrison donation claims; thence west 731.72 feet; thence south $34^{\circ} 55'$ west, 748.91 feet; thence south $50^{\circ} 51'$ west, 987.67 feet; thence $h 32^{\circ} 4'$ west, 1,473.81 feet; thence south $6^{\circ} 35'$ west, 847.24 feet; thence south east, 752.33 feet; thence south $15^{\circ} 14'$ west, 404.38 feet; thence south $58^{\circ} 50'$ west, 1 feet; thence south $86^{\circ} 33'$ west, 1,185.33 feet; thence south $36^{\circ} 51'$ west, 3,247.48 feet; thence south $23^{\circ} 7'$ east, 1,726.92 feet; thence south $48^{\circ} 46'$ east, 904.4 feet; thence south $5^{\circ} 32'$ west, 750.54 feet; thence south $13^{\circ} 26'$ east, 1,467.54 feet; thence $h 34^{\circ}$ west, 252.13 feet; thence south $12^{\circ} 20'$ west, 825.63 feet; thence south 43° east, 783.01 feet; thence south $12^{\circ} 23'$ east, 945.9 feet; thence south $26^{\circ} 55'$ east, 1.84 feet; thence south $29^{\circ} 11'$ east, 2,011.67 feet; thence east 702.94 feet; thence $h 2^{\circ} 16'$ east, 751 feet; thence east 31.85 feet; thence south $23^{\circ} 41'$ east, 208.69 feet; thence south $28'$ west, 200.06 feet; thence south $17^{\circ} 26'$ east, 714.01 feet; thence 345.64 feet; thence north $10^{\circ} 21'$ east, 884.11 feet; thence north $4^{\circ} 22'$ west, 3 feet; thence north $31^{\circ} 43'$ west, 850.95 feet; thence north $9^{\circ} 49'$ west, 703.22 feet; thence east 980.95 feet; thence south $41^{\circ} 40'$ east, 1,305.52 feet; thence south $10'$ east, 2,090.76 feet; thence south $12^{\circ} 47'$ east, 2,498.5 feet; thence east 307.62 feet to a point near high-water mark on or about the quarter-section line of sec-
24.



APPENDIX U U.

IMPROVEMENT OF WILLAMETTE RIVER, OREGON, AND OF LOWER COLUMBIA RIVER AND ITS TRIBUTARIES, OREGON AND WASHINGTON.

REPORT OF MAJOR THOMAS H. HANDBURY, CORPS OF ENGINEERS, OFFICE IN CHARGE, FOR THE FISCAL YEAR ENDING JUNE 30, 1892, WITH OTHER DOCUMENTS RELATING TO THE WORKS.

IMPROVEMENTS.

- | | |
|--|---|
| mouth of Columbia River, Oregon and Washington. | 4. Willamette River above Portland, Oregon. |
| Construction of canal at the Cascades, Columbia River, Oregon. | 5. Cowlitz River, Washington. |
| Columbia and Lower Willamette rivers below Portland, Oregon. | 6. Youngs and Klaskanine rivers, Oregon. |
| | 7. Gauging waters of Columbia River, Oregon and Washington. |

EXAMINATIONS AND SURVEYS.

Willamette River, Oregon, at Clackamas Rapids, Ross Island, and Corvallis.
Lower Willamette and Columbia rivers below Portland, Oregon.
Columbia River near Vancouver, Washington.

HARBOR LINES.

Establishment of harbor lines in Willamette River at Portland, Oregon.

UNITED STATES ENGINEER OFFICE,
Portland, Oregon, July 12, 1892.

GENERAL: I have the honor to submit herewith annual report for fiscal year ending June 30, 1892, upon the work of river and harbor improvement in my charge.

* * * * *
Very respectfully, your obedient servant,

THOS. H. HANDBURY,
Major, Corps of Engineers.

Chief of Engineers,
Chief of Engineers, U. S. A.

U U 1.

THE MOUTH OF MOUTH OF COLUMBIA RIVER, OREGON AND WASHINGTON.

The plan under which this work is being carried on was adopted in 1882, the water being provided a channel across the Columbia River of a depth of 30 feet at mean low tide. This is to be effected by constructing the water flowing over the bar and increasing the currents to such a degree as to procure the desired result. Any work for accomplishing this end must be more or less of a permanent character. The work which is now in progress is the construction of a wide jetty, starting from Fort Stevens, on the South Cape, and extending in a westerly direction, with a slight curve to the south, to Cape Disappointment Spit, for a distance of 4½ miles, more or less, as circumstances may require, to a point about 3 miles south of Cape Disappointment. The jetty is constructed of stone, resting upon a mattress foundation about 40 feet wide and from 2½ to 5 feet thick. The stone extends to a level of 4 feet above mean lower low water. The material thus far has been placed in position from a jetty tramway supported upon piles driven along the line of the jetty and about 2½ feet above the level of low tide. The tramway is a double-track 3-foot gauge road, the tracks being 13 feet between centers, and 28 feet from the plane of mean low water. The material is landed at the jetty and transported to places over these tracks, which are built in advance of the main work.

From the commencement of this work the channel or channels over the bar were very capricious in location and variable in depth. The depths were usually from 19 to 21 feet and the channels varied in number and location through nearly 180° from Cape Disappointment to the Mouth of the Mouth.

The channels already constructed are very marked in the location of Cape Disappointment Spit and in the effects produced by the concentration of the water over the bar. There is now a straight out and back channel, 1,000 feet wide, with a depth nowhere less than 27 feet, and a second channel, a depth of 27 feet. At the end of last season the distance from the 30-foot curve on the outside of the bar to the inside of the bar was 1,000 feet. This distance is now 1,200 feet. These depths refer to the plane of the mean lower low water.

Since the commencement of this improvement was made in the river the amount of cargo shipped July 5, 1884. The total amount appropriated for the improvement for the present fiscal year for carrying out the project is \$250,000. The amount expended, including outstanding liabilities, is \$125,000, leaving a balance of \$125,000 applicable to the further improvement of the river.

At the end of the last fiscal year the jetty tramway was at station 235+40. Station 25+8 was the end of the mattress work at station 235+40. Station 25+8 was the beginning of the jetty proper. During the present season the tramway was extended 1,988 feet. Station 250+2 was the end of the jetty. This is 4½ miles from the beginning of the jetty at station 25+8. From this station east to the end of the present season the distance is 1,080 feet, making the distance from the end of the mattress work to the end of the jetty a little over 4.8 miles.

In view of the results obtained by the jetty at the end of last fiscal year, as shown by the sounding taken at that time on the bar, it was

leemed advisable to stop the construction of the jetty when it arrived at a point $4\frac{1}{2}$ miles from its inner end and await further developments, which would be shown by the sounding to be taken during June of this year. To give to the outer end of the jetty extra strength to resist the action of the waves in that locality, the tramway was widened for 300 feet back from the end so far as to permit of a third track being laid 13 feet south of the south tract. This arrangement permitted the laying of a foundation of mattresses 80 feet wide; on this rock was piled to a level of mean high water. The piles used in this locality were 65 feet long; they were driven into the sand from 25 to 30 feet. The action of the waves and tides would scour the sand down from 10 to 15 feet before the mattress could be put in place. When this was done it would fill back again and as rock was put on keep piling up against it. Extra precautions were taken to double driftbolt and strap the caps and stringers to the piles and put in additional braces from the end back for a distance for 6,000 feet. In the distance of 300 feet at the widened portion of the jetty 30,000 tons of rock were dumped.

Since the commencement of the work in 1884 there has been used in the construction of the tramway and its repairs 377,660 lineal feet of piling and 2,223,580 feet B. M. of lumber. The cost of the tramway has been approximately \$6.50 per lineal foot. There has been used 18,414 cords of fascines. The mattress work in place has cost \$4.50 per lineal foot.

Under the contract dated January 22, 1891, in force with Joseph E. Smith, 150,500 tons of rock were received during the year. The total amount of rock received from all sources since the commencement of the work is 478,890 tons.

About 25,000 tons of this rock was used in securing the root of the jetty and in protecting the buildings and railway between that point and the wharf. The balance has been distributed along the line of the jetty. From the end of the jetty back for a distance of 2,500 feet the rock is raised to a level of 4 feet above datum, for 13,000 feet it is at datum, for 5,200 feet it will average 4 feet above, for the remaining distance it will run from this level to high water. Near the inner end of the jetty it was found to be necessary to pile the rock well up toward the high-water line to protect the piling of the tramway from the heavy drift brought down by the river during the winter and spring. At places along the line of the jetty it was observed that there was a decided tendency during the last of the flood tides and the first of the ebb for the water to flow across the jetty in great volume and with considerable velocity. Where this was the case the sand would not deposit in the vicinity, but would be scoured out, increasing the area of the waterway. At these places rock was dumped in until this action ceased. It was found that when the jetty reached the height of about 4 feet above the mean level of low water the flow during both ebb and flood was under control. The sand was deposited to the level of low water and above, in many instances, on both sides of the jetty.

Under the contract entered into with Richard Hoyt, April 20, 1891, 1,768 cords of fascines and 3,528 poles were received. These were used in making the mattresses placed under the last 1,000 feet of the jetty.

The piles used during the year were purchased in open market at the rate of $9\frac{1}{2}$ cents per lineal foot delivered at Astoria. The lumber was purchased in open market also, at \$10 per 1,000 feet delivered at Fort Stevens.

On account of inclement weather and to give opportunity for general repairs that were needed to the tramway, the barges, towboats, and

rolling stock of the railway, the delivery of rock was suspended from the 20th of December to the first of March. It was necessary to make extensive repairs to four of the oldest barges belonging to the plant in order to keep them in anything like serviceable condition. This was done by written agreement with the lowest bidder after soliciting proposals for the work, at a cost of \$1,750 each. The other repairs were made by the regular employes upon the work.

On June 9, 10, and 11 surveys were made upon the bar at the mouth of the Columbia River with the view to developing the present condition of the channel in that locality. The conditions were favorable for this work and the results make a very satisfactory showing. The soundings were reduced to the plane of the mean of lowest low water and are shown upon the sheet herewith. The general outline of the low-water line as it now exists in the vicinity of the jetty is also shown. This survey shows an increase in the depth of the water over the bar to the northward of the deepest water of last year. The 27-foot channel is now about 1 mile wide and the 25-foot channel about 2 miles. This tendency of the body of the water to pass out more to the northward can be attributed, I think, to the southerly storms that prevail during the winter and spring.

What was the old south channel when the work commenced is generally shoaling up on a line with the end of the jetty. In this vicinity where there was at that time from 19 to 30 and 35 feet, there is now about 17 feet. To the southward this channel is holding at the same depth as formerly. The north channel of that time is closing up, the middle sands, with but 10 feet of water upon them, have entirely disappeared, and where this minimum depth was there is now 34 feet. These sands have been pushed out into the deeper water of the ocean and now there is from 26 to 29 feet of water where they were. No matter of course shoaling has taken place in this deeper water in the case of the fore slope of the old bar, where this was deposited, but not to such an extent as to interfere with navigation. The indications from this survey are all favorable to a permanent depth of channel of at least 30 feet. The shortest distance between the curves of this bar on the two sides of the bar is now but 1,200 feet.

This improvement is already having a very marked effect upon the foreign commerce of the river. The commercial statistics herewith indicate an increase in the number and tonnage of these vessels. The indications for the coming season are that there will be a still greater increase.

Estimates for the year ending June 30, 1894.—It is estimated that \$525,000 will be required to finish this work. Should \$350,000 of this be appropriated for the fiscal year ending June 30, 1893, it is recommended that the balance, \$175,000, be made available for the fiscal year ending June 30, 1894.

The original estimate for the construction of this work was \$3,710,000 of this amount there has already been appropriated to June 30, 1893, \$1,337,500. There was a balance on hand at that date of \$24,333 exclusive of outstanding liabilities.

It is proposed to expend this and future sums appropriated in raising the jetty to a height of 4 feet above low water in those places that are not yet at that height, and in further strengthening the jetty. It is now about 13,000 feet that is at the level of mean low water. Experience has shown that it will not be safe to leave the jetty at this height. The first half of the tides flowing across the jetty, either ebb or flood, will take the sand with them and scour channels. Especially is this

case where there are low places in the jetty. It is only by building up the jetty that this cross-flow can be prevented. About 4 feet above low water seems to be the height required. The jetty towards the outer end will need to be well protected with large rocks to resist heavy seas. During the year Mr. G. B. Hegardt has continued to render intelligent and zealous service in local charge of this work. His report is herewith appended, to which attention is invited for further details pertaining to the work done.

APPROPRIATIONS.

July 5, 1884.....	\$100,000
August 5, 1886.....	187,500
August 11, 1888.....	500,000
February 22, 1890.....	75,000
September 19, 1890.....	475,000
Total.....	1,337,500

Money statement.

July 1, 1891, balance unexpended.....	\$274,710.46
June 30, 1892, amount expended during fiscal year.....	220,913.69
July 1, 1892, balance unexpended.....	53,796.77
July 1, 1892, amount covered by uncompleted contracts.....	29,465.65
July 1, 1892, balance available.....	24,331.12
Amount appropriated by act approved July 13, 1892.....	350,000.00
Amount available for fiscal year ending June 30, 1893.....	374,331.12
{ Amount (estimated) required for completion of existing project.....	175,000.00
{ Amount that can be profitably expended in fiscal year ending June 30, 1894.....	175,000.00
{ Submitted in compliance with requirements of sections 2 of river and harbor acts of 1866 and 1867.	

REPORT OF MR. G. B. HEGARDT, ASSISTANT ENGINEER.

OFFICE OF UNITED STATES ASSISTANT ENGINEER,
Fort Stevens, Oregon, July 1, 1892.

MAJOR: I have the honor to submit the following report of operations on the improvement of the mouth of Columbia River, Oregon and Washington, for the fiscal year ending June 30, 1892:

JETTY TRAMWAY.

At the end of last fiscal year the end of the tramway was at station 239+52 and the bracing carried to station 232+64.

The construction of the tramway was completed in July.

The extension during the year was 1,088 feet, which brings the end of the tramway and jetty to station 250+40. The bracing of the tramway was secured from station 232+64 to the end, a distance of 1,776 feet.

For the purpose of better securing the end of the tramway and the jetty, a spur track was put in at the end, joining the south track 300 feet from the end and having 13-foot centers. Each bent in the spur track was secured to the main bents in the tramway, thus forming a connected structure.

Sixty-five-foot piles were used for the tramway, with the exception of the last six bents, where 70-foot piles were driven.

The mean depth of water in which piles were driven was 12 feet, but before the mats could be sunk the sand would scour away to a depth of from 20 to 24 feet, the bottom generally seeking its level at about 22 feet below low water.

A few bents back from the end a cluster of four 75-foot piles were driven and allowed to stick up about 20 feet above the tracks. Between these four piles another pile, about 40 feet long and resting on the cap of the tracks, was secured with screw bolts and guys. On top of this pile was placed a ball beacon, 8 feet in diameter and

CHIEF OF ENGINEERS, U. S. ARMY.

The top of the beam...

and repairing the tramway had to be done... Several piles were

at 12 stringers through every 100 feet... over the two north piles from the end

from pile to pile between... the rock dumped into the

the south side of the 12 stringer... keeping up the repairs

the piles on the south side were... had to be run out

part of the tramway, having been... during the last

extensive and difficult... It would probably

the work was done... the piles

the work was done... the piles

the work was done... the piles

the work was done... the piles

the work was done... the piles

the work was done... the piles

the work was done... the piles

the work was done... the piles

the work was done... the piles

the work was done... the piles

the work was done... the piles

and probably was, by the rock being washed off by the sea. Part of the gravel has been placed where the settlement or washing off has taken place, it to raise the level of the rock to about 4 feet above low water, but the it towards the end of the jetty. The following approximate amounts were between the different stations: On the shore-protection spur 900 tons; station 44, 1,200; 44+100, 28,000; 100+170, 15,000; 170+200, 2,000; and from station the end the remainder, or 103,000 tons.

At the end of the jetty and back to station 225, a distance of 2,500 feet, the rock is raised to a level of about 4 feet above low water, and from station 96 to 44, a distance of 5,200 feet, to a height of from 3 to 5 feet above low water. At stations 225 and 96 the rock is on an average up to low-water level.

During away of sand has taken place at the end of the jetty. There has, on the contrary, been a filling in of sand on top of the mats of about 12 feet. The mats sink in 25 feet of water at low water, and the water is only from 12 to 14 feet above now. From the survey of the bar, made June 9 to 11 will be seen that the shoals to the westward of the end of the jetty are getting shoaler; and also to be going out toward the sea. On the progress sheet accompanying this report are shown the sand spits that are dry at extreme low water in the vicinity of the jetty. The change is very marked from that of last year, and especially so on the south

The drawing showing the soundings taken on the bar in June this year the soundings at the end of the main spits on the south side of the jetty are shown. The extreme limits of these spits were located by sextants from the surf-boat of the life-boat station here, the boat following the line of breakers as close as possible. The line of the main spit was also located by the use of sextants, from the spit it is a run-out of 2 feet below mean lower low water.

South of the end of the jetty, where there is a break in the line of soundings, the water must be shoal, as it was breaking too heavy there to take any soundings. Some time we were out in the surfboat.

The main portion of the main spit on the south side of the jetty is from 3 to 4½ feet above mean low water. The spit on the north side opposite the main one on the south is about 2 feet above mean lower low water.

Other sands shown as bare at extreme low water are from 1 foot above to 1 foot below mean lower low water.

The quarry of rock was suspended December 21 and again resumed March 1. An attempt was made by the contractor to get rock for the jetty at Bugby Hole quarry. It proved to be very brittle, and could not be gotten out except in small pieces, and was empty was given up. The rock has been furnished from the Willamette River as last year. All the towing of barges has been done by Government boats, and no expense.

The racing of the waves along the line of the jetty has almost entirely ceased, and the waves come only after very severe storms.

Trestle.—In September, about 200 cords of fascines were placed under the trestle between stations 21 and 25, just back of the rock dumped in front of the tracks, and about 300 tons of small rock placed on top of it, to prevent the undermining of the rock in front of the tracks. When heavy sea rolls in, the waves break over the trestle here, and the water, when receding, cuts away the sand from between the tracks and under the rock. Since the above work was done no trouble has been experienced at this place.

During the severe storms we had in December a great deal of rock in front of the trestle tracks between stations 22 and 26 washed off, and it was necessary to bring about 600 tons of rock here to bring it up to the same level it was before the

shore trestle tracks have been subject to decay as much as the first part of the wharf tracks, and required repairing to put them in good condition.

This year nearly all the piles had to be renewed, and this year the ties and some stringers had to be replaced.

At stations 23 and 30 the tracks were torn up and new ties put in for both tracks for the whole distance. From the first old wharf, station 4 to station 23, a number of new ties were put in. The rails of the north track between stations 23 and 30 were so corroded that it was deemed best to take them up. This was done and rails that were in the wharf tracks taken up and put down in the shore track, those from the shore trestle being laid on the wharf instead, the danger not being so great. This work was done in February during suspension of delivery. All the switches were taken up to allow new ties to be put down.

Repairs.—The severe storms experienced during the winter months, accompanied by high tides and heavy swells, did considerable damage to the wharf tracks. In December fifteen piles were washed out from under the wharf approach tracks, and were replaced at the time by new ones being dug in and blocked up. Shortly after delivery of rock had been suspended, in the early part of January, after another



THE
LIFE
OF
SAMUEL JOHNSON
BY
JAMES BOSWELL
IN TWO VOLUMES
THE SECOND VOLUME
LONDON
PRINTED BY A. MILLAR, IN THE STRAND
1791

5—the repairs to this were comparatively small, the locomotive being only a year old.

The boilers of all the locomotives were tested, as were also the steam gauges.

Derricks.—Three new derricks were made and put up in place of those in use, which were about two years old.

An additional derrick, complete, was made and is kept in reserve in case of breakdown.

Pile-driver.—After the repairs to the tramway the driver was laid up and woodwork painted. The iron work of the trucks has been scraped and given two coats of red lead.

Fig.—The deck of the *Mendell* was calked in September.

January 15 the *Mendell* was laid up for repairs, which were completed on the 29th. The repairs consisted of overhauling the high and low pressure cylinders and pistons, the valves, cross-heads, crank pins, and main journals, the condenser cleaned, circulating pump overhauled, boiler sealed and cleaned out. Indicator pipes were taken in, capstan engines repaired, canvass put under pilot-house to prevent leakage, all bunkers repaired, etc. The hull below water line was given one coat of copper paint and the hull and other woodwork painted.

Barges.—All the barges were more or less repaired while engaged in carrying rock in the quarry, the repairs being mostly made to the guards and sheathing.

Shafing plates for the anchors were put on all the barges.

Four additional hatches were cut in each barge to increase the circulation of air. The four oldest barges were taken to Portland for repairs, these being too extensive to be attended to here. The six other barges were taken to Youngs River for the winter, and there repaired by carpenters from here. These repairs consisted of rearing and renewing guards, chocks, bits, towposts, sheathing, etc. Roofs of masts and hulls above water line were painted.

Shop.—The machinery in the shop has been increased by the addition of 12-inch loper, the machinery and engine overhauled and scraped, and the tubes of the boiler painted.

Miscellaneous work done.—One old Dow pump, badly damaged, was repaired by having a new yoke put in, cylinders bored out, and new piston rods made. Two Worthen and one wind-mill pump were also put in good repair.

One of the boiler feed pumps on the steamer *Cascades* was thoroughly overhauled, boring the cylinders bored out, new pins made for the cross head, valves and valve seats reseated, etc.

For the improvement of Coos Bay, Oregon, the following work was done: Eleven dredged, self-righting dump cars were made, completing the construction of the twenty feet here.

Two matcars, used here in the construction of the jetty, were shipped to Coos Bay, there being no further use for them here.

Sixty-four hooks, for suspending inside mats while being made, were also sent at the same time.

Soundings.—June 9, 10, and 11 a series of lines of soundings were taken on the bar to develop the channel. These soundings show a straight-out channel about one-quarter of a mile wide, with a least depth of 29 feet, referred to the plane of the emergence of the lowest low waters. Last year's survey showed a 27-foot channel one-quarter of a mile wide and the 24-foot channel about 2 miles wide.

This year's soundings give the 27-foot channel a width of 1½ miles, the 24-foot channel having the same width as before, or 2 miles. The scouring away of sand from the 27-foot channel of this year has all been to the northern or in a more straight course. Scarcely any change has taken place in the 24-foot channel, the contours being practically where they were last year. The 29-foot channel is about in the same place where the 27-foot channel was last year. Where the 29-foot channel crosses through the crest of the bar the distance between the 30-foot curves has been increased about one-quarter of a mile.

The spit ahead of the jetty is shown to have moved outward, and shoaler soundings were found between the 24-foot curve and the end of the jetty than was the case last year.

The results obtained by the jetty are certainly very gratifying, and it may not be much to expect that, after the freshet in the Columbia River is over, there will be a 30-foot channel over the bar.

Miscellaneous.—Accompanying this report are the following drawings:

Plan and profile showing extent of jetty constructed June 30, 1892.

Survey of the mouth of the river made June 9, 10, and 11.

In closing this report, I wish to acknowledge the efficient and faithful assistance given me in my work by Mr. J. W. Stoneman, overseer.

Very respectfully, your obedient servant,

M. THOS. H. HANDBURY,
Corps of Engineers, U. S. A.

G. B. HEGARDE,
Assistant Engineer.

2816 REPORT OF THE CHIEF OF ENGINEERS, U. S. ARMY.

COMMERCIAL STATISTICS.

The following commercial statistics bearing upon the improvement of the mouth of the Columbia River were collected from the sources indicated, and are the most reliable that can be obtained.

Arrivals and clearances of vessels at Portland, Oregon, during the year ending June 30, 1892.

[From collector of customs Portland, Oregon.]

Vessels.	Coastwise.		Foreign ports.				Total.	
			American.		Foreign.			
	No.	Tons.	No.	Tons.	No.	Tons.	No.	Tons.
Arrived in 1891 and 1892.....	183	228,344	7	4,838	99	129,797	289	362,979
Cleared in 1891 and 1892.....	137	161,594	25	25,090	105	137,459	277	324,143

Commerce, July 1, 1891, to June 30, 1892.

Exports.....	\$1,111,000
Imports entered for consumption.....	1,111,000
Imports entered for transportation to other parts in the United States.....	1,111,000
Imports entered for transit through the United States.....	1,111,000
Total value of exports and imports.....	\$4,444,000
Duties from duties collected.....	1,111,000
Duties from other sources.....	1,111,000
Total receipts.....	\$2,222,000

Arrivals and clearances of vessels and commerce at Astoria, Oregon, during the year ending June 30, 1892.

[From collector of customs, Astoria, Oregon.]

Vessels.	Coastwise.		Foreign ports.				Total.	
			American.		Foreign.			
	No.	Tons.	No.	Tons.	No.	Tons.	No.	Tons.
Arrived in 1891 and 1892.....	100	120,000	10	10,000	10	110,000	20	240,000
Cleared in 1891 and 1892.....	80	100,000	15	15,000	10	80,000	15	205,000

Commerce, July 1, 1891, to June 30, 1892.

Exports.....	\$1,200,000
Imports.....	1,200,000
Total receipts.....	\$2,400,000

The total amount of cargo passing in and out over Columbia River Bar, as reported to the collector of customs at Portland and Astoria, is 2,019,467 tons.

APPENDIX U U—REPORT OF MAJOR HANDBURY. 2817

FROM RECORDS OF THE MERCHANTS' EXCHANGE, PORTLAND, OREGON.

and tonnage inward over Columbia River Bar from June 30, 1891, to June 30, 1892.

1.	Deep-sea vessels.						Steam.				Coasters, American.		Total.	
	American.		British.		German, etc.		American.		Other flags.					
	No.	Tons.	No.	Tons.	No.	Tons.	No.	Tons.	No.	Tons.	No.	Tons.	No.	Tons.
.....			1	1,205			31	26,736	2	2,482	6	2,056	40	32,479
.....	1	1,462	3	3,714			27	20,006	4	4,135	3	868	38	30,185
er	2	3,385	4	6,208			25	19,520	2	2,531	6	1,841	39	33,487
.....			10	10,940	4	4,694	26	21,267	2	1,742	8	3,253	50	41,896
er			21	29,943	1	1,007	25	23,383	2	2,491	2	2,793	51	57,617
r	1	1,036	11	15,736			20	19,059	3	3,782	4	1,330	39	40,943
.....	1	1,469	21	28,809	2	2,248	21	15,994	3	3,404	4	1,773	52	53,697
.....	1	1,931	3	3,781			22	18,233	1	871	6	4,693	33	29,509
.....	1	1,746	3	3,693			19	15,108	1	1,565	4	1,944	28	24,096
.....			1	1,399			25	18,905	1	1,662	5	1,288	32	23,254
.....	4	6,574	2	3,244			27	17,679			6	2,658	39	30,155
.....			4	5,121			23	16,564			2	2,729	29	22,414
al.	11	17,603	84	113,793	7	7,949	291	232,454	21	24,667	56	23,266	470	419,732
us	9	12,393	73	97,954	3	3,969	307	301,360	22	21,448	69	22,103	483	450,247

and tonnage outward over Columbia River Bar from June 30, 1891, to June 30, 1892.

1.	Deep-sea vessels.						Steam.				Coasters, American.		Total.	
	American.		British.		German, etc.		American.		Other flags.					
	No.	Tons.	No.	Tons.	No.	Tons.	No.	Tons.	No.	Tons.	No.	Tons.	No.	Tons.
.....			4	4,724			27	25,560	3	4,004	8	2,188	42	36,536
.....			1	1,312			29	20,443	4	4,266	3	1,647	37	27,608
er			2	1,977			26	20,343	3	3,262	5	1,622	36	27,221
er	2	3,359	7	9,842	1	1,256	24	21,138	2	2,533	6	2,460	42	40,597
r	1	1,488	9	9,717	3	3,438	26	22,630	3	3,362	6	2,649	48	43,294
.....			3	4,904			20	21,308	2	2,436			25	28,648
.....	1	1,036	24	33,499	1	1,007	19	15,226	2	2,533	8	2,531	55	55,832
.....			20	27,787			24	17,974	2	2,217	2	856	48	48,834
.....	1	1,469	3	5,138	2	2,248	16	14,897	2	2,436	5	2,399	29	28,589
.....			4	4,786			27	19,158	1	1,662	6	3,198	38	28,804
.....	1	1,589	3	3,712			29	18,842			4	1,484	37	25,627
.....	1	1,553	2	1,988			22	15,160			3	1,194	28	19,895
l.	7	10,494	82	109,386	7	7,919	269	232,679	24	28,731	56	22,237	465	411,478
us	9	12,393	67	90,683	4	4,963	287	298,845	21	18,807	70	23,218	458	448,009

Comparative statement of principal exports for the past ten seasons.

WHEAT.

	Foreign.		Domestic.		Total.	
	Centals.	Value.	Centals.	Value.	Centals.	Value.
ending July 31—						
.....	1,761,754	\$2,918,458	362,709	\$560,338	2,124,463	\$3,478,796
.....	2,276,809	3,712,253	311,941	477,725	2,588,750	4,199,978
.....	3,147,902	3,720,378	521,834	598,828	3,669,736	4,319,203
.....	3,971,756	4,979,841	1,349,740	1,602,805	5,321,496	5,582,646
.....	3,042,316	3,889,499	711,872	988,936	3,754,188	4,878,435
.....	3,203,108	3,783,208	1,269,263	1,485,189	4,692,371	5,268,397
.....	2,689,686	3,779,644	559,953	741,642	3,249,629	4,521,288
.....	1,752,879	2,210,950	343,690	420,151	2,096,569	2,631,101
1890, to June 30, 1891.	2,786,633	3,814,020	1,235,833	1,536,310	4,022,466	5,350,430
1891, to June 30, 1892.	3,563,562	5,713,710	977,677	1,503,310	4,541,230	7,217,020

2818 REPORT OF THE CHIEF OF ENGINEERS, U. S. ARMY.

Comparative statement of principal exports for the past ten seasons—Contd.

FLOUR.

	Foreign		Domestic.		Total
	Barrels.	Value.	Barrels.	Value.	
1899-1900	1,029	\$1,684,919	179,471	\$708,854	1,209,390
1900-1901	1,074	1,732,784	218,846	845,837	1,587,621
1901-1902	1,038	1,749,439	182,516	659,485	1,407,954
1902-1903	1,028	1,729,877	181,736	689,708	1,411,585
1903-1904	1,028	1,742,508	187,908	548,447	1,290,601
1904-1905	1,028	1,727,928	178,015	798,979	1,411,471
1905-1906	1,028	1,727,928	144,184	584,186	1,243,742
1906-1907	1,028	1,727,928	111,681	417,275	1,110,653
1907-1908	1,028	1,727,928	129,586	522,943	1,250,315
1908-1909	1,028	1,727,928	129,586	621,216	1,350,144

SALMON.

	Foreign		Domestic.		Total
	Cases.	Value.	Cases.	Value.	
1899-1900	1,029	\$1,684,919	289,944	\$1,460,181	1,319,123
1900-1901	1,074	1,732,784	267,784	1,398,629	1,468,413
1901-1902	1,038	1,749,439	267,583	1,484,315	1,511,896
1902-1903	1,028	1,729,877	419,999	1,795,094	1,824,971
1903-1904	1,028	1,742,508	372,112	1,925,500	1,937,618
1904-1905	1,028	1,727,928	382,799	2,241,281	2,269,209
1905-1906	1,028	1,727,928	37,335	1,904,596	1,941,931
1906-1907	1,028	1,727,928	24,724	2,422,210	2,446,938
1907-1908	1,028	1,727,928	279,715	1,441,094	1,720,813
1908-1909	1,028	1,727,928	4,782	1,739,975	1,744,763

Comparative statement of principal exports for the past ten seasons.

	Foreign		Domestic.		Total
	Cases.	Value.	Cases.	Value.	
1899-1900	1,029	\$1,684,919	289,944	\$1,460,181	1,319,123
1900-1901	1,074	1,732,784	267,784	1,398,629	1,468,413
1901-1902	1,038	1,749,439	267,583	1,484,315	1,511,896
1902-1903	1,028	1,729,877	419,999	1,795,094	1,824,971
1903-1904	1,028	1,742,508	372,112	1,925,500	1,937,618
1904-1905	1,028	1,727,928	382,799	2,241,281	2,269,209
1905-1906	1,028	1,727,928	37,335	1,904,596	1,941,931
1906-1907	1,028	1,727,928	24,724	2,422,210	2,446,938
1907-1908	1,028	1,727,928	279,715	1,441,094	1,720,813
1908-1909	1,028	1,727,928	4,782	1,739,975	1,744,763

1,028 1,727,928 279,715 1,441,094 1,720,813

Comparative statement of deep-sea tonnage for the past eight seasons—Continued.

OUTWARD.

Flag.	1884-'85.		1885-'86.		1886-'87.		1887-'88.		1888-'89.		1889-'90.		1890-'91.		1891-'92.	
	No.	Tons.	No.	Tons.	No.	Tons.	No.	Tons.	No.	Tons.	No.	Tons.	No.	Tons.	No.	Tons.
American	20	24,790	14	19,323	7	9,280	3	3,857	12	16,611	5	8,050	9	12,333	7	10,494
British	97	92,655	119	124,073	111	119,716	98	122,344	75	89,731	52	67,428	67	90,683	82	109,586
Norwegian			1	901			2	2,137	2	1,744						
German, etc	4	3,618	3	2,622	1	851	2	2,350	4	3,976	1	974	4	4,963	7	7,949
Total	121	120,963	137	146,919	119	120,847	106	130,688	93	112,062	58	76,452	80	108,039	90	127,820

Note.—From 1884 to 1890 the season begins on July 31; from 1890 the period covered is from June 30.

U U 2.

CONSTRUCTION OF CANAL AT THE CASCADES, COLUMBIA RIVER, OREGON.

The general scope of the improvement which it is desired to effect at the Cascades of the Columbia River includes a reach of about $4\frac{1}{2}$ miles, where the river rushes through a narrow gorge in the Cascade Mountains. The fall in the distance is about 45 feet at high water and 36 feet at low water. The principal obstruction to navigation occurs at the upper end of the reach known as Upper Cascades. The project for the improvement contemplates that the river should be improved below the Upper Cascades by removing bowlders and projecting points in the bed and banks so as to give good navigable water from its lowest up to a 20-foot stage. The fall at the Upper Cascades is to be overcome by digging a canal of 3,000 feet in length across the neck of a low, projecting spur, around which the river is forced at the entrance to the gorge, and placing in this a lock and other suitable structures, which would permit of the passage of boats up to a 20-foot stage of water in the river, this lock and canal to be so arranged that, should the future necessities of commerce so demand, additional structures may be added which will permit of navigation at much higher stages.

The first part of this project, that of improving the river below the foot of the Upper Cascades, is essentially finished.

The difference of level between the head and foot of the canal as now established is 15 feet at high water and 24 feet at low water, and difference in height between high and low water at the foot is 54 feet, and at the head 45 feet. The plan on which the future work in the canal, with its lock and accessories, is to be prosecuted, has for its object to make this portion of the river available for navigation to a stage up to 20 feet at the earliest possible moment, with the funds that are from time to time appropriated for the purpose.

At the commencement of the present fiscal year there was available for the prosecution of the work \$204,691.71. At that time the principal work in progress was cutting stone, there being about fifty cutters employed. Other operations, excepting current repairs, were suspended on account of high water in the Columbia River. By the 1st of August the water had receded sufficiently to permit the lock pit to be pumped out. On the evening of the 7th the water was practically all out of this. Preparations were at once made for resuming the work of placing concrete and setting stone in the north wall of the masonry of the lower lock and guard gate. Quarrying stone was resumed about the 20th of July. Concreting and stone setting was resumed August 20. Work was pushed forward actively until December 5, when it became necessary to suspend the making of concrete and stone setting. On

upper canal walls are nearly completed from the site of the upper guard gate to the upper bulkhead; the slope outside of the berm on the north side of the canal has been paved from the lower bulkhead to beyond the center of the lock chamber a distance, approximately 800 feet, and from the upper bulkhead down a distance of 300 feet; on the south side of the canal the slope has been paved from the lower bulkhead to the lower lock gate, a distance of about 600 feet, and from the upper bulkhead nearly to the upper guard gate a distance of about 500 feet; the excavation of the lower canal from the lower bulkhead to the lower guard gate has been practically completed; the excavation in the lock chamber has been completed, except 2 feet left on the bottom to be removed just before putting down the concrete floor; the excavation for the abutments of the upper lock gate has been partially completed; and from the site of this gate to the upper bulkhead also partially completed. In addition to this a large amount of necessary incidental work has been done, in the way of constructing temporary protection works, quarters, shops, etc., and the establishment of a complete plant for carrying on the work rapidly and economically.

The principal work yet to be done is approximately as follows:

Excavation:

Earth and loose rock	cubic yards..	285,757
Bed rock	do.....	57,782
Total.....	do.....	343,539

Masonry construction:

Granite	do.....	1,210
Basalt dimension stone	do.....	4,287
Basalt face stone	do.....	4,277
Concrete	do.....	46,311
Side wall at entrance	do.....	31,311
Paving slopes	do.....	19,800
Total.....	do.....	107,196

The gates, valves, pumps, engines, and other maneuvering machinery are to be provided. There is also a large amount of filling in and grading to be done. A general idea of the masonry work constructed and yet to be constructed can be had from the tracing herewith, showing work completed and proposed.

There are on hand, June 30, approximately, 3,000 cubic yards of cut stone of various kinds and 457 cubic yards of rough basalt stone ready for cutting.

The buildings and plant are now in a good state of repair and everything is in readiness to push the work forward as rapidly as the funds available and other circumstances will permit.

In my last annual report mention is made of the fact that the agents of the State of Oregon were permitted to enter upon the Government grounds at the Cascades and had commenced the construction of a portage railway thereon, this road with its two inclines was finished about the middle of September. The first boat to reach the upper incline from The Dalles arrived September 15; the first boat to reach the lower incline from Portland arrived September 20. These two boats ran regularly from September 22 to January 12 of this year, their freight and passengers being transported over the portage railway. On this last date the service was discontinued on account of cold, stormy weather, and ice in the river between the Cascades and The Dalles. The boats began their regular trips again February 14 and continued until May 6, when the lower boat was taken off on account of high water, the

REPORT OF LIEUTENANT HARRY TAYLOR, CORPS OF ENGINEERS.

UNITED STATES ENGINEER OFFICE,
Cascades Locks, Oregon, June 30, 1892.

I have the honor to submit the following report of operations for "Improving Columbia River at Cascades, Oregon," for the year ending June 30, 1892.

I assumed local charge of the work on November 14, 1891, relieving First Lieut. Ed. Burr, Corps of Engineers, on that date. At the time I assumed charge it was the end of the working season, but the work was in full progress, with everything running smoothly and systematically.

The river and harbor act approved September 19, 1890, appropriated \$435,000 for this improvement, and the approved project for the expenditure of this appropriation calls for the completion of the masonry of the lower lock and guard gates, and the building of the lock wall and the masonry of the upper lock gates on the land side to the height of about 26 feet, with allowance for necessary additional excavation, and for plant, operating, and incidental expenses; the work to be done hired labor and the purchase of material.

During the winter and early spring of 1891 work had been carried on under the approved project, and up to the end of June, 1891, the work had been the construction of the south wall of the masonry of the lower lock and guard gates, together with necessary excavation, stone cutting, preparation and repair of plant, and all other operations incidental to this work. Preparation had also been made for commencing north wall masonry for these gates, including the cleaning up of the foundation shifting tracks and derricks, both in the pit and overhead. The work was in progress when the annual rise in the Columbia River caused a suspension of all work in the pit on May 4, 1891.

At the beginning of July then there was no work in progress except stone-cutting, overhauling and repairing plant.

The river commenced to fall in the early part of July, and preparations were made preparatory to pumping out the pit as soon as the water should be low enough. The water having fallen sufficiently the pumps were started on August 1, and the water practically out of the pit by the evening of the 7th; but on account of the leak the bulkhead the water was troublesome for about two weeks, until the river had fallen some 5 feet or more, to about 85 on gauge No. 2.

As soon as the water was out of the pit (August 8) preparations were begun for resuming the work of concreting and stone-setting in the north wall of the masonry of the lower lock and guard gates. Concreting was commenced August 19, and stone-setting August 22.

The quarrying had been commenced on July 17th and work in the sand pit on August 14, so that on August 22 the work was in full progress again.

From August 22 until October 16 the work of concreting and stone-setting was confined to the north wall of the masonry of the lower lock and guard gates. This work was practically completed on October 16, and the shifting of the derricks, engines, and tracks to their positions for work on the south (or land side) wall of the lock chamber commenced. These changes were completed on the 19th, and concreting and stone-setting were commenced in the south wall on the 19th and 21st, respectively. A length of about 300 feet was put under construction and completed to reference 108, both concrete and stone, on December 5, 1891.

As soon as the wall was finished the laying up of the plant commenced. Engines and derricks were taken out of the pit, the pump stand moved into position for pumping after the next high water and weighted down, the inclined trestle and sack in the pit weighted and the work suspended for the winter.

The excavation for the extension of the south wall of the lock-chamber wall was commenced on October 30 and carried on until December 16.

The excavation was not quite completed, but as all other work was stopped and the weather very unfavorable it was not considered advantageous to finish the excavation at that time.

The work in the sand pit was suspended December 5, in the quarry November 16, and stone-cutting December 11, 1891.

The office force was moved to Portland December 19, and the work left in charge of the overseer and the necessary watchmen.

The office force was employed in Portland in the preparation of the reports and estimates of the last season's work, in the preparation of the detailed drawings of the masonry of the upper lock and guard gates, and of the shop drawings of the lower lock and guard gates, until April 4, 1892, when it was again moved to Cascade Locks.

As soon as the weather was favorable in the spring, 1892, preparations were made for the resumption of quarrying and stone-cutting. No attempt was made to lay concrete, as the funds remaining from the last appropriation would have been completely exhausted in about a month by the force necessary to carry on that work.

REPORT OF THE CHIEF OF ENGINEERS, U. S. ARMY.

Excavation and stone-cutting were both resumed on April 8, 1892, and have since been continuously done. The stone-cutting during the spring has been on the abutments and for the masonry of the upper guard gate.

The work done for the year ending June 30, 1892, has been the reconstruction and alterations of the lower lock and guard gates, the construction of about 3/4 of the south wall of the lock chamber, the excavation necessary to complete the masonry and cutting stone, the repairs of plant, and all other operations incident to the work.

The character of the work on June 30, 1892, is, in general terms, as follows:

The lower canal walls have been completed to reference 90 from the abutments the lower guard gates nearly to the lower bulkhead, the completed length south wall being 285 feet and the north wall 275 feet, the abutments of the lock and guard gates have been completed to reference 108 on both sides of the south (lock side) wall of the lock chamber has been completed to reference for a length of about 245 feet; the south wing wall of the upper guard gates has been completed to reference 124 for a length of about 97 feet; the upper canal wall has been nearly completed from the site of the upper guard gates to the upper bulkhead to beyond the corner of the lock chamber, a distance of, approximately, 800 feet and from the upper bulkhead down a distance of a little less than 200 feet; on the south side of the canal the slope has been paved from the lower head to the lower lock gates a distance of about 600 feet, and from the upper head nearly to the upper guard gates, a distance of about 500 feet; the excavations for the abutments of the upper lock gates has been partially completed; the excavation in the lock chamber has been completed, except that on the bottom to be removed just before putting down the concrete floor necessary for the abutments of the upper lock gates has been partially completed; from the site of the upper guard gates to the upper bulkhead the bulkhead excavation has been completed.

The work already accomplished, exclusive of all incidentals, such as the construction of temporary protection works, quarters, shops, etc., and the establishment of a complete plant for carrying on the work rapidly and economically, is approximately as follows:

Excavation:

Earth and loose rock	cubic yards..	16
Red rock (conglomerate)	do.....	23
Total	do.....	40

Masonry construction:

Granite	cubic yards..	
Rock of dimension stone	do.....	1
Round face stone	do.....	2
Concrete	do.....	32
Side walls at entrances (not basalt facing rubble backing)	do.....	16
Paving slopes (dry rubble)	do.....	9
Protection wall on right	do.....	24
Total	do.....	77

The work still to be done is approximately as follows:

Excavation:

Earth and loose rock	cubic yards..	285
Red rock	do.....	57
Total	do.....	343

Masonry construction:

Granite	cubic yards..	1
Rock of dimension stone	do.....	4
Round face stone	do.....	4
Concrete	do.....	46
Side walls at entrances	do.....	31
Paving slopes	do.....	15

Total cubic yards

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materials used during the past year, for both concrete and stone masonry, were the same as during the last three months of last year. The character of these materials, proportions, and the plant and methods used in incorporating the concrete, and setting the concrete and stone were so fully and minutely described in Lieutenant Handbury's annual report dated June 30, 1891, that any further description is unnecessary. The only important piece of plant purchased was a pumping outfit, consisting of a centrifugal pump and engine, to replace a similar outfit which had become worn out by long and continuous use. The only change of any note was made in the head traveler in the stone yard. In this the power had been communicated from a steam engine to the driving wheels which gave the motion up and down the length of the shaft through a worm gearing. The speed was slow and the worm gearing had given much trouble, heating and breaking and causing much delay. The worm gearing was taken out and replaced by pinions and wheels. The speed up and down the yard was a little more than three times what it was before, it takes less steam, and it can be run without a hitch. The increased speed of the traveler will allow more cutting to be worked under the sheds than has heretofore been possible.

The amount of concrete made and placed during the year was 17,899 cubic yards. It was all mixed by machinery and placed by chute. Of this amount 836 cubic yards were placed in the south wing wall of the upper guard gates, and the remainder in the abutments of the lower lock and guard gates and the lock chamber walls. The cost of the concrete in the wing wall was somewhat greater than the average of the whole, due to the greater distance of the wall from the mixers, requiring more labor to transfer the concrete from the point of manufacture to the point of use. The proportions varied from 1 of cement, 2 of sand, and 4 gravel or broken stone, to 1 of cement, 4 of sand, and 8 of gravel or broken stone, depending on the final disposition of the concrete. The proportion 1:2:4 was used in the foundations of the lock and in the bottom of the canal in stopping leaks in the bed rock; the proportion 1:3:6 for the lower 14 feet of the lock walls and lower 10 feet of the abutments, and near the top of the walls the proportion was reduced to 1:4:8.

In making 17,899 cubic yards of concrete there were used 15,720 barrels of Portland cement, 8,457 cubic yards of sand, 13,984 cubic yards of gravel, and 1,001 cubic yards of broken stone. The average product was 1.14 cubic yards of concrete per barrel of cement and 1.19 cubic yards per yard of gravel and broken stone. The materials used per cubic yard of concrete were 0.878 barrels of cement, 0.473 cubic yards of sand, 0.781 cubic yards of gravel, and 0.056 cubic yards of broken stone. Cement includes such as was used dry, amounting to 136 barrels.

In connection with the concrete, the manufacturing of the culvert pipe was completed. The pipe was made in sections 3 feet long, 39 inches internal diameter, with 6 inches thick. The concrete was made in the proportion 1:2:4, and the gravel passed through a half-inch screen.

Ninety-seven sections of this pipe were made, containing 51 cubic yards of concrete. The total cost was \$654.51, and the cost per linear foot was \$2.83. The cost of setting the pipes is included in the cost of setting stone.

The sand and gravel for the concrete were obtained from the same pit as last year. The proportions of the two materials varied somewhat at times, but as a rule the proportion of gravel ran higher than it did when the pit was first opened. This was an advantage, as it required proportionally less crushed stone, and the gravel, as shown in previous reports, makes a better and cheaper concrete than the broken

stone. An estimated quantity of 15,457 cubic yards (in place) of material was taken from the pit. After washing and separating there were obtained from this material 8,712 cubic yards of sand, and 13,984 cubic yards of gravel, which were used in the manufacture of the concrete or for making mortar; and in addition 1,035 cubic yards of material were rejected by the screens and used for filling behind the lock walls. The average cost of sand and gravel in the bins, after washing, was 71 cents per cubic



THE
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band, June 30, 1,403.0 cubic yards of cut stone, 457.2 cubic yards of rough basalt, and 1,688.2 cubic yards of stone similar to that which was recut.

Cost in detail of stone laid.

Items.	Granite.		Dimension.		Face.		Culvert.		Canal.		Total.
	Cubic yds.		Cubic yds.		Cubic yds.		Cubic yds.		Cubic yds.		
Transporting	\$181.03	.542	\$352.84	.542	\$1,085.68	.542	\$36.31	.542	\$140.92	.542	\$1,796.78
Setting	755.77	2.263	1,473.07	2.263	4,532.56	2.263	151.61	2.263	588.33	2.263	7,501.34
Cement and sand.	230.83	.691	449.93	.691	1,384.38	.691	40.31	.691	25.50	.691	2,136.05
Stone	24,644.28	73.783	17,583.09	27.009	39,276.41	19.607			704.50	2.710	82,208.28
Other materials	493.33	1.495	973.25	1.495	2,994.65	1.495	100.17	1.495	388.70	1.495	4,956.10
Total	26,311.24	78.771	20,832.18	32.000	49,273.68	24.598	334.40	4.091	1,847.95	7.108	98,599.45
Plant dropped	346.53	1.038	274.55	.422	649.40	.325	4.42	.006	24.42	.094	1,290.32
Engineering and superintendence	583.85	1.748	462.57	.710	1,094.14	.546	7.44	.111	41.15	.159	2,189.15
Incidentals	4,548.19	13.621	3,603.42	5.535	8,523.39	4.255	57.98	.866	320.61	1.231	17,053.59
Aggregate	31,789.81	94.138	25,172.72	38.667	53,510.61	29.724	404.24	6.034	2,234.13	8.592	119,141.51

Stonecutting was in progress at the beginning of July, 1891, and was continued until December 11, with as large a force as could be advantageously worked. Work was then suspended until April 8, 1892, when cutting was again resumed. The work since then has been carried on with about two-thirds the number of cutters that could be worked with the present facilities.

The stonecutters had also been at work during May and June, 1891, while other work was suspended on account of high water. Stone setting did not begin until August 22, but even with nearly four months' cutting ahead, the setting progressed so rapidly that at the beginning of December the masons had very nearly caught up with the cutters. The cutters were also helped by using some of the stone which had been cut in previous years, for use in the side walls of the entrances. The stone selected was of the proper rise and the beds and joints were already cut, but the face had been left rough. All the cutting necessary to fit this stone for use was to cut the face down to rough punched.

Nearly all the stone for the lower lock and guard gate abutments had been cut previous to July, 1891, so that the cutting of last fall was confined principally to face and dimension stone for the lock-chamber walls.

Since the work was resumed in April the cutting had been confined, as far as possible, to the dimension stone for the abutments of the upper lock gate. The dimension stones for one abutment are now almost all cut ready for laying.

The character of the stone and of the cutting were the same as last year.

The following summary gives the details of the cutting for the year, both as to quantities and cost:

	Granite.	Dimension basalt.	Basalt face.	Total.
Stone cut.....number.....	76	1,486	2,647	4,209
Stone cut.....cubic feet.....	2,073.8	27,119.8	50,019.3	79,209.9
Stone cut.....square feet.....	4,262.4	53,575.9	83,089.4	141,527.7
Stonecutters' labor.....hours.....	2,770	29,964	29,669	62,403
Average per hour.....cubic feet.....	.75	.91	1.69	
Average per hour.....square feet.....	1.54	1.82	2.82	
Cost for cutting.....per square foot.....	\$0.50	\$0.44	\$0.30	
Cost for cutting.....per cubic foot.....	1.03	.86	.45	
Cost for stone.....do.....	1.36	.32	.29	
Total cost.....do.....	2.39	1.18	.74	
Total cost of stone cut.....	4,949.37	32,298.37	36,895.99	\$74,053.73

The quarry work was carried on to supply the basalt required for face and dimension stone. The quarrying during the year has been done on the land adjoining the canal grounds on the east, the privilege for which was obtained last year. During the year there were obtained 2,110.5 cubic yards of dimension stone and 603.9 cubic yards of rubble. No rubble was hauled in from the quarry during the last six months as the storage ground for this stone had become very crowded and it will be cheaper to leave it at the quarry until such time as it may be needed than to haul in

new and rehandle it, as much as would be necessary. As the rubble split out incidentally with quarrying, the dimension stone during the spring was not hauled from the quarry, no account is made of it. This increased the cost of the dimension stone somewhat as all the charge for quarrying went to that while heretofore it has been divided between the dimension stone and the rubble.

The total cost of quarrying during the year, including the 14 cents per cubic yard for the quarry right, was \$16,505.89. The average cost per cubic yard was \$6.08; the cost per cubic yard for dimension stone was \$7.25, and per cubic yard for rubble, \$1.93. These items include the cost of transporting the stone to the yards.

The excavation for the year amounted to 8,033 yards. This includes the 2 ft removed under the wall just before putting down the concrete, and the bed rock removed east of the part of the south wall under construction in preparation for the completion of this wall and the south abutment of the upper lock gate. As the high water of 1891 did not cover the guard fence at the lower entrance no sediment was found in the canal when it was pumped out. The total cost of the work was \$10,087.40, which is \$1.26 per cubic yard. The cost here given includes excavation and dumping.

All of the excavated material was used in filling behind the walls. The cobble which were rejected by the sand washers, to the amount of 1,035 cubic yards, were also used as filling, and in addition strippings from the sand pit to the amount of about 462 cubic yards were also used as filling. The total cost of the filling was \$2,378.08.

The metal work for such portions of the lower lock and guard gates as are embedded in the wall had been ordered the year before. These were received and embedded in the masonry.

Snubbing hooks for maneuvering and making fast were manufactured and placed in that portion of the lock wall constructed.

The usual office records were kept to supply all desired details of costs and quantities. The detailed drawings of the masonry are practically completed. The working drawings for the stonecutters for the upper lock gate are all finished and the drawings for the upper guard gate very nearly so. The details of the gates and the snubbing drawings are well advanced.

The cost of the work for the year may be summarized as follows, proportioning engineering, superintendence, and incidentals to the various accounts according to their totals:

Concrete masonry.....	\$106,012.00
Stone masonry.....	119,141.00
Excavating.....	10,087.40
Lock gates.....	5,499.00
Maneuvering machinery and appliances.....	1,721.00
Filling behind walls.....	2,378.08
Total for lock construction.....	244,840.00
Plant in excess of that dropped.....	1,968.00
Total cost for the year.....	242,871.00

Under the authority granted by the Secretary of War the State of Oregon, by its agents, entered upon the canal grounds during May, 1891, and commenced operations upon the construction of a portage railroad. The road was finished during September, and the first boat reached the upper end of the road September 15 at the lower end September 20. The boats in connection with the road commenced running regularly September 22 and continued until January 12, 1892, when they were laid up on account of the cold, stormy weather and the ice in the river between here and The Dalles. The boats began their regular trips again February 14 and continued until May 16, when the lower boat was taken off on account of high water. No transferring by the road has been done since. When the lower boat stopped running in May the river stood at 92 on the lower gauge, just 20 feet above assumed low water. The boat was taken off more on account of the danger and difficulty making a landing than on account of the difficulty of reaching the landing place. Although the boat is of small power she had very little trouble in coming up through the lower rapids, but as she approached the foot of the main rapids she was obliged to keep over near the Washington shore, and to make the landing had to cross the swift current diagonally to the landing place on the Oregon side. The landing at all stages is necessarily on the upstream side of the portage incline and at the high stages a strong current sweeps over the lower end of this incline, putting a boat in danger of being carried down onto the piles of the incline or being driven heavily against the wharf boat. When the canal is completed and the incline removed, it necessarily will be, this trouble will be avoided and boats can easily ascend the lower rapids and make the lower entrance of the canal until the river is considerable above the stage at which the boat stopped this year.

summary of the weather and water-gauge records is transmitted herewith. The following drawings relating to the work are also transmitted: sheet 1, showing the work finished and that proposed. sheet 2, showing the dates and amounts of the work already done on the lock-ber walls and lower-entrance walls.

Very respectfully, your obedient servant,

HARRY TAYLOR,
First Lieutenant of Engineers.

J. THOS. H. HANDBURY,
Corps of Engineers, U. S. A.

U U 3.

IMPROVEMENT OF COLUMBIA AND LOWER WILLAMETTE RIVERS BELOW PORTLAND, OREGON.

The object of this improvement is now to make and maintain a navigable channel from the city of Portland, Oregon, to the sea, having a water depth of 25 feet. There is included in this reach 12 miles of Willamette River and 98 miles of the Columbia, measured along the low-water channel. Before the commencement of the improvement in accordance with previous projects the low-water depth of channel at the shallowest places was between 10 and 15 feet. At the end of the present project there is a low-water depth of 20 feet throughout the entire distance, except at the points in Cathlamet Bay, where there is but 19 feet.

The average rise of the tide in this bay is 7 feet.

The original project under which this improvement has been carried out with modifications and extensions was adopted in 1877. Some preliminary work had been done previous to that time. It contemplated the construction of a navigable low-water channel having a depth of 20 feet should be maintained by means of permanent constructions to protect the banks, contract the rivers in wide places, direct the currents, and control the amount of water that should pass through sloughs having a detrimental influence upon the main channel. While these constructions were being put in place and until their influence was fully developed dredging was resorted to in several localities to give a temporary relief to commerce. Works of a permanent character have been constructed in the Willamette River across the head of Swan Island Chute; in the head of Willamette Slough, to control the amount of water passing into that channel; also along the left bank of the river from that point to its mouth, to prevent erosion and excessive widening of the river; also across other sloughs and channels near the mouth of the Willamette River, to control its waters and those of the Columbia in its vicinity. Permanent structures were placed in the Columbia River at St. Helens, Burke Slough, and Martin Slough, all of which have produced very beneficial effects upon the navigable channel in their vicinity. During the time that these were in contemplation or process of construction dredging was resorted to to give temporary relief.

The amount expended by the United States in this improvement since the adoption of the project of July 1, 1877, is \$649,805.97. It appears from the reports previous to that date \$221,780.46 had been expended in various appropriations looking to the improvement of the Willamette and Columbia rivers between Portland and the sea. The aggregate of the appropriations and allotments, including proceeds from sales of property to the Government, up to June 30, 1892, is \$889,745.71. In addition to this amount the citizens of the city of Portland have in various ways

formed the main part of the dam, the piles preventing any portion of it being washed away. It was the original design that all the piles should be sawed off at a uniform height of 4 feet above low water and connected together in each row by waling pieces bolted to them. As the piles were driven from the north end southward to about the middle of the dam a deposit of sand was made on the lower side, and this built up in some portions 4 and 5 feet above low water. In these portions the waling was either omitted or placed above the 4-foot level. To prevent this deposit from being scoured out at any time a mattress of fascines was put over it and covered with rock. On the south half of the dam where the original design could be carried out, where the waling pieces were bolted on, planks were spiked to the piles on the inside down to low water to prevent stone from rolling out. Fascines were placed between the rows on the top of those already in place and loaded down with rock.

Work was commenced under the contract October 29, and by its terms should have been finished within ninety days from its date. It was pushed as rapidly as practicable, but owing to the river remaining at an unusual height for this season of the year the work could not be finished at the specified time. An extension of time was asked and granted until April 30, 1892. By this time all the material required for the dam was in place according to specifications, excepting that 830 piles remained to be sawed off. The estimated cost of this, at 15 cents each, will be deducted from the retained percentage of the contractors before final settlement is made.

Since the dam was completed it has been submerged by backwater from the Columbia River. Its effect upon the bar at Swan Island would, under this condition, be nothing. The results will be developed after the next Willamette high water.

The following material was furnished in place under this contract:

42,679 linear feet piles, at 14½ cents	\$6,188.45
3,016.11 cords of fascines, at \$2.65	7,992.69
15,058.69 tons of rock, at 60 cents	9,035.20
69,301.4 feet, B. M., lumber, at \$12	831.61
1,769 pounds spikes, at 3½ cents	57.48
5,230 pounds wire, at 3¼ cents	183.05
1,452 bolts, at 30 cents	435.60
Total	24,724.08
Less \$124.50 for piles not sawed off	24,599.58

On the 15th of September the chain bucket dredge belonging to the city of Portland, known as the *City Dredge*, with its tender, the tug *Vaughn*, were placed at the Government service, and, after employing the necessary crew, set to work at the Swan Island Bar in the Willamette River. This shoal was at that time giving trouble to deep-sea vessels. The old cut had not been entirely cleared out to 20 feet depth during the preceding season. Dredging operations were continued at this point until the end of October, when the dredge and outfit were returned to the city authorities and by them turned over to the Port of Portland Commission for work elsewhere. The total amount of material removed from Swan Island Bar during the season was 21,860 cubic yards, at a cost of 12½ cents per cubic yard, including repairs and all other expenses.

A project for improving the Lower Willamette and Columbia rivers so as to secure a navigable channel having at low water a depth of 25 feet from Portland to the mouth of the Columbia, was called for by the river and harbor act approved September 19, 1890. The report upon



The following text is extremely faint and illegible due to low contrast and poor image quality. It appears to be a list or a series of entries, possibly containing names, dates, or technical specifications. The text is arranged in several columns and is difficult to decipher.

ished April 30. This is all the permanent work in the Willamette River, called for by the project. Since their completion the dikes have been covered by back water from the Columbia. Under these conditions these dikes will have little or no effect upon the channel. When the Columbia River recedes and they can direct the currents of the Willamette, favorable results may be expected.

In the Columbia River 11,750 feet of diking was built at Walker Island. This was completed May 28.

The 15,000 feet of diking at Snag Island in Cathlamet Bay, called for by the project, is now under contract by the Port of Portland, and in process of construction.

In addition to the above work the Port of Portland has removed with the city dredge 32,725 cubic yards of material from the bar between the point of Swan Island and St. Johns, leaving at that place a channel 100 feet wide, 21 feet deep at low water; also 40,205 yards from Postoffice Bar, leaving a channel 150 feet wide, 21 feet deep; also 24,650 yards from the bar at Walker Island, leaving a channel 100 feet wide, 20 feet deep.

Of the work called for by the project, there are the additions to the present dike at St. Helens and the dike at Martin Island Bar that have not been commenced, although practically the whole of the dredging is yet to be provided for.

Estimates for the fiscal year ending June 30, 1894.—At the time of writing this report the effect of the work done under the project for obtaining a 25-foot channel from Portland to the sea by the Port of Portland can not be ascertained. The Columbia River is at its high summer stage and over the top of all the dikes, excepting those in process of erection in Cathlamet Bay. Whether this work will have to be strengthened or supplemented with other work in order to attain the object in view can not now be stated. The estimated cost of the work to be done under the project, that has not been entered upon by the Port of Portland, is \$303,600. Should \$150,000 of this amount be provided for expenditure during the fiscal year ending June 30, 1893, then it will be necessary to provide the remaining \$153,600 for carrying on the work during the fiscal year ending June 30, 1894, which amount can be profitably expended during that year.

Money statement.

July 1, 1891, balance unexpended.....	\$61,912.22
June 30, 1892, amount expended during fiscal year.....	43,752.94
July 1, 1892, balance unexpended.....	18,159.28
July 1, 1892, amount covered by uncompleted contracts.....	2,472.40
July 1, 1892, balance available.....	15,686.88
Amount appropriated by act approved July 13, 1892.....	150,000.00
Amount available for fiscal year ending June 30, 1893.....	165,686.88
{ Amount (estimated) required for completion of existing project.....	153,600.00
{ Amount that can be profitably expended in fiscal year ending June 30, 1894.....	153,600.00
{ Submitted in compliance with requirements of sections 2 of river and harbor acts of 1866 and 1867.	

steamers plying on the Upper Willamette, Lower Willamette, Columbia, and Cowlitz rivers, Oregon and Washington.

Name of vessel.	Tons.	Draft.	Name of vessel.	Tons.	Draft.
		<i>Ft. In.</i>			<i>Ft. In.</i>
o. 2.....	150.51	2	Manzanilla.....	129.87	3
.....	41.28	4	Maria.....	184.58	3
.....	85.26	1 8	Muscot.....	199.46	3
.....	213.52		Mayflower.....	23.90	
.....	189.98	2 3	Messeng.....	54.53	
.....	6.61		Michigan.....	597.19	13 6
t.....	20.35	10 9	Milwaukee.....	29.70	2
.....	76.16		Modoc.....	337.55	5
.....	234.24	2 6	Northwest.....	301.98	1 3
.....	40.90		No Wonder.....	235.30	3
.....	4.99		N. S. Bentley.....	401.42	1 6
.....	376.94	6	O. K.....	47.76	4 6
ahaw.....	1.05		O. & C. R. R. Ferry.....	255.92	
.....	14.20		Orient.....	429.76	5
ch.....	10.64		Ocean Wave.....	507.34	
.....	17.07		Ocklahoma.....	394.19	6
alama.....	106.45		Oregon.....	1,642.28	20
Frankfort.....	2.33		Oswego.....	21.43	3 6
.....	183.47		Queen.....	23.75	
a.....	1,746.14	20	Quinnat.....	4.20	
.....	43.55		R. P. Elmore.....	42.76	8
er.....	19.96	5 6	Regulator.....	334.88	
ity.....	566.23		Ramona.....	114.14	
a.....	296.38		R. R. Thompson.....	912.06	10 6
.....	13.47		Rowena.....	4.50	
.....	60.36		Rush.....	12.38	
er.....	25.03		R. Miller.....	41.39	
yer.....	24.44		R. C. Young.....	84.85	
.....	37.01	7 6	Sarah Dixon.....	278.84	
.....	82.38		Sakana.....	8.84	
.....	16.51		State of California.....	1,260.06	20
ayward.....	420.54		Stark St. Ferry.....	174.75	2 10
se.....	456.57		Stark St. Ferry No. 7.....	399.36	2
o. 2.....	137.41		S. G. Reed.....	607.28	8
ight.....	72.92	12	Salem.....	240.08	1 8
.....	7.93		Salem No. 3.....	36.00	1 6
.....	276.41	5	Sea Foam.....	4.95	
.....	32.09	5	T. J. Potter.....	589.60	9
hance.....	71.17		Tacoma.....	1,311.81	
aver.....	276.15	2 4	Totwa.....	6.27	
Canby.....	44.48	9	Telephone.....	413.24	4
r Newell.....	134.43	1 6	Three Sisters.....	327.33	1 1
Moon.....	56.15	1 6	Toledo.....	206.72	1 5
Queen.....	697.04	6	Tonguin.....	18.61	5 6
.....	129.58	2 9	Undine.....	280.48	3
.....	213.40	2	Victorian.....	809.17	
.....	62.03		W. H. Harrison.....	52.86	
est.....	36.84		Wenona.....	34.67	
.....	7.68	5	Western Queen.....	74.72	
ey.....	194.74	4	Willamette Chief.....	523.92	6
t Stephens.....	26.40	5 4	Willapa.....	249.52	
tellog.....	272.12	2 2	Willowa.....	92.05	
is.....	9.96		Wm. M. Hoag.....	451.13	1 6
vaugn.....	21.64	6	Winnington.....	752.07	
.....	23.72	10	Young America.....	42.10	
.....	338.38	6			

U U 4.

IMPROVEMENT OF WILLAMETTE RIVER ABOVE PORTLAND, OREGON.

The project for this improvement was adopted in 1878. It consists of dredging operations, bar scraping, contraction of water over shoals, rock removal, with the object of giving easy navigation for light-boats from Portland to Eugene City, Oregon, a distance of 172 miles.

The mouth of the Yamhill River, 40 miles above Portland, was the scene of an inconvenient low-water navigation in a draft of 2½ feet; a draft of 3 feet could be carried above.

During the low water of last season boats drawing 18 inches draft could ascend as far as Corvallis.

The total amount appropriated for this work to the end of the year since the present project was adopted is \$124,000. At the commencement of the present fiscal year there was a balance on hand \$6,464.42.

Work done during the season.—The work done during the season consisted of snagging operations, covering the river from Portland to Gram Bend, 2 miles below Harrisburg, a distance of 150 miles, a concentrating the water over shoal places by closing chutes and building wing dams. This work was done by the crew of the United States snag boat *Corvallis*. The boat was outfitted early in July with usual snagging and bar-scraping appliances for shallow streams; in addition it was provided with a moveable set of gins and a hammer pile-driving purposes. It left Portland on the 6th of July in command of Capt. George Raabe, who for many years has been engaged in navigation upon this river and is thoroughly conversant with all its difficulties, and knew well where work with the limited funds available be done to the best advantage.

The boat proceeded up the river as far as the stage of water would permit, removing only the most troublesome snags. Arriving at the upper end of the course near Harrisburg July 20, it then dropped down making a more thorough cleaning out of the snags. The remainder of the month and the first week in August were occupied with snag work above Corvallis. A good channel was cleaned out in this region which will extend the period of navigation here fully a month.

The remainder of the season, which closed September 20, was spent in operations below Corvallis. In the vicinity of Weston there were several places particularly troublesome at low-water stages; these were improved by building low dams made by driving short piles into sand to a depth of 5 or 6 feet at intervals of from 6 to 10 feet and filling against these small logs and brush weighted down with gravel, rock, or whatever heavy material might be near at hand. At Chuman Bar a wing dam of this character was built 500 feet in length, also one at Brentano Slough 215 feet in length; also one at Cand Bar 225 feet long. The result of these was to concentrate the water and give immediate relief in this part of the river.

During the season there were 577 snags removed.

The work done by Capt. Raabe seemed to give satisfaction to the men engaged with him in the navigation of the river. It resulted in giving 2½ feet at low water as high up as Salem, and extending the period of navigation above that point fully a month. This work, however, is temporary, and in the nature of things much of it may have to be done over again.

The whole of the funds available for work under this appropriation was not expended at this time, when it might have been to good advantage to the upper portion of the river. A small proportion was left in reserve to relieve any urgent necessity that might arise upon the river below Oregon City.

Estimate for the fiscal year ending June 30, 1891.—If these wing dams and dams at the heads of chutes which are put in to serve the purpose of temporary emergencies during low water could be made stronger and more permanent so as to resist the high-water flow, more satisfactory progress could be made in the improvement of this river. In order to provide for the immediate and urgent needs of navigation during low water, the small amounts usually appropriated must be spent in the manner described in temporary work at many places. An appropriation of \$60,000 in one sum would permit this to be made permanent, and after that yearly appropriations of from \$10,000 to \$12,

would keep the river in reasonably good boating condition during the low-water season.

The amount of commerce on the river and the economical prosecution of the work to be done seem to justify an appropriation of at least \$60,000 for the fiscal year ending June 30, 1894.

APPROPRIATIONS.

Act of—		Act of—	
March 3, 1871	\$16,000	March 3, 1881	\$15,000
March 3, 1873	3,000	August 2, 1882	5,000
June 23, 1874	7,500	July 5, 1884	10,000
March 3, 1875	25,000	August 5, 1886	10,000
August 14, 1876	20,000	August 11, 1888	20,000
August 18, 1878	20,000	September 19, 1890	11,000
March 3, 1879	12,000		
June 14, 1880	12,000	Total	195,500

Money statement.

July 1, 1891, balance unexpended	\$6,464.42
June 30, 1892, amount expended during fiscal year	3,350.49
July 1, 1892, balance unexpended	3,113.93
Amount appropriated by act approved July 13, 1892	30,000.00
Amount available for fiscal year ending June 30, 1893	33,113.93
{ Amount that can be profitably expended in fiscal year ending June 30, 1894	60,000.00
{ Submitted in compliance with requirements of sections 2 of river and harbor acts of 1866 and 1867.	

COMMERCIAL STATISTICS.

This river is in the collection district of Willamette and Oregon. The ports of entry are Portland and Astoria, Oregon. The nearest light-house and works of defense are at the mouth of the Columbia River.

Statement of Upper Willamette River traffic for year ending June 30, 1892.

The following figures are furnished by the transportation companies doing business on the river.

	Tons.
Passengers (111,566)	8,367
Grain	14,894
Lumber	818
Live stock	314
Coal	251
Wool	368
General merchandise	45,860
Total	70,872

U U 5.

IMPROVEMENT OF COWLITZ RIVER, WASHINGTON.

The project for the improvement of this river, which was adopted in 1882, contemplates the removal of sand bars, rocks, snags, overhanging trees, and other obstructions in the channel from its mouth to a point about 50 miles above. Work has been done to Toledo, 30 miles above the mouth. The ruling depth at low water prior to this was 14 inches. It is now 30 inches when the low water can be kept concentrated on one or two troublesome bars. The original estimate was \$5,000 for the first year and an annual expenditure thereafter of \$2,000 per year. The total amount that has been appropriated for this work is \$19,000.

* Of which \$3,000 is to be used in improving Yamhill River.

... was started on the river September 23, and ... operations extended to ... were removed from the chan ... of Turtle River Bar. I ... downstream an ... at that place ... the gravel, with logs an ... was done by there ... of a rise in th ...

... sent into the river again. Jur ... of wing dams ... the purpose of increasin ... the year, June 3 ... the balance of th ...

... and ... The dam at Olequ ... Northern Pacific Railwa ... getting the ...

... reported ... the it ... with the pro ...

U U 6.

IMPROVEMENT OF YOUNGS AND KLASKUINE RIVERS, OREGON.

The project for this improvement contemplates the removal of snags, sunken logs, and overhanging trees from Youngs River as far up as the lower end of the cut-off, a distance of 7 miles; and also from the channel of the Klaskuine as far up as Kamms Wharf, a distance of 2 miles. The latter river empties into the former 6 miles above Youngs Bay.

The estimated cost of this improvement was \$1,600. The total amount appropriated for the work \$1,600. The total amount expended \$1,206.79, which has resulted in a channel of 7 feet depth at high tide to the limits named. The balance now available, \$393.21, will be sufficient to complete the project. No further appropriation is asked.

Money statement.

July 1, 1891, balance unexpended	\$393.21
July 1, 1892, balance unexpended	393.21

U U 7.

GAUGING WATERS OF COLUMBIA RIVER, OREGON AND WASHINGTON.

The object of these gaugings is to ascertain and keep a record of the fluctuations of the Columbia River with the view to gathering information that may be useful in works of improvement on the river, and also by gauges established at various points to indicate to pilots, captains, and others interested in navigation the stage of water on crossings and places of difficult navigation.

A self-registering tide gauge has been in operation at Astoria, Oregon, and one at Cathlamet, Wash., during the entire year.

Daily sheets from the Astoria gauge were exhibited on a bulletin board in that city. These show the stage of water and condition as to roughness on the bar at the mouth of the river and are of great service to commerce.

It is estimated that \$1,500 can be profitably expended in this work during the fiscal year ending June 30, 1894.

APPROPRIATIONS.

August 2, 1882	\$500
July 5, 1884	1,000
August 5, 1886	1,000
August 11, 1888	2,500

Money statement.

July 1, 1891, balance unexpended	\$962.32
July 30, 1892, amount expended during fiscal year	516.00
July 1, 1893, balance unexpended	446.32

{ Amount that can be profitably expended in fiscal year ending June 30, 1894 1,500.00
 { Submitted in compliance with requirements of sections 2 of river and harbor acts of 1866 and 1867.

U U 8.

No. 20. Fifty-second Congress, first session.

IMPROVEMENT OF WILLAMETTE RIVER, OREGON, FOR
THE PURPOSE OF NAVIGATION AT CLACKAMAS RAPIDS AND ROSS
ISLAND, NEAR THE CITY OF CORVALLIS.

UNITED STATES ENGINEER OFFICE,

Portland, Oregon, November 19, 1890.

GENERAL: To comply with your letter of September 20, 1890, directing me to make preliminary examinations and reports upon certain rapids and bars, provided for in the river and harbor act approved September 19, 1890, with a view to ascertaining whether or not they are worthy of improvement by the General Government, I have the honor to report as follows upon—

Willamette River, for improvement of navigation at Clackamas Rapids and Ross Island and near the city of Corvallis.

The falls of the Willamette are located on that river about 12 miles above the city of Portland, Oregon. Around this immense water power there is gradually being built up large manufacturing establishments and commercial interests. There are also large interests invested along the river between the two places, notably the iron manufacturing establishment at Oswego. There is communication by water between Portland and Oregon City at Willamette Falls. At low water boats drawing more than 3 feet can not ascend to Oregon City, and those drawing more than 6 can not ascend to Oswego. In this portion of the river the two formidable obstructions to greater draft boats are the bars at Ross Island and Clackamas Rapids. The Ross Island bar is near the upper or southern limit of the city of Portland. It is caused by a widening of the river bed in this locality, which disperses the waters, diminishes the flood velocities, and causes a deposition of the heavy material carried along. It will be practicable to improve this bar within reasonable limits of expense to such an extent that vessels drawing 10 feet or more can pass over it at low water. This would be of great assistance to the large interests that are being developed along this reach of the river.

The river and harbor act approved September 19, 1890, appropriates \$11,000 for improving Willamette River above Portland, "of which sum \$5,000, or so much thereof as may be necessary, may be used in the district of the Secretary of War, for improvement of the river at Clackamas Rapids and Ross Island." In my project for the expenditure of the amount thus appropriated, which was submitted under date of October 16, 1890, and has been returned to me approved, provision is made for a survey of the Willamette River in the vicinity of the obstructions. The field work of this survey has been done, and its results are given in the report of the Chief of Engineers, U. S. Army, at Corvallis, Oregon, dated November 19, 1890, and will be submitted to the General Government. The results of the examination of the Ross Island bar and Clackamas Rapids will be submitted to the General Government as soon as they are completed.

W. L. G.

additional surveys and investigations will be necessary at this order that plans and estimate of cost of the improvement may be made. For this purpose I recommend that an allotment of \$400 be made from the funds available for examinations and surveys. The present commerce of the river is about 50,000 tons. As it is invested at the falls and at other localities along the river this commerce will of course increase, and this increase will be more or less, according as the river is in a more or less navigable condition.

The city of Corvallis is located on the concave bank of a sharp bend in the Willamette River. For some years past the river has been eroding the banks in the bend next above this and has been threatening a cut-off which if permitted to take place would leave Corvallis some 2 or 3 miles from the river. A portion of this eroded bend has been properly surveyed by the General Government at a cost of \$12,500. During the session of the Willamette of last February the Corvallis Board of Trade was greatly alarmed lest this cut-off would then take place, as a large volume of water was escaping through the fields and across the land opposite the city. Some efforts were then made to obtain legislative action in the matter, but nothing could be done. The city is not so far as it extended protected the bank, but erosion continues above it. It is necessary in the interests of the commerce of the Willamette River that this locality receive further special attention at the hands of the Government. Before deciding what is best to be done a detailed survey covering about 5 miles of the river should be made to determine what is the extent of the change that has taken place here in recent years and what are the real probabilities of a cut-off. The data necessary to the formation of plans and estimates for the work required should be collected. For this work there should be an allotment of \$1,000.

The Willamette River at Clackamas Rapids and Ross Island and the city of Corvallis I believe, for reasons stated herein, to be in need of improvement by the General Government, and so report, having made personal examination at these localities.

I am, very respectfully, your obedient servant,

THOS. H. HANDBURY,
Major, Corps of Engineers.

Gen. THOMAS L. CASEY,
Chief of Engineers, U. S. A.

Through Col. G. H. Mendell, Corps of Engineers, Division Engineer, (Division.)

[First indorsement.]

U. S. ENGINEER OFFICE,
San Francisco, Cal., November 21, 1890.

Respectfully forwarded.

The Willamette River at the three points named—Clackamas Rapids, Ross Island, and Corvallis—is considered to be worthy of improvement.

G. H. MENDELL,
*Colonel, Corps of Engineers,
Division Engineer.*

REPORT FOR THE IMPROVEMENT OF WILLAMETTE RIVER AT ROSS ISLAND, OREGON.

UNITED STATES ENGINEER OFFICE,

Portland, Oregon, September 30, 1899.

GENERAL: In accordance with your instructions based upon the provisions of the river and harbor act approved September 19, 1897, I have the honor to submit the following project for the improvement of the Willamette River at Ross Island. This island is located near S. W. Washington, 1 1/2 miles of Portland, where the river suddenly widens to 1/2 mile, but is restricted and then becomes contracted again to 1/2 mile within a distance of about 2 miles. There are here two islands which divide the river bed into three channel ways, each about 800 feet wide. Across the head of these there is a low flat gravel bar. At these low stages all the water discharged by the river passes down the west channel way. At stages above this the other two carry off portions of the water, and at extreme high stages the island is flooded. As a result of these topographical conditions the depth of water available for navigation is not as great in this portion of the river as in the reaches below, and immediately above where 18 feet a water may be considered as the ruling navigable depth. In the channel where all the water is to be found during the low stages is a shoal bar which limits the draft of vessels to 7 feet.

In this locality the high water of February, 1890, which is the highest of authentic record, reached a height of 30 feet above extreme low water. The volume of discharge at low-water stage is approximately 100,000 cubic feet per second. The extreme rise and fall of low water is about 8 feet.

The nature of the river bed in this locality is so irregular and so highly indurated, offering great resistance to excavation, that the removal of the shoals of stones of coarse sand and gravel is impracticable.

It is proposed to improve this locality by a construction of a dike or dam, 1/2 mile long, which will extend map cover to the west channel way, and will be situated between the surveys made in the 1850's and the surveys made in the 1880's, the latter being the most recent having a bearing of 100 degrees. The dike to be constructed by the surveys was situated between the surveys of 1850 and 1880. A dike from the new survey to the old survey will be constructed. By comparing this with the plan of the dike as shown on the map of the Chief of Engineers, Portland, Oregon, 1888, the changes that have taken place in the sand and gravel have filled in many of the channels and the east channel ways, and the dike will be raised to low water. As a consequence of the dike the water will pass through the west channel, which is a narrow channel, and will be dammed. Although the material here is indurated, it is evident that a considerable erosion has taken place, and this is shown by the exact elevation of the dike as shown on the map. The ruling depth of the river is 7 feet.

The dike will allow vessels that now have occasion to go above the dike to go below it. The Willamette River at this point is subject to the disturbance occasioned by backwater from the rise of the Willamette River from the latter part of May to the August. The extreme of this rise has reached 28 feet. The dike is to be built on the watershed of the Willamette, at N. W. 1/4 Sec. 10, T. 10 N., R. 10 W., No. 28, Forty-second Congress, dis-

ly about the end of December. The extreme of this flood in this locality 30 feet. Before the river can reach its extreme again, except in case of a sudden freeze up about its head subject to backwater from the rise in the Columbia. There are four months in the year when vessels of the draft mentioned go above this shoal.

A channel having a depth of 14 feet at extreme low water can be made, this would doubtless meet all the necessities of the present. The project that I recommend for this is to close the present channel ways or chutes to a height of 4 feet above low water to allow all the water that now passes down these to that height to flow through the new channel; and to assist the erosion that must follow this to the extent that will be necessary to form a cut 100 feet wide and 14 feet deep. The material of which this shoal is composed is compact gravel, the erosion will be slow and the dredging difficult than in the ordinary sand bars. The additional benefit that it is proposed to throw down this channel will maintain the depth to which we may dredge and cause it to improve. These two chutes convey, practically, no water at low stage in the river. At a 4-foot stage they convey fully one-half the total discharge, and at higher stages a larger proportion of the total velocity through the west channel with these chutes is nearly double what it now is at this stage. The dam closing these chutes will be about 3,000 feet long and as shown upon the tracings herewith. The detailed drawings of the channel that it is proposed to dredge are also shown on the next sheet. It is estimated that the dredging to be done to obtain 14 feet at low water will be approximately 26,000

cubic yards above the head of Ross Island and in front of the shoal there is a sand bar over which at low water a ruling of 14 feet is found. The material of this is sand, and can be more easily removed than that which forms the obstruction below. A cut 100 feet wide in length will be necessary here. To obtain a channel 100 feet wide and 14 feet deep it will be necessary to remove 12,000 cubic yards. It is evident that the river bed at this point is too wide for the natural flow of the water to maintain a channel as deep as 14 feet. Dredging will give relief, but to maintain this depth through the bar it may be necessary to build a contracting dike 100 feet in length from one or the other shore in this locality. A rough estimate of the cost of the work here proposed is sub-

Excavation, at \$10.....	\$30,000
26,000 cubic yards, at 50 cents.....	13,000
200 cubic yards, at 25 cents.....	3,000
Material, at \$12.....	4,800
	50,800
	5,100
	55,900

sequent improvements completed vessels of the class considered on the Willamette River to Jennings Bar, which is 10½ miles from Street bridge, Portland. The foot of Clackamas Rap-

... of 12 1/2 miles below the fall ... interests that will be ... 0.8 to 20.8 miles above P ... the output of the ... amounts to 75,000 ... to permit deep-sea vessel ... to improve a large pro ... products of this con ... from the works. I ... of lightering nu ... to the extent the ... there is every reason to believe ... to the advantage and developmer ...

The quantities of lumber at present obtained for the mouth of the Columbia River are located near Oswego. It is expected that during the present year between 150,000 and 200,000 cords will be used. This is transported in barges drawing 9 feet w water depth. Any improvement of the low-water depths over the present will be of great benefit to this work. The commerce that will be benefited by this improvement would seem to warrant that it be undertaken.

Very respectfully, your obedient servant,
THOS. H. HANDBURY,
Major, Corps of Engineers.

Brig. Gen. THOMAS L. CASEY,
Chief of Engineers, U. S. A.
(Through Col. G. H. Mendell, Corps of Engineers, Division of the Pacific.)

[Enclosures 2.]

U. S. ENGINEER OFFICE,
San Francisco, Cal., October 10, 1881.

Respectfully forwarded to the Chief of Engineers, U. S. Army.

G. H. MENDELL,
Colonel, Corps of Engineers, Division of the Pacific.

STATE OF OREGON, COLUMBIA RIVER AT CLACKAMAS RAPIDS.

UNITED STATES ENGINEER OFFICE,
Portland, Oregon, September 10, 1881.

... of the ... following ... will be ... to the ... of the ...

is in the discharge section and the meeting of the current from Clackamas. This, of course, causes a deposition of the material carried along by both streams. The Willamette at its lower end has not sufficient volume or velocity to carry this material farther.

The result is a submerged gravel dam over which boats navigating the river must climb in their passage up the river. The presence of this dam has increased the low-water slope of the river from 12 inches per mile, the average from Portland to the foot of the rapids, to 15 inches and 5 inches per mile, over a distance of about 9,000 feet. The low-water slope from Portland to Oregon City is $7\frac{1}{2}$ inches per mile. The Clackamas River, coming in nearly at right angles to the Willamette, has a slope of $12\frac{1}{2}$ feet per mile. We have no data concerning the volume of its discharge. At low water it is insignificant, but during the rainy season its volume is considerable, bringing down large quantities of heavy gravel, as the results show. The low-water discharge of the Willamette is about 15,000 cubic feet per second.

In the fall of the year 1851 an attempt was made to improve navigation at this rapid by building a dam from the left bank at the head of the rapid and extending it out into the stream so as to force the Willamette water over the right bank where the Clackamas enters, with the design to establish a channel down that side. This resulted in an entire failure, and river navigation terminated at this point until a part of the dam was removed. The dam was made of brush and gravel, and paid for by funds raised by subscription among the citizens of Oregon City. The amount of debris already too much dam in the Willamette caused by the debris it down by the Clackamas. From time to time portions of this dam, which is now entirely submerged at low water, have been removed. The opening through it is now about 200 feet in width with a depth of $3\frac{1}{2}$ feet at low water. Boats of ordinary power ascend this rapid at all stages of the river. At the extreme low stage considerable debris is experienced and sometimes the process of lining has to be resorted to.

As a consequence of this debris brought in by the Clackamas a pool is formed in the Willamette River which extends almost a mile up to the foot of the Willamette Falls. These falls are surmounted by a series of locks, having a lift of 10 feet each. These locks and the water of the falls are owned and operated by a corporation existing under the laws of the State of Oregon. The lower miter sill of these locks is located with reference to the low water in the pool so as to give a depth of 3 feet over it at that stage. Were the whole of this gravel removed, or even so much of it as would accommodate the low-water stage of the river on the same slope that it has from Portland to the foot of the rapids, there would result a lowering of the level of the river at low-water stages which might interfere with the passage of boats over the lower lock. This, however, will not be necessary in order to improve the navigation for all present wants of commerce.

The present low-water channel of the Willamette River through the rapids part of this bar has an average width of about 300 feet with a depth of about 1 on 1,000. Its mean depth is 8 feet, excepting in the section at the summit of the rapid where the old dam is located. The depth here is 500 feet. This channel seems to be all that the low-water stage of the Willamette can maintain against the encroachments of the debris water from the Clackamas.

The problem of improvement resolves itself into two distinct parts, to provide a good low-water channel for the Willamette without affecting the depth of water on the miter sill of the lower lock at Oregon

The removal of the old dam at the head of the rapids can be effected by charges of dynamite; at the same time the gravel in which it is embedded is loosened and washed away. The cost of this work it is difficult to accurately estimate. It may be approximately given at \$3,000. The total estimate of the cost of the improvement in accordance with the project is as follows:

Removal of the old dam.....	\$3,000
Construction of 6,800 feet dike, at \$10.....	68,000
	71,000
Contingencies.....	7,000
	78,000

The traffic to be benefited by this improvement as shown by the information furnished for the year ending June 30, 1891, by the transportation companies doing business on this portion of the Willamette River, was 116,600 tons.

Very respectfully, your obedient servant,
 THOS. H. HANDBURY,
Major, Corps of Engineers.

Brig. Gen. THOMAS L. CASEY,
Chief of Engineers, U. S. A.
 (Through Col. G. H. Mendell, Corps of Engineers, Division Engineer,
 Pacific Division.)

[First indorsement.]

U. S. ENGINEER OFFICE,
San Francisco, Cal., November 2, 1891.

Respectfully forwarded.

The object of the construction is to store in the Willamette River bed gravel brought down by the Clackamas, which is a large stream in freshet, with a steep slope. After a period longer or shorter, perhaps a few years, storage room will be exhausted. A preferable plan, if practicable, is to store the gravel before it reaches the Willamette. It does not appear that an examination showing whether or not this system is practicable has been made. It ought to be made before the project is carried out. An examination may require modification of the project. Subject to this consideration the project is recommended.

G. H. MENDELL,
*Colonel, Corps of Engineers,
 Division Engineer.*

SURVEY OF WILLAMETTE RIVER NEAR CORVALLIS, OREGON.

UNITED STATES ENGINEER OFFICE,
Portland, Oregon, October 30, 1891.

GENERAL: The river and harbor act approved September 19, 1890, provides that an examination and survey be made with the view to improving the navigation of the Willamette River, near the city of Corvallis, Oregon, and also that an estimate of the cost of the necessary improvement be submitted. This duty having been assigned to me by your letter of October 21, 1890, I now have the honor to submit the following report* thereon:

* Map accompanying this report not printed.



The first part of the document discusses the importance of maintaining accurate records of all transactions. It emphasizes that every entry should be supported by a valid receipt or invoice. This ensures transparency and allows for easy verification of the data. The text also mentions that these records are essential for tax purposes and for identifying any discrepancies or errors in the accounting process.

The second part of the document focuses on the role of the accounting department in providing accurate financial information to management. It states that the department should regularly analyze the company's financial performance and provide detailed reports. These reports should include key financial ratios and trends, which can help management make informed decisions about the company's future. The text also highlights the importance of maintaining a strong relationship with external auditors to ensure the reliability of the financial statements.

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The fourth part of the document focuses on the role of the accounting department in providing accurate financial information to management. It states that the department should regularly analyze the company's financial performance and provide detailed reports. These reports should include key financial ratios and trends, which can help management make informed decisions about the company's future. The text also highlights the importance of maintaining a strong relationship with external auditors to ensure the reliability of the financial statements.

The fifth part of the document discusses the importance of maintaining accurate records of all transactions. It emphasizes that every entry should be supported by a valid receipt or invoice. This ensures transparency and allows for easy verification of the data. The text also mentions that these records are essential for tax purposes and for identifying any discrepancies or errors in the accounting process.

with the view to catching drift and checking the velocity of the water when it reached the overflow stage, the idea of this being to assimilate the conditions existing before the trees and underbrush were cut away. The effect of the high water of February, 1890, upon this, and the resulting scouring action of the water in its immediate vicinity, were not such as to warrant the further construction of works of this character. Above the upper end of the revetted portion of the bank there is a hard gravel beach for a distance of about 1,400 feet. This extends back from low water from 50 to 100 feet; then comes the bluff bank from 8 to 10 feet high, eroded more or less at high water. In this locality is the head of a low place in the land through which the water flows at ordinary high stages. Above this for a considerable distance the same gravel bed seems to underlie the bluff bank and protect from erosion except at the higher stages of water. The profile along this low swale shows that the water commences running through it at a 10-foot stage. At that stage the water is $2\frac{1}{2}$ feet above the top of the bank across the lower end of the slough and is backed up to within about 1,700 feet of the point of overflow at the upper end. The fall is then 10 feet. At the low-water stage the surface of the water in the river is on a level with a point 3,500 feet below the head of the swale. This seems to be filled about the head with coarse heavy material brought in during high water. The evidence that this is enlarging or growing deeper is not conclusive.

The total amount expended by the United States in protecting this bend was \$12,496.

From a study of the results of this survey and a personal visit to the ground during high water and again at low water, I can not conclude that there is any danger that the channel of the river at low water will pass over this neck within any reasonable time in the future, or that the city of Corvallis will be deprived of its advantage of water transportation. At high stages of the river the water will continue to pass over these bottom lands as it has done in the past, and no amount of money within reason will prevent it.

The present condition of the river in this vicinity as regards navigation may be classed as very good when compared with many other localities along its course where no improvement is contemplated.

In the first bend above Corvallis we find the lower end of a chute that leads across a narrow neck to the river above. The length of this is 3,000 feet. At a 5-foot stage the water commences to run through this chute. The difference of level in the river at this stage between the head and foot of the chute is 8 feet. It is at present very much obstructed with fallen trees and driftwood. If this were cleared out and the trees and underbrush along its banks cut down for some distance back, it would gradually enlarge and serve as an escape during ordinary high stages for a large portion of the water that now passes down around the bend and over the bottom land in front of Corvallis. This fall of 8 feet in a distance of 3,000 feet will give a very swift current through the chute when it is cleared of the obstructions that now encumber it. The sand and gravel at its head may be expected to be washed away and in time the low-water flow of the river may take this course. This will produce a radical change in the regimen of the river in this vicinity and give great relief to the bend below. Navigation during the low-water stages will probably not be so convenient when this takes place as at present, but in time as the head of the slope is washed down it will be less steep and the velocity of the water diminished.

To the extent of clearing out this chute as indicated above
and that work be done in this locality, and estimate the co
nsumption of \$1,000.

Very respectfully, your obedient servant,

THOS. H. HANDBURY
Major, Corps of Eng

Bvt. Gen. THOMAS L. CASBY,
Chief of Engineers, U. S. A.

Through Col. G. H. Mendell, Corps of Engineers, Division I
(Pacific Division.)

[First Indorsement.]

U. S. ENGINEER OFFICE
San Francisco, Cal., November 17

Respectfully forwarded.

If clearance of the chute A B shall result in a cut-off,
though its length of 3,000 feet will be 8 feet, being at the rate
per mile, whereas the average fall per mile is about 2½ feet. V
then expect development of increased length by erosion of banks
great disturbance of the channel. Indeed, if the new river sh
lower its slope by development of length it would become unn
at the cut-off.

It is not clear that the advantages to be secured can compen
the disturbance to result if the cut-off is to be a result.

It is stated by Major Handbury, in a letter of date subsequent
of report, that indications point to a shifting of the low-water
from the middle to the east branch. If this shall occur or if it
certain that clearance of the chute will not make a cut-off, th
tions herein named will disappear.

Subject to these qualifications the project is recommended.

G. H. MENDELL
Colonel, Corps of Engineers
Division Engineer

U U 9.

PRELIMINARY EXAMINATION AND SURVEY OF THE LOWER W
METTE AND COLUMBIA RIVERS, OREGON, WITH A VIEW OF SEC
5 FEET AT LOW WATER FROM PORTLAND TO THE MOUTH O
COLUMBIA.

(Printed in House Ex. Doc. No. 33, Fifty-second Congress, first session.)

OFFICE OF THE CHIEF OF ENGINEERS,
UNITED STATES ARMY,
Washington, D. C., October 16, 1891

SIR: I have the honor to submit herewith the accompanying
of report dated September 8, 1891, by Maj. Thomas H. Hand
Corps of Engineers, giving results of a survey of the Lower Willa

and Columbia rivers, Oregon, with a view of securing 25 feet at low water from Portland to the mouth of the Columbia, made to comply with provisions of the river and harbor act approved September 19, 1890. A copy of his report of October 6, 1890, of the preliminary examination of the locality is also herewith.

Inviting attention to the accompanying reports of October 14, by The Board of Engineers, and of October 5, by the division engineer, Col. G. H. Mendell, Corps of Engineers, on Major Handbury's report, I have to recommend that the project of Major Handbury, as amended by The Board of Engineers, be submitted to Congress as the project for the improvement of the navigation of the Willamette and Columbia rivers between Portland and the Pacific Ocean, required by the act of September 19, 1890.

The project provides for a channel 25 feet deep. The estimated cost is \$772,464.

Very respectfully, your obedient servant,

THOS. LINCOLN CASEY,
Brig. Gen., Chief of Engineers.

Hon. REDFIELD PROCTOR,
Secretary of War.

PRELIMINARY EXAMINATION OF THE LOWER WILLAMETTE AND COLUMBIA RIVERS, OREGON, WITH A VIEW OF SECURING 25 FEET AT LOW WATER FROM PORTLAND TO THE MOUTH OF THE COLUMBIA.

UNITED STATES ENGINEER OFFICE,
Portland, Oregon, October 6, 1890.

GENERAL: To comply with your letter of September 20, 1890, directing me to make preliminary examinations and reports upon certain rivers and bays with a view to their improvement, provided for in the river and harbor act approved September 19, 1890, I have the honor to report as follows upon—

The Lower Willamette and Columbia rivers, Oregon, with a view to securing 25 feet at low water from Portland to the mouth of the Columbia.

I am familiar with the portion of these rivers mentioned in this item and can safely say that in my opinion they are worthy of being improved as proposed by the General Government. Portland is the principal city of the northwest and is located on the Willamette River, 12 miles from its entrance to the Columbia and 110 miles from the Pacific Ocean. It is at the head of deep-sea navigation on these rivers and has a large and extensive foreign and domestic commerce. A project for the improvement of this reach of these rivers so as to give an available low-water channel of 20 feet has been in process of execution for some years and it is now about completed. Prior to the commencement of this improvement the ruling depth was 12 feet and at that time the depth of water on the bar at the mouth of the Columbia River was from 18 to 20 feet at low water. By reason of the improvement recently effected on the bar, this depth is now 25 feet, with a fair prospect that it will be 30 feet when the works of improvement are finished. The necessities of the commerce and trade of Portland are such that the deepest-draft vessels passing the bar at the mouth of the Columbia River should be permitted to pass to and from that port without light-erage. A low-water depth of 25 feet throughout this reach of 110 miles

2832 REPORT OF THE CHIEF OF ENGINEERS, U. S. ARMY.

is a matter of very easy engineering attainment, and I think can be obtained at very reasonable expense.

A study of the problem has already been entered upon under instructions of January 11, 1890. For the purpose of continuing surveys and investigations now in progress, I request that I be all from the appropriation made by this act for such purpose, the sum \$1,500.

Very respectfully, your obedient servant,

THOS. H. HANDBURY,
Major, Corps of Engineers

Brig. Gen. THOMAS L. CASEY,
Chief of Engineers, U. S. A.

(Through Col. G. H. Mendell, Corps of Engineers, Division Engineer Pacific Division.)

[Second indorsement.]

U. S. ENGINEER OFFICE,
San Francisco, Cal., February 10, 1891

Respectfully returned to the Chief of Engineers.

The Lower Willamette and Columbia rivers are worthy of improvement.

G. H. MENDELL,
Colonel, Corps of Engineers, Division Engineer

SURVEY OF THE LOWER WILLAMETTE AND COLUMBIA RIVERS, OREGON, WITH A VIEW OF SECURING 25 FEET AT LOW WATER FROM PORTLAND TO THE MOUTH OF THE COLUMBIA.

UNITED STATES ENGINEER OFFICE,
Portland, Oregon, September 8, 1890

GENERAL: The river and harbor act approved September 19, 1890, directs that a project shall be submitted, with an estimate of the cost of carrying the same into effect, for obtaining a navigable channel in the Lower Willamette and Columbia rivers from the city of Portland to the sea, having a depth of 25 feet at low water. Having completed duty which was assigned to me by your letter of September 20, 1890, I now have the honor to submit the following report.

The navigable water included in this project extends from the railway bridge in the city of Portland to the crest of the bar at the mouth of the Columbia River, a distance of 114 miles, measured along the line of the present ship channel. Of this distance 12 miles are in the Willamette River, the remaining in the Columbia.

There is in this reach an aggregate of 10 miles, over which it will be necessary to deepen the channel more or less before a depth of 25 feet at low water will exist. Four miles of this are in the Willamette River and six in the Columbia. The principal localities in which improvement in the present channel depths seem now to be necessary are in the vicinity of the head of Swan Island; between the lower end of Swan Island and the John and Post-Office Bar in the Willamette River; and in the vicinity of Martin Island, Walker Island, Cathlamet Island, and the lower Astoria in the Columbia River.

Since a comprehensive study of the subject of this improvement cannot be obtained until it was necessary to make a resurvey of certain portions of each of these rivers. Our latest complete survey of

Columbia River, between Cathlamet and Tongue Point, dated back to 1867-'68. In this reach the river is excessively wide, its channels variable, and shoals are numerous. This has been resurveyed, and great changes from the conditions shown by the old charts have been developed. For use in this work and also the establishment of harbor lines for the city of Portland, a carefully prepared map of the Willamette River, from its junction with the Columbia to the south boundary of the city of Portland, has also been made. Maps of the other localities where works of improvement contemplated by this project are necessary, have recently been made in connection with the work in progress under the present project to obtain a channel 20 feet in depth at low water. These, with the other information on the files of this office, give the data necessary for our present purposes.

An examination of these maps reveals the cause of the shoal water in these several localities and at once suggests the general principle on which the plan of improvement should be based. In the Willamette River below Portland, wherever the width at low water is greater than 1,400 feet, the available channel is less than 25 feet in depth; and in the Columbia River as far down as the head of Cathlamet Bay, wherever this width is greater than 4,000 feet, the channel depth is less than 25 feet. Throughout Cathlamet Bay and from its head to the ocean, where an excessive width pertains with numerous sand bars, the ebb and flood of the tide play an important part in determining the location and depths of the channels. To examine the localities minutely where improvements are contemplated, we will commence at the Swan Island Obstruction and take them in their order going down. At the railroad bridge, $1\frac{3}{4}$ miles above the head of Swan Island, the low-water surface of the river is contracted to a width of 600 feet. At this point the water is so encumbered with wharves and other obstructions that there is practically no increase of width as the river rises. The average low-water depth here is $65\frac{1}{2}$ feet, with a maximum of 80 feet. Below this point the river gradually widens and the depth becomes less. At the distance of 1 mile the width of 1,400 feet is reached, with an average depth of 19 feet and a maximum of 34 feet. One-fourth of a mile farther down the maximum depth of 25 feet is reached, and the low-water width of 1,600 feet, and here is where the first trouble begins. At the head of Swan Island the low-water shores are 3,800 feet apart. The water passes the island in two channels. The south channel is wide and shoal, having scarcely 2 feet at low water. The north, which is the main ship channel, is narrow, but has ample depth after the head of the island is passed. Early surveys show that the south channel had from 5 to 7 feet of water in it. This has gradually shoaled up to its present depths. Its low-water cross section is now 2,600 square feet. It is now practically a high-water chute. The cross section of the north channel at the same stage is 11,140 square feet, or four times as large as the south channel. At a 6-foot stage these sections are about equal, each being 16,000 feet.

The plan adopted in 1877 for the improvement of this part of the river so as to obtain a channel having 20 feet depth provided for closing this south channel or chute to a height of 2 feet above low water, thus forcing all the water flowing in the river to that height down the north channel. The conditions with regard to the amount of money available and the pressing necessities for work at other points have at no time since then until now been such that the funds could be spared for closing this chute. In the meantime a channel having 18 and 19 feet depth has been repeatedly dredged through the bar at the head of

per cent more water down the north channel due to this, with the additional contraction from extending wharves out to the new bar in time open a channel of 25 feet depth. This being difficult to move, it may take some years by the unaided erosive powers of the water, in order to hasten the results after the dam is completed, it will continually improve until attained. The material to be removed in order 150 feet wide will amount to 125,000 cubic yards; in the channel the removal of 236,000 cubic yards; in the channel having a depth of 22 feet at low water to the width at which its natural forces increase, it will be necessary to incur the increased cost. There is an extreme rise of tide here at the water stages of the Willamette, which can be the event of the arrival of vessels drawing more than 22 feet.

Leaving this locality the next point at which the width of river, and with it a less depth than at the mouth of Swan Island and St. John. The width is now 2,400 feet. The velocity acquired during its passage through the north channel is sufficient to maintain a depth of 25 feet for a distance of 2,700 feet below this time its force has become exhausted by the excessive width. The tendency now is to narrow the lower end of the chute behind Swan Island when the upper end is closed to the height built from the right bank commencing about the lower end of the island inclining downstream and thence downstream 3,500 feet, keeping the water in a depth until it reaches the same depth below low water to be 4 feet above low water. The amount of material to be removed in order to maintain a depth of 25 feet for a distance of 2,700 feet below this time its force has become exhausted by the excessive width. The tendency now is to narrow the lower end of the chute behind Swan Island when the upper end is closed to the height built from the right bank commencing about the lower end of the island inclining downstream and thence downstream 3,500 feet, keeping the water in a depth until it reaches the same depth below low water to be 4 feet above low water. The amount of material to be removed in order to maintain a depth of 25 feet for a distance of 2,700 feet below this time its force has become exhausted by the excessive width. The tendency now is to narrow the lower end of the chute behind Swan Island when the upper end is closed to the height built from the right bank commencing about the lower end of the island inclining downstream and thence downstream 3,500 feet, keeping the water in a depth until it reaches the same depth below low water to be 4 feet above low water.

distance between the 25-foot depths above and below this bar is 11,000 feet.

To effect the desired improvement here I have to recommend that a dike be built perpendicularly from the right bank of the river 1,000 feet long, near the point where the 25-foot depth ceases on the upper side of the bar; a second one 3,000 feet below, 600 feet long, on the same side of the river a little below the head of Willamette Slough; a third one 1,200 feet below, still on the right bank, 400 feet long and inclining downstream, and that a training dike 4,500 feet long be built from the outer end of this downstream practically parallel to the present revetment of the concave bank in this locality and 1,300 feet therefrom. The locations of these are shown upon the tracing herewith. If, after construction, it be found that the distance between the first and second dikes is too great to effect the desired concentration throughout the space, a short intermediate dike can be supplemented. The expense of these will be much less than a long training dike built in 18 feet of water. Being on the convex side of the bend, much more satisfactory results may be expected from them than if otherwise located; these dikes, like the ones previously mentioned, to have a height of 4 feet above low water and to be similarly constructed.

In establishing the degree of contraction necessary to insure the 25-foot depth, I have assumed theoretically that the form of the river bed in cross section to be a segment of a circle having a versed sine equal to the depth required for the channel. Taking the area of the cross-section of the river as I find it I determine the length of the chord for this versed sine when the area of the segment is equal to that of the cross-section. For a 25-foot depth at low water I find that the sections in this locality average about 1,200 feet in width, and for a 29-foot depth at a 4-foot stage the average width is about 1,400 feet. These widths correspond very closely with what would be suggested by an examination of the map where a 25-foot channel is found under normal conditions.

The length of diking required at this bar is 6,600 feet. The amount of material to be excavated in order to obtain a low-water channel 150 feet wide and 22 feet deep is 200,000 cubic yards, and to obtain a 25-foot channel this amount is 375,000 yards.

The foregoing provides for the improvement of the Willamette River to its junction with the Columbia. Proceeding down the Columbia River the first locality that will require especial attention is the bar at St. Helens. Under the present project for obtaining a 20-foot channel a permeable jetty 5,500 feet long has been constructed at this point, having for its object the gradual building up of a sand bar behind it which will concentrate the water in the desired direction and increase its depth in the channel. This is slowly accomplishing its object, but before it can effectually open and maintain a channel having 25 feet depth some additional work will be necessary, looking to shutting off a large portion of the water that now escapes through the dike. This may be accomplished by sinking brush mattresses along the dike and holding these in place with stone. This new work should be brought up to the level of low water. Its cost is estimated at \$5 per linear foot, and there is probably 5,000 feet that would require this addition. The amount of material necessary to be excavated here to obtain a 22-foot channel is 12,000 cubic yards; and to obtain the 25-foot channel, 46,000 yards.

Three miles below St. Helens Bar we reach Martin Island Bar. Here the river has a width of 6,000 feet. A portion of the water, estimated to be about one-tenth, was formerly discharged through Burke and

These have been... closed by... dams which... some water over the... will be... To prevent... 25-foot channel to diminish this... done by constructing... the Oregon shore to the west... above low water. The... yards of material... the removal of 150,000...

s of improvement are... Walker Island. Here the... three channels through... depth of water. From... Saltington shore, to the east of... the Oregon shore to the west... head of the island and passes... has a ruling depth of... of the discharge of their... The average rise of tide here... has some weight in determining... The earlier charts of this... in this middle channel... in the two other channels... water passing down either... sort of constraint to prevent... intermediate shoal space... be improved. The channels... by the pilots and... and the sandy shores... in dark or foggy weather... of the Oregon side when...

25-foot channel is maintained... 1,000 feet below the... opposite side of this channel... at low water. The water that... at this point must be what... it begins to feel the influence of... Middle Channel. By restraining this... and contracting it within the... so that the 25-foot channel can... be done by constructing a... shore and 1,000 feet from it, extending... the crossing of the Middle Channel... channel is again found... thence to the head of Walker Island... After this dike is built... will take place on... towards. The... in its passage... to improve... of 6 feet... The amount of material...

Vertical text on the left margin, possibly a page number or reference.

in order to secure a channel having 22 feet depth is 80,000 cubic yards; and to secure 25 feet, 170,000 yards.

Below Walker Island a 25-foot channel may be found until the head of Cathlamet Bay at Jim Crow Point is reached. At this point, which is 29 miles from the crest of the bar, the river commences to widen rapidly, spreading out over numerous shoals separated by channels of varying depths. The width between the mouth of John Day River on the Oregon side and the mouth of Gray River on the Washington side where the widest part is found is 9 miles. Just below this, between Tongue Point and Portuguese Point, it contracts to 4 miles. The main volume of water at Jim Crow Point is divided into two parts, one going down the Washington shore and maintaining the present ship channel along that side for 5 miles farther down, where it crosses over to Tongue Point; the other on being deflected from Jim Crow Point continues in a southwesterly direction, maintaining a 25-foot channel until Snag Island is reached. Here its force is wasted by its being spread out over the adjacent shoals and through smaller channels. Formerly there were two well-defined channels here, one to the north, the other to the south of Snag Island, these joining again below the island. The north channel is now entirely closed, but an opening to the northward is still maintained farther down, through which a large volume of water escapes. At this point the South Channel is shoal. There is a space here of about 3,000 yards length with an average depth of 14 feet, leaving 11 feet to be dredged for a 25-foot channel and 8 feet for the 22-foot channel. After passing this, 25 feet can be carried to a point about 1 mile north of Tongue Point. There is a distance of about 1,000 yards here where dredging may be necessary to give the depth required. At Tongue Point there is a channel leading directly west, having for a short distance at a point near Fort Stevens a least depth of 18 feet, and another passing close around the point and along the shore in front of Astoria. This channel carries greater depths than 25 feet throughout, excepting for a short space just above Upper Astoria. Here the ruling depth is 20 feet.

Following the present ship channel from Jim Crow Point down it inclines south to round the shoal water in the vicinity of Pillar Rock, thence back to the Washington shore, which it follows to the crossing, thence over to Tongue Point. The ruling depth here is practically 20 feet at mean low tide. The average rise of tide is 7 feet.

This channel has the great disadvantage of making a crossing in a wide place in the river bed where the sands are continually working down and changing its direction and capacity. Works erected with the view to holding it in any one position would at times have to contend against the natural inclinations of the river, and would therefore be expensive and uncertain in their results. It would be practicable to force this water to continue down the north shore, and improve the depth of water on that side, should the necessities of commerce warrant the expense. At present the problem is to obtain the depth of 25 feet to the sea by the most practical and economical route, and with the least disturbance of the present existing conditions. Taking the channels in this locality in their different positions as we now find them, and considering the present tendencies of their several courses, together with other facts and interests bearing upon the problem, it would seem best to restore and maintain the one passing to the south of Snag Island. If the water that is diverted by Jim Crow Point into the head of this channel can be continued down and kept from spilling out as it were over the adjacent shoals, and these shoals

o that of extreme tide and covered with grass. When this is done the water will be deep and confined to well-defined channels.

We have every reason to believe that at none of the localities where improvement is required will there be anything more formidable to remove from the desired channel than sand, with perhaps an occasional snag or stump buried in it. The undertaking is not formidable in its character nor are there any difficult abstract engineering problems to be solved. It is merely a question of time and money. If the funds necessary for the execution of the project can be supplied as they can be profitably expended the first results will soon be obtained. We are not wanting in examples from experience, which show clearly that if the funds are not so provided the cost will be greatly increased and the time of procuring the desired results will be greatly prolonged. In the meantime the advantages and profits of the improvement are being lost.

On the tracings which are forwarded herewith I have indicated the positions that seem to me to be the most proper in each case considered for the permanent works to occupy. I also transmit a sheet showing the characteristic cross sections of these works as I would recommend that they be built. These works are all of the same general construction differing only in strength and amount of material used per foot depending upon the forces to be resisted at the location where they are placed. They may be briefly described. Drive two parallel rows of piles to a depth of 12 feet into the bottom of the river. The tops of these to be 4 feet above low water, except at Walker Island where they are to be 6 feet. The rows to be from 4 to 6 feet apart, piles in each row 6 feet apart and each pile opposite an interval in the other row. Piles in each row to be fastened together at top with a waling piece bolted to each one, and also by boards at intervals of 8 inches spiked or fastened to them horizontally on inside, extending from top down to 2 feet below low water. Place one layer of 9-inch fascines 15 feet long between the rows of piles, hold down with rock. Place layers of fascines alternately perpendicular and parallel to rows of piles, to any desired thickness depending upon location; sink these with rock and pile rock on top to height necessary on each side; then fill in between rows of piles to top with fascines and weight down with rock. Hold the two rows of piles together by occasional pieces reaching across the interval and spiked to the waling pieces. In some localities a back brace to resist a tendency to overturn may be necessary. By assembling the material in this manner we can in all cases build a structure proportional in strength to the resistance to be overcome, and with the minimum amount of material. The piles serve as a nucleus around which to build and hold the brush and rock in place. Scouring is provided for and repairs are easily made, and additional strength when necessary is easily given.

The cost of these dikes will vary from \$5 to \$23 per linear foot, depending upon location. A careful estimate has been made for those required in the Willamette and Columbia rivers, separately, based upon the present ruling prices for labor and material and our experience in constructing similar works in this locality.

From the approximate estimate that I have made, it appears that about 400,000 cubic yards of sand will have to be dredged from the channel in the Willamette River and 600,000 from that in the Columbia to secure a 22-foot channel, making 1,000,000 cubic yards in all. The distance that any part of this is to be moved need not in any location exceed 2,000 feet.

To insure early and successful results and economy in carrying out the project herein proposed, it is advisable that the amount estimated for its completion be made available before it is commenced, and the erection of the permanent structures should in all cases precede the dredging. Should funds be provided in this manner, there is no doubt but a healthy competition could be engendered for doing the dredging and for delivering the material for the permanent structures in place.

My estimate for the cost of this work is as follows, and contemplates contracting works which will ultimately produce by the natural process of erosion a channel 25 feet in depth at low water; but to hasten this result and, by taking advantage of the tides, secure the benefits of this depth, I estimate for dredging to a depth of 22 feet at low water where this may be found to be necessary.

WILLAMETTE RIVER.

It is contemplated that the expense of closing the chute on the south side of Swan Island will be paid from the funds now available for work under the present project for improvement, and that this work will be done this fall and winter.

Dredging Swan Island Bar :	
125,000 cubic yards, at 15 cents	\$18,750.00
Between foot of Swan Island and St. John:	
Diking 4,000 feet, at \$10.....	40,000.00
Dredging 65,000 cubic yards, at 15 cents.....	9,750.00
Post-Office Bar :	
Diking 6,600 feet, at \$8.70	57,420.00
Dredging 200,000 cubic yards, at 15 cents.....	30,000.00
Total.....	155,920.00

COLUMBIA RIVER.

St. Helen Bar:	
Additional work necessary to present dike, 5,000 feet at \$5	25,000.00
Dredging 12,000 cubic yards, at 15 cents.....	1,800.00
Martin Island Bar :	
Dike, 2,000 feet, at \$9	18,000.00
Dredging 40,000 cubic yards, at 15 cents.....	6,000.00
Walker Island:	
Diking 13,800 feet, at \$10	138,000.00
Dredging 80,000 cubic yards, at 15 cents	12,000.00
Snag Island:	
Diking 15,000 feet, at \$11.....	165,000.00
Dredging 400,000 cubic yards, at 15 cents.....	60,000.00
Dredging at crossing 1 mile above Tongue Point, 60,000 cubic yards, at 15 cents.....	9,000.00
Dredging in front of Upper Astoria, 20,000 cubic yards, at 15 cents..	3,000.00
Total.....	437,800.00
Total for both rivers.....	593,720.00
Add 20 per cent for contingencies.....	118,744.00
Cost of one dredging machine.....	60,000.00
Total.....	772,464.00

The demand for an increased depth of water from Portland to the sea has kept pace with the growth of that city and the development of the country tributary to it. Previous to 1866 the ruling depth at low water was from 9 to 12 feet. Vessels loaded deeper than this in order to reach Portland must take every advantage of high tide and the shallow water in the river. The usual mode of procedure was to load the vessels rather to St. Helen or the mouth of the Willamette River, and deliver as much of its cargo as would enable it to get over the shallow water above. The efforts at improvement were directed to

ting a channel to the sea 15 feet depth at low water. This being gained it was followed by a demand for 18 feet and then for 20 feet. During these times the ruling depths over the bar at the mouth of the Columbia River were from 19 to 22 feet at low water. Vessels drawing more than 22 feet were rarely chartered for this port. In consequence of the jetty now in process of construction and nearing completion at the mouth of the Columbia River we have now an actual depth of 27 feet over the bar at low water with every reason to believe that we will be 30 feet before the end of the year. There is no apparent reason why the very deepest-draft vessels may not now be chartered to carry cargo at the city of Portland. If the channel in the Columbia and Willamette rivers at the points mentioned in this report be not improved to the depth corresponding to the draft of the vessels which will not exceed 25 feet except in cases so rare that they need not be provided for, then the vessel must be lightered at Astoria in proportion as her draft exceeds the depth of the channel. Deep-sea vessels will always attempt to load as far inland as circumstances of profit will permit. Lightering at best is an expensive operation, and in these days of close competition it is desirable in all cases to keep it at a minimum. The demurrage consequent upon this is always a large item of expense. The total lightering to and from Portland during the year ending June 30, 1891, was 24,300 tons. This is growing less each year as navigation improves. The rate now paid for lightering is 50 cents per ton. The average draft of the foreign vessels passing out over the bar during the year was 21 feet.

Portland, situated at the head of deep-water navigation, over 100 miles inland, possessing great wealth and assured prosperity, is naturally desirous that all products and commodities coming to her port or exported from it shall have every advantage of cheap transportation. The value of the exports from the Columbia River during the year ending June 30, 1891, was \$6,089,585; the imports, \$1,325,400. The gross tonnage passed in and out over the bar at the mouth of the river, as reported by the collectors of customs at Portland and Astoria, was 23,818 tons. In addition to this deep-sea tonnage there is a local tonnage amounting to 1,154,294 tons, making a showing of over 3,000,000 tons for the Willamette and Columbia rivers between Portland and the sea. The total custom-house receipts for last year were \$649,531.83. The jobbing trade of the city of Portland for 1890 is estimated to be \$31,550,000, against \$211,500,000 for 1889; an increase of \$16,550,000. More detailed statistical data bearing upon this improvement will be found in my annual reports on the improvement of the mouth of the Columbia River and the improvement of the lower Willamette and Columbia rivers, submitted to the Chief of Engineers for the year ending June 30, 1891.

Accompanying this report there are transmitted the following tracings: Columbia River from Cathlamet to Tongue Point; Columbia River in vicinity of Walker Island; Columbia River in vicinity of Martin Island; Willamette River from Portland to its junction with the Columbia, two sheets; one sheet showing characteristic sections of proposed channels.

Very respectfully, your obedient servant,

THOS. H. HANDBURY,
Major, Corps of Engineers.

Brig. Gen. THOMAS L. CASEY,
Chief of Engineers, U. S. A.

(Through Col. G. H. Mendell, Corps of Engineers, Division Engineer, Pacific Division.)

Reprinted; printed in House Ex. Doc. No. 38, Fifty-second Congress, first session.

REPORT OF THE CHIEF OF ENGINEERS, U. S. ARMY.
LETTER OF LIEUTENANT W. H. MENDELL, CHIEF OF ENGINEERS, DISTRICT ENGINEER, PACIFIC DIVISION.

UNITED STATES ENGINEER OFFICE,
San Francisco, Cal., October 5, 1881

The following remarks apply to the report and proposals for the improvement of the low-water channel from the mouth of the Willamette River to the mouth of the Columbia River.

1. THE WILLAMETTE RIVER.

It is recommended that the Willamette River be improved, with the following conditions: The proposed bar ought to be parallel instead of perpendicular to the river. Experience has fully demonstrated that the proposed bar is not feasible.

2. COLUMBIA RIVER.

The existing dike at St. Helens be filled in with earth, and the water at and beyond the dike be kept at a depth of 20 feet.

3. DEER ISLAND BAR.

The existing bar when there is less than 20 feet of water is a bar of mud and sand, and is considered as inadequate. It is recommended that the bar be filled in with earth, and the water at and beyond the dike be kept at a depth of 20 feet.

4. ST. HELENS ISLAND.

The existing dike at St. Helens is a dike of earth and stone, and is considered as inadequate. It is recommended that the dike be filled in with earth, and the water at and beyond the dike be kept at a depth of 20 feet.

5. DEER ISLAND BAR.

The existing bar when there is less than 20 feet of water is a bar of mud and sand, and is considered as inadequate. It is recommended that the bar be filled in with earth, and the water at and beyond the dike be kept at a depth of 20 feet.

6. ST. HELENS ISLAND.

The existing dike at St. Helens is a dike of earth and stone, and is considered as inadequate. It is recommended that the dike be filled in with earth, and the water at and beyond the dike be kept at a depth of 20 feet.

component in this part of the river exceeds the fresh-water drainage. A wide and deep channel in Cathlamet Bay, able to pass the flood without needless retardation, would give a greater rise of tide at all points above, and work improvement in all channels below, including the bar at the entrance. But any channel to be permanent through the wide expanse of sands needs to be maintained by constructions which shall insure a common path for its proportion of the flood and ebb.

There have been several channels through this bay, each used for a time, then obstructed beyond the point of convenient navigation, and abandoned for another. There has always been a minimum depth of about 14 to 16 feet somewhere in Cathlamet Bay, which has retarded and delayed navigation, no matter where the channel has been. The channel now used is on the north shore, crossing the river to Tongue Point. This has been in use for a number of years. For a long time the channel was in the middle of the bay, varying its position from time to time. The processes by which one channel deteriorates and another is bettered have not been clearly established. It is by no means certain that the sand which obstructs the channel is brought down above, down stream. Nor is it established that the sand is brought down below by the flood tide, although there is good reason to suppose that this is at least partially true. Whatever its direction the movement is in the deepest water. The fact that a portion of a tidal channel lies across the axis of an estuary is, in my judgment, neither a good nor a bad symptom. In selecting a channel for improvement, it is thought that the volume of tidal water as exhibited by the width and depth is a controlling consideration. Another consideration is the convenience of navigation. The channel on the north shore is on the whole easier to navigate in dark or thick weather.

Selection of the channel to be improved ought to be made at the time when the works are about to be constructed. In regard to the features of the proposed improvement it may be said that the dredged channel ought, for convenience of navigation, to be straight, and wider than 150 feet. Its curved trace seems to have been adopted to take advantage of a better depth for a short distance. This is not recommended as being really economical.

There seems to be no necessity in these tidal waters to carry construction to a greater height than 2 or 3 feet above low tide.

Subject to these considerations the project, so far as it relates to Cathlamet Bay, is recommended.

LOWER BAY.

Vessels have always passed to and from the ocean below Tongue Point by way of the southern channel which lies in front of Astoria. This channel appears to have deteriorated. It is obstructed in front of Astoria by the Gilman Reef, and by the wreck of the *Sylvia de Grasse* and the rock upon which she struck. These features, together with cross channels leading from the Astoria Channel to the Middle Channel, seem sufficient to account for the dwindling that is apparent from comparisons of channel sections at Smiths Point and others above as far as Tongue Point. Preservation of the capacity of this channel in its former integrity is regarded as an important feature of this system of river improvement. The dredging proposed just north of as well as below Tongue Point are palliatives which may temporarily arrest decay, but it is thought to be insufficient for permanent improvement.

Martins Island, Walker Island, or below, can not project for the Willamette. Judicious improve the influence of the tide at Portland, and thus in the Willamette to some extent; but there is nothin which requires a modification of the works project River.

It is to be observed that while the project is n low stage of river, the estimates are, so far as th based on a depth of 22 feet, in a narrow channel. and indeed it is to be expected, that a narrowed se by natural action, yet it may also be said that wherever there are other sections situated so as t ment by deposits of scoured material, to make th dredging.

The constructions proposed consist of walls of tion is temporary in character, as the parts alte will have a life of a few years. A better but mor would be to make the work all in stone and brush

Inasmuch as these improvements are on the line ing commerce, it is worth consideration whether be made permanent once for all.

The estimates, both of values and quantities, Handbury are accepted without revision.

Very respectfully, your obedient servant.

G.

Colonel, Corps of Engineers,
Brig. Gen. THOMAS L. CASEY,
Chief of Engineers, U. S. A.

this is a convex bank, the Board is of opinion that spurs are preferable to parallel works.

In reference to the points of difference at Martins Island Bar the Board is of opinion that the single spur dike proposed is not sufficient. It should be placed about 1,000 feet below the location shown on the tracing, and a longitudinal dike should be constructed from its outer end downstream to give a proper direction to the currents, the length to depend upon the effects while the works are under construction.

At Walkers Island Bar the Board concurs with the division engineer in his proposed modification of the project.

At Cathlamet Bay the Board concurs with the following recommendations by the division engineer:

Selection of the channel to be improved ought to be made at the time when the works are about to be constructed. In regard to the features of the proposed improvement it may be said that the dredged channel ought, for convenience of navigation, to be straight and wider than 150 feet. Its curved trace seems to have been adopted to take advantage of a better depth for a short distance. This is not recommended as being really economical.

There seems to be no necessity in these tidal waters to carry construction to a greater height than 2 or 3 feet above low tide.

Subject to these considerations the project, so far as it relates to Cathlamet Bay, is recommended.

As to making the dikes permanent the Board is of opinion that if sufficient funds are supplied this is, in the end, the most economical.

Respectfully submitted.

HENRY L. ABBOT,

*Colonel of Engineers, Bvt. Brig. Gen., U. S. A.,
President of the Board,*

C. B. COMSTOCK,

Colonel of Engineers, Bvt. Brig. Gen., U. S. A.

D. C. HOUSTON,

Colonel of Engineers,

G. L. GILLESPIE,

Lieut. Col. of Engineers.

Brig. Gen. THOMAS L. CASEY,

Chief of Engineers, U. S. A.

U U 10.

[Printed in House Ex. Doc. No. 36, Fifty-second Congress, first session.]

PRELIMINARY EXAMINATION OF COLUMBIA RIVER, FROM THE MOUTH OF WILLAMETTE RIVER TO THE UPPER LIMITS OF THE CITY OF VANCOUVER, WASHINGTON, WITH A VIEW OF ESTABLISHING A SHIP CHANNEL.

UNITED STATES ENGINEER OFFICE,
Portland, Oregon, October 11, 1890.

GENERAL: To comply with your letter of September 20, 1890, directing me to make preliminary examinations and reports upon certain rivers and bays, provided for in the river and harbor act approved September 19, 1890, with the view to their improvement, I have the honor to report as follows, upon:

Columbia River, from the mouth of the Willamette River to the upper limits of the city of Vancouver, with a view to establishing a ship channel.

I am familiar with this reach of the Columbia River, having frequently had my attention called to the obstructions therein for the removal of which an estimate is desired. A call for this estimate was made by resolution of the Senate during its last session, but no funds being available from which the expenses of a survey could be paid, the information desired could not at that time be supplied.

The difficulty to navigation at this point lies in a sand bar located between Vancouver and the mouth of the Willamette River, about 10 miles below. There is but 9 feet on this bar at low water. When removed vessels drawing 20 feet or more could easily ascend above Vancouver. The commerce of this city is growing rapidly, and it is improbable that within a short time it may assume considerable proportions. In fact, with the removal of this bar I see no reason why deep-sea going vessels may not habitually load and unload at the foot of this city. I regard this portion of the Columbia River as being worthy of improvement by the Federal Government, and therefore recommend that I be authorized to make the necessary surveys and estimate of costs of the improvement to be made.

To make this survey and estimate that the sum of \$10,000 be appropriated therefor. Plans and estimate of cost, I enclose herewith.

Very respectfully,
J. H. Mendenhall

My servant,

THOS. H. HANDBURY,
Major, Corps of Engineers.

Brig. Gen. THOMAS L. CLAYTON,
Chief of Engineers,
(Through Col. G. H. Mendell, Chief of Engineers, Division Engineer,
Pacific Division.)

[First endorsement.]

PORTLAND, OREGON, October 13, 1890.

Respectfully forwarded, recommended,

G. H. MENDELL,
Colonel, Corps of Engineers,
Division Engineer, Pacific Division.

SURVEY OF COLUMBIA RIVER, FROM THE MOUTH OF WILLAMETTE RIVER TO THE UPPER LIMITS OF THE CITY OF VANCOUVER, WASHINGTON, WITH A VIEW OF ESTABLISHING A SHIP CHANNEL.

UNITED STATES ENGINEER OFFICE,
Portland, Oregon, October 29, 1891

GENERAL: The river and harbor act approved September 19, 1890, provides that an examination and survey be made with an estimate of the cost of improvement of the—

Columbia River, from the mouth of the Willamette River to the upper limit of the city of Vancouver, with a view to establishing a ship channel.

This duty having been assigned to me by your letter of October 18, 1890, and being now completed, I have the honor to submit the following report:

The city of Vancouver, Wash., is located upon the right bank of the Columbia River, 103 miles above its mouth, and 5 miles above the mouth of the Willamette River. There is now practically a 20-foot channel

roughout this distance until a point $2\frac{1}{2}$ miles above the mouth of the Willamette is reached, where the water suddenly shoals to 9 feet. Passing this shoal, which is about 3,600 feet wide at its narrowest part, a channel of 20 feet depth or more is again found and can be carried above the limits of Vancouver. This shoal is evidently caused by the withdrawal from this portion of the river of the large proportion of its water which passes behind Hayden Island. The upper end of this island is opposite Vancouver. It has a length of $4\frac{1}{2}$ miles and a width of 2,500 feet at its widest part, with an average width of about 1,800 feet. Its top is about 18 feet above low water, and is covered during the high tides. The bottom lands in this vicinity on the Oregon side, and also on the Washington side below Vancouver, are also covered during these tides.

The low-water discharge of the Columbia River at this point is approximately 77,000 cubic feet per second. Of this amount 28,000 cubic feet pass behind Hayden Island, and 49,000 down the main channel. These amounts are the results of rough observations, and may be changed by more carefully collected data. They are sufficiently near correct, however, for the purpose of this report. The extreme high water of 1876 reached a level of 30 feet above the low-water datum. We have no data regarding the quantity discharged at that stage. The area of a low-water section across the head of the slough behind Hayden Island is 25,000 square feet, and that of a section of the main river at its widest place between Vancouver and the mouth of the Willamette is 38,460 square feet. Immediately above the head of the slough the river has a low-water width of 3,400 feet, with a channel 1,000 feet wide and a depth varying from 20 to 30 feet. Where the slough again joins the main river at the foot of Hayden Island we find the same width between banks with two channels, one 800 feet wide with depths over 63 feet, the other 1,000 feet wide, with depths from 20 to 23 feet. The widest distance between banks on the north side of the island where the difficulties to navigation are experienced is 3,200 feet. In this locality there is a sand bar extending out from the right bank and occupying the middle of the river down as far as the foot of Hayden Island. Portions of this are bare at low water, and on other portions the depths vary from zero to 9 and 10 feet. This is evidently caused by the diminution in velocity and volume of water, due to that which passes behind the island during the low and medium stages of the river. The sections above the head of the chute and below the foot of the island show that if the low-water discharge of the chute, which is 36 per cent of the total discharge of the river, can be restored to the main channel, there must necessarily be an improvement in the navigable capacity of this channel. Should the slough be closed to a level with the top of the island, the main channel would eventually be scoured out to a section something like what is found below the foot of the island. There is no necessity for this, however, as the channel depth here is excessive.

As a means of obtaining the necessary depth of water over this obstruction for deep-sea going vessels I recommend that the chute behind Hayden Island be closed by a dam having a height of 4 feet above low water; this dam to be built of piles, brush, and rock; to have a width of 6 feet at the top and a base from 25 to 30 feet in width in the deepest water.

The length of the dam will be 3,000 feet. The banks at the two ends should be protected for a distance of 250 feet against erosion at high

hing the place now is the bar just below the town, which it is proposed by this project to remove.

Very respectfully, your obedient servant,
 THOS. H. HANDBURY,
Major, Corps of Engineers.

rig. Gen. THOMAS L. CASEY,
Chief of Engineers, U. S. A.

through Col. G. H. Mendell, Corps of Engineers, Division Engineer,
 (sic Division.)

[First indorsement.]

U. S. ENGINEER OFFICE,
San Francisco, Cal., November 2, 1891.

respectfully forwarded, recommended.

G. H. MENDELL,
*Colonel, Corps of Engineers,
 Division Engineer.*

U U II.

ESTABLISHMENT OF HARBOR LINES IN WILLAMETTE RIVER AT PORTLAND, OREGON.

THE CHAMBER OF COMMERCE OF PORTLAND, OREGON,
Portland, Oregon, December 13, 1890.

DEAR SIR: I have the honor to inclose herewith memorial of this number, calling your attention to the necessity of establishing harbors on both sides of the Willamette River in front of our city. Copies of this same have been sent to our Senators and Representatives and I pray your earnest attention to the subject.

Very respectfully, etc.,

D. D. OLIPHANT,
Secretary.

THE HON. SECRETARY OF WAR.

[Second indorsement.]

OFFICE CHIEF OF ENGINEERS,
 U. S. ARMY,
December 20, 1892.

Respectfully returned to the Secretary of War.

The chamber of commerce of the city of Portland, Oregon, requests the Secretary of War to establish harbor lines for the harbor of Portland, on both sides of the Willamette River, in front of the cities of Portland, East Portland, and Albina, and as far down the river as St. Johns, under the provisions of section 12 of the river and harbor act approved September 19, 1890.

It is recommended that a Board of Engineers be appointed to consider and report upon the subject of harbor lines at Portland within the limits above designated, the Board to consist of Col. G. H. Mendell, Major Thomas H. Handbury, and Capt. Thomas W. Symons, the expenses

...ar just below the town, which it is pro-

bedient servant,

THOS. H. HANDBURY,
Major, Corps of Engineers.

Y,
U. S. A.

, Corps of Engineers, Division Engineer,

[First indorsement.]

U. S. ENGINEER OFFICE,
San Francisco, Cal., November 2, 1891.
commended.

G. H. MENDELL,
Colonel, Corps of Engineers,
Division Engineer.

U U II.

...OR LINES IN WILLAMETTE RIVER AT PORT-
LAND, OREGON.

COMMERCE OF PORTLAND, OREGON.
Portland, Oregon, December 13, 1890.

In honor to inclose herewith memorial of this
attention to the necessity of establishing harbor
Willamette River in front of our city. Copies
sent to our Senators and Representatives and
attention to the subject.
etc.,

D. D. OLIPHANT,
Secretary.

...RY OF WAR.

[Second indorsement.]

OFFICE CHIEF OF ENGINEERS,
U. S. ARMY,
December 20, 1892.

...rned to the Secretary of War.

...commerce of the city of Portland, Oregon, requests
War to establish harbor lines for the harbor of Port-
es of the Willamette River, in front of the cities of
ortland, and Albina, and as far down the river as St.
provisions of section 12 of the river and harbor act
umber 19, 1890.

...that a Board of Engineers be appointed to consider
rbor lines at Portland within the lim-
o consist of Col. G. H. Mendell, Maj.
t. Thomas W. Symons, the expenses

of the Board to be paid from appropriation for "improving lower Willamette and Columbia rivers in front and below Portland, Oregon."

With the approval of the Secretary the order convening the Board will be issued from this office.

THOS. LINCOLN CASEY,
Brig. Gen., Chief of Engineers.

[Third indorsement.]

WAR DEPARTMENT,
December 23, 1890.

Approved as recommended by the Chief of Engineers.
By order of the Assistant Secretary of War:

JOHN TWEEDALE,
Chief Clerk.

MEMORIAL OF PORTLAND, OREGON, CHAMBER OF COMMERCE.

PORTLAND, OREGON, *December 8, 1890.*

The chamber of commerce of the city of Portland desires to invite the attention of the Secretary of War to the fact that it is essential to the preservation of the harbor of Portland that harbor lines be established on both sides of the Willamette River in front of the cities of Portland, East Portland, and Albina, and as far down the river as St. Johns.

It is apparent to the chamber of commerce, and it is hoped that it will also be to the Secretary of War, that the different municipalities bordering on the river should be guided in the establishment of wharves, and that all encroachments upon the river should be restrained by some definite plan fixed by authority above them all; also that a plan for the improvement of the river and the fixation of the channel on the northeasterly side of Swan Island should be definitely determined in the general interest of the harbor and of commerce.

The chamber of commerce of Portland therefore prays that the Secretary of War will, in conformity with the authority granted to him by section 12 of the river and harbor act of September 19, 1890, cause to be fixed the harbor lines of the harbor of Portland within the limits above designated.

The Hon. SECRETARY OF WAR.

REPORT OF BOARD OF ENGINEERS.

PORTLAND, OREGON, *May 23, 1892.*

GENERAL: The Board of Engineer Officers convened by virtue of Special Orders No. 88, dated Headquarters, Corps of Engineers, U. S. Army, Washington, D. C., December 24, 1890, for the purpose of establishing harbor lines for the city of Portland, Oregon, has the honor to submit the following report:

The city of Portland is located upon the Willamette River, about 110 miles above its junction with the Columbia and 110 miles inland from the Pacific Ocean. It is the head of navigation for deep seagoing vessels. The city as at present constituted includes both sides of the Willamette River for a distance of about $4\frac{3}{4}$ miles. In its southern portion the city limits extend from the west bank only to the middle of the channel for a distance of about $1\frac{1}{2}$ miles north of its southern limit, and in the northern portion these limits extend from the east bank only to the middle of the channel for a distance of about $4\frac{1}{4}$ miles south of its northern limit. By this arrangement there is a part of the right bank of the river in the southern portion of the city and a part of the left

ank in its northern portion that are not at present within its limits. The total length of the river between the extreme northern and southern boundary lines is $10\frac{1}{2}$ miles.

Near the southern boundary the river bed between the main banks is occupied by two islands, Ross Island and East Island, which divide its waters at the higher stages into three channel ways. These islands are overflowed at a stage about 17 feet above low water. At the low stage practically the whole of the water passes down the most westerly channel. A project is now before Congress awaiting appropriation, which proposes to close the middle and east channel way to all water up to a height of 4 feet, by a dam to extend from the right bank across to the head of Ross Island. This work is projected with the view to improving navigation at this point to a depth of 14 feet.

Near the northern portion of the reach the river bed is divided by Swan Island into two channel ways; the westerly one being a high-water chute now closed by a dam across its upper end to all water above a 4-foot stage. The easterly channel way is the one used by deep-sea vessels, and in fact all craft coming to Portland from below. Swan Island is overflowed at a stage about 12 feet above low water. At all other points within the reach under consideration the river flows in a single channel way unobstructed save by the wharves and bridges that have been erected at various points for the accommodation of its commerce and the convenience of traffic across it. The normal width of the river at a bank full stage is about 1,600 feet. At its narrowest point in its original condition the width was 800 feet. By the construction of bridges and wharves natural widths have been very much encroached upon. At its narrowest point, which is the location of the steel railroad bridge, the river is now but 600 feet wide.

It has been a fortunate circumstance for the adjacent property in times of flood that the bottom of the river is susceptible to erosion. What has been lost in available discharge area by wharf encroachments and bridge piers has in general been compensated for by a deepening of the river. In the heart of the city where the greatest encroachments have taken place depths are now found as great as 95 feet. The low-water discharge of the river is approximately 15,000 cubic feet per second. It has been estimated from rough discharge observations taken two days after the highest water reached in February, 1890, which is the highest of which there is any authentic record, that at the extreme of this flood the discharge was approximately 400,000 cubic feet per second. This flood in the Willamette reached at Portland a height of 28.4 feet above low water. The area of cross section at the steel railroad bridge, which is the narrowest point of the river within the limits of Portland, is at low water 37,825 square feet and at extreme high water 55,000 square feet. The original area of cross section at this point was at low water 29,000 square feet and at extreme high water 40,000 square feet. By contracting the width of the river from 800 feet to 600 and putting in the bridge piers scouring has been induced to such an extent that the high-water cross section has actually been increased 5,000 square feet, and the average low-water depth increased $7\frac{1}{2}$ feet.

A map of the Willamette River between the extreme north and south limits of the city of Portland, showing the bridges, streets, wharves, and other features along the banks, has been prepared for the Board. Tracing from this map, with the harbor lines recommended for approval marked in red, is herewith transmitted.* The necessary data

influential citizens owning property bordering upon the river and interested in the commerce of the city.

The argument of Mr. George on this occasion was to the effect that, inasmuch as public convenience seemed to demand that free bridges be constructed across the Willamette River in the city of Portland for the purpose of easy communication from one portion of the city to the other, and that these bridges to a certain extent would obstruct free navigation on the river, and their piers and abutments would necessarily occupy a considerable percentage of the cross section available for the flow of the water at these narrow places, which circumstances might tend to increase the overflow on the adjacent property during freshets, the harbor lines should be placed well back upon the adjacent wharves, and it should be required that these, being private interests, should be eventually removed, in order that the evil effects anticipated from the obstruction which the proposed bridge piers will offer to the flow of the water may be made as small as possible. This idea is embodied in a series of resolutions passed by the committee of which he is chairman, and presented to the Board. A copy of these resolutions accompanies this report.

At this meeting Mr. R. Koehler, manager Southern Pacific Company, in Oregon, called attention to the fact that the proposed lines as they are located between the steel bridge and East Ankeny street in East Portland would subject his company to serious loss of property and materially interfere with the shipping business transacted at its wharf just above the east end of the steel bridge. Mr. Koehler's views are set forth in a written communication to the Board, copy of which accompanies this report.

Communications were received by the Board from Mr. J. E. Haseltine, owner of the property situated between East Pine and East Oak streets, on the east side of the river, which is crossed by the proposed harbor line; and also from Mr. C. A. Dolph, a part owner and representing the owners of the property directly opposite on the west side, which is crossed by the proposed line. Copies of these communications are herewith inclosed. It is claimed in both these cases that the existing structures do not extend beyond the harbor lines authorized by the city ordinances; that they have been in existence for some years, and were erected at considerable expense to their owners for the accommodation of general shipping business; that to require these to be removed back to the harbor lines proposed by the Board, in order to accommodate shipping at their docks, would necessitate the use of powder to blast away the bottom and bank of the river, which is a hard, compact conglomerate that is not eroded, except with the greatest difficulty, by the action of the water, and that to do this blasting would endanger the stability of the remaining parts of the structure. It is also claimed in all these cases that the projecting portions of the wharves form but a small percentage of what would otherwise be the available area for the flow of the water.

These three cases occur, unfortunately, in the narrowest portions of the river and in the heart of the business portion of the city, where every foot that can be gained which will facilitate the flow of the flood waters is of the utmost importance. In this locality there is already the steel railroad bridge, the piers of which occupy $6\frac{1}{2}$ per cent of the high-water cross section of the river, and plans are now before the Secretary of War for another to cross at Burnside street, about 2,200 feet above this. The piers of this bridge will occupy 9 per cent of the high-water cross section at its locality. Twenty-one hundred feet farther up

is the Morrison Street Bridge, and 1,400 feet above this is the Malis Street Bridge. In order to a clearer understanding of the situation this locality the Board has prepared a map on a larger scale, showing these objectionable structures and their relations to harbor lines established by city ordinances and the lines proposed by the Board. These are plotted sections of the river at the points where these wharves are located. A tracing from this is herewith inclosed.

In the case of the Southern Pacific Wharf it is observed that if the outer line be produced it will intersect the bridge at a point 80 feet outside of the eastern abutment. As the north end of this wharf within 35 feet of the bridge the result is to practically diminish the width of the waterway at this point 12 per cent and the area of cross section of the river at high water 6 per cent. The wharf line proposed by the Board intersects the axis of the bridge at a point 40 feet inside the outer line of the wharf and 25 feet inside the established city harbor line.

The bank at this point is a hard, compact gravel, very difficult to move except by blasting, which breaks it up into its small component parts, that are easily washed away. The water being extremely deep immediately in front, it is not probable that dredging to any considerable extent will be necessary in order to get rid of the blasted material.

At the point on the center line, of East Pine street where the proposed harbor line crosses Mr. Haseltine's wharf the shortest distance between the dock lines established by the city on the two sides of the river is 680 feet. The city dock line, with which the outer edge of this wharf coincides, is 60 feet in advance of the proposed harbor lines at the center of East Pine street. By this arrangement the northwest corner of this wharf is set back and its abnormal projection into the river removed. About 9 per cent is gained in the width of the river and about 3 per cent in the high-water area of cross section.

On the west side of the river and almost immediately opposite the wharf just considered is the third, which projects abnormally into the stream. At this point the river is 740 feet wide between established dock lines. Originally a dock line was established by the city of Portland between Washington and Pine streets, practically upon the line proposed by the Board; subsequently this was moved out to its present position, with which the projecting wharf coincides. At the center of Pine street this extends beyond the proposed line 40 feet. By setting this line back nearly $5\frac{1}{2}$ per cent is gained in the width of the river and 2 per cent in the area of high-water cross section.

Respectfully submitted.

THOS. H. HANDBURY,
Major, Corps of Engineers
THOMAS W. SYMONS,
Captain, Corps of Engineers

Brig. Gen. THOMAS L. CASEY,
Chief of Engineers, U. S. A.

[First indorsement.]

OFFICE CHIEF OF ENGINEERS,
U. S. ARMY,
July 16, 1891

Respectfully submitted to the Secretary of War.

It having been made manifest to the Secretary of War that the establishment of harbor lines is essential to the preservation and pro-

tion of the harbor at Portland, Oregon, a Board of Engineers was constituted by Special Orders No. 88, Headquarters, Corps of Engineers, December 24, 1890, to consider and report upon this subject; and the within report by the majority of the Board recommends for approval of the Secretary of War the harbor lines described in the inclosed paper and delineated upon the accompanying chart.

It is recommended that the lines selected be approved, and that the Secretary place his approval both upon this report and the tracing submitted.

THOS. LINCOLN CASEY,
Brig. Gen., Chief of Engineers.

WAR DEPARTMENT,
August 9, 1892.

Approved.

L. A. GRANT,
Acting Secretary of War.

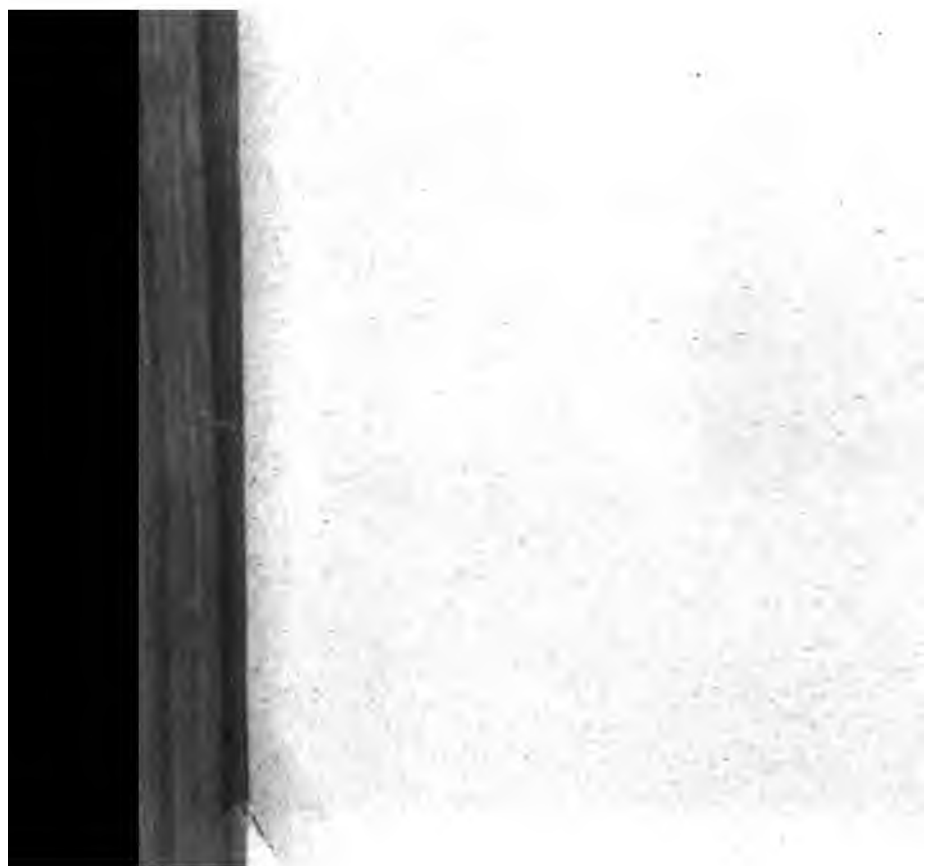
DESCRIPTION OF PIER HEAD LINES ALONG THE WILLAMETTE RIVER BETWEEN THE NORTH AND SOUTH BOUNDARY LINES OF THE CITY OF PORTLAND, OREGON, RECOMMENDED FOR ADOPTION AND APPROVAL OF THE SECRETARY OF WAR BY THE BOARD OF ENGINEER OFFICERS CONVENEED BY VIRTUE OF SPECIAL ORDERS NO. 88, DATED HEADQUARTERS CORPS OF ENGINEERS, U. S. ARMY, WASHINGTON, D. C., DECEMBER 24, 1890.

The location of the lines, with distances and bearings mentioned in this description, are shown in red ink upon a map on file in the United States Engineer office in Portland, Oregon. A tracing from this was forwarded to the Chief of Engineers, U. S. Army, with report dated May 23, 1892.

Left bank, going north.—Commencing at a point in the center line of Madison street which shall be 300 feet from the intersection of said center line with the center line of Front street, measured in an easterly direction; thence to a point on the center line of Morrison street which shall be 200 feet from the intersection of said center line with the center line of Front street, measured in an easterly direction; thence to a point on the center line of Washington street which shall be 222 feet from the intersection of said center line with the center line of Front street, measured in an easterly direction; thence to a point on the center line of Stark street which shall be 250 feet from the intersection of said center line with the center line of Front street, measured in an easterly direction; thence to a point on the center line of Pine street which shall be 285 feet from the intersection of said center line with the center line of Front street, measured in an easterly direction; thence to a point on the center line of Ash street which shall be 282 feet from the intersection of said center line with the center line of Front street, measured in an easterly direction; thence to a point on the center line of Ankney (A) street which shall be 245 feet from the intersection of said center line with the center line of Front street measured in an easterly direction; thence to a point which shall be 130 feet north of the center line of Davis (D) street and 212 feet east of the center line of Front street; thence to a point on the center line of Flanders (F) street which shall be 395 feet from the intersection of said center line with the center line of First street, measured in an easterly direction; thence to a point which shall be 110 feet north of the center line of Glisan (G) street and 265 feet east of the center line of First street; thence to a point on the center line of the present steel railway bridge, which shall be 169 feet from the center of the pivot pier, measured in a westerly direction; thence to a point which shall be 380 feet northeasterly from the west side of North Front street, measured on a line which shall be drawn perpendicular to said west side at its intersection with the center line of Johnson (J) street; thence to a point which shall be 340 feet easterly from the westside of North Front street, measured on a line which shall be drawn perpendicular to said west side at its point of intersection with the east side of North Seventh street; thence to a point which shall be 500 feet northeasterly from the west side of North Front street, measured on a line which shall be drawn perpendicular to said west side at its point of intersection with the east side of North Seventeenth street; thence to a point, on the prolongation northward of the center line of Blackstone street, which shall be 1,750 feet from the intersection of said center line

tion westward of the center line of Russell street which shall be also on the prolongation southward of outer face of the docks of the North Pacific Terminal Company and 700 feet, more or less, from the intersections of said center line of Russell street with the center line of Loring street; thence along the outer face of said Pacific Terminal Company Docks, direction N. $39^{\circ} 45'$ W., distance 3,120 feet, to a point; thence in a direction N. $31^{\circ} 30'$ W., distance 2,630 feet, to a point; thence in a direction N. 26° W., distance 2,000 feet, to a point; thence N. 44° W., distance 1,500 feet, to a point; thence in a direction N. 53° W., distance 5,280 feet, to a point; thence in a direction N. $88^{\circ} 30'$ W., distance 400 feet, to a point; thence in a direction N. 69° W., distance 800 feet, to a point; thence in a direction S. $84^{\circ} 30'$ W., distance 500 feet, to a point; thence in a direction N. 66° W., distance 1,100 feet, to a point; thence in a direction N. $51^{\circ} 30'$ W., distance 5,000 feet, to a point; thence to a point on the prolongation southwesterly of the south side of Oneonta (Main) street, in the town of St. John, which shall be 320 feet from the intersection of said street with the east side of Front street; thence to a point on the prolongation southwesterly of the center line of Burlington (Washington) street which shall be 240 feet from the intersection of said center line with the east side of Front street; thence to a point on the prolongation westward of the north side of Baltimore (Fanny) street which shall be 430 feet from the intersection of the said north side with the east side of Front street.

Swan Island.—Commencing at the point of intersection of the line of the Government dam at the head of Swan Island Chute with a line drawn parallel to Sherlock street and 1,560 feet therefrom; thence, in a direction N. $58^{\circ} 45'$ W., distance 5,960 feet, to a point; thence in a direction N. $44^{\circ} 30'$ W., distance 4,650 feet, to a point; thence in a direction due north, distance 360 feet, to a point; thence in a direction S. 80° E., distance 1,750 feet, to a point; thence in a direction N. 80° E., distance 1,500 feet, to a point; thence in a direction S. 61° E., distance 500 feet, to a point; thence in a direction S. 53° E., distance 4,000 feet, to a point; thence in a direction S. 30° E., distance 4,036 feet, to a point; thence in a direction S. 60° W., distance 456 feet, to a point of beginning.



APPENDIX V-V.

SUPERVISION OF THE HARBOR OF NEW YORK.

REPORT OF CAPTAIN FREDERICK RODGERS, U. S. NAVY, SUPERVISOR
OF THE HARBOR OF NEW YORK, FOR THE FISCAL YEAR ENDING
JUNE 30, 1892.

OFFICE OF THE SUPERVISOR OF THE HARBOR OF NEW YORK.

New York, September 19, 1892.

GENERAL: In accordance with instructions dated August 26, 1892, I have the honor to submit the following annual report:

As this report only covers the period included in the past fiscal year, and as I did not assume charge of this office until July 7, I can only submit a report of the operations of the office as shown by the records thereof.

I have to state that, upon assuming charge of the office, I found three of the four vessels employed under its jurisdiction were not in efficient condition for service, and the fourth one, the tug *Nimrod*, the only really efficient vessel of the lot for general patrol work, had been so continually employed and under steam that she badly needed a general overhauling, in order to preserve her for future service.

I am informed that lack of necessary funds for the purposes prevented the necessary repairs upon these vessels.

I concluded that it was most desirable and a measure of economy to put all these vessels in as efficient condition as practicable for work during the present year, and, with the authority of the office of the Chief of Engineers, the two launches, *Active* and *Alert*, have been thoroughly overhauled and are now in excellent order. The *Argus* is now under repairs and it is proposed to give the *Nimrod* a general overhauling when the services of the former vessel become available.

In reference to the repairs to the *Argus* on this and the last occasion, it was a question whether she was worth the expenditure. It is possible that one more year's service may be got out of her, but I doubt whether it would be expedient to spend any more money on her. However,

unless repaired and put in a reasonably serviceable condition, but vessel would be available for patrol work in the lower bay.

I find from the records, that the *Argus* originally cost \$21,950 including \$700 for plans, and her equipment \$1,227.75, making a total cost of \$23,177.75. She appears to have been commissioned on November 9, 1889, and since that date the sum of \$7,186.12 appears to have been spent in repairs.

I am satisfied from the experience I have had so far, that the recommendation of the supervisor in his report of July 9, 1891, is well founded, viz.:

I have to renew my suggestion in the preceding report, that two more tug boats be furnished, one of which will be required to replace the *Argus* before many months and the other as an auxiliary to relieve broken-down patrolers and perform emergency service, which is frequently necessary. It is a well-known fact that no steam lines can be handled properly without an extra boat, and the same obtains for service of this office.

There should be, for an efficient performance of the duties required in patrolling the harbor and adjacent waters, at least three light-drawers but seaworthy, propellers of about 100 feet in length, with fair speed and proper accommodations for the crew to live on board. In addition to the *Nimrod*, two more vessels of about the same size would therefore be required. These vessels would cost \$45,000 each, and \$12,000 annually for crew and maintenance, including repairs. An appropriation for this purpose, in addition to the usual one of \$33,000 is recommended.

The work of the office, and that of those under its jurisdiction, appears to have been efficiently performed.

There has been moved and deposited into and at legally designated places outside the harbor and behind bulkheads in the neighborhood of New York, during the fiscal year ending June 30, 1892, the amount of 10,072,311 cubic yards of material, mud, city refuse, garbage, and dirt, ashes, acids, lime, and other matter as per the following recapitulation:

Place of deposit.	Kind of material.	Amount
Mud buoy	Mud, etc.	Cubic 5.17
Refuse Buoy	City refuse, garbage, etc.	1.58
Scotland Light-ship	Mud, etc.	45
Long Island Sound	do.	79
North River	Dirt, ashes, etc., behind bulkheads and on shore for filling.	40
East River	do.	20
Harlem River	do.	40
Hudson River	do.	13
Raritan River	do.	11
New York Bay	do.	9
Staten Island Sound	do.	27
Newark Bay	do.	23
Swinburne Island	do.	
Pelham Bay	do.	
Jamaica Bay	do.	
Shoal Harbor, New Jersey	do.	
Shrewsbury River	do.	
Passaic River	do.	17
Barren Island	Acids, dead animals, etc., on shore in store	5
Total		10 07

Permits issued

From the foregoing detailed statement it will be seen that 7,954 cubic yards of city refuse, mud, etc., was deposited in the ocean at the mouth of the harbor, and that 2,061,470 cubic yards of cellar

shes, and other inoffensive material was utilized for filling in behind bulkheads, reclaiming land, etc., for which special permits were issued.

The following is a statement to July 1, 1892, of the appropriations made for "prevention of obstructive and injurious deposits within the harbor and adjacent waters of New York City":

Appropriation, 1888-'89, act of June 29, 1888	\$30,000.00	
Expended to July 1, 1890	29,957.09	
Balance July 1, 1890	42.91	
Appropriation, 1889-'90, act of March 2, 1889	31,070.00	
Expended to July 1, 1890	22,315.22	
Balance July 1, 1890	11,754.78	
Outstanding liabilities, contracts, orders, etc	8,584.04	
Balance after deducting liabilities	3,170.74	
Appropriation, 1889-'90, act of March 2, 1889, for the purchase or construction of a vessel	60,000.00	
Expended to July 1, 1890	36,520.00	
Balance July 1, 1890	23,480.00	
Appropriation, 1890-'91, act of August 30, 1890:		
For pay of crew and maintenance of steamer <i>Argus</i>	\$8,000.00	
Expended to July 1, 1891	6,379.08	1,620.92
For pay of crew and maintenance of new vessel to be purchased or constructed	10,000.00	
Expended to July 1, 1891	9,621.61	378.39
For pay of inspectors and deputy inspector, office force, and expenses of office	15,000.00	
Expended to July 1, 1891	11,785.40	3,214.60
Outstanding liabilities		5,213.91
Balance July 1, 1891		165.00
Balance July 1, 1891		5,048.91
Appropriation, 1891-'92, act of March 3, 1891:		
For pay of inspectors and deputy inspectors, office force, and expenses of office	\$15,000.00	
Expended to July 1, 1892	10,609.52	\$4,390.48
Outstanding liabilities		2,456.05
Balance July 1, 1892		1,934.43
For pay of crew and maintenance of steamer <i>Argus</i>	\$8,000.00	
Expended to July 1, 1892	7,093.75	906.25
Outstanding liabilities		883.22
Balance July 1, 1892		23.03
For pay of crew and maintenance of steamer <i>Nimrod</i>	\$10,000.00	
Expended to July 1, 1892	8,616.34	1,383.66
Outstanding liabilities		1,312.59
Balance		41.07
Total balance appropriation 1891-'92		2,051.01

The following is a list of the employés of the harbor, with the date of their appointment, rating, pay, and

Name.	Rating.	Pay.
John G. Morison.....	Chief clerk and inspector.....	1.
A. C. Murphy.....	Stenographer.....	
Aaron Elliott.....	Messenger.....	
E. C. Hager.....	Master and deputy inspector.....	1.
James Quirk.....	do.....	1.
E. McGovern.....	Pilot and deputy inspector.....	
F. T. Stillwell.....	do.....	
G. R. Barker.....	do.....	
William Halliday.....	do.....	
Fred. A. Tappan.....	do.....	
John B. Polan.....	Fireman.....	
George Murphy.....	do.....	
J. T. Talt.....	do.....	
John Saunders.....	do.....	
Charles Halliday.....	Engineer.....	1.
R. Hammond.....	Fireman.....	
R. Humber.....	Second deckhand.....	
James Bailey.....	Cook.....	
Bernard Kelly.....	Fireman.....	
W. A. Bloomer.....	Chief engineer.....	1.
Charles Kelly.....	Fireman's helper.....	
F. J. Wren.....	Carpenter.....	
George Wyman.....	Cook.....	

I strongly recommend that two additional positions be created in the force of this office, as suggested in a preceding report.

The following is an estimate of appropriate expenditures for the fiscal year ending June 30, 1894, by the Chief of Engineers, New York:

Detailed objects of expenditure, and explanation

Prevention of obstructive and injurious deposits within the adjacent waters of New York City:

For pay of inspectors, deputy inspectors, office force, and clerks of office.....	
For pay of crew and maintenance of steamer <i>Nyasrod</i>	
For pay of crew and maintenance of steamer <i>Aegus</i>	
For purchase or construction of two steam tugs.....	
For pay of crew and maintenance of two steam tugs to be purchased or constructed.....	
Total.....	

Very respectfully, your obedient servant

I
Capt

Brig. Gen. THOMAS L. CASEY,
Chief of Engineers, U. S. A.

CORRESPONDENCE RELATING TO DUMPI
HOOK.

OFFICE OF THE SUPERVISOR
HARBOR
NEW YORK

GENERAL: I respectfully call to your attention that many of the inward bound freight steamers discharge their ballast outside the bar.

I have lately received a communication from the Harbor Commissioners on this subject, and they have requested

roaches to the harbor within which the discharge of all ballast shall be regarded as illegal.

I am of opinion that the continual deposit of large quantities of this material near the entrance of the harbor will, in course of time, be a source of danger and inconvenience to navigation. In a case recently reported to me by the pilot commissioners, the British steamer *Hurth* discharged her ballast outside the bar; and the pilot states that the master informed him that he was ordered by his owners to do so outside the jurisdiction of the United States authorities before entering. Four hundred tons of stone were thrown overboard from this steamer about 3½ miles from the bar. I think there can be no doubt that the dumping of such large quantities of stone, or any other heavy material generally used for ballast in 7 or 8 fathoms of water should be prohibited.

The British steamer *Elmville* is also reported by one of the inspectors of this office as having discharged her ballast about 5 miles to the seaward of Sandy Hook light-ship. From the best information at my disposal, I believe that this practice is almost universal, and that in some instances it is probable that vessels are discharged much within the 3-mile limit.

After consultation with Lieut. Col. Gillespie, United States Engineer in charge of the improvement of the harbor, I am of opinion that no deposits of ballast should be permitted within 15 fathoms of the Sandy Hook light-ship, or in less than 16 fathoms of water.

I request that this limit may be approved as defining the adjacent waters of the harbor.

Very respectfully, your obedient servant,

H. B. ROBESON,
Captain, U. S. Navy, Supervisor.

Brig. Gen. THOMAS L. CASEY,
Chief of Engineers, U. S. A.

[First indorsement.]

OFFICE CHIEF OF ENGINEERS,
U. S. ARMY,
December 24, 1891.

Respectfully submitted to the Acting Secretary of War with recommendation for approval.

THOS. LINCOLN CASEY,
Brig. Gen., Chief of Engineers.

WAR DEPARTMENT,
JUDGE-ADVOCATE-GENERAL'S OFFICE,
Washington, D. C. December 29, 1891.

Respectfully returned to the Secretary of War.

Section 6 of the river and harbor act of September 19, 1890 (26 Stat., 1033), provides—

That it shall not be lawful to cast, throw, empty, or unload, or cause to be cast, thrown, emptied, or unloaded, either from or out of any ship, vessel, lighter, barge, boat, or other craft, or from the shore, pier, wharf, furnace, manufacturing establishments, or mills of any kind whatever, any ballast, stone, slate, gravel, earth, rubbish, wreck, filth, slabs, edgings, sawdust, slag, cinders, ashes, or other waste of any kind, into any port, road, roadstead, harbor, haven, bay, gable river, or navigable waters of the United States, which shall tend to obstruct navigation, or to deposit or place, or cause, suffer, or procure to be deposited, any ballast, stone, slate, gravel, earth, rubbish, wreck, filth, slabs, edgings, sawdust, or other waste in any place or situation on the bank of a navigable

LETTER OF THE ATTORNEY-GENERAL.

DEPARTMENT OF JUSTICE,
Washington, D. C., January 6, 1892.

SIR: I have by reference the letter of December 23, written by H. B. Robeson, supervisor of the harbor of New York, to Gen. Casey, Chief of Engineers, with the indorsements, touching the matter of the deposit of ballast outside of New York Harbor, and at a distance of more than 3 miles from the shore, at low-water mark.

You ask my opinion as to whether or not the supervisor of the harbor, or the War Department, can interfere to prevent these deposits.

In answer, I have to say: I know of no statute authorizing such interference, nor of any power to so interfere in the absence of statute. The indorsement of Col. Lieber, Acting Judge-Advocate-General, seems to cover the subject-matter.

Very respectfully,

W. H. H. MILLER,
Attorney-General.

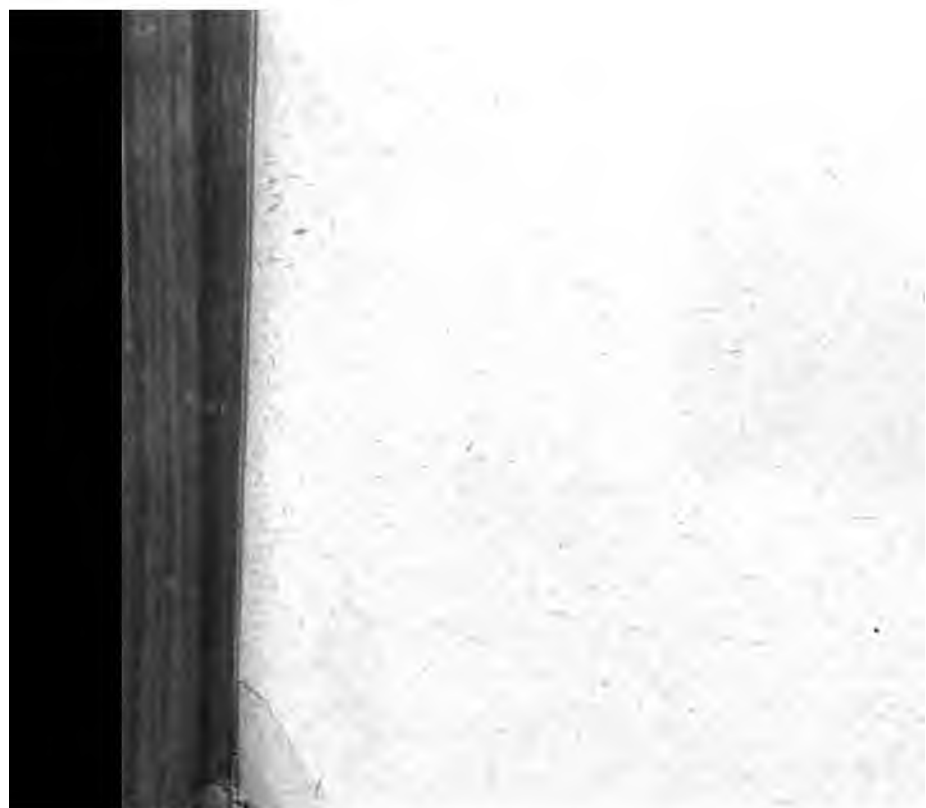
The SECRETARY OF WAR.

PROPOSED DRAFT OF AN ACT TO PREVENT THE DUMPING OF BALLAST OR OTHER BULKY MATERIAL IN THE APPROACHES TO NEW YORK HARBOR AND BAY OFF SANDY HOOK.

Be it enacted by the Senate and House of Representatives of the United States of America in Congress assembled, That any vessel or vessels from which may be dumped or discharged ballast or other bulky material in the approaches to New York Harbor and Bay within fifteen miles of the Sandy Hook light-ship, or in less than sixteen fathoms of water, shall not be allowed by the collectors of customs to receive passengers or cargo, or clear from New York or any port adjacent to New York Harbor and Bay, for the period of six months from the time of the commission of such dumping or discharging of ballast or other bulky material.

SEC. 2. That it shall be the duty of the supervisor of the harbor of New York, appointed and acting under the provisions of an act entitled "An act to prevent obstructive and injurious deposits within the harbor and adjacent waters of New York City, by dumping or otherwise, and to punish and prevent such offenses," approved June twenty-ninth, eighteen hundred and eighty-eight, to detect by proper measures any vessel or vessels which may dump or discharge ballast or other bulky material within the limits specified in the foregoing section of this act, and upon being satisfied of the commission of such act or acts to report the same to the collectors of customs at ports adjacent to New York Harbor and Bay for action by them in pursuance of the provisions in the said foregoing section; and the provisions of the said act approved June twenty-ninth, eighteen hundred and eighty-eight, and the jurisdiction of the said supervisor of the harbor of New York, are hereby extended, so far as they are applicable and consistent with the provisions of this act, to embrace the waters included within the limits specified in section one of this act.

SEC. 3. That the collectors of customs at any or all the ports adjacent to New York Harbor and Bay, upon receipt of information from the said supervisor of the harbor of New York that any vessel or vessels have dumped or discharged ballast or other bulky material within the limits specified in section one of this act, are hereby authorized and directed not to allow said vessel or vessels to receive passengers or cargo or clear from the respective ports within six months from the time of commission of such dumping or discharging of ballast or other bulky material, as provided in section one of this act.



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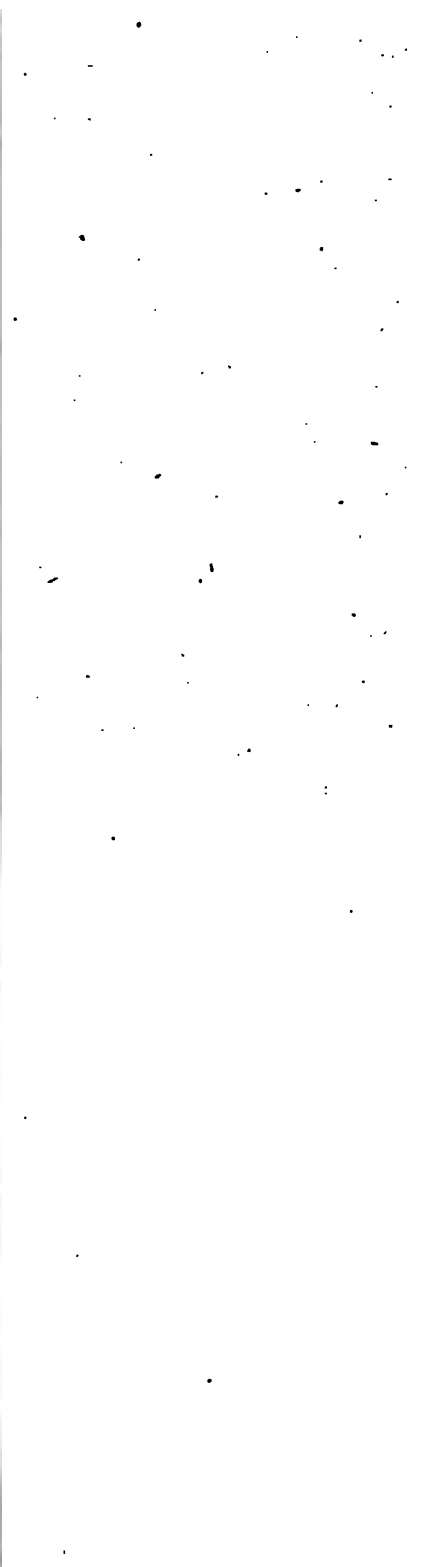
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