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## SUPPLEMENT

TO THE

## Manual of Instructions

## FOR THE <br> Survey of Canada Lands

DETERMINATION OF THE ASTRONOMICAL AND MAGNETIC MERIDIANS

PROBLEMS CONNECTED WITH THE SYSTEMS OF SURVEY)

TABLES

Issued by authority of the Honourable the Minister of
Mines and Technical Surveys


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Page
Preface to 1952 Edition ..... V11
Preface to 1917 Edition ..... viii
Index to Notation ..... ix
CHAPTER I - THE ASTRONOMICAL MERIDLAN
Determination of the Astronomical Meridian ..... 1
To Find the Pole Star ..... 1
Pole Star Observation for Azimuth ..... 3
Specimen Observations ..... 4
Determination of the Watch Correction ..... 6
Determination of the Watch Correction by the Meridian Transit of a Star ..... 6
Determination of the Watch Correction by theMeridian Transit of the Sun9
Determination of the Watch Correction by Radio Time Signals ..... 11
The Watch Rate ..... 12
Observation on the Sun for Azimuth ..... 13
Computation for Sun Observation ..... 14
Specimen Observations ..... 16
Observation of Polaris for Azimuth on Governing Surveys ..... 20
Watch Correction ..... 21
Correction for Striding Level ..... 21
Determination of the Value of One Turn of the Micrometer ..... 22
Computation for Azimuth Observation ..... 23
General Remarks on Observing ..... 30
CHAPTER Il - DETERMINATION OF THE MAGNETIC MERIDLAN
Directions for Observing ..... 31
General Remarks ..... 31
Explanation of Specimen Observation ..... 33
Setting a Transit by Means of the Compass ..... 35
Compass of Cooke Transit ..... 36
The Wild Double lmage Prism Compass ..... 36
CHAPTER 111 - INSTRUMENTS
Transit Theodolite for Governing Surveys ..... 38
"Optical" Theodolites ..... 38
Instruments for Use on Winter Surveys ..... 39
General ..... 39
Sidereal Watch ..... 40
Steel Tapes ..... 40
Stadia Rods ..... 41
Clinometer ..... 42
CHAPTER IV - PROBLEMS CONNECTED WITH THE SYSTEM OF SURVEY
Correction for Height Above Sea-Level ..... 43
Latitudes and Longitudes of Points in the System ..... 44
Page
Latitude ..... 44
Longitude, Third System ..... 44
Longitude, First System ..... 45
Longitude, Second and Fourth Systems ..... 45
Effect of Errors of Survey ..... 46
Given the Latitude and Longitude of a Point, to find its Position with regard to the System of Survey Second, Third and Fourth Systems ..... 46
First System of Survey ..... 47
Fractional Township or Range between parts of the Country Surveyed under different Systems of Survey ..... 47
Fractional Township ..... 48
Fractional Range ..... 48
First Example ..... 49
Second Example ..... 51
Fractional Sections Adjoining an Initial Meridian ..... 51
Geodetic Positions ..... 52
Spheroidal Co-ordinates of Section Corners as Determined by the Geodetic Survey of Canada (Table) ..... 53
CHAPTER V - CONSTRUCTION AND USE OF THE TABLES
Table I Lengths of Arcs of Meridians, Parallels, etc., in different Latitudes ..... 57
Radius of Curvature of a Section of the Spher- oid inclined at any angle to a Meridian ..... 59
Radius of Spherical Curvature ..... 59
Table II Corrections to Table I for change in elements of Figure of Earth ..... 59
Table III Latitudes of Base and Correction Lines and Lengths of Arcs of Meridians, Parallels, etc., for First and Second Systems of Survey. ..... 60
Table IV Latitudes of Base and Correction Lines, etc., for Third and Fourth Systems of Survey ..... 62
Table V Chord Azimuths, etc., for Base Lines, First and Second Systems of Survey ..... 62
Table VI Chord Azimuths, etc., for Base Lines, Third and Fourth Systems of Survey ..... 64
Table VII Chord Azimuths, Jogs, etc., for Correction Lines, First and Second Systems of Survey ..... 64
Table VIII Chord Azimuths, Jogs, etc., for Correction Lines, Third and Fourth Systems of Survey ..... 65
Table IX Latitudes, and Widths in Chains, of Northern Boundaries of Sections in First and Second Systems of Survey ..... 66
Page
Table $X$ Latitudes, and Widths in Chains, of Northern Boundaries of Sections in Third and Fourth Systems of Survey ..... 66
Table XI Difference of Latitude between Township Corners and Section and Quarter Section Corners ..... 67
Table XII For Converting Logarithmic Tangents of Small Arcs into Logarithms of Seconds of Arc ..... 67
Table XIII $\log \frac{1}{1-m}$ tabulated with $\log m$ as argument ..... 67
Table XIV Deflection of Trial Line for Deviations from 1 to 149 Links at the end of Eighty-One Chains ..... 68
Table XV Corrections in Links to Slope Measurements ..... 68
Table XVI Table for Laying Out Roads One Chain Wide ..... 68
Table XVII To Convert Time into Arc ..... 68
Table XVIII To Convert a Mean Time Interval to the Equiv- alent Sidereal Time Interval ..... 69
THE ASTRONOMICAL FIELD TABLES
Field Tables for Solar Observations ..... 69
Field Tables for Star Observations ..... 70
The Apparent Motion of Polaris ..... 70
Computation of the Azimuth and Altitude of Polaris ..... 72
TABLES
I Radii of Curvature of Meridians and Parallels, etc. ..... 76
II Corrections to be applied to the Logarithms of $\mathrm{R} \sin 1^{\prime \prime}$ and $\mathrm{N} \sin 1^{\prime \prime}$ in Table I for Clarke's later values of the Dimensions of the Earth ..... 84
III Latitudes, etc., of Base and Correction Lines. First and Second Systems of Survey ..... 85
IV Latitudes, etc., of Base and Correction Lines. Third System of Survey ..... 86
$V$ Chord Azimuths, Deflections, Deflection Offsets, etc., for Base Lines. First and Second Systems of Survey ..... 92
VI Chord Azimuths, Deflections, Deflection Offsets, etc., for Base Lines. Third System of Survey ..... 93
Page
VII Chord Azimuths, Deflections, Deflection Offsets, Jogs, etc., for Correction Lines. First and Second Systems of Survey ..... 95
VIII Chord Azimuths, Deflections, Deflection Offsets, Jogs, etc., for Correction Lines. Third System of Survey ..... 96
IX Latitude, with Logarithms of Secant and Tangent for the North Boundary of each Section, and the Widths of Quarter Sections on such Boundaries. First and Second Systems of Survey ..... 100
X Latitude, etc., for the North Boundary of each Section. Third System of Survey ..... 107
XI Showing the difference of Latitude between Town- ship Corners and Section and Quarter Section Posts on a Township Chord ..... 148
XII For Converting Logarithmic Tangents of Small Arcs into Logarithms of Seconds of Arc ..... 149
XIII $\log \frac{1}{1}$ tabulated with $\log m$ as argument ..... 150
XIV Deflection of a Trial Line for Deviations from 1 to 149 Links at the end of Eighty -one Chains ..... 156
XV Corrections in Links to Slope Measurements ..... 157
XVI Table for Laying Out Roads One Chain Wide ..... 162
XVII To Convert Time into Arc ..... 163
XVIII To Convert a Mean Time Interval to the Equiv- alent Sidereal Time Interval ..... 164

## Preface to 1952 edition

In 1930 the Dominion Lands in Manitoba, Saskatchewan, Alberta, and British Columbia were transferred to the respective provinces. In I950, the Territorial Lands Act, describing the public lands of Yukon Territory and Northwest Territories as "Territorial Lands" was passed and the Dominion Lands Act repealed. In December 1951 the Canada Lands Surveys Act became law and superseded the Dominion Lands Surveys Act which was repealed. The Canada Lands Surveys Act generally applies to the public lands in Yukon and Northwest Territories, National Parks, and surrendered lands and reserves as defined in the Indian Act. This revision of the 1917 Supplement is, therefore, called the Supplement to the Manual of Instructions for the Survey of Canada Lands.

Chapter I has been slightly revised to agree with modern practice and instruments.

To Chapter II has been addeda short description of the Wild Double Image Prism Compass.

Chapter III has been considerably revised. Information has been added on modern optical transits, the care of instruments, and the correction for stretch and sag to measurements made by steel tapes.

A table listing the geodetic latitudes and longitudes of section corners has been added to Chapter IV.

The tables have been extended northward to township 244 in latitude $70^{\circ} 17^{\prime}$. Columns in the table tabulating data in degrees and decimals have been omitted since transits with this type of division are no longer in use in Canada. In tables Nos. V, VI, VII, and VIII the convergence of meridians for 100 chains of longitude has been added as a further convenience incertain calculations. Tables have been added for the conversion of time to arc and of mean time intervals to sidereal time intervals.

The table to reduce chains to decimals of a township side has been omitted as it was seldom used.
B. W. Waugh,

Surveyor General.

## Preface to 1917 edition

The first Manual of Instructions for the survey of the Dominion Lands, a small 12 mo pamphlet of thirty-two pages, was prepared in 1871 by Col. J. S. Dennis, Surveyor-General; the title was "Manual showing the System of Survey adopted for the Public Lands of Canada in Manitoba and the North-West Territories, with Instructions to Survey ors." It was published by authority of the Honourable the Secretary of State, the Dominion Lands office being then a branch of his department. The Manual contained only one table, "showing the departure in running 81 chains 50 links at any course from 1 to 60 minutes."

The second edition was prepared in 1881 , under the direction of Mr. Lindsay Russell, Surveyor-General, by Dr. Deville; it was considerably enlarged, forming a large octavo book of 86 pages. By that time tive need of tables specially adapted to the survey of Dominion Lands had become imperative: thirteen tables were calculated by Dr. Deville and Dr. King and were appended to that edition.

A number of editions followed, the fourth, published in 1892 , containing six additional tables, or nineteen altogether. The fifth and sixth editions, issued in 1903 and 1905 respectively, contained only eight tables. The tables left out were seldom used and it was considered that when needed they could be consulted in the 1892 edition.

In 1908, when the stock of the fourth edition of the Manual (1892) was exhausted, a reprint of the tables became necessary. They were published as a supplement, and their construction and use fully explained. Problems connected with the system of survey originally published by Dr. King in the Report of the Department of the Interior for 1891, were appended.

For this edition, the Supplement has been completely revised. There have been added two chapters on observing and a chapter briefly describing the instruments kept in stock at the head office, for sale to surveyors employed by the Department. These chapters replace matter which formerly appeared in the Manual of Instructions. The additional chapters and the revision are the work of H. Parry, D. L.S.; he has carefully checked the tables and it is hoped that they will now be found free from errors.

## INDEX TO THE NOTATION

```
a.
    equatorial semi-diameter of the earth
b........ polar semi-diameter of the earth.
c........ earth's compression.
e........ eccentricity.
h........ altitude of a star.
K. ........ distance.
L......... latitude.
l........ elevation above sea-level.
M ........ longitude.
N. ....... length of normal to the meridian.
P........ radius of parallel of latitude; also polar distance.
p........ polar distance.
R.A....... right ascension.
R........ radius of curvature of meridian.
S........ radius of curvature.
t ......... hour angle.
Z........ azimuth.
```


## CHAPTER I

## DETERMINATION OF THE ASTRONOMICAL MERIDIAN

The reference of lines to an astronomic meridian, in order to determine their direction, or to check the accuracy of their production, is most readily made by observations on the Pole star.

The telescopes of most modern surveying transits are amply powerful for observing Polaris at any time during a clear day with the exception of a few hours before or after noon. Many of them are provided with lighting arrangements to facilitate observations at night. For best results observations should be taken within a few hours before dusk or after dawn when the air is not quivering and accurate pointings on the reference object may be made. For night observing the reference object requires to be well lighted. Directing the headlights of an automobile on a line or traverse picket at short range is effective.

## TO FIND THE POLE STAR

Whether observations are made in the daytime or at night the star must first be found. To set the telescope at solar focus, focus the eyepiece so that the cross hairs or diaphragm markings are clear and distinct. Then point the telescope on a well defined object a half mile or more distant and adjust the objective focus until the image of the object is as sharp as possible. By slight movements of the eyepiece and objective focussing screws continue the adjustment until the sharpest possible images are obtained of the object and the cross hairs which will remain in constant relation to each other when the eye is moved from side to side, i.e., when there is no parallax.

With the sidereal time and the latitude of the place as arguments the azimuth of the Pole star may be taken from the Astronomical Field Tables, a sample page of which is shown in Figure I; its altitude may be obtained from the same tables.

If the observation is being taken along a survey line, the azimuth of the line will be known and the transit may be set to read the azimuth of Polaris. Otherwise, it may be necessary to orient it by means of the magnetic compass making due allowance for the magnetic declination of the place of observation. When the horizontal plate is properly set, it is merely necessary to tilt the telescope to the proper altitude in order to bring the star into the telescope's field of view.

Practice and patience are required to discover the star in daylight. A slight to and fro motion of the horizontal tangent screw gives a relative motion to the star which aids in its perception and insures that it is not behind the vertical wire of the diaphragm. Once the star is found, the focus of the telescope may be readjusted if necessary. When the solar focus is properly set it is good practice, where possible, to mark the focussing screw with a knife cut or other device so that it can again be brought to the same setting. On some instruments this is not possible. Correct focus is necessary for finding the star in daytime.

ASTRONOMICAL FIELD TABLES

|  |  | AZIMUTH OF POLE STAR |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | $\begin{array}{\|c\|} \hline \mathbf{L a t} \\ \mathbf{4 4} \\ \hline \end{array}$ | $\begin{array}{l\|} \hline \text { Lat. } \\ \mathbf{4 6} 6^{\circ} \end{array}$ | $: \begin{aligned} & \text { Lat. } \\ & \mathbf{4 8}^{\circ} \end{aligned}$ | $\begin{aligned} & \text { Lat. } \\ & \mathbf{5} 0^{\circ} \end{aligned}$ | $\frac{\text { Lat. }}{\mathbf{5 2}}$ | $\frac{\text { Lat. }}{54^{\circ}}$ | $\left\lvert\, \begin{gathered} \text { Lat. }^{\circ} \\ \mathbf{5} \mathbf{6}^{\circ} \end{gathered}\right.$ | $: \begin{aligned} & \text { Lat } \\ & \mathbf{5 8} \\ & \hline \end{aligned}$ | $\begin{aligned} & \text { Lat } \\ & 60^{\circ} \end{aligned}$ |  |
| 27 |  | 10. | 12.5 | 15.2 | 18.2 | 21.6 | $25 \cdot 4$ | 297 | 34.6 | $40 \cdot 2$ |  |
| ＝ 25 | 10 | 11.6 | 14.1 | 16.9 | 20.0 | 23.5 | 27.4 | 31.8 | 36.8 | 42.5 |  |
| $\bigcirc$ | 20 | 13.1 | 15.6 | $18 \cdot 5$ | 21.6 | 25.2 | 29.2 | $33 \cdot 7$ | 38.8 | 44.6 |  |
| 空 ${ }^{\text {Ha }}$ | 30 | 14.4 | 17.0 | 19.9 | 23.1 | 26.7 | 30.8 | 35.4 | 40.6 | 46.5 |  |
| －518 | 40 | 15.6 | 18.2 | 21.2 | 24.5 | 28.1 | 32.3 | 37.0 | 42.2 | $48 \cdot 3$ |  |
| 最 16 | 50 | 16.6 | 19.3 | 22.3 | $25 \cdot 6$ | 29.4 | 33．6 | 38．3 | $43 \cdot 7$ | 49.8 |  |
| 边 13 | 1900 | 17.5 | 20.2 | 23.3 | 26.6 | 30．4 | 34.7 35 | 39.5 | 44.9 | 51．2 |  |
| 08 | $1 \begin{aligned} & 10 \\ & 20\end{aligned}$ | 18.2 18.8 | 21.0 21.6 | 24.1 24.7 | 27.5 28.2 | 31.3 32.0 | 35.6 36.4 | 40.5 41.3 | $\begin{aligned} & 46 \cdot 0 \\ & 46 \cdot 8 \end{aligned}$ | 52.3 53.2 |  |
| 06 | 30 | 19.3 | 22.1 | 25.2 | 28.7 | 32.6 | 37. | 41．9 | 47.5 | 53．9 |  |
| 03 | 40 | 19.6 | 22.4 | 25.5 | 29.0 | 33.0 | 37.4 | 42.3 | 48.0 | 54.4 |  |
| 01 | 50 | 19.7 | 22.6 | 25.7 | 29.2 | 33.2 | 37.6 | $42 \cdot 6$ | 48.2 | 54.7 |  |
| 02 | 2000 | 19.7 | 22.6 | 25.7 | 29.2 | 33.2 | 37.6 | 42.6 | 48.2 | 54.7 |  |
| 04 | 10 | 19.6 | 22.4 | 25.6 | 29.1 | 33.0 | 37.4 | 42.4 | 48.1 | 54.6 |  |
| 07 | 20 | 19.3 | 22.1 | 25.2 | 28.7 | $32 \cdot 7$ | 37.1 | 42.0 | 47.7 | 54.2 |  |
| 09 | 30 | 18.8 | 21.6 | 24.7 | 28.2 | 32.2 | $36 \cdot 5$ | 41.5 | 47.2 | 53.6 |  |
| 12 | 40 | 18.2 | 21.0 | 24.1 | 27.6 | 31.5 | 35.8 | 40.7 | 46.4 | 52.8 |  |
| 14 | 50 | 17.4 | 20.2 | 23.3 | 26.7 | 30.6 | 34.9 | 39.8 | 45 | 51．7 |  |
| 16 | 2100 | 16.5 | 19.3 | 22.3 | $25 \cdot 7$ | 29.5 | 33.8 | 38.7 | 44.2 | 50.5 |  |
| 19 | 10 | 15.5 | 18.2 | 21.2 | 24.6 | 28.3 | 32.6 | 37.3 | 42.8 | 49.0 |  |
| 21 | 20 | 14.3 | 16.9 | 19.9 | 23.2 | 26.9 | 31.1 | 35.8 | 41.2 | 47.3 |  |
|  | 30 | 12.9 | 15.5 | 18.5 | 21.7 | 25.4 | 29.5 | 34.1 | 39.4 <br> 37.4 | 45.4 |  |
| 28 | 50 | 11.4 09.8 | 14.0 12.3 | 16.9 15.1 | 20.1 | 23.7 21.8 | 27.7 | 32.2 30.2 | 37.4 35.2 | 43.3 41.0 |  |
| － 30 | 2200 | 08.0 | 10.5 | 13.2 | $16 \cdot 3$ | 19.7 | 23.5 | 27.9 | 32.9 | 38.5 |  |
| － 32 | 10 | 06.1 | 08.5 | 11.2 | 14.2 | 17.5 | 21.3 | 25.5 | 30.3 | $35 \cdot 9$ |  |
| 34 | 20 | 04.1 | 06.4 | 09.0 | 11.9 | 15.1 | 18.8 | 22.9 | 27.6 | 33.0 |  |
| － 36 | 30 | 02.0 | 042 | 06.7 | $09 \cdot 5$ | 12.6 | 16.2 | 20.2 | 24.7 | 29.9 |  |
| 38 | 40 | 59.7 | O1．9 | 043 | 07．0 | 10.0 | 13.4 | 17.2 | $21 \cdot 6$ | 26.6 |  |
| 40 | 50 | 57.3 | 59.4 | 01.7 | 04.3 | 07.2 | 10.5 | 14.1 | 18. | 23.2 |  |
| 42 | 2300 | 54.8 | 56.8 | 59.0 | 01．5 | 04.3 | 07.4 | $10 \cdot 9$ | 15.0 | 19.6 |  |
| 44 | 10 | 52.2 | 54.1 | 56.2 | 58.6 | 01.2 | 04.2 | 07.6 | 11.4 | 15.8 |  |
| 45 | 20 | 49.5 | 51.3 | 53.3 | 55．5 | 58.0 | 00.9 | 04.1 | 07.7 | $11 \cdot 9$ |  |
| 47 | 30 | 46.7 | 48.4 | 503 | 52.4 | 54.7 | 57.4 | 00.5 | 03．9 | 07.8 |  |
| 48 | 40 | 43.8 | 45.4 | 47.1 | 49.1 | 51．3 | 53.8 | 56.7 | 59.9 |  | $\bigcirc$ |
| 49 | 50 | 40.8 | 42.2 | 43.9 | 45.8 | 47 | 50.2 | 52.8 | 55.8 | 59.3 |  |

## ALTITUDE AND AZIMUTH OF POLARIS

November，December ．．．．．．．．．． 1951
September，October ．．．．．．．．．．．．．． 1952
July，August 1953

## POLE STAR OBSERVATION FOR AZIMUTH

The maximum error in the azimuth of the Pole star, as determined from the field tables, is about 0.5 minutes. Where azimuth observations are required to an accuracy of one or two minutes only, the calculations may be made from the data contained in them.

For all astronomical work, whether for time or azimuth, the instrument must be very firmly set up and carefully levelled. In survey ing a lot, subdividing a township, or making a traverse, an observation for azimuth is made with one of the survey lines as a reference line. In each case the bearing by account is generally near enough to the azimuth for the purpose of setting the instrument and finding the star. The reference object should be about one half mile or more distant, so that it can be sighted without parallax when the instrument is at solar focus.

## SPECIMEN OBSERVATIONS

Specimen azimuth observations are shown on page 4 The following notes refer to the observation on the upper half of the page.

The observation is supposed to have been made with a double centre transit to determine the bearing of the westerly boundary of lots 14 to 20 , range 1 , in the townsite of Waskesiu very near the northeast corner of section 8 , township 57 , range 1 , west of the 3 rdinitial meridian. The bearing of this reference line is to be referred to the astronomical meridian through the centre of the township.

The bearing of the reference line is known to be about $177^{\circ} 08^{\prime}$. Set the vernier of the horizontal circle to read $177^{\circ} 08^{\prime}$. Using the lower clamp and tangent screw, and with the vertical circle to the right, direct the telescope on the reference line. Read verniers A and B and enter their mean under the heading, H.C.R. on Ref. Line, Circle Right ( $177^{\circ} 08^{\prime}$ ).

From Table X, in this Supplement, note that the latitude of the place is $53^{\circ} 55^{\prime}$; the sidereal watch reads about 19 h 10 m . Entering the Astronomical Field Tables (Figure I), with these arguments, the azimuth of the Pole star is given as $1^{\circ} 35^{\prime} .6$. Loosen the upper clamp and set the vernier of the horizontal plate to read $1^{\circ} 35^{\prime}$.

At sidereal time $19 \mathrm{~h} 10^{\mathrm{m}_{\mathrm{it}}}$ is necessary to subtract $1^{1}$ from the latitude of the place to obtain the altitude of Polaris (Figure I); therefore, the altitude is $53^{\circ} 55^{\prime}-11^{\prime}=53^{\circ} 44^{\prime}$. Set the telescope at this altitude and Polaris should be in the field of view.

With the vertical tangent screw bring the star to a point immediately above or below the horizontal cross hair, then bisect the star with the vertical cross hair using the upper tangent screw. Note and enter the watch time of the bisection to the nearest second under the heading, Watch Time, Circle Right ( $19^{\mathrm{h}} 12^{\mathrm{m}} 16^{\mathrm{s}}$ ).

Since the vertical cross hair will entirely cover the star, it is good practice to make two or three trial pointings before making the final setting, which in this operation as in all others, should be made by

## POLE STAR OBSERVATIONS FOR AZIMUTH



| Date. Dec. $10^{\text {th }}$. 1951 Refline N.By.Sec.24,99,2, W. $6^{\text {th }}$ Place 20 Chs W. of NE:Cor.Sec, 24 - Approx, Lat. $57^{\circ} 37^{\prime}$ Observer d. Doe ....Instrument Wild T.S. No. 500 |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Circle | H.C.R. onRef line |  | H.C.R.on Polaris |  | Watch Time |  |
| Right | $295{ }^{\circ}$ |  | $23^{\circ}$ | $49^{\prime}$ | $9^{\text {h }} 58^{m}$ | $26^{5}$ |
| Left | 115 | 19 | 203 | 50 | 10 01 | 05 |
| Mean | 295 | 20 | $23^{\circ}$ | 49.5 | 959 | 45 |
| Tab.az.for 9 h .50 m Lat. $56^{\circ}$ <br> Difference for 8 m .35 s . <br> Difference for $1^{\circ} 37^{\prime}$ Lat. <br> Azimuth of Polaris |  |  |  | Watch Corr. Sid Time | $\begin{array}{ccc} - & 1 & 10 \\ 9 & 58 & 35 \end{array}$ |  |
|  |  |  |  | NOTE: If, after observation, any deflection in the line was made, the following information should be supplied. <br> Amount. 3.o Direction N Place NE.Cor: Sec, 21 |  |  |
|  |  |  | $\begin{array}{cc} 358^{\circ} & 30^{\prime} 2 \\ 23 & 49.5 \\ \hline \end{array}$ |  |
|  |  |  |  |  |  |  |  |
|  |  | $\begin{array}{r} -\quad 25 \quad 193 \\ 295 \quad 20 \end{array}$ |  |  |  |  |
| Correction to H.C.R. H.C.R.on Ref. line |  |  |  |  |  |  |  |  |
| Azimuth of Ref. line |  | $270^{\circ} 00.7$ |  |  |  |  |
| Convergence for $23 / 4 \mathrm{mi}$. |  | - 3.7 |  |  |  |  |
| Bearing of Ref. line |  | $269^{\circ}$ 57'0 |  |  |  |  |

turning the tangent screw in a positive, or clockwise, direction.

Read the horizontal plate verniers $A$ and $B$ and enter the mean under the heading, H. C. R. on Polaris, Circle Right ( $1^{\circ} 34^{\prime}$ ).

This completes only the first half of the observation because, for azimuth work, it is essential that all observations be made in the two positions of the instrument - circle right, and circle left.

To complete the observation, transit the telescope, set the horizontal circle to read the azimuth of Polaris plus $180^{\circ}$, and reset the altitude in the telescope's new position. Repeat the setting on Polaris as before, entering the Watch Time ( $19 \mathrm{~h} 15^{\mathrm{m}} 24^{\mathrm{s}}$ ), and the H.C.R. on Polaris ( $181^{\circ} 35^{\prime}$ ) in their respective places. Point the telescope on the reference line, read the horizontal circle, and enter under Circle Left ( $357^{\circ} 07^{\prime}$ ).

The observation is now complete and the next stepis to calculate the bearing of the reference line.

First, mean the horizontal circle readings on the reference line $\left(177^{\circ} 07!5\right)$, and on Polaris $\left(1^{\circ} 34!5\right)$, and mean the watch times ( $19^{\mathrm{h}} 13^{\mathrm{m}} 50^{\mathrm{s}}$ ). Let the correction to the watch be $+12^{\mathrm{s}}$, which makes the sidereal time $19^{\mathrm{h}} 14^{\mathrm{m}} 02^{\mathrm{s}}$ (methods of determining the watch correction are described in the next section).

From the Field Tables (Figure I), the azimuth of the Pole star at $19^{\mathrm{h}} 10^{\mathrm{m}}$ for latitude $52^{\circ}$ is $1^{\circ} 31!3$, and at $19^{\mathrm{h}} 20^{\mathrm{m}}$ it is $1^{\circ} 32!0$. By direct proportion, the correction to obtain the azimuth at $19{ }^{\mathrm{h}} 14^{\mathrm{m}} 02^{\mathrm{s}}$ is $+0!3$. Similarly, by direct proportion, the correction to obtain the azimuth at latitude $53^{\circ} 55^{\prime}$, the latitude of the place, is $+4!1$. With these corrections, the azimuth of Polaris is $1^{\circ} 35!7$ at the mean time of the observation.

The mean H. C. R. on Polaris, as calculated above, is $1^{\circ} 34!5$. Therefore, the correction to the horizontal circle readings to give true azimuths is $1^{\circ} 35!7-1^{\circ} 34!5=+1!2$. Applying this correction to the mean H. C. R. on the reference line gives the true azimuth as $177^{\circ} 08!7$.

The reference meridian for the survey is the meridian through the centre of the township, almost exactly 1 mile east of the observation point. From the convergence scale of the Field Tables (Figure 2), the convergence per mile at latitude $53^{\circ} 55^{\prime}$ is $1!2$. The bearing of the reference line referred to the central meridian of the township is, therefore, $177^{\circ} 08!7+1!2=177^{\circ} 09!9$, or $177^{\circ} 10^{\prime}$.

Since the observation was taken for the purpose of determining the bearing only of a survey line, no deflection is to be made and the word 'nil' is entered under the proper heading.

The observation on the lower half of page 4 is supposed to have been taken on the north boundary of section $24, \mathrm{Tp} .99, \mathrm{R} .2, \mathrm{~W}$. of the 6th meridian. The chord has been run east from the control meridian and the place of observation was a point 20 chains W . of the N.E.
corner of the section, the reference line lying to the west. Because a single-centre instrument was used, the horizontal plate could not be set to read azimuths.

The bearing of the reference line is known to be about $270^{\circ} 00^{\prime}$ making the horizontal circle reading of $295^{\circ} 21^{\prime}$ on the line $25^{\circ} 21^{\prime}$ greater than the true bearing. From the Astronomical Field Tables (the relevant page is not reproduced here), the azimuth of Polaris for the time and place is about $358^{\circ} 34^{\prime}$, to which must be added $25^{\circ} 21^{\prime}$ to obtain the setting for the horizontal plate in order to find the star.

The remainder of the proceedings and calculations are the same as in the preceding observation, except that as the bearing of the line is required to be $270^{\circ} 00^{\prime}$ it must be corrected by a deflection northward of 3 minutes. Entries recording deflections must never be neglected.

## DETERMINATION OF THE WATCH CORRECTION

The watch may be set approximately to local sidereal time by several means. One of these, which involves a knowledge of the local standard time, is given in the Astronomical Field Tables (Figure 2). If the standard time is not known it may be possible to obtain it from radio time signals. Otherwise, an approximation of apparent mean noon and hence of mean noon may be obtained by observing the time of the highest altitude of the sun.

A convenient fact to remember is that around March 21 the local mean time and the local sidereal time are the same, and about September 21 the sidereal time is 12 hours in advance of mean time.

When the sidereal time is known approximately, an observation on the Pole star for azimuth will provide an approximate value of the astronomical meridian on which the time may be observed by the methods that follow. The watch correction thus obtained may be used in re-working the azimuth observation and thus providing a more accurate meridian from which the time may be re-observed. The watch correction should be determined shortly before or after an azimuth observation.

## DETERMINATION OF THE WATCH CORRECTION by The meridian transit of a star

During the progress of a survey the bearings of the lines ${ }^{\circ}$ surveyed are known. By applying the convergence from the reference meridian, the azimuth of the astronomical meridian may be calculated and the instrument set on it. If the telescope is then set at the altitude of a time star, the watch time of its transit of the meridian may be observed. The sidereal times of transit of a number of suitable stars are given in the Astronomical Field Tables (not reproduced here). The magnitudes and Polar distances of the stars are also given. The altitude on the meridian to the south may be calculated by subtracting the Polar distance from the supplement of the latitude. In selecting a time star to be used in daylight, it should be remembered that stars are more difficult to see south of the zenith than north of it, and that this difficulty increases as the altitude decreases.

## ASTRONOMICAL FIELD TABLES



CONVERSION STANDARD TIME TO LOCAL SIDEREAL TIME CONVERGENCE
FIGURE 2

For the first specimen of azimuth observation given in the preceding section, a convenient star would be a Aquilae. From the Astronomical Field Tables, its magnitude is 1 , so that it should be easy to see; its time of meridian transit, for Nov. 15, 1951, is $19 \mathrm{~h} 48^{\mathrm{m}} 26^{\mathrm{s}}$, which allows sufficient time to set the transit after completing the azimuth observation.

Its altitude on the meridian is $\left(180^{\circ}-53^{\circ} 55^{\prime}\right)-81^{\circ} 16^{\prime}$ (Polar distance $)=44^{\circ} 49^{\prime}$.

The bearing by account of the reference line is $177^{\circ} 08^{\prime}$ and the convergence to be subtracted to obtain azimuths is $1^{t} .2$. Set the horizontal circle at $177^{\circ} 07^{\prime}$, and sight it on the reference line with the lower horizontal movement. Loosen the upper clamp and turn the transit so that the vernier reads $180^{\circ}$. Set the altitude of the star on the vertical circle. Note the time the star passes the vertical wire of the transit, and calculate the watch correction:

| Watch time of meridian transit of a Aquilae | $19^{\mathrm{h}} 48^{\mathrm{m}} 14^{\mathrm{s}}$ |
| :---: | :---: |
| Sidereal time of meridian transit |  |
| of a Aquilae | $\underline{19 \mathrm{~h}} 48^{\mathrm{m}} 26^{\mathrm{s}}$ |
| Watch Correction, Nov. 15 |  |
| (watch slow) . . . . . . . . . . . . + | $12^{5}$ |

Since, from the azimuth observation, the bearing by account was in error by about 2 minutes of arc, the setting for the astronomical meridian was also in error by 2 minutes. The error thus introduced into the watch correction can be calculated from the formula $4 \times 2 \mathrm{x} \cos \mathrm{h} \sec \delta$ in seconds, where $h$ and $\delta$ are the altitude and declination of the star respectively. Substituting, we get, $4 \times 2 \times \cos$ $44^{\circ} 49^{\prime} \sec 8^{\circ} 44^{\prime}=5.6$ seconds. Since the bearing by account is $2 \mathrm{~min}-$ utes less than the true bearing, the azimuth of the astronomic meridian set on the transit will actually be $180^{\circ} 02^{\prime}$ and the sidereal time of the observation will be $19^{\mathrm{h}} 48^{\mathrm{m}} 32^{\mathrm{s}}$ instead of $19^{\mathrm{h}} 48^{\mathrm{m}} 26^{\mathrm{s}}$ making the watch correction $+18^{5}$.

The effect of the error of the watch correction ( 5.6 seconds) on the observed azimuth would be less than .01 minute and can be neglected.

To obtain the watch correction for the second specimen azimuth observation, a Leonis, magnitude 1.3 , would be suitable although there is a very small interval of time after the completion of the observation to set the transit for the time star. The reading on the horizontal circle when the transit is set on the reference line with circle right is $295^{\circ} 21^{\prime}$. The bearing of the reference line by account is $270^{\circ} 00^{\prime}$ and the convergence to the reference meridian is $3!7$. The azimuth of the reference line is therefore $270^{\circ} 03!7$. The angle between the reference line and the astronomic meridian towards the south is $270^{\circ} 03!7-180^{\circ} 00^{\prime}=$ $90^{\circ} 0317$. To set the transit in the astronomic meridian, the horizontal plate should be made to read $295^{\circ} 21^{\prime}-90^{\circ} 03!7=205^{\circ} 17^{\prime}$. The altitude setting for the time star is $\left(180^{\circ}-57^{\circ} 37^{\prime}\right)-77^{\circ} 48^{\prime}=44^{\circ} 35^{\prime}$.

| of a Leonis | $10^{\mathrm{h}} 07 \mathrm{~m} 00^{\text {s }}$ |
| :---: | :---: |
| Sidereal time of meridian transit of a Leonis .. | $10^{\mathrm{h}} 05^{\mathrm{m}} 50^{\mathrm{s}}$ |
| Watch Correction, Dec. 10 (watch fast). | $-1^{\mathrm{m}} 10^{\mathrm{s}}$ |

The correction due to the error in azimuth is not calculated.

## DETERMINATION OF WATCH CORRECTION BY THE MERIDIAN TRANSIT OF THE SUN

The determination of the watch correction by means of the meridian transit of the sun is convenient for several reasons: the observation is made at a time of the day when the instrument is usually on line and can be readily set in the meridian; there is no difficulty in finding the sun; and it can be observed through light clouds or haze when stars are invisible.

The observation is simple. The telescope is set in the meridian as for a star, and the sun glass is attached to the eyepiece. The watch time when each limb of the sun crosses the vertical thread of the diaphragm is noted, and the mean of the two gives the watch time of the meridian transit. The sidereal time of the observation equals the Apparent Right Ascension of the Sun. It may be calculated from Apparent Right Ascension and Declination of the Sun, in the Astronomical Field Tables - a sample page of which is shown in Figure 3.

An example, based on the first specimen of azimuth observation, follows:


From the Astronomical Field Tables (Figure 3), the sun's apparent right ascension at Greenwich apparent noon, on Nov. 15, 1951, is $15 \mathrm{~h} 19^{\mathrm{m}_{33} \mathrm{~s}}$, and the variation in 1 hour is +10 S 2 .

The place of observation is near the northeast corner of section 8 , township 57, range 1 , west of the 3 rd initial meridian. The longitude of the third meridian is $106^{\circ} \mathrm{W}$. The northeast corner of section 8 is four sixths of a range west of the 3 rd meridian, which from Table IV of the Supplement, is 0.1 degrees ( $05^{\prime} 57^{\prime \prime}$ ) west, fixing the longitude of the place as $106.1^{\circ} \mathrm{W}$.

The elapsed time equivalent to 106.1 degrees of longitude

## ASTRONOMICAL FIELD TABLES THE SUN'S APPARENT RIGHT ASCENSION

at Greenwich apparent noon and variation for one hour.

| 등 | 1951 |  |  |  |  |  |  |  | 든 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 合 | September |  | October |  | November |  | December |  | $\stackrel{+}{\circ}$ |
|  | h. m. ${ }_{\text {s. }}$ | 9. | h. m. ${ }^{\text {m }}$ | s. |  | s. | m. s. | s. |  |
| 1 | 103933 | 9.1 | 122731 | $9 \cdot 0$ | 142329 | 9.8 | 162658 | 10.8 | 1 |
| 2 | 104311 | 9.1 | 123108 | 9.1 | 142724 | 9.8 | 163117 | $10 \cdot 8$ | 2 |
| 3 | 104648 | 9.1 | 123446 | $9 \cdot 1$ | 143120 | 9.8 | 163536 | $10 \cdot 8$ | 3 |
| 4 | 105025 | 9.0 | 123824 | $9 \cdot 1$ | 143516 | 9.9 | 163957 | 10.9 | 4 |
| 5 | 105402 | 9.0 | 124202 | $9 \cdot 1$ | 143914 | 9.9 | 164418 | $10 \cdot 9$ | 5 |
| 6 | 105739 | 9.0 | 124541 | 9.1 | 144312 | 9.9 | 164839 | 10.9 | 6 |
| 7 | 1110115 | 9.0 | 124920 | $9 \cdot 1$ | 144711 | 10.0 | 165301 | 10.9 | 7 |
| 8 | 110451 | 9.0 | 125259 | 9.1 | 145111 | 10.0 | 165724 | 10.9 | 8 |
| 9 | 110827 | 9.0 | 125639 | $9 \cdot 2$ | 145512 | 10.0 | 170146 | 11.0 | 9 |
| 10 | 111203 | 9.0 | $\begin{array}{llll}13 & 00 & 19\end{array}$ | $9 \cdot 2$ | 145913 | 10.1 | 170610 | 11.0 | 0 |
| 11 | 111539 | 9.0 | 130359 | 9.2 | 150315 | 10.1 | 171034 | 11.0 | 11 |
| 12 | 111914 | 9.0 | 130740 | $9 \cdot 2$ | 150719 | $10 \cdot 2$ | 171458 | 11.0 | 2 |
| 13 | 112249 | 9.0 | 131122 | $9 \cdot 2$ | 151123 | $10 \cdot 2$ | 171922 | 11.0 | 3 |
| 14 | 112625 | $9 \cdot 0$ | $13 \quad 1504$ | 9.3 | 151528 | 10.2 | 172347 | 11.0 | 4 |
| 15 | 113000 | 9.0 | 131846 | $9 \cdot 3$ | 151933 | $10 \cdot 2$ | 172812 | 11.1 | 5 |
| 16 | 113335 | 9.0 | 132229 | $9 \cdot 3$ | 152340 | $10 \cdot 3$ | 173238 | 11.1 | 6 |
| 17 | 113710 | 9.0 | 132613 | $9 \cdot 3$ | 152747 | $10 \cdot 3$ | 173704 | 11.1 | 17 |
| 18 | 114045 | 9.0 | 132957 | $9 \cdot 4$ | 153156 | 10.4 | 174130 | 11.1 | 18 |
| 19 | 114420 | 9.0 | 133342 | 9.4 | 153605 | 10.4 | 174556 | 11.1 | 9 |
| 20 | 114755 | 9.0 | 133728 | 9.4 | 154015 | 10.4 | 175022 | 11.1 | 20 |
| 21 | 115131 | 9.0 | 134114 | $9 \cdot 4$ | 154426 | 10.5 | 175449 | 11.1 | 21 |
| 22 | 115506 | 9.0 | 134501 | 9.5 | 154837 | 10.5 | 175915 | 11.1 | 22 |
| 23 | 115841 | 9.0 | 134848 | 9.5 | 155250 | 10.5 | 180342 | 11.1 | 23 |
| 24 | 120217 | 9.0 | 135236 | 9.5 | 155703 | 10.6 | 180808 | 11.1 | 24 |
| 25 | 120553 | 9.0 | 135625 | 9.6 | $\begin{array}{llll}16 & 01 & 17\end{array}$ | 10.6 | 181235 | 11.1 | 25 |
| 26 | 120929 | 9.0 | 140015 | 9.6 | 160532 | 10.6 | 181702 | 11.1 | 26 |
| 27 | 121305 | 9.0 | 140405 | 9.6 | 160948 | $10 \cdot 7$ | 182128 | 11.1 | 27 |
| 28 | $\begin{array}{lllll}12 & 16 & 41\end{array}$ | 9.0 | 140757 | 9.6 | 161404 |  | 182554 | 11.1 | 28 |
| 29 | 122017 | 9.0 | 141149 | 9.7 | 161821 | $10 \cdot 7$ | 183021 | 11.1 | 29 |
| 30 | 122354 | 9.0 | 141541 | 9.7 | 162239 | $10 \cdot 8$ | 183447 | 11.1 | 30 |
| 31 | 122731 | 9.0 | 141935 | 9.7 | 162658 | 10.8 | 183912 | 11.1 | 31 |
| 32 |  |  | 142329 | 9.8 |  |  | 184338 | 11.1 | 32 |

is equal to $\frac{106.1}{15}=7.07$ hours. The variation in the sun's apparent right ascension in 7.07 hours is $7.07 \times 10.2=+72^{s}=+1^{\mathrm{m}} 12^{\mathrm{s}}$. Hence the sidereal time at apparent noon is $15^{\mathrm{h}} 19^{\mathrm{m}} 33^{\mathrm{s}}+1^{\mathrm{m}} 12^{\mathrm{s}}=15^{\mathrm{h}} 20^{\mathrm{m}} 45^{\mathrm{s}}$, and the watch correction is $15^{\mathrm{h}} 20^{\mathrm{m}_{4}} 45^{\mathrm{s}}-15^{\mathrm{h}} 20^{\mathrm{m}_{3}} 33^{\mathrm{s}}=+12^{\mathrm{s}}$ (watch slow).

An error of 0.1 degrees, or 6 minutes, in longitude would give an error in time of only 1 second.

A nother example, based on the second specimen of azimuth observation, follows:


Sidereal time at apparent noon

$$
=17^{\mathrm{h}} 06^{\mathrm{m}} 10^{\mathrm{s}}+11.0 \times \frac{(118+0.2)}{15}=17^{\mathrm{h}} 07^{\mathrm{m}} 37^{\mathrm{s}}
$$

Watch correction (watch fast) $=\quad-1^{\mathrm{m}} 10^{\mathrm{s}}$
If, because of clouds or other reason, only one limb can be observed, a correction for the semi-diameter of the sun in sidereal time has to be applied to the observed watch time to give the watch time of transit. The semi-diameter of the sun in sidereal time is given in the Nautical Almanac for every day of the year.

Suppose, for example, that in the above observation the watch time for the first limb only was observed. In that case, it would be necessary to add the semi-diameter in sidereal time, namely $1^{\mathrm{m}} 10^{\mathrm{s}} .86$, to the watch time, $17^{\mathrm{h}} 07^{\mathrm{m}} 36^{\mathrm{s}}$, to give the watch time of transit, $17^{\mathrm{h}}$ $08^{\mathrm{m}} 47^{\mathrm{s}}$. If the second limb only were observed the semi-diameter would have to be subtracted.

## DETERMINATION OF THE WATCH CORRECTION BY RADIO TIME SIGNALS

Radio time signals are broadcast from a number of stations in Canada and United States on several wave lengths and at various times. The stations which are most used by surveyors in Canada are stations of the CBC network; CHU Ottawa; NSS Washington; NPG San Francisco; and WWV Washington.

As an example of the usage of time signals, suppose the 1 p. m., Eastern Stand Time, radio signal by CBC were to be heard at the place of the first specimen of azimuth observation, longitude $106^{\circ}$ $05^{\prime} 57^{\prime \prime}$, on Nov. 15 , at watch time $14^{\mathrm{h}} 31^{\mathrm{m}} 27^{\mathrm{s}}$.

| From the Astronomical Field Tables (Figure 2) the sidereal time at noon Eastern Standard Time, Nov. 11 is . | $15^{\mathrm{h}} 20^{\mathrm{m}} 07^{\mathrm{s}}$ |
| :---: | :---: |
| Correction for 4 days $=4 \times 3{ }^{\text {m }}$ 56. ${ }^{\text {s }} 6$ | $15^{\mathrm{m}} 46^{\text {s }}$ |
| Correction to 1 p.m. $=1+\frac{1}{24} \times 3^{m_{5}} 56^{s} .6 \ldots$. | $1^{\mathrm{h}} 00^{\mathrm{m}} 10^{\text {s }}$ |
| Sidereal time of signal, longitude $75^{\circ}$ | $16^{\mathrm{h}} 36^{\mathrm{m}} 03^{\mathrm{s}}$ |
| $\begin{aligned} \text { Correction for longitude } & =106^{\circ} 05^{\prime} 57^{\prime \prime}-75^{\circ} \\ & =31^{\circ} 05^{\prime} 57^{\prime \prime} \ldots \ldots \end{aligned}$ | $2^{\mathrm{h}} 04^{\mathrm{m}} 24^{\mathrm{s}}$ |
| Sidereal time of signal at place of observation | $14^{\mathrm{h}} 31^{\mathrm{m}} 39^{\mathrm{s}}$ |
| Watch time of signal | $14^{\text {h }} 31^{\mathrm{m}} 27^{\mathrm{s}}$ |
| Hence, watch correction is | $12^{5}$ |

## THE WATCH RATE

It will be noted in the two specimen observations for azimuth that comparatively large errors in the watch correction have little effect on the resulting azimuth. An influence on the error is the apparent rate of travel of Polaris. In the first specimen, this rate is 0.7 minutes of arc in 10 minutes of time and in the second 2.7 minutes in 10 minutes. At the times of upper and lower culmination, i.e., at about $1^{\mathrm{h}} 50^{\mathrm{m}}$ and $13^{\mathrm{h}} 50^{\mathrm{m}}$, the rate is about 4.6 minutes of arc for 10 minutes of time, or nearly 30 seconds of arc for 1 minute of time. In the more precise observations for governing surveys where azimuths are calculated to seconds, it is evident that the watch correction is required with considerable accuracy.

It is not always possible to obtain a time observation and the accuracy of a critical azimuth observation may be adversely effected by an inaccurate watch correction. As a precautionary measure it is good practice to observe for time as opportunities occur, and establish a daily rate for the watch. The daily rate of the watch is the number of seconds it gains or loses in 24 hours. This can be done quite simply in surveying a meridian but in all other surveys allowance must be made for changes in longitude.

In the preceding specimen observations for azimuth a watch correction of $+12^{\mathrm{s}}$ was determined at longitude $106^{\circ} 05^{\prime} 57^{\prime \prime}$ at $15^{\mathrm{h}} 20^{\mathrm{m}}$ $45^{s}$ on Nov. 15 (page 8 ) and again determined as $-1^{\mathrm{m}} 10^{\mathrm{s}}$ at longitude $118^{\circ} 10^{\prime} 12^{\prime \prime}$ at $17^{\mathrm{h}} 07^{\mathrm{m}_{37} \mathrm{~s}}$ on Dec. 10 (page 9). At the instant of the second determination, the sidereal time at the place of the first observation would be $17^{\mathrm{h}} 07^{\mathrm{m}} 37^{\mathrm{s}}+\frac{1}{15}\left(118^{\circ} 10^{\prime} 12^{\prime \prime}-106^{\circ} 05^{\prime} 57^{\prime \prime}\right)=17^{\mathrm{h}} 55^{\mathrm{m}}$ $54^{\mathrm{s}}$. The watch correction would be $17^{\mathrm{h}} 55^{\mathrm{m}} 54^{\mathrm{s}}-17^{\mathrm{h}} 08^{\mathrm{m}} 47^{\mathrm{s}}=+0^{\mathrm{h}}$ $47^{\mathrm{m}_{07}} \mathrm{~s}$. This means that the watch has lost $47^{\mathrm{m}} 07^{\mathrm{s}}-12^{\mathrm{s}}=46^{\mathrm{m}} 5^{\mathrm{s}}$ in $25+\left(17^{\mathrm{h}} 55^{\mathrm{m}}-15^{\mathrm{h}} 20^{\mathrm{m}}\right) \frac{1}{24}=25.1$ days, giving a daily rate of $-46^{\mathrm{m}} 55^{\mathrm{s}}$ $\div 25.1$ or $-1^{\mathrm{m}} 52$. ${ }^{\mathrm{s}} 1$

It is not probable that the watch would maintain an even daily
loss of $1^{\mathrm{mi}} 52.1$ over such a long period as 25 days. In order to establish a reliable rate, time should be observed at intervals not exceeding three or four days.

To illustrate the usage of the daily rate, suppose an azimuth observation was taken at $20^{\mathrm{h}} 30^{\mathrm{m}}$ on Dec. 12 at longitude $119^{\circ} 06^{\prime} 1^{\prime \prime}$, and that it is required to know the watch correction at the time of the observation.

$$
\begin{aligned}
& \text { Watch correction, longitude } 118^{\circ} 10^{\prime} 12^{\prime \prime} \\
& \text { at } 17^{\mathrm{h}} 07^{\mathrm{m}} \text {, Dec. } 10 \ldots . . . . . . . . . . . . . . . . \\
& \text { Daily rate, - } 1^{\mathrm{m}} 52 . \frac{\mathrm{S}}{} \text {; elapsed time, } 2.1 \\
& \text { days; watch loss ........................................ } 3^{\mathrm{m}}{ }_{55^{s}} \\
& \text { Watch correction at time of observation } \ldots \ldots . . .+2^{\mathrm{m}} 45^{\text {s }} \\
& \text { Correction for longitude }= \\
& \text { Hence, required watch correction is . . . . . . . . . . . . . . }-0^{m} 59^{s}
\end{aligned}
$$

## OBSERVATION OF THE SUN FOR AZIMUTH

It may happen that star observations are prevented by smoke, haze, or light clouds, and the only method available for the determination of azimuth will be observation on the sun.

The method is not recommended when Polaris can be observed because it is not as accurate and, as it involves more calculation, is subject to a greater number of errors.

The following explanation is based on the use of the inverted eyepiece.

The instrument, carefully set up at the station and levelled, is directed on the reference line, and the horizontal circle is read and recorded, as usual, under the heading H. C. R. on Ref. Line. A sun glass must then be attached to the eyepiece and the instrument directed on the sun.

The next few steps are easy enough if performed methodically. Ingeneral, they consist of placing the image of the sun in the angle formed by the crosshairs of the diaphragm, first in the upper left quadrant with the instrument in circle right position, second in the lower right quadrant with the instrument in the circle left position (Figure 4). This procedure should be followed for an observation in the forenoon. In the afternoon, the other two remaining quadrants, as shown in Figure 5 should be used. In each case, when the cross hairs are tangent to the limbs of the sun's image, the circle readings and the approximate time should be taken.


Rules for Observing:

1. Commence with the sun on the leftof the vertical thread and impinging uponit, above the horizontal thread in the forenoon , and below in the afternoon.
2. Follow the sun with the slow motion screw of the vertical circle until the vertical thread also becomes tangent to the disc. The rules are reversed in the second position of the instrument.
3. Place the sun on the right of the vertical thread and imping ing upon the horizontal thread, below it in the forenoon and above it in the afternoon.
4. Follow the sun with the slow motion screw of the upper plate until the horizontal thread also becomes tangent to the disc.

The readings of the vertical circle on the sun, and of the horizontal circle on both the sun and the reference object, generally one of the line pickets, must be taken in both positions of the instrument and the approximate time of the observation noted.

## COMPUTATION FOR THE SUN OBSERVATION

The following formula may be used for the calculation:
$\cos \frac{a}{2}=\sqrt{\cos S \cos (S-P) \sec L \sec h}$
where $S=\frac{h+L+P}{2}$
$h^{\prime}=s^{\prime} n^{\prime} s$ true altitude,
$P=s^{\prime} n^{\prime} s$ polar distance,
$\mathrm{L}=$ latitude of observation station,
$a=$ angle sun makes with the meridian east or west from the north.

Then the azimuth from the north through the east, south and west is the same as "a" for forenoon observations, and is "360-a" for afternoon observations.

The latitude, and the logarithm of its secant, are given in Table $X$ for the north side of every section.

The first step in the reduction of the observation is to calculate the means of the time, H. C. R. on Reference Line, Sun's altitude, and H.C. R. on Sun. Sample observations are shown on pages 16 and 17 .

The sun's true altitude is the mean of the observed altitudes corrected for refraction and parallax. A table of the cormbined mean refraction and parallax is given in the Astronomical Field Tables, a sample page of which is shown in Figure 6. The table also gives the corrections which should be applied for temperature and pressure, in order to find the mean refraction and parallax for the atmospheric conditions at the time of observation. The mean of the observed altitudes is $28^{\circ} 51^{\prime} .5$ (Page 16 ). The combined refraction and parallax for this altitude is given as 1'.6 (Figure 6). For a barometric pressure of 29 inches a correction of -0.1 must be applied. For a temperature of $70^{\circ} \mathrm{F}$ a correction of $-0!1$ is required. Hence the resultant correction for refraction and parallax is 1'4 - a correction which must always be subtracted from the observed altitude. The sun's true altitude, therefore, is $28^{\circ} 50^{\prime} .1$. The means of the Times, H. C. R. on Ref. Line, and H. C. R. on Sun, can be written in directly.

The next step is to find the sun's Polar distance. The Astronomical Field Tables (Figure 7) give the sun's declination for $0^{h}$ Greenwich civil time, and the variation for one hour. We must therefore know the time after $0^{h}$ Greenwich civil time at which the observation was taken. This is obtained by adding the local standard time to the longitude of the reference meridian at the time zone, expressedin time. Thus in the example, the observation was taken at $7^{\mathrm{h}} 19^{\mathrm{m}} \mathrm{a} . \mathrm{m}$., mountain standard time ${ }^{\text {c }}$, which is the local time for longitude $105^{\circ} \mathrm{W}$, and is 7 h behind Greenwich civil time. The Greenwich civil time of the observation is therefore $7^{\mathrm{h}} 19^{\mathrm{m}}+7^{\mathrm{h}}=14^{\mathrm{h}} 19^{\mathrm{m}}=14^{\mathrm{h}}$. 3 . From the table (Figure 7) the sun's declination at $0^{h}$ on June 1,1951 , is $\mathrm{N} 21^{\circ} 54.9$, and the variation for one hour, $+0!36$. For $14 ?_{3}$ the variation is $5^{!} 1$. Thus, the declination North is $22^{\circ} 0^{\circ} 0$, that is, a Polar distance of $68^{\circ} 00^{\circ} 0$.

The observation sheet (page 16 ) shows a convenient method of working out the formula. The reduction of the observation then proceeds as for the observation on the Pole star, and needs no further explanation.

The best time for observing is when the sun is on the prime vertical, as an error in the altitude has then the least effect upon the azimuth. Useful observations, however, may be made at other times within certain limits set by practical considerations. For instance, it is considered that for an altitude lower than eight degrees the refraction correction is too uncertain in value. Again, the altitude should not be greater than that at which the rate of change of the azimuth is double the rate of change of the altitude because at higher altitudes, an error in altitude would produce too large an error in azimuth.

When local time is used, as in the observation on page 17 the approximate longitude of the place is required.

## SUN OBSERVATIONS FOR AZIMUTH



## SUN OBSERVATIONS FOR AZIMUTH



## ASTRONOMICAL FIELD TABLES

Table of corrections to Apparent Altitude of sun for REFRACTION and PARALLAX

| Mean Refraction Sun's Parallax Bar. $30^{\circ}$ Tem. $50^{7}$ |  | Correction to the Mean Refraction. |  |
| :---: | :---: | :---: | :---: |
|  |  | For Height of Barometer | For Height of Thermometer |
| App. <br> Alt. | Refr. | Barometer Reading (inches) | Thermometer Reading (Fahr.) |
|  |  | 262728293031 | $-10^{\circ} 10^{\circ} 30^{\circ} 50^{\circ} 70^{\circ} 90^{\circ}$ |
| $6^{\circ}$ | 8.4 | -1.1-0.8-0.6-0.3 $0.00+0.3$ | $+1.1+0.7+0.30 .0-0.3-0.6$ |
| 7 | 7.3 | $\begin{array}{lllll}-1.0 & 0.7 & 0.5 & 0.3 & 0.0+0.3\end{array}$ | $+1.00 .600 .30 .0-0.30 .5$ |
| 8 | 6.4 | $\begin{array}{llllll}0.9 & 0.6 & 0.4 & 0.2 & 0.0+0.2\end{array}$ | $\begin{array}{llllll}+0.9 & 0.6 & 0.3 & 0.0 & -0.2 & 0.5\end{array}$ |
| 9 | 5.7 | $\begin{array}{llllll}-0.8 & 0.6 & 0.4 & 0.2 & 0.0+0.2\end{array}$ | +0.8 0.5 |
| 10 | 5.2 | $\begin{array}{lllll}-0.7 & 0.5 & 0.4 & 0.2 & 0.0+0.2\end{array}$ | +0.7 055 |
| 11 | 4.7 | $\begin{array}{lllll}-0.6 & 0.5 & 0.3 & 0.2 & 0.0+0.2\end{array}$ | $\begin{array}{llllll}+0.6 & 0.4 & 0.2 & 0.0-0.2 & 0.4\end{array}$ |
| 12 | 4.3 | $\begin{array}{lllll}-0.6 & 0.4 & 0.3 & 0.2 & 0.0+0.2\end{array}$ | $\begin{array}{llllll}+0.6 & 0.4 & 0.2 & 0.0 & 0.2 & 0.3\end{array}$ |
| 13 | 4.0 | $\begin{array}{lllll}-05 & 0.4 & 0.3 & 0.1 & 0.0+0.1\end{array}$ | +0.5 $0.4 \begin{array}{llllll} & 0.2 & 0.0-0.2 & 0.3\end{array}$ |
| 14 | 3.7 | $\begin{array}{lllll}-0.5 & 0.4 & 0.3 & 0.1 & 0.0+0.1\end{array}$ | $\begin{gathered}+0.5 \\ 0.3 \\ 0.3 \\ 0.2\end{gathered} 00.0-0.10 .3$ |
| 15 | 3.4 | $\begin{array}{lllll}-0.5 & 0.3 & 0.2 & 0.1 & 0.0+0.1\end{array}$ | $+0.50 .30 .20 .0-0.1 .0 .3$ |
| 16 | 32 | $\begin{array}{lllll}-0.4 & 0.3 & 0.2 & 0.1 & 0.0+0.1\end{array}$ | +0.4 0.3 0.3 0.1 |
| 17 | 3.0 | $\begin{array}{lllll}-0.4 & 0.3 & 0.2 & 0.1 & 0.0+0.1\end{array}$ | +0.4 $0.31011000-0.1$ |
| 18 | 2.8 | $\begin{array}{lllll}-0.4 & 0.3 & 0.2 & 0.1 & 0.0+0.1\end{array}$ | +0.4 0.3 0.1 $0.10 .0^{-0.1} 10.2$ |
| 19 | 2.7 | $\begin{array}{lllll}-0.4 & 0.3 & 0.2 & 0.1 & 0.0+0.1\end{array}$ | +0.4 0.2 0.1 $00.0-0.1 \quad 0.2$ |
| 20 | 2.5 | $\begin{array}{lllll}-0.3 & 0.3 & 0.2 & 0.1 & 0.0+0.1\end{array}$ | $+0.30 .2 \begin{array}{lllll} \\ +0.1 & 0.0-0.1 & 0.2\end{array}$ |
| 25 | 1.9 | $\begin{array}{lllll}-0.3 & 0.2 & 0.1 & 0.1 & 0.0+0.1\end{array}$ | +0.3 0.2 0.1 0.1 |
| 30 | 1.5 | $\begin{array}{lllll}-0.2 & 0.2 & 0.1 & 0.1 & 0.0+0.1\end{array}$ | $+0.2 \begin{array}{lllll} \\ +0.2 & 0.1 & 0.0-0.1 & 0.1\end{array}$ |
| 35 | 1.3 | -0.2 00.1 0.1 00.0 | +0.2 0.1 0.1 $0.0 .0-0.100 .1$ |
| 40 | 1.0 | $\begin{array}{lllll}-0.1 & 0.1 & 0.1 & 0.0 & 0.0+0.0\end{array}$ | +0.1 0.1 0.1 |
| 45 | 0.9 | -0.1-0.1-0.1-0.0 0.0+0.0 | $+0.1+0.1+0.0 \quad 0.0-0.0-0.1$ |

FIGURE 6

## ASTRONOMICAL FIELD TABLES

THE SUN'S APPARENT DECLINATION
FOR Oh GREENWICH CIVIL TIME AND VARIATION FOR ONE HOUR
( 0 h Greenwich Civil Time is twelve hours before Greenwich Mean Noon of the same date)

|  | 1951 |  |  |  |  |  |  |  | 気 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 䦎 | May |  | June |  | July |  | August |  | - |
| 1 | N. $14^{\circ} 45^{\prime} \cdot 8$ | 0.77 | N. $21{ }^{\circ} 54.9$ | $0 \cdot 36$ | N. $23^{\circ} 11^{\prime} \cdot 0$ | $0^{\prime} \cdot 15$ | N. $18^{\circ} 17^{\prime} \cdot 2$ | $0 \cdot 62$ | 1 |
| 2 | $1504 \cdot 1$ | 0.76 | $2203 \cdot 2$ | 0.34 | 2307.2 | 0.17 | $18 \quad 02 \cdot 3$ | 0.63 | 2 |
| 3 | 1522.1 | 0.75 | $2211 \cdot 2$ | 0.32 | 2303.0 | 0.18 | 1747.0 | 0.64 | 3 |
| 4 | $\begin{array}{lll}15 & 39.9\end{array}$ | 0.74 | $2218 \cdot 8$ | 0.31 | 2258.4 | $0 \cdot 20$ | 1731.5 | 0.65 | 4 |
| 5 | 1557.5 | 0.72 | $22.26 \cdot 0$ | 0.29 | 2253.4 | 0.22 | $1715 \cdot 7$ | 0.66 | 5 |
| 6 | $\begin{array}{llll}16 & 14.7\end{array}$ | 0.71 | $2232 \cdot 8$ | $0 \cdot 28$ | 2248.0 | 0.23 | 1659.6 | 0.68 | 6 |
| 7 | $\begin{array}{lll}16 & 31.7\end{array}$ | 0.70 | $2239 \cdot 3$ | 0.26 | 2242.2 | 0.25 | $1643 \cdot 2$ | 0.69 | 7 |
| 8 | 1648.4 | 0.69 | $2245 \cdot 3$ | 0.24 | $2235 \cdot 9$ | 0.27 | $1626 \cdot 6$ | 0.70 | 8 |
| 9 | 1704.9 | 0.68 | 2250.9 | 0.23 | 2229.3 | 0.28 | $1609 \cdot 7$ | 0.71 | 9 |
| 10 | $17 \quad 21 \cdot 0$ | 0.67 | 2256.1 | 0.21 | 2222.4 | 0.30 | 1652.5 | 0.72 | 10 |
| 11 | 1736.9 | 0.65 | $2300 \cdot 9$ | 0.20 | $2215 \cdot 0$ | 0.32 | 1535.1 | 0.73 | 11 |
| 12 | 1752.5 | 0.64 | $23 \quad 05 \cdot 3$ | 0.18 | $2207 \cdot 2$ | 0.33 | $15 \quad 17 \cdot 4$ | 0.74 | 12 |
| 13 | $18 \quad 07.7$ | 0.63 | 2309.3 | 0.16 | 2159.1 | 0.35 | 1459.5 | 0.75 | 13 |
| 14 | 1822.7 | 0.62 | 2312.9 | 0.14 | $2150 \cdot 6$ | 0.36 | 1441.4 | 0.76 | 14 |
| 15 | 1837.4 | 0.61 | 2316.1 | 0.12 | 2141.7 | 0.38 | 1423.0 | 0.77 | 15 |
| 16 | 1851.7 | 0.59 | $2318 \cdot 9$ | 0.11 | 2132.5 | 0.39 | 1404.4 | 0.78 | 16 |
| 17 | $1905 \cdot 7$ | 0.58 | $2321 \cdot 2$ | 0.09 | $2122 \cdot 8$ | 0.41 | $1345 \cdot 6$ | 0.79 | 17 |
| 18 | 1919.4 | 0.56 | $2323 \cdot 2$ | 0.07 | 2112.9 | 0.42 | 1326.5 | 0.80 | 18 |
| 19 | 1932.8 | 0.55 | 23 24.7 | 0.06 | 2102.5 | 0.44 | $1307 \cdot 3$ | 0.81 | 19 |
| 20 | 1945.8 | 0.54 | $23 \quad 25 \cdot 8$ | 0.04 | $2051 \cdot 8$ | 0.44 | $1247 \cdot 8$ | 0.82 | 20 |
| 21 | 1958.5 | 0.52 | 23 26.6 | 0.02 | $2040 \cdot 8$ | 0.47 | $1228 \cdot 1$ | 0.82 | 21 |
| 22 | $2010 \cdot 8$ | 0.51 | $23.26 \cdot 9$ | 0.01 | 2029.4 | 0.48 | 1208.3 | 0.83 | 22 |
| 23 | 2022.9 | 0.49 | $2326 \cdot 8$ | 0.01 | 2017.7 | 0.50 | 1148.2 | 0.84 | 23 |
| 24 | 2034.5 | 0.48 | 23 26.2 | 0.03 | 2005.6 | 0.51 | 1128.0 | 0.85 | 24 |
| 25 | $2045 \cdot 8$ | 0.46 | 2325.3 | 0.05 | 1953.2 | 0.52 | 1107.5 | 0.86 | 25 |
| 26 | 2056.8 | 0.45 | 2323.9 | 0.06 | 1940.4 | 0.54 | 1046.9 | 0.86 | 26 |
| 27 | 2107.4 | 0.43 | $23 \quad 22 \cdot 2$ | 0.08 | $1927 \cdot 4$ | 0.55 | 1026.1 | 0.87 | 27 |
| 28 | 21.17 .6 | 0.42 | $2320 \cdot 0$ | 0.10 | 1914.0 | 0.56 | $\begin{array}{lll}10 & 05 \cdot 2\end{array}$ | 0.88 | 28 |
| 29 | $21 \quad 27.5$ | 0.40 | $23 \quad 17 \cdot 4$ | 0.12 | $1900 \cdot 2$ | 0.58 | $944 \cdot 1$ | 0.88 | 29 |
| 30 | 2137.0 | 0.39 | 2314.4 | 0.13 | 1846.2 | 0.59 | 922.8 | 0.89 | 30 |
| 31 | 2146.1 | 0.37 | N. 2311.0 | 0.15 | 1831.9 | 0.60 | 901.4 | 0.90 | 31 |
| 32 | N. 2154.9 | 0.36 |  |  | N. 18 17.2 | 0.62 | N. 839.8 | 0.90 | 32 |

FIGURE 7

In a sun observation for azimuth, the ordinary surveyor's transit reading to one minute cannot be expected to give results with an accuracy better than three or four minutes. Recently, however, a Solar PrismAttachment has been developed, and when placed over the object glass, it enables the observer to make more accurate pointings on the sun. With a good modern instrument, reading to one second in altitude and in azimuth and without a striding level, azimuths may be determined with an accuracy of about 20 seconds, or less, under good conditions. For results of this accuracy, forward and reverse determinations must be calculated separately, i.e., the altitudes and horizontal circle readings on the sun cannot be meaned, and the sun's declination should be obtained from the Nautical Almanac. The latitude of the place should be known within a tolerance of about 5 seconds.

## OBSERVATION OF POLARIS FOR AZIMUTH ON GOVERNING SURVEYS

Ongoverning surveys where great precision is required, the observation for azimuth is made with a six-inch transit theodolite, and the degree of accuracy required makes the observation somewhat more complex in all its details than the methods previously described. Some of the special precautions and refinements are mentioned below.

For accurate work a good solid set-up for the instrument is essential.

When it is intended to read the angle between two pointings on the horizontal circle, care should be taken, on turning the instrument in azimuth, to use the same forward or backward motion for each such pair. This tends to neutralize the effect of any yield in the instrument stand caused by that part of the impulse of revolution which passes down through the foot screws to the stand head.

Loose foot screws are a source of similar error. The pinch screws should always be tightened before finally adjusting the levelling screws, so that the latter turn stiffly in their nuts. Even though this may be less convenient to the observer in bringing quickly, and with nicety, the level bubbles to the desired position, it will eliminate with certainty one source of error.

The tangent and micrometer screws should always be turned so as to push against the counteracting spring, because, in turning in the opposite direction, the spring might fail to bring back the plate until some time in the interval between the observation and the reading of the drum.

The reference object for azimuth work should be, if possible, at such a distance that the telescope is at solar focus when the pointing is made on the reference object.

* For a description of the Solar Prism Attachment see Astronomy Applied to Land Surveying by R. Roclofs, 1950, N. V. Wed. J. Ahrend \& Zoon - Amsterdam, Holland.


## In observing for azimuth on governing surveys the following

 program is recommended:1. Level the instrument very carefully using the striding level for this purpose, so that the level correction may be small.
2. Point on the reference object and read the microscopes three times each on forward and backward graduations.
3. Point approximately on Polaris and place the striding level in position (zero of graduation to the right, or east). Point accurately on Polaris noting the time by sidereal watch. Read the striding level, reverse it, and read it again. Read the microscopes three times each on forward and backward graduations.
4. Reverse the telescope in altitude, turn the instrument $180^{\circ}$ in azimuth, and repeat as in No.3. The striding level must, of course, be removed while the telescope is being transited.
5. Same as No. 2.

Such an observation, under favourable conditions, will give a result correct to within a few seconds. However, observations in the field are seldom taken under ideal conditions and it is recommended strongly that two or more observations be taken at a station whenever weather conditions will allow. This precaution should always be adopted when the bearing of the line is much in doubt; the range of results will then provide some criterion of the accuracy of observation.

It will be found convenient, in order to prevent mistakes, always to begin the observation with the same position of the instrument.

## WATCH CORRECTION

The watch correction should be known with more than usual precision. In observing Polaris near upper or lower transit an error of one second of time corresponds, in the latitude of the western provinces, to an error in azimuth of about half a second of arc.

An observation for time should be taken either shortly before or shortly after every azimuth observation. The instrument should be carefully levelled and the observed transit corrected for azimuth error according to the formula $\Delta \mathrm{t}=4 \times \Delta \mathrm{a} \cos \mathrm{h} \sec \delta$, where $\Delta \mathrm{t}$ is the correction in seconds to the observed time, $\Delta$ a the azimuth error in minutes of arc, and h and $\delta$ the altitude and declination of the star. The correction is added for all stars south of the zenith if the azimuth correction is plus and subtracted if minus.

The result of observations for time must always be entered in the format the front of the record book of astronomical observations.

## CORRECTION FOR STRIDING LEVEL

The striding level is graduated from zero at one end, continuously upwards to the other end. Representing by $w$ and $e$ the readings of the westorleft and eastor right extremities of the bubble respectively when the zero of the graduation is at the east or right end and by $w^{\prime}$ and $e^{\prime}$, the corresponding westandeast readings after the level is reversed,
that is to say, when the zero of the graduation is at the west or left end, $d$ being the value of one division in seconds of arc, the level correction is:

$$
\frac{d}{4}\left\{\left(w-w^{\prime}\right)+\left(e-e^{\prime}\right\} \tan h\right.
$$

Tan $h$, the inclination factor for Polaris, is tabulated in the azimuth observation book.

The level correction is applied to the horizontal circle readings according to sign. The level vials are usually chambered and the length of the bubble should be adjusted to about twenty divisions prior to the observation.

The determination of the value of one division of the level in seconds of arc is ordinarily made by the National Research Council. lf, however, the surveyor has no knowledge of his level value and wishes to determine it in the field, he may adopt the following method:

The level is placed on the upper plate, parallel to the plane of revolution of the telescope, and a mark is set up in the direction of one of the foot screws and at a distance such that the telescope may be in solar focus. By turning the foot screw, the bubble is brought close to one end of its run. The telescope is pointed approximately on the mark and firmly clamped.

A more careful pointing is now made with the movable thread of the eyepiece micrometer, and the readings of the micrometer and level are noted.

The foot screw is then turned until the bubble is close to the other end of its run; the drum of the eyepiece micrometeristurned until the movable thread bisects the mark; and the micrometer and level readings are again noted. The difference of micrometer readings gives the angular displacementfrom which the value of one division of the level may be derived. The operation should be repeated several times. The level may be reversed end for end during the course of the determination if desired.

Instead of a distant point, the pointings may be made on the telescope of a transit or level used as a collimator.

## DETERMINATION OF THE VALUE OF ONE TURN OF THE MICROMETER

To reduce the micrometer readings to arc, the value of one turn is required. This is ordinarily determined for solar focus by the National Research Council. If, however, the surveyor has no knowledge of this value, he may determine it by methods described in standard text books on astronomy.

The following method will be found convenient:
Set the movable wire of the micrometer close to one end of its run
and move the upper part of the instrument by means of the tangent screw until the movable wire bisects some distant object (solar focus) at the same level as the transit; read the micrometer once and the horizontal circle microscopes three times. Now bring the movable wire close to the other end of its run and again bisect the same object by means of the tangent screw, reading the micrometer and circle microscopes as before. The horizontal angle, as shown by the microscope readings, divided by the difference of micrometer turns gives the value of one turn of the micrometer.

The operation should be repeated a number of times and, in order to decrease the effect of periodic errors of the circle graduation, the instrument should be revolved, by means of the shifting head on the stand, to give readings on different parts of the circle.

The uniformity of the micrometer screw may be tested by measuring the value of one turn over different parts of the screw.

Another transit or a level may be used as a collimator and gives a better reference object than a distant point. Set up the collimator a few feet from the transit to be tested, so that the two telescopes are at the same level. Adjust both to solar focus, and point on the object glass of the transit. Looking now at the collimator through the telescope of the transit, the cross wires or points of the collimator telescope will be seen as at an infinite distance. These cross wires or points make an excellent reference object.

## COMPUTATION FOR AZIMUTH OBSERVATION

When the above observation for azimuth has been taken with due attention to the special precuations, it can be reduced by the following formula:

$$
\tan Z=-\frac{\tan P \sec L \sin t}{1-\tan P \tan L \cos t}
$$

where $Z, P, L, t$, are azimuth, polar distance, latitude and hour angle, respectively. ${ }^{*}$

Writing $m$ for $\tan P \tan L \cos t$ the formula may be written

$$
\tan Z=-\left(\frac{1}{1-m}\right) \tan P \sec L \sin t
$$

Table XIII gives the values of $\log \frac{1}{1-m}$ tabulated with $\log m$ as argument. In using this table attention must be paid to the sign of $m$ which is the same as that of cos $t$; when $t$ lies between $0^{\mathrm{h}}$ and 6 h , or $18^{\mathrm{h}}$ and $24^{\mathrm{h}}, \mathrm{m}$ is positive and the half of the table as given on pages 150 ,

[^0]In the spherical triangle defined by the star, the zenith and the pole, the hour angle $t$ is measured from upper culmination, and the azimuth Z is positive or negative according as the star is east or west of the meridian.

152 and 154 must be used; when $t$ lies between $6^{h}$ and $18^{h}$, m is negative and the half of the table as given on pages 151,153 and 155 must be used.

Since $m$ is always less than unity, $\frac{1}{1-\mathrm{m}}$ is always positive, and therefore $\tan \mathrm{Z}$ is always of opposite $\operatorname{sign}$ to $\tan \mathrm{P} \sec \mathrm{L} \sin \mathrm{t}$. Hence when $t$ is between $0^{\mathrm{h}}$ and $12^{\mathrm{h}}$, $\tan \mathrm{Z}$ is negative, indicating that Polaris is west of the meridian; and when $t$ is between $12^{\mathrm{h}}$ and $24^{\mathrm{h}}, \tan \mathrm{Z}$ is positive, indicating that Polaris is east of the meridian. In the specimen observations (page 26 ), the suffix ' $n$ ' has been added to $\log \frac{1}{1-m}$ thus representing $\log -\left(\frac{1}{1-m}\right)$ in the above formula.

Denoting by a the angle which Polaris makes with the meridianeast or west from north, then loga (in seconds) is obtained directly from $\log \tan \mathrm{Z}$ by the use of Table XII, and Polaris is east or west of the meridian according as a is positive or negative.

The logarithms of secant and tangent $L$ are given in Tables IX and X for the north side of every section. For points outside the township system, the latitudes can usually be determined with sufficient accuracy from available maps.

A table giving the Right Ascension and log $\tan \mathrm{P}$ for Polaris for everytendays, is pasted in each record book of astronomical observations sent out to surveyors.

The observations on pages 26 to 29 show the form of record and method of computation.

In the form, R.O. is for reference object;H.C.R. for horizontal circle reading; coll. for collimation; R.A. for right ascension;


By the ordinary formulae of spherical trigonometry
whence $\tan Z=-\frac{\sin P \cos L \sin t}{\cos P-\sin h \sin L}$
Eliminating the altitude h by
$\sin \mathrm{h}=\cos \mathrm{P} \sin \mathrm{L}+\sin \mathrm{P} \cos \mathrm{L} \cos \mathrm{t}$
after reduction

$$
\tan Z=-\frac{\sin t}{\cot P \cos L-\cos t \sin L}
$$

which may also be written

$$
\tan Z=-\frac{\tan P \sec L \sin t}{1-\tan P \tan L \cos t}
$$

and $F$. and B. for forward and backward readings of the microscopes.
The specimen observationsare taken on a base line according to the program laid down.

The correction for run ${ }^{*}$ of the microscopes in each case is deduced from the means of the forward and backward readings and applied to the forward reading.

In reducing observations taken on base lines, the convergence must be applied to the mean azimuth to reduce it to a bearing as required by the Manual. For observations taken on initial meridians the results may be left as azimuths, for convenience.

A correction in the direction of a survey line, found necessary from the azimuth observation, may be made most easily by offsetting the transit stations in the required direction perpendicular to the line. The amount of the offsets are obtained by multiplying the tangent of the deflection angle by the distances of the transit stations from the point of deflection.

With modern instruments in which the microscopes are automatically meaned, single entries only will be made in the record form under the headings, Microscope A, and Microscope B. For good results three readings should be made and entered for each pointing.

An azimuth observation less in accuracy than that described above may be obtained without a striding level. The programof observing is slightly altered to secure the best results. First sight Polaris approximately. Then bring the plate bubble perpendicular to the line of sight to accurate centre and make the pointing to the star. Next read the reference object, transit the telescope, and repeat the operation. The resulting azimuth, calculated by the method used in governing surveys, will have an accuracy dependent on the sensitivity of the plate bubble. If the calculations are made from the Astronomical Field Tables results

Let $F=$ forward or apparent left reading $B=$ backward or apparent right reading

Then distance between readings $=5^{\prime}+F-B$

$$
\begin{aligned}
& \text { Error of run }=F-B \\
& \text { Correction to } F=-F \times \frac{F-B}{5^{\prime}+F-B} \\
& \text { Corrected reading }=F \times \frac{5^{\prime}}{5^{\prime}+F-B}
\end{aligned}
$$

When $F-B$ is not larger than, say, ten seconds, this formula may be written in a more convenient form, without appreciable error. Thus,

$$
\text { Corrected reading }=F-\frac{F}{5}(F-B)
$$

## AZIMUTH OBSERVATION

cho
Place Sta 68-.61:52. W. of N.E. cor sec. 34 tp. 84-1-5
r.о.. S.ta. 70-60.80. .".... ....". .....". ....". . 33 ..". ......"

Date. 12-7.-11
oberever. f. Smith DL.S


## FOR GOVERNING SURVEYS

Instrument \#1!2
One turn of micrometer $166^{\prime \prime}: 36$
One division of striding level. 4*.O.


## AZIMUTH OBSERVATION

Place Sta. 76-66:7.1. W. of N.E. con. sec. 33 tp. 8.4-2-5. r.о. Sta. 74-74.10..."..................".............................".



## FOR GOVERNING SURVEYS

Instrument \# 511
One turn of micrometer . 164:"O7
One division of striding level $, 2,6$

will be less accurate because values aregiven to a tenth of a minute only and since errors may result in interpolation.

## GENERAL REMARKS ON OBSERVING

The instrument should be firmly set up with sufficient clearance to permit the unrestricted movement of the observer, and it should be in good adjustment. In azimuth observation on governing surveys, a recorder is necessary to speed the operation and to read the watch at the instant of pointing on the star. The observation point should be protected from wind, and the transit from direct sunlight. When determining the solar focus, the reference object should be one-half mile or more distant and atmospheric conditions should be such that the air is not quivering. When possible a set should include at least three complete observations.

An observation for time should be made as near as possible to the time of observation. The necessity for having an accurate watch correction increases with the nearness of Polaris to the meridian.

## CHAPTER II

## DETERMINATION OF THE MAGNETIC MERIDIAN


#### Abstract

Although the compass is not allowed for establishing lines of Canada lands surveys, it is employed for other purposes and a knowledge of the direction of the magnetic meridian or of the magnetic declination is useful. For the determination of this direction, transit theodolites are fitted with especially sensitive needles. As the observation can be made in a few minutes and with very little trouble, it is desired that all surveyors should observe whenever theycan do so without inconvenience.


The observation and the recording formare arranged for the determination of the azimuth of the magnetic needle instead of the magnetic declination. The arrangement is made for the sake of simplicity in observing and recording, the bearing in question being, subject to instrumental corrections, the angle read on the horizontal circle of the transit. Moreover, it is not liable to errors of sign, as in adding or subtracting the declination.

## DIRECTIONS FOR OBSERVING

1. Place the instrument on a survey line, and after adjustment, set the vernier to read the bearing of the line.
2. Release the lower clamp, direct the telescope on the line, and fasten the lower clamp.
3. Release the vernier clamp, and turn the vernier plate until the north end of the magnetic needle observed with a magnifying glass, is seen exactly opposite the zeromark. Tap the trough lightly with the pencil, or preferably rit the milled part of one of the footscrews with the finger nail, to be sure that the needle has taken the position of rest. Note the reading of the horizontal circle. Take several readings by repeating the operation.
4. Repeat ops ration No. 3 for the south end of the needle.
5. Enter in the notes the place of observation, date, hour of the day, kind of time used, nature of the weather and any other remarks deemed necessary. It is important to record auroras occurring within 24 hours of the time of observation.

## GENERAL REMARKS

For saving trouble and calculations, it is suggested that observations be made on any line of which the azimuth is known.

The direction of the magnetic needle is subject to a daily fluctuation called the diurnal variation. During the greater part of the night the direction is not far from normal. In the early morning, the north end of the needle in Canada moves toward the east, reaching its maximum deflection about. 7 or $8 \mathrm{a} . \mathrm{m}$. The motion is now reversed, the north end travelling westwards, and crossing the normal direction
about 10 or $11 \mathrm{a} . \mathrm{m}$. The extreme western position is reached in the afternoon and then the needle comes back to its normal position at some time after 5 or $6 \mathrm{p} . \mathrm{m}$. This march is subject to wide variations during magnetic storms. The magnitude of the diurnal variation is not constant. In the inhabited parts of Canada, it may exceed 20 minutes. Observations at both eastern and western elongations of the needle on the same day, that is between 7 and $8 \mathrm{a} . \mathrm{m}$. and between 1 and $2 \mathrm{p} . \mathrm{m}$. give the best results, and it is desirable that when convenient they may be taken then. This gives not only the best value for the declination, but also the diurnal variation which it is most useful to know. Failing this, however, the best time to observe is after $5 \mathrm{p} . \mathrm{m}$., when the needle is about in its normal position. It is true that the normal position is crossed generally between 10 and $11 \mathrm{a} . \mathrm{m}$., but the motion being very rapid and the time of crossing uncertain, the afternoon observation is preferable.

Usually when the instruments are sent out from the office the magnetic needle is balanced for Ottawa and the index correction known. If at any time the needle should require rebalancing, the surveyor should proceed as follows:

Raise the needle with the lifter and remove the brass cover. This cover is secured to the trough by four screws, two on each side; when these screws are removed the cover may be lifted off. Now remove the end cover glasses. To do this scrape off the white lead putty around the edges, slide the cover glasses toward the centre and lift them out. The needle may now be taken out of the trough and the counterweight shifted. Then the lifter being still raised, place the needle upon it and lower the lifter gently. If the needle is not yet balanced repeat the operation until a satisfactory balanceis obtained. A carefully balanced needle should give no parallax in reading.

The steel pivct on which the needle swings during observations is made of hard steel shaped to a very sharp point. At the centre of the needle a cupped piece of agate is inserted. Although the needle is made as light as possible, the actual intensity of the pressure between the pivot and agate is probably many tons per square inch, and it is not surprising that in the majority of cases sluggishness in the needle is traceable to a damaged jewel or pivot. Therefore great care must be observed in lowering the needle very gently on the pivot. On no account should the compass be carried with the needle resting on its pivot.

In taking the needle out of the trough whether to rebalance the needle or to clean the agate, care should be taken to see that it is put back in its proper position. If replaced in the reverse position the index correction would be altered. For this reason, to safeguard against error, the position of the compass, whether "compass west"or "compass east" should be enteredin the remarks after each observation when observing.

If the needle is sluggish, the observation cannot be accurate. The sluggishness is generally due to a dull pivot or a scratched cap. To keep both in proper condition, the needle must always be lowered gently on its pivot and never be allowed to play, except when actually in use.

There are instances of the polarity of the needle being reversed by transporting an instrument on an electric car. It is difficult to conceive that a needle may be brought into such an intense magnetic field as that of an electric car without its magnetism being affected in some way; therefore, it is preferable to avoid this mode of transportation.

The place of observation must be at least three or four hundred yards away from wires carrying directelectric current. There must be no iron near the instrument. The observermust scrutinize his clothing and make sure that he has no iron or nickel on his person. Iron is found in buttons, as wire in hat brims, in some forms of neckties, in watches, chains and other articles of jewellery. The pivot in folding reading glasses is frequently made of iron. In case of doubt, the object may be tried close to the compass, measuring the distance at which an appreciable deflection is first produced. If the object is not brought closer than fifteen or twenty times this distance, the effect on the needle is negligible in observations of this kind.

The needle may be deflected by static electricity developed in cleaning the glass cover of the compass trough or the rubber frame of the reading glass. This electricityis dissipated bybreathing on the glass or rubber frame.

When the telescope points to magnetic north, the needle should, if the instrument were accurately constructed, be exactly opposite its zero mark, but it seldom is. The deviation of the needle from the zero mark is the magnetic index correction; it is positive or + when the north end of the needle is to the left or west of the zero mark; when on the right or east, it is negative or - .

With the needle opposite the zero mark, the telescope points in a direction which, in the following explanation, is called "compass north." To bring the telescope into the direction of the magnetic north, it must, if the index correction is positive, be turned to the right by an angle equal to the correction - hence the rule that the index correction is to be algebraically added to the azimuth of compass north in order to obtain the azimuth of magnetic north (azimuth reckoned from $0^{\circ}$ to $360^{\circ}$ ). Inv rsely, the index correction must be algebraically subtracted from the azimuth of magnetic north, such for instance as is taken from a magnetic map, in order to obtain the azimuth of compass north.

The index correction is ascertained by comparison with a standard unifilar magnetometer at the Dominion Observatory. When possible, it is well to have it determined both at the beginning and at the end of a survey.

## EXPLANATION OF SPECIMEN OBSERVATION

(a) H.C.R. of compass north.

This is the average of the mean north and south end readings. The transit was adjusted to read correctly the bearing of the survey line, so that the horizontal circle reading of compass north is the

## OBSERVATION FOR MAGNETIC DECLINATION



 Time_--.-_15_p.m._-_Instrument No.__2216
Bearing of reference line.-. $89^{\circ}-59^{\prime}$


REMARKS
A few clouds - Windy.
No aurora.
Circle E - compass W.
Mean local time
bearing of compass north. If the transit had not been so adjusted a correction to this reading would have been required.
(b) Correction for convergence.

The correction for convergence is applied in order to reduce the bearing read on the horizontal circle to an azimuth. The value of the correction is taken from the diagram in the Astronomical Field Tables. It is added when the point of observation is to the east of the reference meridian and subtracted if to the west. The rule given in the Manual to convert an azimuth to a bearing is here reversed, the object in this case being to convert a bearing to an azimuth.
(c) Azimuth of compass north.

The bearing has now been reduced to an azimuth.
(d) Index correction.

In the example given, the index correction being negative is subtracted from the azimuth of compass north to obtain the azimuth of magnetic north. If the index correction were positive, it would be added to the azimuth of compass north. The index correction is furnished witheachinstrument after comparison with the unifilar magnetometer.
(e) Azimuth of magnetic north,

The azimuth of magnetic north is the angle formed by the astronomical and magnetic meridians.

## SETTING A TRANSIT BY MEANS OF THE COMPASS

In connection with surveys of Canada lands, the most frequent use of the compass is for checking the courses of a traverseor for setting up the transit to read azimuths.

In the first case, it is sufficient to make sure that there is no abnormal change in the reading of the compass north: any sudden change indicates a probable mistake in some of the last courses.

The second case arises when it is desired to observe the Pole star in day time at a place where there is no line of known azimuth. The problem consists in setting up the transit so that it shall read azimuths. If the surveyor has already ascertained the azimuth of compass north with his instrument, he merely sets his vernier to read this azimuth, releases the lower clamp, turns the whole instrument till the needle is exactly opposite the zero mark, fastens the lower clamp and releases the vernier clamp. With the instrument (No.2216) used for the specimen observation and anywhere near the place where the observation was taken, the vernier would be set to read $27^{\circ} 11!8$ or rather $27^{\circ} 12^{\prime}$.

It may be, however, that the surveyor has not ascertained
the azimuth of compass north with his own instrument and has to resort to the azimuth of magnetic north taken from a map or determined by another surveyor. Then the surveyor must, from the azimuth of magnetic north, deduce the azimuth of compass north by applying the index correction of his own instrument after changing the sign. Starting with $27^{\circ} 06$ : 0 for azimuth of magnetic north in the case already cited, and the index correction being -5.8 , the surveyor would add 5.8 to $27^{\circ} 06!0$, which would give him $27^{\circ} 11: 8$ for the azimuth of compass north. He would then proceed as already explained.

All these corrections, it may be observed, are generally small and in practice are frequently disregarded.

The above remarks apply particularly to instruments supplied with the trough pattern of compass.

## COMPASS OF COOKE TRANSIT

Some Cooke instruments are fitted with a compass of telescopic pattern which may be briefly described. The outer shell of the compass is a brass tube on one end of which an ordinary Ramsden eyepiece is attached. There is a glass diaphragm on which are etched two close parallel vertical lines. The needle is of the regular edge bar type with one end bent up at right angles and ground to a very fine edge. This end swings sufficiently close to the glass diaphragm to give a good definition of the bent up edge of the needle when the eyepiece is focussed on the lines of the diaphragm. A pointing is made by bisecting the space between the two vertical lines with the needle. Only one end of the needle can of course be read. It is found however that this is more than compensated for by the increased accuracy of the readings. The needle lifter is operated by means of a milled head screw at the end of the compass remote from the eyepiece. The method of fastening this compass to the standard is an improvement on that used with the trough compass and assures better permanency of the index correction.

To rebalance the needle or clean the agate loosen the three small central screws and slide the tube apart. Unscrew the large central screw which ordinarily serves to keep the needle on the pivot. The needle may now be removed for balancing or cleaning. The same precautions and delicacy of handling must be observed as with the trough pattern of compass.

## THE WLLD DOUBLE IMAGE PRISM COMPASS

Because the Wild transit has a steel centre a separate compass has been made available which may be attached to thetripod when the transit is removed. The attachment has a small telescope for sighting the reference object, a clampand tangent screw for horizontal movement, and a ball and socket levelling device with a circular plate bubble.

The compass circle is graduated in divisions of two degrees of arc. Both sides of the circle are visible through the viewer in the relation shown below. The compass may be read to the nearest degree, and tenths of a degree may be estimated.


Reading 67.3 degrees.
To read the compass the first upright figure on the left is the tens of degrees (60). The number of full divisions to the right from this division to the inverted image of its supplementary angular value $\left(60^{\circ}+180^{\circ}=240^{\circ}\right)$ gives the digits of the reading (7). The tenths of a degree may be estimated more accurately in the centre of the image (0.3). The full reading of 67.3 degrees is the compass azimuth of the reference object. Ten readings should be taken at each observation point. The azimuth of compass north is obtained by subtracting the compass azimuth from the astronomic azimuth, or the astronomic azimuth plus 360 degrees. The index correction must then be applied to obtain the azimuth of magnetic north.

The compass may be balanced by loosening, and moving radially, the balancing screws placed on the face of the circle. The same care must be exercised to protect the steel pivot as with other compasses.

## CHAPTER III

## INSTRUMENTS

Survey instruments are no longer specially designed for the requirements of Canadian lands surveys. This chapter is therefore confined to certain general observations on the care of instruments likely to be used.

## TRANSIT THEODOLITE FOR GOVERNING SURVEYS

For the survey of governing lines no entirely satisfactory replacement has yet been developedfor the six-inch micrometer transit theodolite of 1912. A full description of it is given in Topographical Survey Bulletin No. 34.

These transits are no longer made and, as those still in use become worn, the features most likely to give trouble are the footscrews and the horizontal circle clamp. Wear in these parts may introduce sighting errors when the elevation of the telescope is changed. A check may be made by pressing the telescope sidewise with the finger. The telescope should spring back to the original sight line when the pressure is released. Should an appreciable error be revealed, its source can often be found by pressing in turn the tribach, clamp, standard, etc., and noticing the effect. If the footscrews show looseness, they can sometimes be tightened by more vigorous action on the small binding screws. Otherwise they should be returned to the shop for repair.

Another test, which is particularly useful in the case of clamp trouble, is to sight the telescope on a point and then move it up and down in altitude once or twice, afterwards checking to see if the point is still on the vertical crosswire. If the error reverses after the horizontal tangent screw has been usedinopposite senses, it indicates a worn centre pivot, tangent=screw, or clamp. In such cases the instrument should be sent to a competent instrument maker for repair.

Care should be taken that the micrometer screw drum spindles do not become slightly bent causing the drum to touch the index. Serious errors in angle measurement will be introduced if this occurs.

## "OPTICAL" THEODOLITES

Many "optical" theodolites are now on the market, all more or less based on the designs made for the Zeiss firm by Wild. These instruments have more factory adjustments and fewer field ones than in the case of the older types. Striding levels for the se instruments have not yet been sufficiently refined for accurate azimuth observation; the best available has a value of about five seconds of arc per division. The length of the bubble cannot be adjusted.

The "optical" theodolites are characterized by accurately graduated circles, cut on glass, with a device for optically bringing into the field of the micrometer microscope the images of two diametrically opposite points on the circle, or, in some more recent designs, cut on
two circles simultaneously. The various types of micrometer used with this eccentricity-compensating device permit very rapid circle reading with little or no eyestrain.

Due to the complicated nature of the optical trains inside these instruments, and the use of factory adjustments, totally enclosed bearings, etc., verylittle can be done in the field to effect repairs after damage or other trouble. On no account should an attempt be made by the surveyor to dissect any part of an optical theodolite unless he has had previous experience in taking it apart under office conditions. Unless the function of each screw is known, there is grave risk of seriously injuring the instrument, or losing some of the small parts.

## INSTRUMENTS FOR USE ON WINTER SURVEYS

Certain greases, originally developed for military purposes and suitable for use over a wide temperature range, are now being used in instruments likely to be required on winter surveys. Before lubricating an instrument with low-temperature grease, all trace of the old lubricant must be removed from every surface by means of ether, or other solvent. All micrometer screws and nuts must be so cleaned, as otherwise a film of the old grease may remain and cause binding at very low temperatures. If a cold chamber is available, the instrument, after being winterized, should be left in it for several hours and all movements checked before removal.

Instruments which have been subjected to extreme cold should never be quickly exposed to warmth because this results in the condensation of moisture which may be particularly harmful in "optical" theodolites having steel parts. If an instrument has to be transferred suddenly from cold to heat, the best course is to pack it first in its box or other fairly air-tight container, and leave it for several hours before removal. In this way the instrument will reach room temperature without an excessive amount of saturated air coming into contact with it.

Most motions of an instrument, even when winterized, will be stiffer at low temperatures. In some examples of the current models (1951) of the Wild theodolite, stiffness at sub-zero points occurs to such a degree, due to differential contraction between the trunnions and their bearings, as to render the instrument unusable.

When an instrument shows excessive stiffness at any temperature, on no account should force be used, as serious damage may result. The only safe treatment is to dismantle the parts carefully, if facilities are available, and find the cause of the trouble.

## GENERAL

Extreme care should always be exercised in handling both transits and levels. Sudden jars may break delicate parts or disturb fine adjustments. No weight should be placed on an instrument box whether an instrumentis within itor notand the instrument boxes should always be kept ina safe place. It is preferable to box a transit in moving from one instrument station to another. Otherwise the centre may be 78897-4
strained or the instrument may be damaged through a fall or by parts becoming entangled in tree limbs.

The instruments should be kept dry, clean, and freshly oiled at all times using a fine watch oil. The interval between overhauls should not exceed two years. Lenses should be cleaned with a brush or very lightly rubbed with a clean cloth or tissue to prevent scratching the surface. Clamps should never be excessively tight. In boxing an instrument they should never be tightened more than just sufficient to prevent movement. Tripod head screws should be loosened when the instrument is not in use.

In winter operations the transitman should avoid breathing on the eyepieces of verniers. If the object glass becomes frosted on the inside the frost may sometimes be removed by pointing the telescope towards the sun.

In camp, instruments and instrument boxes must always be kept in a safe place and should never be handled except by, or under the direction of, a competent instrument man.

SIDEREAL WATCH

The sidereal watch is an 18 -size 19 -jewel movement in an open face nickel case. The dial is divided into twenty-four hours.

Before being accepted, each watch is tested at the Dominion Observatory to ascertain if the adjustments have been made with the necessary accuracy.

No timepiece will give good service without reasonable care. Great changes of temperature must be avoided; this can be accomplished by carrying it constantly in an inner pocket where it is maintained at an even temperature by the heat of the body. The pocket must be clean and reserved exclusively for the watch which should be inserted always in the same position. It is a good plan, as a protection against dust, to keep the watch in a tight-fitting case of chamois skin. If exposed to a very low temperature, it may not only stop, but be injured permanently. It must be kept away from electric motors or dynamos, which might magnetize the balance. Windingevery day as nearly as possible at the same hour is essential; this is to be done by turning the crown or the key and not by turning the watch. A watch must be cleaned and oiled at least every fourth year. A watch, particularly of a higher grade, may be ruined easily by an incompetent workman; too much care cannot be exercised in selecting the man to whom it is entrusted. When repairs are required, it is best to have them made through the head office.

STEEL TAPES

The steel tapes most commonly used are $0!' 25$ wide by $0!102$ thick and in lengths of 400 links, 500 links, or 300 feet. Each tape has its correctlength in terms of the Dominion measure of length determined by the National Research Council on the flat (fully supported), under a tension of twenty pounds, and at a temperature of $68^{\circ} \mathrm{F}$. The coefficient
of thermal expansion is about $0.000,006$ per $1^{\circ} \mathrm{F}$., so that the correction for a $10^{\circ} \mathrm{F}$. change in temperature is about 0.006 feet per 100 feet. The weight per 100 feet is 1.68 pounds. Due to variation in cross section the tapes actually in use vary from 1.3 to 1.7 pounds per 100 feet. Young's modulus of elasticity, $E$, is $30,000,000$ per square inch. Cards giving temperature corrections in tabulated form are available at head office. Sag (catenary) and stretch corrections are calculated from the above data.

The formulae are:

$$
\text { Stretch }=\frac{L \times\left(P-P_{0}\right)}{A \times E}
$$

Where $L$ is the original length of the tape section, $P$ the tension applied in pounds, $P$ o the tension applied in standardizing the tape ( 20 lbs .), A the area of the cross section of the tape in square inches ( $0!125 \times 0!{ }^{\prime} 02$ ) and $E$ the modulus of elasticity of the tape $(30,000,000)$.

$$
\text { Correction for sag }=\frac{W^{2} L}{24 P^{2}}
$$

Where $W=$ weight of length of tape section in pounds, $L$ the length of the tape section, and $P$ the tension applied in pounds.

Measurements made with steel tapes are subject to correction for temperature, slope, stretch, and sag. The accuracy required in a survey will determine whether any or all of the corrections should be applied. In governing surveys all four corrections should be used.

Steel tapes require considerable care in the field. When not in use they should either be reeled up or put in a safe place to one side of the survey line. To avoid breaks, tapes should never be jerked for straightening purposes, and the chainman should always be on the alert for kinks. At frequent intervals they should be cleaned with an oily rag. In the vicinity of salt water this should be done every evening to prevent pitting. At the close of the season, they should be cleaned thoroughly and fairly heavily greased with vaseline. Repair kits for breakages should always be available.

## STADIA RODS

Stadia rods are fifteen feet in length and three inches wide. They fold in the middle. They are graduated in feet and tenths. There are no figures on the rod, the colour scheme being so arranged that they are unnecessary. A folding level is attached to the back of the rod as an aid for holding it vertical.

The stadia wires of the transit theodolite are set by the makers in the supposed ratio of $1: 100$ between outside wires and $1: 200$ between middle and outer wires. As a matter of fact, however, they
are rarely in this exact ratio. The true ratios are furnished for each diaphragm. These ratios are used for calculating a table of the corrections to be applied to the distances read oa the rod. If the surveyor should be without the true stadia constants, he can prepare his table of corrections by chaining a base on level ground and measuring with the stadia the distance of a number of points on the base; the difference between the two measurements gives the correction for each distance. The table is completed by interpolation. The measurement must be made when the air is quite steady and the conditions favourable.

With modern internal focussing instruments the formula, $k r \cos ^{2} V+(f+c) \cos V$, becomes $k r \cos ^{2} V$, since $f+c$ may be neglected and the formula for the vertical component becomes $k r \cos V \sin V$, or $1 / 2 k r \sin 2 V$, where $k$ is the stadia constant, $r$ the rod intercept, and $V$ the angle of inclination of the sight.

In making stadia measurements the stadia rod must be held vertical by centering the cross bubbles in the level attached to the rear of the rod. The bubbles may be adjusted by erecting the rod in a vertical position by means of a long plumb line and centering the bubbles by the adjusting screws. When the rod is being held for sighting it should be turned to catch as much sunlight as possible and yet present sufficient surface to the instrument man on which to make the reading. For best results, the lengths of measurements should not exceed 1,000 feet; the full intercept between the outside stadia lines should be read, and then checked by the summation of the two one-half intercepts; the line of sight through the lower stadia line should always clear the intervening ground by at least three feet.

## CLINOMETER

The clinometer, or abneylevel, is used to measure the slope of the tape. In governing surveys, both front and rear chainman should measure and record the slope. All slopes greater than 7 degrees should be measured with a transit.

## CHAPTER IV

## PROBLEMS CONNECTED WITH THE SYSTEM OF SURVEY

## CORRECTION FOR HEIGHT ABOVE SEA-LEVEL

The tables have been calculated from the dimensions of the earth's surface at sea-level.

The township sides are actually measured on surfaces elevat ed above sea-level, and therefore the differences of latitude and longitude calculated from the tables are greater than those actually covered by the township sides.

Any measured distance may be reduced to sea-level by subtracting the correction $\frac{l}{S} x, x$ being the distance, $l$ the elevation above sea-level, and $S$ the radius of curvature of the line under consideration.

In general $N$ (see Table i) can be used instead of $S$.

Base lines when the system of survey is exactly followed are established by direct measurement from the 49 th parallel, northward along an initial meridian.

Hence the latitude of a base line should be less than that given in the table by $\left(L-49^{\circ}\right) \frac{l}{R}$, where $l$ is the mean elevation of the initial meridian between the 49 th parallel and the base under consideration.

Many base lines, however, have been established, not by this direct measurement, but by the survey of township meridians from other bases. If the actual latitudes of these base lines are required, account must be taken of the elevations of all the north and south lines through which the connection with the 49 th parallel has been made. It is obvious, however, that the average elevation of the country above the sea will give a sufficiently accurate result, since the small errors due to difference of elevation are masked by errors of survey.

On the base lines the effect of elevation above sea-level is to decrease the difference of longitude covered by one range, and this must be allowed for inestablishing an initial meridian by means of chainage along a base line or in estimating the accuracy of measurement of a base line by its closing on an initial meridian, since the initial meridians, except the first, have been placed approximately on even degrees of longitude (every fourth degree). The longitude covered by one range at an elevation $l$, may be obtained by multiplying the differences of longitude given in Tables III and IV by ( $1-\frac{l}{N}$ ).

The correction for elevation above sea-level is, in latitude $51^{\circ}, 0.00382$ chains for one mile distance at an elevation of 1,000 feet, and varies directly as the elevation and distance. It changes somewhat with the latitude, but slightly, and the correction in any particular case may be taken as the same as that for latitude $51^{\circ}$. If extreme accuracy be required, the formula given above, $\frac{l}{S} x$, may be used.

The error in the length of township chords of course involves an error in deflection angles and azimuths, but this is too small to be appreciable.

## LATITUDES AND LONGITUDES OF POINTS IN THE SYSTEM

By "points in the system" is meant the corners of specified sections, or points referred to them by connecting lines. In the latter case the lines, if short, may be reduced to latitude and longitude by means of "latitude and departure" from a traverse table, and by using Table XI.

Thus the problemis reduced to the determination of the latitude and longitude of any section corner.

## LATITUDE

The latitude of a section corner can be taken directly from Table IX and Table X.

Since the section corners are presumed to be at a distance of even sections from the north and south boundaries of the township, being established by survey from those boundaries, the latitude found as above must, when the section corner is not on the meridian outline of the township, be increased by the correction given by Table XI.

In the first system the sections are not measured on meridians from the north or south boundary of the township, but on lines parallel to the eastern boundary of the township. Hence, theoretically, the difference of latitude between the given corner and the township outline should be decreased in the ratio of cosine azimuth of the section line to unity; but this correction is insignificant.

The correction for sea-level may also be applied.
LONGITUDE, THIRD SYSTEM

In the second and third systems, the section lines are true meridians from the base line north and south two townships. Hence the longitude of a section corner is the same as that of the corresponding corner on the base line from which the township has been surveyed.

Then if $d M$ be the longitude covered by one range on that base line, and if $n$ be the number of the range in which the section lies, $m$ the number of sections lying between the given section and the eastern boundary of the township, the number of ranges which intervene between the initial meridian and the eastern boundary of the given section is $n-1+\frac{m}{6}$ and the difference in longitude between it and the initial meridian is $\left(n-1+\frac{m}{6}\right) d M$. This added to the longitude of the initial meridian gives the longitude of the eastern boundary of the section.

The longitude of the Principal or Firstmeridian is $97^{\circ} 27^{\prime} \quad 28!' 4$.
The longitudes of the Second, Third, Fourth, etc., meridians
are $102^{\circ}, 106^{\circ}, 110^{\circ}$, etc., subject to certain errors of survey, which cannot be discussed at present.

The difference of longitude should be corrected for height above sea-level if precision is required. This can be done by multiplying it by $\left(1-\frac{l}{N}\right)$.

For example:
The NE corner of sec.16, tp.23, r.17, W. of the Fourth meridian (third system of survey). Here $n=17, m=3$, and the township is surveyed from the 7 th base, for which we find from Table IV, $d M=$ $8^{\prime} 22!' 411=502!' 411$. Therefore longitude of the section line

$$
=110^{\circ}+\left(502!^{\prime} 411 \times 163 / 6\right)=112^{\circ} 18 \cdot 09!^{\prime} 78
$$

The NE corner of sec. 16 is in approximately the same latitude as the NE corner of Sec. 13, and is 3 sections distant from the bounding meridian of the township.

| de of NE cor. sec. 13 tp. 23 (Table X) | $50^{\circ} 57 \prime 56!\prime 05$ |
| :---: | :---: |
| Correction for 3 sections (Table XI) | 0!'07 |
| Latitude of NE cor. sec. 16 tp. 23 | $50^{\circ} 57 \cdot 56{ }^{\prime} 12$ |

## LONGITUDE, FIRST SYSTEM

In the first system the procedure for the longitude is a little different. The section lines are drawn parallel to the east side of the township, so that the difference of longitude between the section line and the east boundary of the township is not the same as on the base line, but is equal to the actual distance from the boundary of the township divided by $\mathbf{P} \sin 1^{\prime \prime}, \mathbf{P} \sin 1^{\prime \prime}$ being taken from Table I for the actual latitude of the section post. Thus using the same notation as before
difference of longitude from initial meridian

$$
=(\mathrm{n}-1) \mathrm{dM}+\frac{81.50 \mathrm{~m}}{\mathrm{P} \sin 1^{\prime \prime}}
$$

dM being taken from Table III (lst system) for the governing base line, or it may be calculated by the equivalent formula difference of longitude

$$
=\left(n-1+\frac{m}{6}\right) d M+\frac{Q}{P \sin 1^{\prime \prime}}
$$

where $Q=2 m(40-w)$, $w$ being the width of quarter sections as taken from the last column of Table IX.

## LONGITUDE, SECOND AND FOURTH SYSTEMS

Longitudes in the second system are calculated in the same way as those in the third, taking $d M$ from Table III instead of Table IV. In the fourth system the process is the same as for the third system, and the same table is used - Table IV.

## EFFECT OF ERRORS OF SURVEY

An error in the latitude of the base line, or an error in the longitude of the initial meridian, of course increases or decreases by the amount of error in the latitude or longitude of the section corner. Similarly, a chainage error on the base line affects the longitudedirectly. In the computation all known errors of this kind must be allowed for.

An error in the latitude of the base line also affects the longitude covered by 486 chains (or 489 chains) measured along the base line, since 486 chains covers a greater longitude if the base line be moved north. The manner in which the effect of an error of this kind may be estimated is shown in the following example.

Suppose the 6th base line (third system) to be placed 10 chains too far north, we find from Table IV

| dM for 6th base line | $=$ | $498!662$ |
| :--- | :--- | :--- |
| dM for 6th correction line | $=$ | $500!527$ |

The 6th correction line is two townships, i.e. 966 chains north of the 6th base line, and the difference in dM for these lines is $1 \because 865$. Therefore, dM for the actual position of the 6 th base line, 10 chains north of its theoretical position, is

$$
498!662+1!865 \times \frac{10}{966}=498!681
$$

The correction, in the case supposed, to dM for one range is $0!019$, and in 29 ranges (about the distance apart of two initial meridians) it amounts to $0!019 \times 29=0!55$, or 54 links.

GIVEN THE LATITUDE AND LONGITUDE OF A POINT, TO FIND ITS POSITION WITH REGARD TO THE SURVEY SYSTEM, i.e. to find in what section it is, and the township and range, and its distance from the NE corner of the section.

## SECOND, THIRD AND FOURTH SYSTEMS

This is the converse of the preceding problem. The first step is to find, from Table IX or $X$, the latitude of the section line next north of the given latitude. The difference between these two latitudes is reduced to chains by Table I. This gives the distance $(x)$ in chains to be measured from the point to find the north boundary of the section. For great accuracy the small corrections for altitude and from Table XI may be applied to $x$.

The number of sections by which the section line is north of the southern boundary of the township in which it lies is to be noted. Call this number a, and the number of the township $t$.

We also know the number of the nearest base line, i.e., the base line on which depends the survey of township $t$. From Table IV we take out dM for this base line.

From the given longitude of the point subtract the longitude of the initial meridian. Divide the difference oy dM , with quotient $n$ and remainder $r$. Divide $r$ by $\frac{d M}{6}$ with quotient $b$ and remainder $s$. Then $s$, reduced from seconds of longitude to chains by Table I, with argument, latitude of the given point, gives the distance ( $y$ ) to be measured east from the point to find the eastern line of the section.

We now know that the given point is $x$ chains south and $y$ chains west of the north-east corner of some section in township No. $t$ and range No. $(n+1)$ west of the initial meridian; and also that the northern boundary of the section is a sections north of the southern boundary of the township, and that the eastern boundary is bsections west of the eastern boundary of the township.

It is now easy by means of a skeleton township diagram to determine the number of the section, e.g., if $a=5, b=3$, the section is 28.

Without a township diagram, the section number can be found from the formula

$$
\text { No. of section }=1 / 2\{12 a-5 \pm(2 b-5)\}
$$

The upper sign is taken when a is odd, and the lower when a is even. These two rules are comprised in the general formula

$$
\text { No. of section }=1 / 2\left\{(12 a-5)-(-1)^{a}(2 b-5)\right\}
$$

The calculation for the second systemis the same as above, using the proper tables for that system. It is also the same for the fourth system.

In this manner have been computed the positions of a great many section corners in British Columbia (fourth system of survey) with reference to points along the line of the Canadian Pacific Railway, the latitudes and longitudes of these points having been first determined by a traverse survey.

## FIRST SYSTEM OF SURVEY

The procedure in this system is the same as above, except that the total difference of longitude from the eastern boundary of the township (instead of the nearest section line) must be reduced to chains, and from the distance in chains must be subtracted the nearest multiple of 81.50 .

## FRACTIONAL TOWNSHIP OR RANGE BETWEEN PARTS OF THE COUNTRY SURVEYED UNDER DIFFERENT SYSTEMSOF SURVEY

Townships of the first and second systems adjoin each other without overlap or deficiency, since the townships in the se two systems are of the same dimensions. Similarly of the third and fourth systems.

But where townships surveyed under the latter systems abut on townships of the first or second system, a fractional townshipor
range occurs. It is only necessary to consider the case of the third system abutting on the first or second, since the fourth does not occur in juxtaposition with these latter systems.

## FRACTIONAL TOWNSHIP

Townships of the third system are 6 chains shorter, measured north and south than the others. The townships in both cases are meas ured north from the 49th parallel, and hence the third system falls short of the other by 6 chains for each township, and the northern boundary of a township of the third system is therefore south of the northern boundary of the same township of the first or second system by 6 chains multiplied by the number of the township.

Thus the 5 th correction line (tp. 18), as surveyed under the third system, is $6 \times 18=108$ chains south of its position under the second system. For twelve ranges west of the Second meridian, the territory from the 5 th correction line northward to the 8 th correction line was surveyed under the second system, while the country south of the former line has been surveyed under the third system. There is therefore an additional township (measuring 108 chains from north to south) lying between township 18 of the third system and township 19 of the second system. (This fractional township is called township 19A, and is subdivided according to the third system. See Manual of Surveys.)

## FRACTIONAL RANGE

Townships of the third system are 3 chains narrower (measured east and west along the base line) than those of the first and second systems. The overlap of the latter systems over the third, however, is not equal to 3 chains multiplied by the number of ranges, but exceeds this, since the widths are laid off along base lines which lie in different latitudes, and hence the convergence of meridians comes into play.

The readiest method of calculating this overlap is as follows:
Let $d M_{1}$ be the longitude covered by one range of the base line in the first or second system as found from Table III.

Let $d M$ be the same quantity for the base line of the third system (from Table IV)

Then $d M_{1}-d M$ is the difference of the longitude between the exterior meridians of range one, as surveyed under the two systems.
The difference of longitude at the eastern boundary of the nth range will be

$$
(n-1)\left(d M_{1}-d M\right)
$$

This reduced to chains is

$$
(n-1)\left(d M_{1}-d M\right) P \sin 1^{\prime \prime}
$$

P $\sin 1^{\prime \prime}$ being taken from the proper table for the latitude of the base or section line on which the overlap is required.

## FIRST EXAMPLE

The meridian outline between ranges 12 and 13 , west of the Second meridian, from township 19 to township 22, inclusive, is the western boundary of a tract of country surveyedunder the second system of survey. Required: the width of range 13, as surveyed under the third system, on the northern boundaries of townships $19,20,21$ and 22.

The base line on which this meridian outline is based is th. 6th base line, or northern boundary of township 20.

$$
\begin{aligned}
\text { From Table III, } d M_{1} & =8^{\prime} 21:^{\prime} 972 \\
\text { " } \quad " \mathrm{IV}, \mathrm{dM} & =\frac{8^{\prime} 18!^{\prime} 662}{3:^{\prime} 310} \\
\text { whence } d M_{1}-d M & =\frac{1}{2}
\end{aligned}
$$

and at the eastern boundary of the thirteenth range, the difference of longitude is $3.310 \times 12=39: 172$.

We have then for the northern boundary of township 19 (third system):
$\log 39.72=1.5990092$
Table IV, $\log \mathbf{P} \sin 1^{\prime \prime}=\frac{9.9896352}{1.5886444}$
Nat. number $=38.783$
For the northern boundary of township 20:
$\log 39.72=1.5990092$
$\log P \sin 1^{\prime \prime}=9.9888297$
1.5878389

Nat. number $=38.711$
For the northern boundary of township 21:
$\log 39.72=1.5990092$
$\log P \sin 1^{\prime \prime}=9.9880192$
1.5870284

Nat. number $=38.639$
For the northern boundary of township 22:

$$
\log 39.72=1.5990092
$$

$\log P \sin 1^{\prime \prime}=\frac{9.9872086}{1.5862178}$
Nat. number $=38.567$
Hence townships $19,20,21$ and 22 , surveyed under the third system in range 13, have their eastern tiers of sections narrowed by $38.783,38.711,38.639$ and 38.567 chains respectively, along the north boundaries of the different townships.

Now, the full widths of the se sections when regular is got from Table $X$, by multiplying the "width of quarter section" by two.

Thus, the width of the eastern tier of sections in range 13 is:
Along N. boundary of tp. 19, 80.15-38.78 $=41.37$ chains

| $" 1$ | $"$ | $20,80.00-38.71=41.29$ |  |
| :--- | :--- | :--- | :--- |
| $" 1$ | $"$ | $21,79.85-38.64=41.21$ | $"$ |
| $"$ | $"$ | $22,79.69-38.57=41.12$ |  |

These widths must be increased by one chain for road, if the widths from post to post are required.

For the township lines to the north of the correction line, viz. : $23,24,25$ and 26 , the width of range 13 may be found in the same way, using the dM from Tables III and IV for the 7th instead of the 6th base line.

If the width of the fractional section on the north side of the 6 th correction line is required, that is, the south boundary of township 23 , it must be remembered that here, on account of the correction line being thrown south, from the less depth of the townships of the new system, the southern boundary of township 23 of the third system, which is brought from the 7 th base line, intersects the second system south of the correction line, i.e., on a line brought from the 6 th base line.

Therefore we have
For the second system, Table III, $\mathrm{dM}_{1} 6$ th base $=8^{\prime} 21!972$ " third " " IV, dM 7th base $=\underline{8^{\prime} 22!411}$

$$
\mathrm{dM}_{1}-\mathrm{dM}=-0!439
$$

and for twelve ranges, $12\left(\mathrm{dM}_{1}-\mathrm{dM}\right)=-5^{\prime}: 268$
With the difference of longitude, $5!268$, and the $P \sin 1^{\prime \prime}$ for the 6 th correction line, third system, we get the required jog.

It will be noticed that the overlap is negative, i.e., instead of there being a fractional township there is a surplus.

The heavy lines represent the second system, the dotted ones the third. The line $A^{\prime} B^{\prime}$ is the one which we have just considered; it falls to the east of AB, but to the west of CD.


The lines in the figure are all township lines. Thus it will be seen that there is a small piece of land, $B^{\prime} C^{\prime}$, which is in fact a township of itself. Its designation would be township 23 A , range 12 A .

## SECOND EXAMPLE

Required: the depth, north and south, of township 27, range 19, west of the principal meridian.

The north boundary of township 26 is the northern boundary of a tract of country surveyed under the first system.

Since each township of the third system is 6 chains shorter north and south than one of the first system, the northern boundary of township 26 in the third system is $6 \times 26=156$ chains south of the same boundary under the first system.

Therefore the distance from the north boundary of township 26, first system, to the northeast corner of section 12 , township 27 , third system, is $161-156=5$ chains.

Since 1.50 chains must be allowed for road, 3.50 chains is the available width of the strip of land.

## FRACTIONAL SECTIONS ADJOINING AN INITIAL MERIDIAN

The longitude of the Principal meridian at the intersection of the 4 th base line is $97^{\circ} 27^{\prime} 28^{\prime \prime} 4$.

The Second, Third, etc., meridians were laid down by survey from the Principal Meridian, with the intention to place them at every fourth degree of longitude $-102^{\circ}, 106^{\circ}, 110^{\circ}$, etc. There is also the Second meridian east of the Principal meridian, laid down by survey from it, in approximate longitude $94^{\circ}$.

The actual longitudes, by astronomical observation, of such as have been determined are:

Second meridian at the north boundary of sec. 13, tp. $15,102^{\circ} 00^{\prime} 16^{\prime}!5$. Third meridian at the north boundary of sec. 13 , tp. $46,106^{\circ} 00^{\prime} 10!1$. Fourth meridian at the north boundary of sec. 36, tp. $49,110^{\circ} 00^{\prime} 18^{\prime \prime} 0$. Fifth meridian at the north boundary of sec. 36 , tp. $52,114^{\circ} 00^{\prime} 07!7$.

The discrepancies from the intended values are due in part to error in the assumed longitude of the Principal meridian, in part to errors of survey. The longitudes of these meridians at points other than those stated, will of course vary with the azimuthal error in surveying the meridians.

The width of the last range in seconds, on a given base line, when closing on an initial meridian is got by subtracting from the difference in longitude (in seconds) between the initial meridians, the nearest integral multiple of dM from Table III or Table IV (according to the system of survey in question).

Thus for the width of the last range on the 18 th base line between the Third and Fourth meridians (third system of survey) we have from Table IV, $\mathrm{dM}=549$ ! 123 for one range. Assuming the Third and Fourth meridians to be in the above stated longitudes at the 18 th base line, we divide the difference of longitude $4^{\circ} 00^{\prime} 07!9$ or $14407!9$ by $549^{\prime \prime} 123$ with quotient 26 and remainder $130^{\prime \prime} 7$. That is, the width of range 27 on the 18 th base line or the difference of longitude between the meridian forming the eastern boundary of townships 67, 68, 69 and 70, range 27 and the Fourth meridian is $130^{\prime \prime} 7$.

A better result could be obtained by considering the actual latitude of the base line, and its elevation above sea-level. Thus it is
known that the 18 th base line between the Third and Fourth meridians is approximately six chains south of its latitude as given in the tables, and has a mean elevation of about 1,700 feet. Using these figures and proceeding as already explained on pages 43 and 45 , correcting for latitude displacement

$$
\mathrm{dM}=549^{\prime \prime} 123-\frac{6}{966} \times 2!365=549^{\prime \prime} 108
$$

and correcting for altitude

$$
\mathrm{dM}=549^{\prime!} 108\left(1-\frac{l}{\mathrm{~N}}\right)=549^{\prime!} 064
$$

Proceeding now as before using 549:'064 as the longitude covered by one range we find the width of range 27 to be $132:^{\prime} 2$. This difference of longitude can be converted into chains by multiplying by $P$ $\sin l^{\prime \prime}$ for the section line whose length is required, whether the southern boundary of township 67, or the northern boundary of township 70, or any of the intermediate township or section lines.

If the width of the last broken section be required, then if dealing with the third system of survey, integral multiples of $1 / 6 \mathrm{dM}$ (difference of longitude covered by one section) must be subtracted from the width of the fractional township until the remainder is less than $1 / 6 \mathrm{dM}$. This remainder may then be converted to chains by multiplying by $P \sin 1^{\prime \prime}$ taken out of the table for the latitude of the line under consideration. The reason for this is that the widths in seconds of longitude are the same for all sections from the base line to the correction line (second and third systems).

## GEODETIC POSITIONS

Owing to the unequal distribution of mass and density in local areas of the earth's surface, the normal to the spheroid does not coincide with the plumb line vertical. In consequence the latitude and longitude of a point determined by astronomical observations may differ from the latitude and longitude as determined by geodetic measurements. Since the amount of the plumb line deflection varies from place to place the distance between any two points calculated from their astronomical positions may not agree with the distance actually measured on the ground. It is therefore desirable that positions in the Canada Lands surveys systems should be geodetic rather than astronomic.

A number of section corners have been tied into the Geodetic Survey of Canada's networks oftriangulation and the resulting spheroidal co-ordinates ( 1927 North American Datum) are listed in the following table. As the networks are extended further ties will be made and the results tabulated.

Spheroidal Co-ordinates of Section Corners as Determined by the Geodetic Survey of Canada (1927 North American Datum)
West of Principal Meridian.

| Tp. | R . | Section | Latitude | Longitude |
| :---: | :---: | :---: | :---: | :---: |
| 1 | 10 | 22, 1/4 N. By. | $49^{\circ} 03{ }^{\prime} 32!773$ | $98^{\circ} 43^{\prime} 41!' 22$ |
| 1 | 17 | 10, 1/4 E. By. | 490117.40 | $9939 \quad 37.95$ |
| 1 | 27 | 26, 1/4 E. By. | $\begin{array}{llll}49 & 03 & 57.93\end{array}$ | 1005852.01 |
| 1 | 28 | 28, 1/4 E. By. | $49 \quad 0358.05$ | 1010937.41 |
| 1 | 30 | 15, N.E. Cor. | 490238.59 | 1012424.27 |
| 1 | 33 | 28, 1/4 E. By. | $49 \quad 0356.42$ | 1014955.55 |
| 2 | 1 | 10, N. E. Cor. | $\begin{array}{llll}49 & 07 & 05.37\end{array}$ | $\begin{array}{llll}97 & 30 & 16.51\end{array}$ |
| 2 | 3 | 23, 1/4 E. By. | $49 \quad 08 \quad 27.30$ | $\begin{array}{llll}97 & 45 & 04.16\end{array}$ |
| 2 | 14 | 29, N. E. Cor. | $49 \quad 0945.63$ | $99 \quad 18 \quad 06.86$ |
| 2 | 16 | 12, N. E. Cor. | $\begin{array}{llll}49 & 07 & 03.78\end{array}$ | $99 \quad 28 \quad 52.04$ |
| 3 | 5 | 2, N.E. Cor. | 4911132.19 | $\begin{array}{llll}98 & 01 & 24.67\end{array}$ |
| 3 | 7 | 19, N.E. Cor. | $\begin{array}{llll}49 & 14 & 12.51\end{array}$ | $\begin{array}{llll}98 & 23 & 07.88\end{array}$ |
| 3 | 18 | 8, 1/4 E. By. | $49 \quad 1201.51$ | $99 \quad 51 \quad 20.95$ |
| 7 | 17 | 31, N. E. Cor. | $\begin{array}{llll}49 & 37 & 14.60\end{array}$ | $9945 \quad 24.77$ |
| 11 | 18 | 35, N. E. Cor. | $4958 \quad 29.92$ | 994912.79 |

West of Second Meridian

| Tp. | R . | Section | Latitude | Longitude |
| :---: | :---: | :---: | :---: | :---: |
| 1 | 1 | 16, 1/4 N. By. | $49^{\circ} 02^{\prime} 35!' 32$ | $102^{\circ} 05^{\prime} 03!\cdot 51$ |
| 1 | 3 | 12, 1/4 E. By. | 490116.00 | $10216 \quad 29.49$ |
| 1 | 5 | 1, 1/4 S. By. | 485957.18 | 1023316.10 |
| 1 | 6 | 13, 1/4 E. By. | 490209.83 | 1024040.32 |
| 1 | 12 | 24, 1/4 N. By. | $4903 \quad 26.95$ | 1032910.91 |
| 1 | 13 | 10, 1/4 N. By. | 490141.84 | 1033950.62 |
| 1 | 14 | 8, 1/4 N. By. | 490142.87 | 1035033.82 |
| 1 | 16 | 26, N.E. Cor. | $\begin{array}{llll}49 & 04 & 23.28\end{array}$ | 1040154.93 |
| 1 | 18 | 28, N.E. Cor. | $49 \quad 04 \quad 20.68$ | 1042035.05 |
| 1 | 23 | 17, 1/4 E. By. | 490208.65 | 1050158.66 |
| 1 | 27 | 7, 1/4 E. By. | $4901 \begin{array}{lll}49 & 17.63\end{array}$ | $10535 \quad 21.87$ |
| 2 | 7 | $8,1 / 4 \mathrm{~N}$. By. | $4907 \quad 01.64$ | 1025445.67 |
| 2 | 10 | 10, N.E. Cor. | $49 \quad 06 \quad 56.90$ | $\begin{array}{llll}103 & 15 & 09.83\end{array}$ |
| 2 | 26 | 12, 1/4 N. By. | $49 \quad 06 \quad 57.34$ | $\begin{array}{llll}105 & 21 & 20.10\end{array}$ |
| 2 | 29 | 22, N.E. Cor. | 490843.11 | $\begin{array}{llll}105 & 47 & 22.72\end{array}$ |
| 5 | 29 | 14, N.E. Cor. | $\begin{array}{llll}49 & 23 & 30.21\end{array}$ | 1054738.56 |
| 6 | 26 | 25, 1/4 S. By. | $49 \quad 2937.41$ | $105 \quad 2246.01$ |
| 16 | 23 | 24, N.E. Cor. | $\begin{array}{llll}50 & 22 & 03.34\end{array}$ | 1050148.25 |
| 16 | 29 | 33, 1/4 E. By. | $\begin{array}{llll}50 & 23 & 22.45\end{array}$ | $105 \quad 55 \quad 25.86$ |
| 17 | 20 | 14, 1/4 E. By. | $\begin{array}{llll}50 & 25 & 59.75\end{array}$ | $1 \begin{array}{ll}104 & 38 \quad 25.97\end{array}$ |
| 18 | 24 | 23, 1/4 E. By. | $\begin{array}{llll}50 & 32 & 06.50\end{array}$ | $\begin{array}{llll}105 & 11 & 27.43\end{array}$ |
| 20 | 26 | 20, N. E. Cor. | $\begin{array}{llll}50 & 43 & 01.67\end{array}$ | $\begin{array}{ll}105 & 33 \\ 105\end{array}$ |
| 21 | 24 | 34, N. E. Cor. | $\begin{array}{llll}50 & 50 & 01.05\end{array}$ | $\begin{array}{llll}105 & 14 & 20.97\end{array}$ |
| 22 | 28 | 24, N.E. Cor. | $\begin{array}{lll}50 & 53 & 30.48\end{array}$ | $10544 \quad 50.34$ |
| 26 | 26 | 6, N.E. Cor. | $\begin{array}{llll}51 & 11 & 51.27\end{array}$ | $\begin{array}{llll}105 & 36 & 45.77\end{array}$ |
| 28 | 26 | 7, 1/4 N. By. | $\begin{array}{llll}51 & 23 & 13.37\end{array}$ | 1053902.66 |
| 28 | 27 | 34, N. E. Cor. | $\begin{array}{llll}51 & 26 & 43.03\end{array}$ | 1054234.06 |
| 29 | 22 | 32, N.E. Cor. | $\begin{array}{llll}51 & 31 & 57.18\end{array}$ | 1050311.40 |


| Tp. | R. |  | Section | Latitude |  |  | Longitude |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 41 | 27 | 27. | N. E. Cor. | 52 | 33 | 56.08 | 105 | 47 | 38.98 |
| 45 | 20 | 23, | N. W. Cor. | 52 | 53 | 59.84 | 104 | 48 | 31.98 |
| 45 | 22 | 22, | N.E. Cor. | 52 | 56 | 54.41 | 105 | 07 | 19.81 |
| 45 | 22 | 18, | 1/4 W. By. | 52 | 55 | 35.27 | 105 | 13 | 08.93 |
| 45 | 24 | 23, | $1 / 4 \mathrm{E}$. By. | 52 | 53 | 34.20 | 105 | 21 | 54.30 |
| 46 | 20 | 22, | N. E. Cor. | 52 | 59 | 14.18 | 104 | 48 | 33.09 |
| 47 | 22 | 31. | $1 / 4 \mathrm{E}$. By. | 53 | 05 | 47.57 | 105 | 11 | 47.74 |

West of Third Meridian

| Tp. | R. | Section | Latitude | Longitude |
| :---: | :---: | :---: | :---: | :---: |
| 1 | 14 | 4, 1/4 E. By. | $49^{\circ} 00^{\prime} 25!^{\prime} 42$ | $107^{\circ} 48^{\prime} 32!^{\prime} 63$ |
| 1 | 22 | 34, N. E. Cor. | $\begin{array}{llll}49 & 05 & 12.37\end{array}$ | 1085117.34 |
| 2 | 3 | 12, N. E. Cor. | $49 \quad 06 \quad 59.32$ | $\begin{array}{lll}106 & 16 & 25.29\end{array}$ |
| 2 | 5 | 33, N. E. Cor. | $\begin{array}{llll}49 & 10 & 28.07\end{array}$ | $10636 \quad 26.39$ |
| 2 | 9 | 20, 1/4 E. By. | $\begin{array}{llll}49 & 08 & 17.34\end{array}$ | 1070949.58 |
| 2 | 16 | 31, N.E. Cor. | $\begin{array}{llll}49 & 10 & 27.53\end{array}$ | $10807 \quad 14.09$ |
| 2 | 19 | 4, 1/4 E. By. | $4905 \quad 39.48$ | $108 \quad 28 \quad 34.96$ |
| 2 | 24 | 33, N.E. Cor. | $\begin{array}{llll}49 & 10 & 27.37\end{array}$ | $10908 \quad 39.63$ |
| 2 | 25 | 34, N.E. Cor. | $49 \quad 10 \quad 27.21$ | $109 \quad 15 \quad 20.54$ |
| 3 | 1 | 3, 1/4 E. By. | $49 \quad 10 \quad 54.44$ | 1060300.62 |
| 3 | 7 | 2, S.E. Cor. | $\begin{array}{llll}49 & 10 & 28.40\end{array}$ | $10650 \quad 07.99$ |
| 3 | 18 | 9, N.E. Cor. | $\begin{array}{llll}49 & 12 & 12.24\end{array}$ | 1082134.24 |
| 3 | 20 | 14, N. E. Cor. | $\begin{array}{llll}49 & 13 & 04.75\end{array}$ | 1083501.35 |
| 4 | 10 | 11, 1/4 W. By. | $\begin{array}{llll}49 & 17 & 27.72\end{array}$ | 1071500.88 |
| 4 | 22 | 21, N.E. Cor. | $\begin{array}{llll}49 & 19 & 11.37\end{array}$ | 1085351.33 |
| 7 | 1 | 36, N. E. Cor. | $4936 \quad 39.40$ | 1060013.50 |
| 10 | 1 | 36, S.E. Cor. | 495130.72 | $10600 \quad 16.65$ |
| 10 | 1 | 36, N. E. Cor. | 495222.91 | $10600 \quad 16.60$ |
| 40 | 4 | 33, N. E. Cor. | $\begin{array}{llll}52 & 29 & 34.49\end{array}$ | $10630 \quad 26.70$ |
| 40 | 6 | 23, N.E. Cor. | $\begin{array}{llll}52 & 27 & 49.57\end{array}$ | 1064450.33 |
| 41 | 4 | 31, N. E. Cor. | $\begin{array}{llll}52 & 34 & 48.60\end{array}$ | $\begin{array}{llll}106 & 33 & 19.43\end{array}$ |
| 42 | 2 | 26, 1/4 E. By. | $\begin{array}{llll}52 & 38 & 45.40\end{array}$ | $10610 \quad 16.66$ |
| 42 | 3 | 22, N. E. Cor. | $\begin{array}{llll}52 & 38 & 18.61\end{array}$ | $\begin{array}{llll}106 & 20 & 21.55\end{array}$ |
| 42 | 5 | 21, N.E. Cor. | $\begin{array}{llll}52 & 38 & 18.28\end{array}$ | 1063904.72 |
| 43 | 15 | 28, N.E. Cor. | $\begin{array}{llll}52 & 44 & 24.42\end{array}$ | 1080628.70 |
| 44 | 10 | 21, N.E. Cor. | $\begin{array}{llll}52 & 48 & 45.48\end{array}$ | 1072256.48 |
| 44 | 12 | 36, N.E. Cor. | $\begin{array}{llll}52 & 50 & 30.59\end{array}$ | $10735 \quad 59.68$ |
| 46 | 20 | 19, N.E. Cor. | $\begin{array}{llll}52 & 59 & 14.19\end{array}$ | 1085256.06 |
| 47 | 24 | 15, N.W. Cor. | $\begin{array}{llll}53 & 03 & 34.56\end{array}$ | $10926 \quad 30.14$ |
| 50 | 1 | 12, 1/4 E. By. | $\begin{array}{llll}53 & 17 & 59.38\end{array}$ | 1060012.97 |
| 50 | 26 | 3, N.E. Cor. | $\begin{array}{llll}53 & 17 & 32.86\end{array}$ | 1094237.01 |

West of Fourth Meridian

| Tp. | R. | Section | Latitude | Longitude |
| :---: | :---: | :---: | :---: | :---: |
| 8 | 3 | 5, 1/4 E. By. | $49^{\circ} 37^{\prime} 05!82$ | $110^{\circ} 21 \prime 55!71$ |
| 8 | 7 | 11, 1/4 E. By. | $4937 \quad 57.77$ | 1105022.29 |
| 9 | 10 | $32,1 / 4 \mathrm{~N} . \mathrm{By}$. | 494709.50 | 1111928.44 |
| 10 | 10 | 25, 1/4 N. By. | 495132.36 | 1111403.98 |
| 14 | 10 | 7, $1 / 4 \mathrm{~N}$. By. | $\begin{array}{llll}50 & 09 & 51.73\end{array}$ | $\begin{array}{llll}111 & 21 & 25.77\end{array}$ |
| 15 | 4 | 5, 1/4 E. By. | $\begin{array}{llll}50 & 13 & 47.53\end{array}$ | $\begin{array}{ll}110 & 30\end{array} 28.92$ |


| Tp. | R. | Section |  |  |  |  |  |  |  |  | Latitude |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |

West of Fifth Meridian

| Tp. | R. | Section | Latitude | Longitude |
| :---: | :---: | :---: | :---: | :---: |
| 27 | 2 | 23, N. E. Cor. | $51^{\circ} 19 \prime 44!' 94$ | $114^{\circ} 09^{\prime} 53!^{\prime} 86$ |
| 31 | 4 | 10, N.E. Cor. | 513857.67 | 1142823.83 |
| 33 | 1 | 8, N.E. Cor. | $\begin{array}{llll}51 & 49 & 26.42\end{array}$ | $\begin{array}{llll}114 & 05 & 43.57\end{array}$ |
| 34 | 1 | 28, 1/4 N. By. | $\begin{array}{llll}51 & 57 & 18.47\end{array}$ | 1140500.74 |
| 51 | 4 | 32, 1/4 E. By. | $53 \quad 2648.21$ | 1143229.07 |
| 51 | 23 | 21, N.E. Cor. | $\begin{array}{llll}53 & 25 & 29.15\end{array}$ | 1171901.44 |
| 52 | 1 | $8,1 / 4 \mathrm{~N}$. By. | $\begin{array}{llll}53 & 28 & 58.63\end{array}$ | 1140640.53 |
| 52 | 3 | 14, N.E. Cor. | $53 \quad 2951.11$ | $\begin{array}{llll}114 & 19 & 12.62\end{array}$ |
| 52 | 24 | 31, N.E. Cor. | $5332 \begin{array}{ll}53 & 27.39\end{array}$ | 1173047.86 |
| 53 | 1 | 24, N.E. Cor. | 5313556.94 | 1140003.48 |
| 53 | 2 | 1, N.E. Cor. | $\begin{array}{llll}53 & 33 & 20.25\end{array}$ | 1140854.74 |
| 53 | 1 | 36, N.E. Cor. | $\begin{array}{llll}53 & 37 & 41.74\end{array}$ | 1140003.50 |
| 53 | 8 | 11, 1/4 E. By. | $\begin{array}{llll}53 & 33 & 48.05\end{array}$ | 1150325.10 |
| 53 | 12 | 23, N.E. Cor. | $53 \quad 3556.66$ | 11583846.64 |
| 53 | 13 | 17, N.E. Cor. | $\begin{array}{llll}53 & 35 & 04.92\end{array}$ | 1155204.11 |
| 54 | 6 | 32, 1/4 N. By. | 534255.97 | 1145053.00 |
| 54 | 7 | $30,1 / 4 \mathrm{~N}$. By. | 5314203.16 | 1150109.90 |
| 54 | 9 | 17, N.E. Cor. | $\begin{array}{llll}53 & 40 & 19.52\end{array}$ | $\begin{array}{llll}115 & 16 & 40.54\end{array}$ |
| 54 | 12 | 9, N.E. Cor. | $53 \quad 3926.92$ | 1154145.22 |
| 54 | 13 | 8, N.E. Cor. | $\begin{array}{llll}53 & 39 & 26.47\end{array}$ | $115 \quad 5204.21$ |
| 54 | 18 | 2, N.E. Cor. | $\begin{array}{llll}53 & 38 & 33.94\end{array}$ | 1163152.02 |
| 55 | 11 | 32, 1/4 E. By. | $\begin{array}{llll}53 & 47 & 44.26\end{array}$ | 1153511.89 |
| 55 | 20 | 19, N.E. Cor. | $\begin{array}{llll}53 & 46 & 25.60\end{array}$ | 1165654.19 |
| 56 | 18 | 19, 1/4 N. By. | 535139.31 | 1163948.23 |

West of Sixth Meridian
$451 \quad 9$, N.E. Cor. $\quad \begin{array}{llllllll}52 & 52 & 19.26 & 118 & 04 & 17.13\end{array}$

## CHAPTER V

## CONSTRUCTION AND USE OF THE TABLES

The geodetic tables of the Supplement have been based on the dimensions given by Col. Clarke (1866) for the figure of the earth.

In the computation of the tables, no account has been taken of the irregularities and errors of survey, or of the altitude above sealevel at which the surveys are made. The surveys are considered as based on a parallel of latitude of $49^{\circ}$, and the different townships and sections as having their theoretic dimensions.

The tables therefore, do not strictly represent the geodetic quantities for the different points of the Canada lands system. The method of applying corrections for altitude and known errors of survey is briefly treated on pages 43 and 45 . The actual errors of survey and the imperfections of all geodetical assumptions are too complicated for treatment here.

These discrepancies, however, are always small, and will exert no appreciable influence on the field work of a surveyor. With the exception of the latitudes and differences of longitudes, the errors of the tables are negligible.

## TABLE I

LENGTHS OF ARCS OF MERIDIANS, PARALLELS, ETC., IN DIFFERENT LATITUDES

According to Col. A. R. Clarke,R.E., in his "Comparison of Standards of Length" (1866), the spheroid of revolution most nearly approaching the form of the earth has for its major or equatorial semiaxis $20,926,062$ feet, and for its minor or polar semi-axis $20,855,121$ feet.

Representing the semi-major andsemi-minor axis by a and b respectively, we have for the compression

$$
\begin{aligned}
& \mathrm{c}=\frac{\mathrm{a}-\mathrm{b}}{\mathrm{a}}=\frac{1}{294.98} \text {, and the eccentricity } \mathrm{e} \text { is given by the formula } \\
& \mathrm{e}^{2}=\frac{\mathrm{a}^{2}-\mathrm{b}^{2}}{\mathrm{a}^{2}}=0.0067686
\end{aligned}
$$

The unit of measure in the Canada lands surveys is the Gunter's, or 66 -foot chain. The equatorial semi-axis in chains is $317,061.545$.

Representing by $L$ the geographical latitude of a place, or the angle which its vertical line makes with the plane of the equator, we have for the radius of curvature of the meridian

$$
R=\frac{a\left(1-e^{2}\right)}{\left(1-e^{2} \sin ^{2} L\right)^{3 / 2}}
$$

for the length of the normal to the meridian terminated by the minor axis

$$
N=\frac{a}{\left(1-e^{2} \sin ^{2} L\right)^{1 / 2}}
$$

and for the radius of the parallel of latitude $L$

$$
P=N \cos L
$$

The length in chains of one second of latitude is equal to R sin 1"; one second of the great circle perpendicular to the meridian is equal to $N \sin 1^{\prime \prime}$; and one second of longitude is equal to $P \sin l^{\prime \prime}$. The logarithms of these quantities are placed in the second, third and fourth columns of Table I. They have been calculated by means of the logarithmic expansions of $R$ and $N$.

Thus putting $n$ for $\frac{a-b}{a+b}$ we have

$$
\begin{array}{r}
\log \left(R \sin 1^{\prime \prime}\right)=\log \left[a(1-n)^{2}(1+n) \sin 1^{\prime \prime}\right] \\
-3 \mu n \cos 2 L+3 / 2 \mu n^{2} \cos 4 L-e t c .
\end{array}
$$

where $\mu$ is the modulus of the commonsystem of logarithms, and powers of $n$ higher than the second are neglected as being insensible in the eighth decimal place.

Substituting the value of a in chains, as given above, and taking

$$
\begin{aligned}
& n=\frac{a-b}{a+b}=\frac{1}{588.96}, \text { we get } \\
& \begin{aligned}
\log \left(R \sin l^{\prime \prime}\right)=0.18597916 & -0.00221218 \cos 2 L \\
& +0.00000188 \cos 4 L
\end{aligned}
\end{aligned}
$$

In calculating the last two terms by logarithms five places are sufficient.

For $N \sin 1^{\prime \prime}$ we have
$\log \left(N \sin 1^{\prime \prime}\right)=1 / 3 \log \left(R \sin 1^{\prime \prime}\right)+2 / 3\left\{\log a+\log \sin 1^{\prime \prime}+2 \mu n\right\}$
$=1 / 3 \log \left(\mathrm{R} \sin 1^{\prime \prime}\right)+0.12546215$,
and for $P \sin 1^{\prime \prime}$
$\log \left(P \sin 1^{\prime \prime}\right)=\log \left(N \sin 1^{\prime \prime}\right)+\log \cos L$.
The calculation has been made to eight places of decimals to ensure accuracy in the seventh place. In tabulating the eighth figure has been dropped.

The calculation of the logarithms of $R \sin 1^{\prime \prime}$ and $N \sin l^{\prime \prime}$ has also been made directly from the formulae for $R$ and $N$, by the use of a subsidiary angle.

Thus, finding an angle $\psi$ such that $\sin \psi=e \sin L$, we have

$$
\begin{aligned}
& \mathbf{R} \sin 1^{\prime \prime}=a\left(1-e^{2}\right) \sec ^{3} \psi \sin 1^{\prime \prime} \\
& N \sin 1^{\prime \prime}=a \sec \psi \sin 1^{\prime \prime}
\end{aligned}
$$

Seven figure logarithms were used, and consequently the results could not be depended upon to the seventh figure, but they have been serviceable as a check upon the series computation.
$\log \mathrm{N} \sin 1^{\prime \prime}, \log \mathrm{P} \sin 1^{\prime \prime}$ and $\log \mathrm{R} \sin 1^{\prime \prime}$ are given in the table for every $10^{\prime}$ of latitude from $42^{\circ}$ to $70^{\circ}$. Their values for intermediate latitudes can be obtained by simple interpolation. Where, however, $\log P \sin l^{\prime \prime}$ is required with accuracy for an intermediate latitude, it is better first to obtain $\log \mathrm{N} \sin l^{\prime \prime}$ for the latitude by interpolation from the table and then to add log cos L.

Under the heading 'Chains in $1^{\prime \prime \prime}$ are given the natural numbers corresponding to the logarithms of $R \sin l^{\prime \prime}$ and $P \sin l^{\prime \prime}$. These natural numbers are useful in reducing small differences of latitude and longitude to chains by simple multiplication, being preferable in many cases to the logarithms.

The converse operation of reducing short distances north and south or east and west to seconds of latitude or longitude may be performed by multiplying by the quantities in the two columns headed "Seconds in one Chain." These columns contain the reciprocals of the quantities in the columns "Chains in l"'.

In the last two columns of the table are given the lengths of one degree of latitude and longitude in English miles.

## RADIUS OF CURVATURE OF A SECTION OF THE SPHEROID INCLINED AT ANY ANGLE TO A MERIDIAN

In some operationsit is necessaryto find the radius of curvature of the trace on the earth's surface of a "straight" or "transit" line, making a given angle with the meridian.

Representing this radius of curvature by $S$, and $\theta$ being the angle with the meridian, we have the formula

$$
\frac{1}{S}=\frac{\cos ^{2} \theta}{R}+\frac{\sin ^{2} \theta}{N}
$$

and introducing an auxiliary angle $X$ determined by the formula

$$
\begin{aligned}
& \tan X=\sqrt{\frac{R \sin 1^{\prime \prime}}{N \sin 1^{\prime \prime}}} \tan \theta, \text { we have } \\
& S \sin 1^{\prime \prime}=N \sin 1^{\prime \prime} \frac{\sin ^{2} X}{\sin ^{2} \theta}
\end{aligned}
$$

a formula adapted for ready calculation by logarithms.

## RADIUS OF SPHERICAL CURVATURE

The mean of the values of $S$ when $\theta$ is given all possible values is $\sqrt{N R}$. This is the radius of curvature of the surface or the radius of the sphere to the surface at a given point. Its logarithm is readily found from Table $I$, being the arithmetical mean of the logarithms of $N$ and $R$.

TABLE II

## CORRECTIONS TO TABLE I FOR CHANGE IN ELEMENTS OF FIGURE OF EARTH

In Table I the data used are Clarke's 1866 values, viz. :
$a=20,926,062$ feet
$\mathrm{n}=\frac{1}{588.96}$
and all the following tables are based on Table I, and therefore on these values. Clarke's later values (Geodesy, 1880) are,

$$
\begin{aligned}
& \mathrm{a}=20,926,202 \text { feet } \\
& \mathrm{n}=\frac{1}{585.93}
\end{aligned}
$$

If, for any purpose, it is desired to use the se values, Table I can be
corrected by means of Table II, which has been computed thus:
Differentiating the formulae,
$\log R \sin 1^{\prime \prime}$
$=\log a+\log \sin 1^{\prime \prime}-\mu\left(n+3 / 2 n^{2}\right)-3 \mu n \cos 2 L+3 / 2 \mu n^{2} \cos 4 L$
$\log \mathrm{N} \sin 1^{\prime \prime}$

$$
=\log a+\log \sin 1^{\prime \prime}+\mu\left(n-1 / 2 n^{2}\right)-\mu n \cos 2 L+1 / 2 \mu n^{2} \cos 4 L
$$

and putting $\frac{1}{n}=p$, we have

$$
\begin{aligned}
& d\left(\log R \sin 1^{\prime \prime}\right)=\mu \frac{d a}{a}+\mu n^{2} d p+3 \mu n^{2} \cos 2 L d p \\
& d\left(\log N \sin 1^{\prime \prime}\right)=\mu \frac{d a}{a}-\mu n^{2} d p+\mu n^{2} \cos 2 L d p
\end{aligned}
$$

$\mu$ being the modulus of the common system of logarithms. Terms involving the cubes and higher powers of $n$ are insensible and may be neglected.

To change Clarke's earlier to his later values, we have

$$
\begin{aligned}
\mathrm{da} & =+140 \text { (feet) } \\
\mathrm{d} p & =-3.024 \\
\mathrm{a} & =20926062 \text { (feet) } \\
\mathrm{n} & =\frac{1}{588.96}
\end{aligned}
$$

$$
\text { and } \mu=0.43429448
$$

whence $d \log \left(R \sin 1^{\prime \prime}\right)=-.00000088-.00001136 \cos 2 L$
$d \log \left(N \sin 1^{\prime \prime}\right)=+.00000669-.00000379 \cos 2 L$
These quantities are tabulated in Table 11, with the proper signs of application to $\log \mathrm{R} \sin 1^{\prime \prime}$ and $\log \mathrm{N} \sin 1^{\prime \prime}$ in Table I.

## TABLE 111

LATITUDES OF BASE AND CORRECTION LINES AND LENGTHS OF ARCS OF MERIDIANS, PARALLELS, ETC. , FOR FIRST AND SECOND SYSTEMS OF SURVEY

This table is constructed for the first and second systems of survey only. 1t accordingly stops at the 13 th base, township 48, north of which there are no surveys under these systems.

Each township measuring 489 chains each way, the lst correction line is 978 chains north of the 49 th parallel.

The latitude of the 1 st correction line is therefore

$$
49^{\circ}+\frac{978}{R \sin 1^{\prime \prime}}
$$

Here $R \sin 1^{\prime \prime}$ must be taken from Table I for the middle latitude between the 1 st base and the lst correction line. For accuracy it is necessary therefore to compute an approximate difference of latitude, using an approximate value of R sin 1 ". For instance $\mathrm{R} \sin \mathrm{l}^{\prime \prime}$ may be taken from the table for latitude $49^{\circ}$.

The approximate difference of latitude being thus determined, the middle latitude is found from it (this being a sufficiently close
approximation), and the final $R \sin 1^{\prime \prime}$ is taken from Table I for that latitude. Then dividing 978 by this we have a very close approximation to the difference of latitude between the base and the correction line.

From the latitude of the lst correction line, that of the 2nd base line is found by a similar process, and so on in succession as far as the table extends.

The table is checked by applying the same process to a longer distance than 978 chains. For example, the latitude of the 6 th base can be directly determined from that of the first by using 9,780 chains instead of 978 . When long distances are thus taken, a second approximation to the middle latitude may become necessary.

The columns $\log N \sin 1^{\prime \prime}$ and $\log R \sin 1^{\prime \prime}$ are taken from Table I by interpolation, and $\log P \sin 1^{\prime \prime}$ is found by adding $\log \cos \mathrm{L}$ to $\log N \sin 1^{\prime \prime}$.

The width of a township along a base line is 489 chains. The longitude corresponding to this length measured along the parallel of latitude is given in the column headed 'Longitude covered by 489 chains of westing", not only for the base lines but also for the correction lines.

The longitude for 489 chains, along a base line, is the longitude covered by one range of townships. Along a correction line it does not correspond to the longitude covered by a range, since the width of a township along a correction line is greater or less than 489 chains according as the township north or south of the correction line is considered. The tabulated quantity, however, for correction lines can be used to calculate the narrowing or widening of sections at the correction lines.

The township width, 489 chains, is measured along the base line which has such azimuth that its terminal point falls in the same latitude as its initial point.

Thus every township corner along a base line has the same latitude, and the base line is a succession of chords of the latitude circle.

The difference of longitude between one township corner and the next is given by the formula

$$
\mathrm{dM}=\frac{489}{P \sin 1^{\prime \prime}}
$$

It is assumed here that the chord of the arc of the latitude circle is equal to the arc. That the difference between the chord and the arc is inappreciable may be shown thus:

By spherical trigonometry

$$
\sin \frac{\text { chord }}{2 N}=\sin \frac{d M}{2} \cos L
$$

whence chord $=N \cos L d M-N \cos L \sin ^{2} L \frac{d M^{3}}{24}$

$$
=\operatorname{arc}-\operatorname{arc} \times \frac{d M^{2}}{24} \sin ^{2} L
$$

so that the difference between the chord and the arc is equal to

$$
\operatorname{arc} \times \frac{d M^{2}}{24} \sin ^{2} L
$$

dM being in circular measure.

For any township chord this amounts to less than one fiftieth of a link.

The chord always lies north of the arc. The distance between them is greatest at their middle points, amounting there to about 10 links. Hence, at the international boundary line, which is the first base line, since the actual territorial boundary is the curve, and the base line a series of chords, the road allowance which lies along the north side of this base is increased in width by 10 links at the middle of the chords.

The non-coincidence of the chord and arc also has the effect of increasing and decreasing the widths of roads on correction lines, since on account of the jog, the township corners north and south of the road are not opposite one another. The increase or decrease in the width of the road along correction lines, when required, may be easily found by an appice ion of Table XI.

In the first column of Table III are given, for convenience, the numbers of the townships corresponding to the several base and correction lines. Thus the 6th base is the northern boundary of township 20, and so on.

TABLE IV

## LATITUDES OF BASE AND CORRECTION LINES, ETC., FOR THIRD AND FOURTH SYSTEMS OF SURVEY

This table is similar to Table III, except that it is made for the third system of survey, where the widths of townships are 486 instead of 489 chains, and their depths, in a north and south direction, 483 instead of 489 chains.

The table also applies, without change, to the fourth system (British Columbia).

In this table, as well as in Table III, the latitudes given are those of the line of posts on the south side of the road allowance. To get the latitude of the posts north of the road on correction lines, the latitude of the correction line, as given in the table, must be corrected by adding the equivalent in latitude of the width of the road, i.e., one chain and a-half for the first and second systems (Table III), and one chain for the third system (Table IV).

## TABLE V

## CHORD AZIMUTHS, ETC., FOR BASE LINES, FIRST AND SECOND SYSTEMS OF SURVEY

The extremities of the township chord, as above stated, are in the same latitude. Hence the chord is equally inclined to the meridians passing through its terminal points, and its azimuth, east or west of north, is equal to the complement of half the change in azimuth, that is, of half the "convergence of meridians."

Let $d Z$ represent the change in azimuth or convergence of meridians, dM the difference of longitude, and L the latitude.

Then, by spherical trigonometry

$$
\tan 1 / 2 d Z=\tan 1 / 2 d M \sin L
$$

whence, by expansion of the tangents in terms of the arcs,

$$
d Z=d M \sin L+\frac{d M^{3}}{12} \sin L \cos ^{2} L
$$

or, if dZ and dM be expressed in seconds,

$$
\mathrm{dZ}=\mathrm{dM} \sin \mathrm{~L}+\frac{\mathrm{d} \mathrm{M}^{3}}{12} \sin L \cos ^{2} L \sin ^{2} 1^{\prime \prime}
$$

The second term is inappreciable, amounting in latitude $51^{\circ}$ to less than one ten-thousandth of a second.

$$
\therefore d Z=d M \sin L
$$

The convergence or "deflection" ( $\mathrm{d} Z$ ), given in Table V , is thus calculated from the difference of longitude ( dM ) in Table III.

The "chord azimuth" is the complement of half the deflection.
The chord azimuth,convergence for 100 chains and the deflection are given in the table in degrees, minutes and seconds.

In the survey of a base line, the surveyor, when he arrives at a township corner, deflects his line to the north through an angle equal to the "deflection," and thus establishes in azimuth the chord across the next range of townships.

This deflection angle may be turned with the instrument, but more readily by the use of the "deflection offsets" in the table. The tabulated offset is the linear distance in inches between one of the chords and the prolongation of the other, at one chain from the township corner.

Their distance apart at any point is found by multiplying the tabulated offset by the distance, expressed in chains, of the point from the township corner.

For example, if the instrument stand on the prolongation of the first chord at 15 chains past the corner, and the back picket at 40 chains on the other side of the corner, that is, behind the corner, then the instrument must be moved north fifteen times, and the back picket south forty times, the "deflection offset for one chain". The line of the instrument and picket will then be in the correct bearing for the prolongation of the base line.

The angle is thus turned as accurately as a straight line can be produced with the instrument, and much more accurately than the angle can be measured with the graduated arc, while the setting of the instrument at the corner (which may be in low ground, unsuitable for accurate line production) is rendered unnecessary.
"Longitude covered by one range" in the seventh column is merely the longitude in the seventh column of Table III, reduced to time by dividing by 15 . This gives the number of seconds which a watch will gain or lose on local time in being carried across a range. The gain or loss intravelling over any other distance along the base line is proportional to the distance. The column is added for astronomical purposes, especially the determination of azimuth by observation of Polaris at any hour angle.

Table V applies to the first and second systems of survey.

## TABLE VI

CHORD AZIMUTHS, ETC., FOR BASE LINES, THIRD AND FOURTH SYSTEMS OF SURVEY

This table is similar to Table $V$, but is made for the third system of survey.

The calculation is made by the same formulae, changing only the width of the range, which is 486 instead of 489 chains, and using the latitudes of the base lines from Table IV, instead of those from Table III.

$$
\mathrm{dM}=\frac{486}{P \sin 1^{\prime \prime}}, \quad \mathrm{dZ}=\mathrm{dM} \sin L
$$

The table also applies to the fourth system.

## TABLE VII

CHORD AZIMUTHS, JOGS, ETC., FOR CORRECTION LINES, FIRST AND SECOND SYSTEMS OF SURVEY

This table gives quantities for correction lines similar to those given in Table $V$ for base lines. It applies to the first and second systems of survey.

The correction lines are posted on both sides of the road. The chord azimuths and deflections are given for the south side of the road, which is that side for which the latitudes of correction lines are given in Table III.

The calculation of the chord azimuth for correction lines is somewhat different from that for base lines.

For the base lines we have

$$
\begin{aligned}
& \mathrm{dM}=\frac{489}{P \sin 1^{\prime \prime}} \\
& \text { deflection }=d M \sin L
\end{aligned}
$$

For the correction lines, one range is not 489 chains, but the distance between meridians which include 489 chains on the nearest base line.

Hence in the formulae: -

$$
\begin{aligned}
& \mathrm{dM}=\frac{489}{P \sin 1^{\prime \prime}} \\
& \text { and deflection }=d M \sin L=\frac{489}{P \sin 1^{\prime \prime}} \sin L
\end{aligned}
$$

we must take P sin $l^{\prime \prime}$ for the next base line south of the correction line, if the difference of longitude and the deflection for the south side of the correction line road are required; while for the north side of that road we must take $P \sin l^{\prime \prime}$ for the next base line north. L of course, is the latitude of the correction line itself.

The length of one range on the correction line is $d M \times P \sin 1^{\prime \prime}$.
If, then, $P_{1}$ and $P_{2}$ represent the radii of parallels for the base lines next north and south, respectively, and $P$ that for the correction line itself; then

$$
\mathrm{d} M_{1}=\frac{489}{P_{1} \sin 1^{\prime \prime}} \quad, \quad \mathrm{dM}_{2}=\frac{489}{P_{2} \sin 1^{\prime \prime}}
$$

and we have for the length of one range on the correction line: -

$$
\begin{aligned}
& \text { North side }=\frac{489}{P_{1} \sin 1^{\prime \prime}} \times P \sin 1^{\prime \prime} \\
& \text { South side }=\frac{489}{P_{2} \sin 1^{\prime \prime}} \times P \sin 1^{\prime \prime}
\end{aligned}
$$

The values of the se quantities are tabulated in the sixth and seventh columns of Table VII.

For extreme accuracy P sin 1 "for the north side of the road should be taken out for a latitude greater by 1.50 chains, or $0!' 98$ greater than that tabulated in Table III; but the difference in the result would be almost inappreciable, being less than one quarter of a link per township.

The difference of lengths of the township lines north and south of the correction line road gives the overlap or jog.

The jog for one range is given in the eighth column of the table. As this jog occurs in each range of townships, its value at any range is the product of the jog for one range by the number of ranges.

The excess of the length of the north side over, or the defect of the south side from 489 chains, is the linear divergence or convergence of the township lines. Since there are twelve half sections in a township side, the convergence or divergence for one half section is one-twelfth of the convergence or divergence for the township, or one twenty-fourth of the jog, the excess of the north side and the defect of the south side being very nearly, though not quite, equal.

This convergence or divergence for one half section is entered in the ninth column of the table. It is used in the second system, where the surplus or deficiency caused by the convergence of meridians is divided equally among all the quarter-sections, Hence, in surveying a correction line under the second system, the width of each quarter section (exclusive of the roads) is forty chains plus or minus this tabulated quantity. The surplus or deficiency on the township line midway between the base and the correction line ishalf of that on the correction line.

In the first system the whole of the surplus or deficiency is thrown into the western tier of quarter sections. This surplus or deficiency is the difference between 489 chains and the quantities in the sixth and seventh columns of Table VII. For example, on the north side of the road on the lst correction line the surplus is 1.75 chains, and the we sterly quarter section of the townshipis therefore 41.75 , all the others being 40 chains.

It is to be observed thatin all cases the whole divergence or convergence is applied to the section itself, and that the road allowance retains its width of 1 chain or $11 / 2$ chains, with the exception of the roads on correction lines, which are subject to a widening or narrowing as explained under Table III.

TABLE VIII
CHORD AZIMUTHS, JOGS, ETC., FOR CORRECTION LINES, THIRD AND FOURTH SYSTEMS OF SURVEY

This table gives for the third and fourth systems the same quantities as are given in Table V1I for the first and second systems.

The surplus or deficiency is in all cases divided equally among all the quarter sections.

## TABLE IX

## LATITUDES, AND WIDTHS IN CHAINS, OF NORTHERN BOUNDARIES OF SECTIONS IN FIRST AND SECOND SYSTEMS OF SURVEY

This table, with Table XI, gives the latitudes in degrees, minutes and seconds for the northern boundaries of all sections in the first and second systems.

The sections numbered in the second column are those adjacent to the eastern boundary of the township. The latitudes of the northern boundaries of interior sections lying west of these are approximately the same. Thus the northern boundaries of sections $14,15,16$, 17 and 18 have very nearly the same latitude as the north boundary of 13 , and sofor the other east and west tiers of sections. The small corrections required to the latitudes of Tables IX and X to obtain the latitudes of the northeast corners of sections not on the bounding meridians of townships are given in Table XI.

These latitudes are computed by interpolating from the latitudes given in Table III.

The logarithmic secant and tangent of the latitude are given in the table for use in calculation of azimuth observations.

In the last column of the table are given the widths of the north boundaries of the quarter sections (in the second system of survey). These are calculated for the correction lines in the mannerexplained under Table VII, and for the intermediate lines by interpolation.

For quarter sections adjoining correction lines the usual width is given for the north boundary of the quarter section to the south of the correction line; bracketed with it is also given the width, measured along the south boundary, of the quarter section immediately to the north. That is, the two lengths bracketed are the lengths of quarter section sides measured along the south limit and the north.limit, respectively, of the road on correction lines.

TABLE X
LATITUDES, AND WIDTHS IN CHAINS, OF NORTHERN BOUNDARIES OF SECTIONS IN THIRD AND FOURTH SYSTEMS OF SURVEY

This table gives for the third system the same quantities as are given in Table IX for the first and second.

The table may also be applied to the fourth system by correcting the latitudes of the alternate section lines, viz., the north boundaries of sections 1,13 and 25 in each township, by subtracting therefrom $0!\cdot 33$, the equivalent in arc of 50 links. The change in the logarithmic secant and tangent is inappreciable, as these logarithms are given to only five places of decimals. The widths of quarter sections in the last column must be increased by 50 links.

## TABLE XI

## DIFFERENCE OF LATITUDE BETWEEN TOWNSHIP CORNERS AND SECTION AND QUARTER SECTION CORNERS

This table is used when it is required to find the latitude of any point on a township chord, or within a township, as when it is desired to find the error of the survey lines by connecting with an astronomically determined point.

$$
\begin{aligned}
\text { Let } l= & \text { length of chord, chains. } \\
c= & \text { distance along chord from either end to point at } \\
& \text { which latitude difference is required, chains. } \\
\theta= & \text { convergence of meridians per chain, seconds of } \\
& \text { arc. } \\
\mathrm{dL}= & \text { approximate distance from the parallel to the } \\
& \text { chord, in links. }
\end{aligned}
$$

then $d L=.00024(l-c) c \theta$ (approximately).
The angular difference of latitude may be obtained by use of the conversion factors given in Table I.

Table XI can be used for all systems.

## TABLE XII

## FOR CONVERTING LOGARITHMIC TANGENTS OF SMALL ARCS INTO LOGARITHMS OF SECONDS OF ARC

This gives the logarithm of the ratio of a small arc expressed in seconds of arc, to its tangent; by adding it to the log tangent, the logarithm of the arc is obtained, and the arc itself is found with a table of logarithms of numbers, without having to compute proportional parts.

## TABLE XIII

$\log \frac{1}{1-m}$ tabulated with $\log m$ as argument.
These tables are useful in abridging the work of time-azimuth observations on Polaris; they give by inspection the value of

$$
\log \frac{1}{1-\tan P \tan L \cos t}
$$

when $\log \tan P \tan L \cos t$ is known. The quantity $\tan P \tan L \cos t$ has been represented by $m$, so that the azimuth formula may be written

$$
\tan Z=-\frac{1}{1-m} \tan P \sec L \sin t
$$

It will be noted that $\log \frac{1}{1-m}$ must be taken out with regard to the sign of m .

## DEFLECTION OF A TRLAL LINE FOR DEVIATIONS FROM 1 TO 149 LINKS AT THE END OF EIGHTY-ONE CHAINS

This is useful in deflecting trial lines. It gives the angular deflection of a line for deviations of 1 to 149 links at the end of eightyone chains

## TABLE XV

## CORRECTIONS IN LINKS TO SLOPE MEASUREMENTS

This table has been computed for the use of surveyors working in mountainous country where the slopes are measured with the traisit; it is not well adapted to ordinary clinometer chaining.

The table has been compiled with the correction as argument, to give an accuracy of one-tenth of a link per chain. A greater degree of accuracy may, of course, be obtained by interpolation to the measured slope, but it is seldom necessary. The corrections are given for every chain length up to nine chains, the object being to simplify the surveyor ${ }^{\dagger}$ s calculation in the field. A convenient method of using the tables is illustrated by the following example.

Required: the slope correction for 3.682 chains at $26^{\circ} 09^{\prime}$.
This slope lies between the tabulated slopes $26^{\circ} 06^{\prime}$ and $26^{\circ} 14^{\prime}$. Taking out the slope corrections for a slope of $26^{\circ} 06^{\prime}$ and the differences of the corrections for $26^{\circ} 14^{\prime}$ and $26^{\circ} 06^{\prime}$ :
Correc. for $3 \mathrm{chs} .=30.6 \mathrm{lks}$. with diff. for $8^{\prime}$ of 0.3 lks .
$" 0.6 \quad "=6.12 \mathrm{n} \quad \mathrm{n} \quad \mathrm{n} \quad \mathrm{n} \quad 0.06 \mathrm{n}$
" $\underline{0.08} "=\underline{0.82} " \quad$ " $\quad \underline{0.01}$ "
$" \overline{3.68} "=\overline{37.54} " \quad " \quad " \quad \overline{0.37} "$
Difference for $3^{\prime}$ is $3 / 8 \times 0.37=0.14 \mathrm{lks}$.
Correc. for 3.68 chs . at $26^{\circ} 09^{\prime}=37.54+0.14=37.68 \mathrm{lks}$.
$=0.377 \mathrm{chs}$.

TABLE XVI

## TABLE FOR LAYING OUT ROADS ONE CHAIN WIDE

Roads are normally posted at points of change of direction, at the intersections of the road limits, as explained in the Manual of Instructions for the Survey of Dominion Lands (Art. 157). Table XVI correlates the diagonal distance between the points of intersection of the limits of road to the angle of deflection, or change of direction. It applies to a road of constant perpendicular width of one chain but may be used for other widths by increasing or decreasing the tabulated distances in direct proportion to the width required.

TABLE XVII

## TO CONVERT TIME INTO ARC

For convenience in converting time into arc the equivalents
of hours, minutes and seconds are tabulated in degrees, minutes and seconds.

## TABLE XVIII

TO CONVERT A MEAN TIME INTERVAL TO THE EQUIVALENT SIDEREAL TIME INTERVAL

The number of minutes and seconds to be added to a mean time interval to obtain the equivalent sidereal time interval are tabulated with days, hours and minutes as arguments.

Example: To converta mean time interval of 2 days, 6 hours, 12 minutes and 20 seconds to the equivalent sidereal time interval.

|  | Days | Hours | Min's | Sec's |
| :--- | :---: | :---: | :---: | :---: |
| Mean Time Interval | 2 | 6 | 12 | 20 |
| Tabulated addition for |  |  |  |  |
| 2 days |  |  |  | 7 |

## THE ASTRONOMICAL FIELD TABLES

The Field Tables are issued in two sets, one giving data for sun observations, and the other for star observations. Both sets should be in the hands of all surveyors engaged on Canada lands surveys.

## FIELD TABLES FOR SOLAR OBSERVATIONS

The Field Tables for solar observations are issued each year. They give the sun's apparent declination at $0^{h}$ Greenwich Civil Time with its variation for one hour and the sun's apparent right ascension at Greenwich apparent noon with its variation for one hour. These data, which are taken direct from the American Ephemeris, are tabulated for every day of the year. There is also a table of mean refraction for different altitudes, with the corrections required thereto for temperature and barometric pressure.

The tables of the sun'sdeclination and of the mean refraction are required for observations on the sun for azimuth; the table of the sun's right ascension is required for obtaining the sidereal time by observing the meridian transit of the sun. For both these purposes the approximate longitude is necessary; if not known from the survey it generally can be obtained with sufficient accuracy from a large-scale map of the area.

Full instructions for solar observing with specimen observations are given in Chapter 1.

## FIELD TABLES FOR STAR OBSERVATIONS

The Field Tables for star observations are issued for short periods in different years as explained in a subsequent paragraph "Apparent Motion of Polaris'". They contain a table for finding the Pole star and the astronomical meridian, a list of "time stars", the sidereal time at noon, eastern standard time ( $75^{\circ}$ longitude) at ten day intervals, and a diagram showing the convergence of meridians per mile of longitude at latitudes from $44^{\circ}$ to $60^{\circ}$ inclusive.

Table for Finding the Pole Star and the Astronomical Meridian. - The table is entered with the sidereal time as argument. The first column gives the number of minutes to be added to or subtracted from the latitude to obtain the altitude of the star. In the second column is the argument, the local sidereal time for every ten minutes. In the other columns is the azimuth of the star forevery even degree of latitude for $44^{\circ}$ to $60^{\circ} .^{*}$ The table enables the Pole star to be readily found in day time and when it is found and observed, provides an easy means of determining its bearing. When the position of the astronomical meridian is known approximately, as is the case on most surveys, the transit can be setin the direction of the star and to the proper altitude by means of the table. When it is not known, however, the compass needle may be used, the magnetic declination being taken from the current "Magnetic Map of Canada" or from the magnetic diagram shown on many of the large-scale maps.

The method of observing and the use of the field tables are fully explained in Chapter 1.

Time Stars - The Star Field Tables give a list of time stars taken from the American Ephemeris which are suitable for Canada lands survey work. The method of observing the sidereal time of meridian transit of a star is described in Chapter 1.

Latitude and Convergence Per Mile of Longitude. - The diagram is convenient in determining the convergence for referring an observed azimuth to the meridian of the centre of the township or to any other reference meridian.

## THE APPARENT MOTION OF POLARIS

The path described by the Pole star on the celestial sphere from 1949 to 1955 is shown on the diagram "Apparent Motion of the Pole Star". It is the combined effect of precession, nutation, aberration, and proper motion. This constant variation in the position of the star produces a slow change in the azimuth from any point, irrespective of the daily variation of azimuth caused by the earth's rotation. Hence, at the same sidereal time on two successive days the star will not have quite the same azimuth. If the azimuth of the star be considered at the same sidereal time each day for a month, there will be found a change which for latitude $60^{\circ}$ may amount to $25^{\prime \prime}$, according to the month and the sidereal time chosen. Taken overa whole year this change may be anything up to $80^{\prime \prime}$ for latitude $49^{\circ}$ or up to $110^{\prime \prime}$ for latitude $60^{\circ}$ according to the sidereal time considered. Hence, if the Field Tables were made out taking a mean position of Polaris for a year, they would be subject to a maximum error of half this amount, that is, about $40^{\prime \prime}$ at latitude $49^{\circ}$ and about $55^{\prime \prime}$ at latitude $60^{\circ}$. It will be observed from the diagram * Tables are available for latitudes $62^{\circ}, 64^{\circ}$, and $66^{\circ}$ for the months of April to October inclusive, for each year.

## APPARENT MOTION OF THE POLE STAR



The mean pasition of the Pole Star adopted for each table is indicated by
FIGURE 8
(Figure 8) that the star crosses its path again and again, occupying approximately the same positions during certain periods of consecutive years. Because of this peculiarity two star tables are issued each year, one for January, February and March of one year and April, May and June of the next, the other for November and December, September and October, and July and August of three successive years. With this arrangement the maximum error at latitude $49^{\circ}$ is $24^{\prime \prime}$ and for latitude $60^{\circ}$ is $32^{\prime \prime}$.

## COMPUTATION OF THE AZIMUTH AND <br> ALTITUDE OF POLARIS

Azimuth of the Pole Star. - The azimuth is computed by the formula:

$$
\tan Z=-\frac{\tan P \sec L \sin t}{1-\tan P \tan L \cos t}
$$

Whence, $\cot Z=\sin L \cot t-\cot P \cos L \operatorname{cosec} t$ where $Z$, $P$, $L$, $t$ denote the azimuth, polar distance, latitude and hour angle respectively.

The path of Polaris is plotted for each year by its right as censiol, and declination taken directly from the American Ephemeris. In order to determine a mean position of Polaris for the period specified on each table, the path of the star must be plotted for the three successive years being used. The American Ephemeris for the third year is not available at the time the tables are being prepared. The values for Polaris for this year are extrapolated for the first day of each month from the values tabulated for the foregoing years. The results are sufficiently accurate for plotting the diagram. From the points on the path representing the limiting dates of the table to be calculated, the mean position of Polaris for the table may be obtained graphically and its right ascension and declination read directly from the borders of the diagram.

On the diagram (Figure 8) the mean position, A, of the Pole star for the table comprising the following periods:

| November, December | 1951 |
| :--- | :--- |
| September, October | 1952 |
| July, August | 1953 |

has the values

$$
\begin{aligned}
\mathrm{RA} & =1^{\mathrm{h}} 51^{\mathrm{m}} 03^{\mathrm{s}} 5 \\
\mathrm{P} & =90^{\circ}-89^{\circ} 02^{\prime} 38!^{\prime \prime} 4=0^{\circ} 57^{\prime} 21!^{\prime} 6
\end{aligned}
$$

Since the argument in the Field Tables is the sidereal time and not the hour angle, the values of $t$ to be used in the calculation for the azimuth should be the sidereal time minus the right ascension of the mean position.

Altitude of the Pole Star. - The correction to be applied to the latitude to obtain the altitude is given by
$h-L=P \cos t-1 / 2 P^{2} \sin 1^{1} \tan L \sin ^{2} t$, terms involving higher powers of $P$ being inappreciable.

Since the term containing $L$ is small and since the altitude of Polaris is required only to find the star, a value for the mean latitude ( Lm ) may be used in the calculation. The values of $P$ and $t$ are the same as those used in the corresponding calculation for azimuth.

TABLES
TABLE I

| Latitude | $\log \mathrm{N} \sin 1^{\prime \prime}$ | $\log \mathrm{P} \sin 1^{\prime \prime}$ | $\log R \sin 1{ }^{\prime \prime}$ | Chains in 1" |  | Seconds <br> in one Chain |  | English Miles in one Degree |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  | Latitude | Longitude | Latitude | Longitude | Latitude | Longi tude |
| - ' |  |  |  |  |  | " | " |  |  |
| 4200 | 0.1873775 | 0.0584510 | 0.1857461 | 1.5337 | 1.1441 | 0.6520 | 0.8741 | 69.02 | 51.48 |
| 10 | 3818 | 73144 | 7589 | 38 | 1.1411 | 20 | 0.8764 | . 02 | 51.35 |
| 20 | 3860 | 61711 | 7717 | 38 | 1.1381 | 20 | 0.8787 | . 02 | 51.21 |
| 30 | 3903 | 50212 | 7845 | 39 | 1.1351 | 20 | 0.8810 | . 02 | 51.08 |
| 40 | 3946 | 38645 | 7973 | 39 | 1.1320 | 19 | 0.8834 | . 03 | 50.94 |
| 50 | 3989 | 27010 | 8101 | 39 | 1.1290 | 19 | 0.8857 | . 03 | 50.81 |
| 4300 | 4031 | 15306 | 8230 | 40 | 1.1260 | 19 | 0.8881 | . 03 | 50.67 |
| 10 | 4074 | 0.0503534 | 8358 | 40 | 1.1229 | 19 | 0.8905 | . 03 | 50.53 |
| 20 | 4117 | 0.0491693 | 8487 | 41 | 1.1199 | 19 | 0.8930 | . 03 | 50.39 |
| 30 | 4160 | 79782 | 8615 | 41 | 1.1168 | 18 | 0.8954 | . 04 | 50.26 |
| 40 | 4203 | 67802 | 8744 | 42 | 1.1137 | 18 | 0.8979 | . 04 | 50.12 |
| 50 | 4245 | 55750 | 8872 | 42 | 1.1106 | 18 | 0.9004 | . 04 | 49.98 |
| 4400 | 4288 | 43629 | 9001 | 43 | 1.1075 | 18 | 0.9029 | . 04 | 49.84 |
| 10 | 4331 | 31437 | 9129 | 43 | 1.1044 | 18 | 0.9054 | . 04 | 49.70 |
| 20 | 4374 | 19173 | 9258 | 44 | 1.1013 | 17 | 0.9080 | . 05 | 49.56 |
| 30 | 4417 | 0.0406838 | 9387 | 44 | 1.0982 | 17 | 0.9106 | . 05 | 49.42 |
| 40 | 4460 | 0.0394430 | 9515 | 44 | 1.0951 | 17 | 0.9132 | . 05 | 49.28 |
| 50 | 4503 | 81949 | 9644 | 45 | 1.0919 | 17 | 0.9158 | . 05 | 49.14 |


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| Latitude | $\log \mathrm{N} \sin 1^{\prime \prime}$ | $\log \mathrm{P} \sin 1^{\prime \prime}$ | $\log \mathrm{R} \sin 1^{\prime \prime}$ | Chains in 1" |  | Seconds in one Chain |  | English Miles in one Degree |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  | Latitude | Longitude | Latitude | Longitude | Latitude | $\begin{gathered} \text { Longi } \\ \text { tude } \end{gathered}$ |
| 49 <br> 9 <br>  <br>  <br>  <br>  <br> 20 <br>  <br>  <br>  <br> 40 <br> 40 <br>  <br> 50 | 0.1875572 | 0.0045001 | 0.1862852 | 1.5356 | 1.0104 | 0.6512 | 0.9897 | 69.10 | 45.47 |
|  | 5615 | 30469 | 2980 | 57 | 1.0070 | 12 | 0.9930 | . 11 | 45.32 |
|  | 5657 | 15849 | 3107 | 57 | 1.0037 | 12 | 0.9964 | . 11 | 45.16 |
|  | 5699 | 0.0001143 | 3234 | 58 | 1.0003 | 11 | 0.9997 | .11 | 45.01 |
|  | 5742 | 9.9986351 | 3361 | 58 | 0.9969 | 11 | 1.0031 | . 11 | 44.86 |
|  | 5784 | 71470 | 3488 | 58 | 0.9935 | 11 | 1.0066 | . 11 | 44.71 |
| $\begin{array}{rr}50 \quad 00 \\ & 10 \\ & 20 \\ & 30 \\ 40 \\ & 50\end{array}$ | 5826 | 56501 | 3615 | 59 | 0.9900 | 11 | 1.0101 | . 12 | 44.55 |
|  | 5869 | 41444 | 3742 | 59 | 0.9866 | 11 | 1.0136 | . 12 | 44.40 |
|  | 5911 | 26296 | 3869 | 60 | 0.9832 | 10 | 1.0171 | . 12 | 44.24 |
|  | 5953 | 9.9911058 | 3995 | 60 | 0.9797 | 10 | 1.0207 | . 12 | 44.09 |
|  | 5995 | 9.9895730 | 4122 | 61 | 0.9763 | 10 | 1.0243 | . 12 | 43.93 |
|  | 6037 | 80309 | 4248 | 61 | 0.9728 | 10 | 1.0279 | . 13 | 43.78 |
| $51 \quad 00$ | 6079 | 64797 | 4374 | 62 | 0.9693 | 10 | 1.0316 | . 13 | 43.62 |
|  | 6121 | 49192 | 4500 | 62 | 0.9659 | 10 | 1.0353 | . 13 | 43.46 |
|  | 6163 | 33493 | 4625 | 63 | 0.9624 | 09 | 1.0391 | .13 | 43.31 |
|  | 6205 | 17701 | 4751 | 63 | 0.9589 | 09 | 1.0429 | . 13 | 43.15 |
|  | 6247 | 9.9801813 | 4876 | 63 | 0.9554 | 09 | 1.0467 | . 14 | 42.99 |
|  | 6289 | 9.9785830 | 5002 | 64 | 0.9519 | 09 | 1.0506 | . 14 | 42.83 |


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TABLE I - Continued.

| Latitude | $\log \mathrm{N} \sin 1{ }^{\prime \prime}$ | $\log P \sin 1^{\prime \prime}$ | $\log \mathrm{R} \sin 1^{\prime \prime}$ | Chains in 1" |  | Seconds in one Chain |  | English Miles in one Degree |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  | Latitude | Longitude | Latitude | Longitude | Latitude | Longi tude |
| 56 00 | 0.1877310 | 9.9352927 | 0.1868065 | 1.5375 | 0.8616 | 0.6504 | ${ }_{1.1607}$ | 69.19 | 38.77 |
| 10 | 7350 | 34177 | 8184 | 75 | 0.8579 | 04 | 1.1657 | . 19 | 38.60 |
| 20 | 7389 | 9.9315310 | 8304 | 76 | 0.8541 | 04 | 1.1708 | . 19 | 38.44 |
| 30 | 7429 | 9.9296324 | 8422 | 76 | 0.8504 | 04 | 1.1759 | . 19 | 38.27 |
| 40 | 7468 | 77216 | 8541 | 76 | 0.8467 | 03 | 1.1811 | . 19 | 38.10 |
| 50 | 7508 | 57987 | 8659 | 77 | 0.8429 | 03 | 1.1863 | . 20 | 37.93 |
| 5700 | 7547 | 38635 | 8777 | 77 | 0.8392 | 03 | 1.1916 | . 20 | 37.76 |
| 10 | 7586 | 9.9219158 | 8894 | 78 | 0.8354 | 03 | 1.1970 | . 20 | 37.59 |
| 20 | 7625 | 9.9199557 | 9012 | 78 | 0.8317 | 03 | 1.2024 | . 20 | 37.43 |
| 30 | 7664 | 79829 | 9129 | 78 | 0.8279 | 03 | 1.2079 | . 20 | 37.26 |
| 40 | 7703 | 59974 | 9245 | 79 | 0.8241 | 02 | 1.2134 | . 20 | 37.09 |
| 50 | 7742 | 39991 | 9361 | 79 | 0.8203 | 02 | 1.2190 | . 21 | 36.92 |
| 5800 | 7781 | 9.9119877 | 9478 | 80 | 0.8166 | 02 | 1.2247 | . 21 | 36.75 |
| 10 | 7819 | 9.9099633 | 9593 | 80 | 0.8128 | 02 | 1.2304 | . 21 | 36.57 |
| 20 | 7858 | 79257 | 9709 | 81 | 0.8090 | 02 | 1.2362 | . 21 | 36.40 |
| 30 | 7896 | 58747 | 9824 | 81 | 0.8051 | 02 | 1.2420 | . 21 | 36.23 |
| 40 | 7934 | 38102 | 0.1869938 | 81 | 0.8013 | 01 | 1.2479 | . 22 | 36.06 |
| 50 | 7972 | 9.9017321 | 0.1870053 | 82 | 0.7975 | 01 | 1.2539 | . 22 | 35.89 |


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TABLE I - Concluded.

| Latitude | $\log \mathrm{N} \sin 11$ | $\log P \sin 11$ | $\log \mathrm{R} \sin 1^{\prime \prime}$ | Chains in 1" |  | Seconds in one Chain |  | English Miles in one Degree |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  | Latitude | Longi tude | Latitude | Longitude | Latitude | Longitude |
| 6300 | 0.1878884 | 9.8449352 |  |  |  | 0.6497 | 1.4291 |  |  |
| 10 | 8919 | 9.8449352 9.8424503 | 0.1872789 2893 | 1.5391 92 | 0.6997 | 0.6497 | 1.4291 | 69.26 | 31.49 |
| 20 | 8954 | 9.8399475 | 2996 | 92 | 0.6957 0.6917 | 97 | 1.4373 1.4456 | . 26 | 31.31 31.13 |
| 30 | 8988 | 74262 | 3100 | 93 | 0.6877 | 97 | 1.4540 | . 27 | 31.13 30.95 |
| 40 | 9022 | 48866 | 3202 | 93 | 0.6837 | 97 | 1.4626 | . 27 | 30.77 |
| 50 | 9056 | 9.8323282 | 3305 | 93 | 0.6797 | 96 | 1.4712 | . 27 | 30.59 |
| 6400 | 9090 | 9.8297510 | 3407 | 94 | 0.6757 | 96 | 1.4800 | . 27 | 30.41 |
| 10 | 9124 | 71546 | 3508 | 94 | 0.6717 | 96 | 1.4888 | . 27 | 30.41 |
| 20 | 9158 | 45389 | 3609 | 94 | 0.6676 | 96 | 1.4978 | . 27 | 30.04 |
| 30 | 9191 | 9.8219035 | 3709 | 95 | 0.6636 | 96 | 1.5069 | . 28 | 30.04 29.86 |
| 40 | 9225 | 9.8192482 | 3809 | 95 | 0.6596 | 96 | 1.5162 | . 28 | 29.68 |
| 50 | 9258 | 65730 | 3909 | 95 | 0.6555 | 95 | 1.5256 | . 28 | 29.50 |
| 6500 | 9291 | 38774 | 4008 | 96 | 0.6514 | 95 |  |  |  |
| 10 | 9324 | 9.8111610 | 4107 | 96 | 0.6474 | 95 | 1.5351 1.5447 1.544 | . 28 | 29.32 29.13 |
| 20 | 9356 | 9.8084240 | 4205 | 96 | 0.6433 | 95 | 1.5447 1.5544 | . 28 | 29.13 28.95 |
| 30 | 9389 | 56659 | 4302 | 97 | 0.6392 | 95 | 1.5644 | . 29 | 28.77 |
| 40 | 9421 | 28862 | 4399 | 97 | 0.6352 | 95 | 1.5744 | . 29 | 28.58 |
| 50 | 9453 | 9.8000850 | 4496 | 97 | 0.6311 | 95 | 1.5846 | . 29 | 28.40 |


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TABLE II

| Latitude | $d\left(\log R \sin 1^{\prime \prime}\right)$ | $d\left(\log N \sin 1^{\prime \prime}\right)$ | Latitude | $d\left(\log R \sin 1{ }^{\prime \prime}\right)$ | $d\left(\log N \sin 1^{\prime \prime}\right)$ |
| :---: | :---: | :---: | :---: | :---: | :---: |
| - |  |  | - |  |  |
| 42. | -0.0000021 | +0.0000063 | 56....... | +0.0000034 | +0.0000081 |
| 43. | 17 | 64 | 57. | 37 | 82 |
| 44. | 13 | 66 | 58. | 41 | 84 |
| 45. | 09 | 67 | 59. | 45 | 85 |
| 46. | $05$ | 68 | 60. | 48 | 86 |
| 47. | -0.0000001 | 70 | 61...... | 51 | 87 |
| 48. | +0.0000003 | 71 | 62..... | 55 | 88 |
| 49. . | 07 | 72 | 63. | 58 | 89 |
| 50. | 11 | 74 | 64....... | 61 | 90 |
| 51. | 15 | 75 | 65. | 64 | 91 |
| 52. | 19 | 76 | 66. | 67 | 92 |
| 53. | 23 | 77 | 67........ | 70 | 93 |
| 54. | 26 | 79 | 68....... | 73 | 94 |
|  | 30 | 80 | 69........ | 76 | 95 |
| 55......... |  |  | 70....... | 78 | 96 |

TABLE III

| No. of Township | Number of Line | Latitude |  |  | Log $\mathrm{N} \sin 1^{\prime \prime}$ | Log P sin 1" | Log R sin 1" |  | ngitude ered by westing |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | - | 1 | " |  |  |  | - | " |
| 0 | 1st Base. | 49 | 00 | 00.00 | 0.1875572 | 0.0045001 | 0.1862852 | 8 | 03.959 |
| 2 | 1st Correction. |  | 10 | 36.86 | 5618 | 0.0029573 | 2988 |  | 05.681 |
| 4 | 2nd Base. |  | 21 | 13.71 | 5662 | 0.0014047 | 3123 |  | 07.421 |
| 6 | 2nd Correction |  | 31 | 50.53 | 5707 | 9.9998425 | 3258 |  | 09.177 |
| 8 | 3rd Base. |  | 42 | 27.34 | 5752 | 9.9982704 | 3393 |  | 10.951 |
| 10 | 3rd Correction. |  | 53 | 04.12 | 5797 | 9.9966886 | 3527 |  | 12.743 |
| 12 | 4th Base. | 50 | 03 | 40.89 | 5842 | 9.9950968 | 3662 |  | 14.552 |
| 14 | 4th Correction |  | 14 | 17.63 | 5887 | 9.9934951 | 3797 |  | 16.379 |
| 16 | 5th Base |  | 24 | 54.36 | 5932 | 9.9918832 | 3931 |  | 18.225 |
| 18 | 5th Correction |  | 35 | 31.07 | 5976 | 9.9902611 | 4065 |  | 20.089 |
| 20 | 6th Base |  | 46 | 07.75 | 6021 | 9.9886289 | 4199 |  | 21.972 |
| 22 | 6th Correction |  | 56 | 44.42 | 6065 | 9.9869863 | 4333 |  | 23.875 |
| 24 | 7th Base | 51 | 07 | 21.07 | 6110 | 9.9853334 | 4466 |  | 25.796 |
| 26 | 7th Correction |  | 17 | 57.69 | 6154 | 9.9836701 | 4600 |  | 27.737 |
| 28 | 8th Base |  | 28 | 34.30 | 6199 | 9.9819962 | 4733 |  | 29.698 |
| 30 | 8th Correctio |  | 39 | 10.89 | 6244 | 9.9803117 | 4866 |  | 31.678 |
| 32 | 9 th Base |  | 49 | 47.46 | 6288 | 9.9786165 | 4999 |  | 33.680 |
| 34 | 9th Correction. | 52 | 00 | 24.01 | 6332 | 9.9769105 | 5132 |  | 35.701 |
| 36 | 10th Base |  | 11 | 00.54 | 6376 | 9.9751935 | 5264 |  | 37.744 |
| 38 | 10th Correction. |  | 21 | 37.05 | 6420 | 9.9734658 | 5396 |  | 39.808 |
| 40 | 11th Base |  | 32 | 13.54 | 6464 | 9.9717268 | 5529 |  | 41.894 |
| 42 | 11th Correction. |  | 42 | 50.02 | 6508 | 9.9699768 | 5660 |  | 44.001 |
| 44 | 12th Base. |  | 53 | 26.47 | 6552 | 9.9682156 | 5792 |  | 46.130 |
| 46 | 12th Correction. | 53 | 04 | 02.90 | 6596 | 9.9664431 | 5923 |  | 48.282 |
| 48 | 13th Base |  | 14 | 39.32 | 0.1876640 | 9.9646592 | 0.1866055 | 8 | 50.456 |

TABLE IV
Latitudes, etc., of Base and Correction Lines. Third System of Survey.

| No. of Township | Number of Line | Latitude |  |  | Log $\mathrm{N} \sin 1^{\prime \prime}$ | Log P $\sin 1^{\prime \prime}$ | Log R sin $1^{\prime \prime}$ | Lo cov 486 of | gitude ered by chains |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | 49 | 00 | "'00 |  |  |  | ' | " |
| 0 | 1st Base. | 49 | 00 | 00.00 | 0.1875572 | 0.0045001 | 0.1862852 | 8 | 00.990 |
| 2 | 1 st Correction. |  | 10 | 29.05 | 5617 | 0.0029763 | 2986 |  | 02.681 |
| 4 | 2nd Base. |  | 20 | 58.08 | 5661 | 0.0014430 | 3119 |  | 04.388 |
| 6 | 2nd Correction |  | 31 | 27.09 | 5706 | 9.9999002 | 3253 |  | 06.112 |
| 8 | 3rd Base. |  | 41 | 56.08 | 5750 | 9.9983479 | 3386 |  | 07.852 |
| 10 | 3rd Correction |  | 52 | 25.06 | 5795 | 9.9967860 | 3519 |  | 09.610 |
| 12 | 4th Base | 50 | 02 | 54.01 | 5839 | 9.9952144 | 3652 |  | 11.385 |
| 14 | 4th Correction |  | 13 | 22.95 | 5883 | 9.9936331 | 3785 |  | 13.178 |
| 16 | 5th Base. |  | 23 | 51.86 | 5927 | 9.9920419 | 3918 |  | 14.988 |
| 18 | 5th Correction. |  | 34 | 20.76 | 5971 | 9.9904408 | 4050 |  | 16.816 |
| 20 | 6th Base. . |  | 44 | 49.63 | 6016 | 9.9888298 | 4182 |  | 18.662 |
| 22 | 6th Correction. |  | 55 | 18.49 | 6060 | 9.9872087 | 4315 |  | 20.527 |
| 24 | 7 th Base | 51 | 05 | 47.33 | 6104 | 9.9855775 | 4447 |  | 22.411 |
| 26 | 7th Correction. |  | 16 | 16.15 | 6148 | 9.9839362 | 4578 |  | 24.313 |
| 28 | 8th Base |  | 26 | 44.95 | 6192 | 9.9822845 | 4710 |  | 26.235 |
| 30 | 8th Correction. |  | 37 | 13.73 | 6235 | 9.9806225 | 4842 |  | 28.176 |
| 32 | 9 th Base. |  | 47 | 42.49 | 6279 | 9.9789501 | 4973 |  | 30.136 |
| 34 | 9th Correction. |  | 58 | 11.24 | 6323 | 9.9772672 | 5104 |  | 32.117 |
| 36 | 10th Base... | 52 |  | 39.96 | 6366 | 9.9755738 | 5235 |  | 34.118 |
| 38 | 10th Correction. |  | 19 | 08.66 | 6410 | 9.9738696 | 5366 |  | 36.139 |


|  | $\begin{array}{llll} \sharp \sim & m & N & \text { nn } \\ \sim \end{array}$ |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \circ \text { o } 0 \infty \\ & \text { N in } \\ & \text { in o } \hat{\sim} \text { in in } \end{aligned}$ |  | $\begin{aligned} & \infty \\ & \infty \underset{\sim}{\infty} \underset{\sim}{\circ} \stackrel{\infty}{\sim} \stackrel{\sim}{\sim} \end{aligned}$ | $\begin{aligned} & \text { n } N \infty \text { が } \\ & \sim \\ & \sim \\ & \sim \\ & \sim \end{aligned}$ |  |
|  |  |  |  |  <br>  <br>  <br> m m m N N <br> ののののの <br> かのかの |
|  |  |  |  |  |
|  |  |  |  |  |
| $\begin{gathered} \text { No 웅 } \\ \text { in } \\ \text { n } \end{gathered}$ | $\underset{\sim}{N} \tilde{m} \underset{\sim}{n} \text { in } \hat{0}$ | $\pm \begin{gathered}\text { din } \\ \text { ¢ }\end{gathered}$ | $\begin{aligned} & \circ \sim \sim \infty \times \infty \\ & \text { in } \\ & \text { in } \end{aligned}$ |  |
|  |  |  | $\begin{array}{r:c} \vdots & \vdots \end{array}: \vdots$ |  |
|  |  | $0 \sim \overrightarrow{0} 0 \infty$ | ○NホN | $0_{\infty}^{\sim} \sim_{\infty} \infty_{\infty}^{\infty}$ |

TABLE IV - Continued

| No. of Township | Number of Line | Latitude |  |  | Log $\mathrm{N} \sin 1^{\prime \prime}$ | Log $P \sin 1^{\prime \prime}$ | Log R sin $1^{\prime \prime}$ | Lon cov 486 of | gitude red by chains sting |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | - | ' | " |  |  |  | 1 | " |
| 90 | 23rd Correction | 56 | 51 | 28.47 | 0.1877514 | 9.9255142 | 0.1868676 | 9 | 36.929 |
| 92 | 24th Base. | 57 | 01 | 56.68 | 7555 | 9.9234857 | 8800 |  | 39.630 |
| 94 | 24th Correction. . |  | 12 | 24.87 | 7596 | 9.9214437 | 8923 |  | 42.362 |
| 96 | 25th Base |  | 22 | 53.04 | 7637 | 9.9193880 | 9045 |  | 45.125 |
| 98 | 25th Correction. |  | 33 | 21.19 | 7677 | 9.9173186 | 9168 |  | 47.919 |
| 100 | 26th Base. |  | 43 | 49.33 | 7718 | 9.9152352 | 9290 |  | 50.747 |
| 102 | 26th Correction. |  | 54 | 17.45 | 7759 | 9.9131377 | 9411 |  | 53.607 |
| 104 | 27th Base.... | 58 | 04 | 45.55 | 7799 | 9.9110260 | 9533 |  | 56.500 |
| 106 | 27th Correction. |  | 15 | 13.63 | 7839 | 9.9088999 | 9654 | 9 | 59.427 |
| 108 | 28th Base. |  |  | 41.70 | 7879 | 9.9067593 | 9774 | 10 | 02.389 |
| 110 | 28th Correction. |  | 36 | 09.75 | 7920 | 9.9046041 | 0.1869894 |  | 05.386 |
| 112 | 29th Base.... . . |  | 46 | 37.78 | 7960 | 9.9024341 | 0.1870014 |  | 08.418 |
| 114 | 29th Correction. |  | 57 | 05.80 | 7999 | 9.9002491 | 0134 |  | 11.487 |
| 116 | 30th Base. | 59 |  | 33.79 | 8039 | 9.8980491 | 0253 |  | 14.592 |
| 118 | 30th Correction. |  |  | 01.77 | 8079 | 9.8958338 | 0371 |  | 17.735 |
| 120 | 31st Base..... |  | 28 | 29.74 | 8118 | 9.8936032 | 0489 |  | 20.916 |
| 122 | 31 st Correction. |  | 38 | 57.68 | 8157 | 9.8913570 | 0607 |  | 24.136 |
| 124 | 32nd Base. |  | 49 | 25.61 | 8196 | 9.8890950 | 0725 |  | 27.395 |
| 126 | 32nd Correction |  | 59 | 53.52 | 3235 | 9.8868172 | 0842 |  | 30.695 |
| 128 | 33rd Base. | 60 | 10 | 21.42 | 8274 | 9.8845233 | 0959 |  | 34.035 |





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TABLE IV - Concluded.

| No. of Township | Number of Line | Latitude |  |  | $\log \mathrm{N} \sin 1^{\prime \prime}$ | Log $P \sin 1^{\prime \prime}$ | Log $\mathrm{R} \sin 1^{\prime \prime}$ |  | gitude ered by chains westing |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | - | , | " |  |  |  |  |  |
| 180 | 46th Base | 64 | 42 | 20.98 | 0.1879233 | 9.8186215 | 0.1873833 | 12 | 17.929 |
| 182 | 46th Correction. |  | 52 | 48.45 | 9267 | 9.8158183 | 3937 |  | 22.708 |
| 184 | 47th Base | 65 | 03 | 15.90 | 9302 | 9.8129928 | 4040 |  | 27.556 |
| 186 | 47th Correction. |  | 13 | 43.33 | 9336 | 9.8101448 | 4143 |  | 32.474 |
| 188 | 48th Base |  | 24 | 10.75 | 9370 | 9.8072739 | 4245 |  | 37.465 |
| 190 | 48th Correction. . . . . |  | 34 | 38.16 | 9404 | 9.8043799 | 4347 |  | 42.529 |
| 192 | 49th Base. . . . . . . . . |  | 45 | 05.55 | 9437 | 9.8014625 | 4448 |  | 47.668 |
| 194 | 49th Correction. |  | 55 | 32.92 | 9471 | 9.7985213 | 4549 |  | 52.885 |
| 196 | 50th Base. | 66 | 06 | 00.28 | 9504 | 9.7955559 | 4649 | 12 | 58.180 |
| 198 | 50 th Correction |  | 16 | 27.63 | 9538 | 9.7925661 | 4749 | 13 | 03.556 |
| 200 | 51 st Base |  | 26 | 54.96 | 9571 | 9.7895516 | 4848 |  | 09.014 |
| 202 | 51 st Correction. |  | 37 | 22.28 | 9604 | 9.7865120 | 4947 |  | 14.556 |
| 204 | 52nd Base |  | 47 | 49.58 | 9636 | 9.7834470 | 5045 |  | 20.183 |
| 206 | 52nd Correction |  | 58 | 16.87 | 9669 | 9.7803561 | 5142 |  | 25.898 |
| 208 | 53 rd Base. | 67 | 08 | 44.14 | 9701 | 9.7772390 | 5239 |  | 31.703 |
| 210 | 53 rd Correction |  | 19 | 11.40 | 9733 | 9.7740953 | 5335 |  | 37.600 |
| 212 | 54th Base |  | 29 | 38.65 | 9765 | 9.7709246 | 5431 |  | 43.591 |
| 214 | 54th Correction. |  | 40 | 05.89 | 9797 | 9.7677266 | 5526 |  | 49.678 |
| 216 | 55th Base.......... |  | 50 | 33.11 | 9329 | 9.7645009 | 5620 | 13 | 55.864 |
| 218 | 55th Correction. . . . . | 68 | 01 | 00.31 | 9860 | 9.7612470 | 5714 | 14 | 02.150 |



TABLE V
Chord Azimuths, Deflections, Deflection Offsets, etc., for Base Lines.
First and Second Systems of Survey.

| No. of Base Line | Chord Azimuth |  | Convergence for 100 Chains | Deflec tion | Deflection Offset For one Chain Distance | Longitude covered by one Range | No. of Township |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | - ' | " | " | ' ${ }^{\prime}$ | inches | s. |  |
| 1 | 8956 | 57.4 | 74.69 | 605.2 | 1.402 | 32.3 | 0 |
| 2 |  | 55.1 | 75.63 | 09.8 | 1.420 | 32.5 | 4 |
| 3 |  | 52.8 | 76.58 | 14.5 | 1.438 | 32.7 | 8 |
| 4 |  | 50.4 | 77.54 | 19.2 | 1.456 | 33.0 | 12 |
| 5 |  | 48.0 | 78.52 | 24.0 | 1.474 | 33.2 | 16 |
| 6 |  | 45.6 | 79.51 | 28.8 | 1.493 | 33.5 | 20 |
| 7 |  | 43.1 | 80.52 | 33.8 | 1.512 | 33.7 | 24 |
| 8 |  | 40.6 | 81.55 | 38.8 | 1.531 | 34.0 | 28 |
| 9 |  | 38.1 | 82.58 | 43.8 | 1.551 | 34.2 | 32 |
| 10 |  | 35.5 | 83.64 | 49.0 | 1.570 | 34.5 | 36 |
| 11 |  | 32.9 | 84.71 | 54.3 | 1.591 | 34.8 | 40 |
| 12 |  | 30.2 | 85.80 | 59.6 | 1.611 | 35.1 | 44 |
| 13 |  | 27.5 | 86.91 | 705.0 | 1.632 | 35.4 | 48 |

## TABLE VI

Chord Azimuths, Deflections, Deflection Offsets, etc., for Base Lines.
Third System of Survey


TABLE VI - Concluded
Chord Azimuths, Deflections, Deflection Offsets, etc., for Base Lines.
Third System of Survey.

| No. of Base Line | Chord <br> Azimuth |  | Convergence for 100 Chains | Defi | flec ion | Deflection Offset For one Chain Distance | Longitude covered by one Range | No, of Township |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | - ' | " | " | ' | " | inches | s. |  |
| 36 | 8955 | 13.0 | 118.12 | 9 | 34.1 | 2.204 | 43.7 | 140 |
| 37 |  | 08.8 | 119.84 |  | 42.4 | 2.236 | 44.2 | 144 |
| 38 |  | 04.5 | 121.60 |  | 51.0 | 2.269 | 44.7 | 148 |
| 39 |  | 00.1 | 123.40 |  | 59.7 | 2.303 | 45.2 | 152 |
| 40 | 8954 | 55.7 | 125.24 |  | 08.7 | 2.337 | 45.7 | 156 |
| 41 |  | 51.1 | 127.13 |  | 17.8 | 2.372 | 46.2 | 160 |
| 42 |  | 46.4 | 129.06 |  | 27.2 | 2.408 | 46.8 | 164 |
| 43 |  | 41.6 | 131.04 |  | 36.9 | 2.445 | 47.4 | 168 |
| 44 |  | 36.6 | 133.07 |  | 46.7 | 2.483 | 48.0 | 172 |
| 45 |  | 31.6 | 135.15 |  | 56.8 | 2.522 | 48.6 | 176 |
| 46 |  | 26.4 | 137.28 |  | 07.2 | 2.562 | 49.2 | 180 |
| 47 |  | 21.1 | 139.47 |  | 17.8 | 2.603 | 49.8 | 184 |
| 48 |  | 15.6 | 141.71 |  | 28.7 | 2.645 | 50.5 | 188 |
| 49 |  | 10.0 | 144.02 |  | 39.9 | 2.688 | 51.2 | 192 |
| 50 |  | 04.3 | 146.39 |  | 51.5 | 2.732 | 51.9 | 198 |
| 51 | 8953 | 58.4 | 148.82 | 12 | 03.3 | 2.777 | 52.6 | 200 |
| 52 |  | 52.3 | 151.33 |  | 15.5 | 2.824 | 53.3 | 204 |
| 53 |  | 46.0 | 153.90 |  | 28.0 | 2.872 | 54.1 | 208 |
| 54 |  | 39.6 | 156.56 |  | 40.9 | 2.921 | 54.9 | 212 |
| 55 |  | 32.9 | 159.29 |  | 54.1 | 2.972 | 55.7 | 216 |
| 56 |  | 26.1 | 162.10 | 13 | 07.8 | 3.025 | 56.6 | 220 |
| 57 |  | 19.1 | 165.00 |  | 21.9 | 3.079 | 57.4 | 224 |
| 58 |  | 11.8 | 167.99 |  | 36.4 | 3.135 | 58.3 | 228 |
| 59 |  | 04.3 | 171.08 |  | 51.4 | 3.192 | 59.3 | 232 |
| 60 | 8952 | 56.5 | 174.26 |  | 06.9 | 3.252 | 60.2 | 236 |
| 61 |  | 48.5 | 177.56 |  | 22.9 | 3.313 | 61.2 | 240 |
| 62 |  | 40.3 | 180.96 |  | 39.5 | 3.377 | 62.3 | 244 |

TABLE VII
Chord Azimuths, Deflections, Deflection Offsets, Jogs, etc., for Correction Lines.

|  | Chord Azimuth | Convergence for 100 Chains on South side of Road | Deflec tion | Deflection Offset for one Chain Distance | Length of one Range on Correction Line |  | Jog for <br> 1 Range |  | No. of Township |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  | North side of Road | South side of Road |  |  |  |
|  | - 11 | 11 | ' 11 | inches | chains | chains | chains | links |  |
| 1 | 895656.9 | 75.16 | 606.2 | 1.406 | 490.751 | 487.266 | 3.485 | 14.5 | 2 |
| 2 | 84.6 56.9 | 76.10 | 10.8 | 1.424 | . 773 | . 244 | . 529 | 14.7 | 6 |
| 3 | 52.3 | 77.06 | 15.5 | 1.442 | . 796 | . 222 | . 574 | 14.9 | 10 |
| 4 | 49.9 | 78.03 | 20.2 | 1.460 | . 818 | . 200 | . 618 | 15.1 | 14 |
| 5 | 47.5 | 79.02 | 25.0 | 1.478 | . 841 | . 177 | . 664 | 15.3 | 18 |
| 6 | 45.1 | 80.02 | 29.8 | 1.497 | . 865 | . 154 | . 711 | 15.5 | 22 |
| 7 | 42.7 | 81.03 | 34.7 | 1.516 | . 888 | . 131 | . 758 | 15.7 | 26 |
| 8 | 40.2 | 82.06 | 39.7 | 1.535 | . 913 | . 107 | . 806 | 15.9 | 30 |
| 9 | 37.6 | 83.11 | 44.8 | 1.554 | . 937 | . 083 | .r. 54 | 16.1 | 34 |
| 10 | 35.0 | 84.17 | 50.0 | 1.574 | . 962 | . 058 | . 904 | 16.3 | 38 |
|  | 32.4 | 85.26 | 55.2 | 1.594 | 490.987 | . 034 | 3.953 | 16.5 | 42 |
| 12 | 32.4 29.7 | 86.36 | 700.6 | 1.615 | 491.012 | 487.008 | 4.004 | 16.7 | 46 |

TABLE VIII
Chord Azimuths, Deflections, Deflection Offsets, Jogs, etc., for Correction Lines.

|  | Chord <br> Azimuth | Convergence for 100 Chains on South side of Road | $\begin{gathered} \text { Deflec - } \\ \text { tion } \end{gathered}$ | Deflection Offset for one Chain Distance | Length of one Range on Correction Line |  | Jog for1 Range |  | No. of Township |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  | North side of Road | South side of Road |  |  |  |
|  | - 1 1 | " | ' $"$ | inches | chains | chains | chains | links |  |
| 1 | 895658.0 | 75:15 | 604.0 | 1.398 | 487.719 | 484.298 | 3.421 | 14.3 | 2 |
| 2 | 55.7 | 76.08 | 08.5 | 1.415 | . 740 | . 277 | . 463 | 14.4 | 6 |
| 3 | 53.5 | 77.03 | 13.0 | 1.432 | . 762 | . 255 | . 507 | 14.6 | 10 |
| 4 | 51.2 | 77.99 | 17.6 | 1.450 | . 784 | . 234 | . 550 | 14.8 | 14 |
| 5 | 48.8 | 78.96 | 22.3 | 1.468 | . 806 | . 212 | . 594 | 15.0 | 18 |
| 6 | 46.4 | 79.95 | 27.1 | 1.486 | . 829 | . 189 | . 640 | 15.2 | 22 |
| 7 | 44.0 | 80.95 | 31.9 | 1.505 | . 852 | . 167 | . 685 | 15.4 | 26 |
| 8 | 41.6 | 81.97 | 36.8 | 1.524 | . 875 | . 144 | . 731 | 15.5 | 30 |
| 9 | 39.1 | 83.00 | 41.8 | 1.543 | . 899 | . 120 | . 779 | 15.7 | 34 |
| 10 | 36.5 | 84.05 | 46.9 | 1.562 | . 923 | . 097 | . 826 | 15.9 | 38 |
| 11 | 34.0 | 85.12 | 52.0 | 1.582 | . 947 | . 072 | . 875 | 16.1 | 42 |
| 12 | 31.4 | 86.20 | 57.2 | 1.602 | . 972 | . 048 | . 924 | 16.4 | 46 |
| 13 | 28.7 | 87.30 | 702.6 | 1.622 | 487.997 | 484.023 | 3.974 | 16.6 | 50 |
| 14 | 26.0 | 88.42 | 07.9 | 1.643 | 488.023 | 483.998 | 4.025 | 16.8 | 54 |
| 15 | 23.3 | 89.56 | 13.4 | 1.664 | . 049 | . 972 | . 077 | 17.0 | 58 |



TABLE VIII - Concluded
Chord Azimuths, Deflections, Deflection Offsets, Jogs, etc., for Correction Lines.

|  | Chord Azimuth | Convergence for 100 Chains on South side of Road | Deflection | Deflection Offset for one Chain Distance | Length of one Range on Correction Line |  | Jog for 1 Range |  | No. of Township |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  | North side of Road | South side of Road |  |  |  |
|  | ' | " | $1{ }^{\prime}$ | inches | chains | chains | chains | links |  |
| 41 | 895450.6 | 128.09 | 1018.8 | 2.376 | 488.935 | 483.104 | 5.831 | 24.3 | 162 |
| 42 | 45.9 | 130.04 | 28.2 | 2.412 | 488.980 | . 060 | 5.920 | 24.7 | 166 |
| 43 | 41.1 | 132.05 | 37.8 | 2.449 | 489.026 | 483.015 | 6.011 | 25.0 | 170 |
| 44 | 36.1 | 134.10 | 47.7 | 2.487 | . 074 | 482.969 | . 105 | 25.4 | 174 |
| 45 | 31.1 | 136.21 | 57.8 | 2.526 | . 122 | . 922 | . 201 | 25.8 | 178 |
| 46 | 25.9 | 138.37 | 1108.1 | 2.565 | . 172 | . 873 | . 299 | 26.2 | 182 |
| 47 | 20.6 | 140.58 | 18.8 | 2.606 | . 223 | . 823 | . 400 | 26.7 | 186 |
| 48 | 15.1 | 142.86 | 29.7 | 2.648 | . 276 | . 772 | . 503 | 27.1 | 190 |
| 49 | 09.5 | 145.19 | 40.9 | 2.691 | . 330 | . 720 | . 610 | 27.5 | 194 |
| 50 | 03.8 | 147.60 | 52.4 | 2.735 | . 385 | . 666 | . 719 | 28.0 | 198 |
| 51 | 895357.9 | 150.07 | 1204.2 | 2.781 | . 442 | . 610 | . 832 | 28.5 | 202 |
| 52 | 51.8 | 152.61 | 16.4 | 2.828 | . 501 | . 553 | 6.947 | 28.9 | 206 |
| 53 | 45.5 | 155.22 | 28.9 | 2.876 | . 561 | . 495 | 7.066 | 29.4 | 210 |
| 54 | 39.1 | 157.91 | 41.8 | 2.925 | . 623 | . 434 | . 189 | 30.0 | 214 |
| 55 | 32.4 | 160.68 | 55.1 | 2.976 | . 687 | . 372 | . 315 | 30.5 | 218 |


| $\begin{array}{lllll} N & 0 & 0 & H & \infty \\ N & N & M & N & N \end{array}$ | $\underset{\sim}{N}$ |
| :---: | :---: |
| $\begin{array}{ll} 0 & N \\ \dot{m} & n \\ m & \dot{N} \\ m \end{array}$ | $\stackrel{o}{m}$ |
|  | $\begin{aligned} & 0 \\ & \underline{0} \\ & \cdots \end{aligned}$ |
|  | $\begin{aligned} & 0 \\ & n \\ & \dot{0} \\ & \stackrel{1}{\infty} \\ & \underset{i}{n} \end{aligned}$ |
| mーNはの $\begin{array}{llll}n & n & 0 & 0 \\ \sim & \infty & \infty & 0\end{array}$ $\therefore \infty \infty$ $\begin{array}{ll} 0 & 0 \\ \alpha & 0 \\ \infty & 0 \\ q_{1} & \dot{4} \end{array}$ | $\begin{aligned} & \text { I } \\ & \underset{0}{2} \\ & \text { g } \end{aligned}$ |
| $\begin{array}{ccccc} a & m & \infty & 0 & 0 \\ N & \infty & m & 0 & n \\ 0 & 0 & \ddots & \ddots & n \\ \dot{m} & \dot{m} & \dot{m} & \dot{m} & \dot{m} \end{array}$ | $\stackrel{N}{m}$ |
| $\infty \infty$ サれの $\infty$ N～NN $0 \sim m$ in 0 $9$ | $\begin{aligned} & a \\ & \underset{N}{n} \end{aligned}$ |
| $\begin{array}{llll} \dot{r} & \infty & n & 0 \\ 0 \\ n & 0 & 0 & 0 \\ \dot{m} & 0 & \dot{j} & \dot{N} \\ \underline{0} & i n \\ -1 & -1 & n \end{array}$ | $\begin{aligned} & \dot{N} \\ & \text { N } \\ & \dot{N} \end{aligned}$ |
| $\begin{array}{lll} 0 & 0 & \infty \\ i n \\ n & 0 \\ n & m & 0 \\ 0 & n \end{array}$ | $\begin{aligned} & 0 \\ & \infty \\ & +1 \end{aligned}$ |
| $0 \sim \infty$ o o in in in in 0 | $\overrightarrow{0}$ |

TABLE IX

Latitude, with Logarithms of Secant and Tangent for the North Boundary of each Section, and the widths of Quarter

Sections on such Boundaries.
First and Second Systems of Survey.

| Township | Section | Latitude L. | Log Sec L. | Log Tan L. | Quarter |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 36 |  | 0.18306 | 0.06084 | $\begin{aligned} & \text { chains } \\ & 40.000 \end{aligned}$ |
|  | 1 | $00 \quad 53.07$ | 19 | 0.06106 | 39.988 |
|  | 12 | 0146.14 | 31 | 29 | . 976 |
|  | 13 | $\begin{array}{lll}02 & 39.22\end{array}$ | 44 | 51 | . 964 |
|  | 24 | $03 \quad 32.29$ | 57 | 74 | . 952 |
|  | 25 | $04 \quad 25.36$ | 70 | 97 | . 940 |
|  | 36 | $05 \quad 18.43$ | 83 | 0.06219 | . 928 |
| 2 | 1 | 0611.51 | 96 | 42 | . 916 |
|  | 12 | 0704.58 | 0.18409 | 64 | . 904 |
|  | 13 | 0757.65 | 22 | 87 | . 892 |
|  | 24 | 0850.72 | 35 | $0.063 \quad 09$ | . 880 |
|  | 25 | 0943.79 | 48 | 32 | . 868 |
|  | 36 | $10 \quad 36.86$ | 60 | 55 | (39.856 |
|  |  |  |  |  | (40.146 |
| 3 | 1 | $11 \quad 29.93$ | 73 | 77 | . 134 |
|  | 12 | $12 \quad 23.01$ | 86 | 0.06400 | . 122 |
|  | 13 | 1316.08 | 99 | 22 | .109 |
|  | 24 | $14 \quad 09.15$ | $0.185 \quad 12$ | 45 | . 097 |
|  | 25 | 1502.22 | 25 | 68 | . 085 |
|  | 36 | $15 \quad 55.29$ | 38 | 90 | . 073 |
| 4 | 1 | $16 \quad 48.36$ | 51 | $0.065 \quad 13$ | . 061 |
|  | 12 | 1741.43 | 64 | 35 | . 049 |
|  | 13 | $18 \quad 34.50$ | 77 | 58 | . 036 |
|  | 24 | $19 \quad 27.57$ | 90 | 81 | . 024 |
|  | 25 | $20 \quad 20.64$ | 0.18603 | 0.06603 | . 012 |
|  | 36 | $21 \quad 13.71$ | 16 | 26 | 40.000 |
| 5 | 1 | $22 \quad 06.78$ | 29 | 48 | 39.988 |
|  | 12 | $22 \quad 59.85$ | 42 | 71 | . 976 |
|  | 13 | $23 \quad 52.92$ | 55 | 94 | .963 |
|  | 24 | $24 \quad 45.98$ | 68 | 0.06716 | . 951 |
|  | 25 | $25 \quad 39.05$ | 81 | 39 | . 939 |
|  | 36 | $26 \quad 32.12$ | 94 | 61 | . 927 |
| 6 | 1 | $27 \quad 25.19$ | $0.187 \quad 07$ | 84 | 39.915 |
|  | 12 | $28 \quad 18.26$ | 21 | $0.068 \quad 07$ | . 902 |
|  | 13 | 2911.33 | 34 | 29 | . 890 |
|  | 24 | $30 \quad 04.40$ | 47 | 52 | . 878 |
|  | 25 | $30 \quad 57.46$ | 60 | 75 | . 866 |
|  | 36 | $31 \quad 50.53$ | 73 | 97 | (39.854 |
|  |  |  |  |  | (40.148 |

TABLE IX - Continued

Latitude, with Logarithms of Secant and Tangent, etc.
First and Second Systems of Survey.

| Township | Section | Latitude L. | Log Sec L. | Log Tan L. | Quarter Section |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 7 |  | - 1" |  |  | chains |
|  | 1 | $\begin{array}{lll}49 & 32 & 43.60\end{array}$ | 0.18786 | 0.06920 | .135 |
|  | 12 | $\begin{array}{lll}33 & 36.67\end{array}$ | 99 | 42 | . 123 |
|  | 13 | $34 \quad 29.74$ | 0.18812 | 65 | . 111 |
|  | 24 | $\begin{array}{lll}35 & 22.80\end{array}$ | 25 | 88 | . 099 |
|  | 25 | $\begin{array}{lll}36 & 15.87\end{array}$ | 38 | $0.070 \quad 10$ | . 086 |
|  | 36 | $\begin{array}{lll}37 & 08.94\end{array}$ | 52 | 33 | . 074 |
| 8 | 1 | $38 \quad 02.00$ | 65 | 56 | . 062 |
|  | 12 | $\begin{array}{lll}38 & 55.07\end{array}$ | 78 | 78 | . 049 |
|  | 13 | $39 \quad 48.14$ | 91 | 0.07101 | . 037 |
|  | 24 | $40 \quad 41.20$ | 0.18904 | 24 | . 025 |
|  | 25 | $41 \quad 34.27$ | 17 | 46 | . 012 |
|  | 36 | $42 \quad 27.34$ | 30 | 69 | 40.000 |
| 9 | 1 | $43 \quad 20.40$ | 44 | 92 | 39.988 |
|  | 12 | $\begin{array}{ll}44 & 13.47\end{array}$ | 57 | $0.072 \quad 14$ | . 975 |
|  | 13 | $45 \quad 06.54$ | 70 | 37 | . 963 |
|  | 24 | $45 \quad 59.60$ | 83 | 60 | . 951 |
|  | 25 | $46 \quad 52.67$ | 96 | 82 | . 938 |
|  | 36 | $47 \quad 45.73$ | $0.190 \quad 10$ | 0.07305 | . 926 |
| 10 | 1 | $\begin{array}{lll}48 & 38.80\end{array}$ | 23 | 28 | . 914 |
|  | 12 | $49 \quad 31.86$ | 36 | 50 | . 901 |
|  | 13 | $50 \quad 24.93$ | 49 | 73 | . 889 |
|  | 24 | $\begin{array}{lll}51 & 17.99\end{array}$ | 63 | 96 | . 877 |
|  | 25 | $52 \quad 11.06$ | 76 | 0.07418 | . 864 |
|  | 36 | 5304.12 | 89 | 41 | $\left\{\begin{array}{l}39.852 \\ 40.150\end{array}\right.$ |
| 11 | 1 | $53 \quad 57.19$ | 0.19102 | 64 | . 137 |
|  | 12 | $54 \quad 50.25$ | 16 | 86 | . 125 |
|  | 13 | $\begin{array}{lll}55 & 43.32\end{array}$ | 29 | $0.075 \quad 09$ | . 112 |
|  | 24 | $\begin{array}{lll}56 & 36.38\end{array}$ | 42 | 32 | . 100 |
|  | 25 | $\begin{array}{lll}57 & 29.44\end{array}$ | 55 | 54 | . 087 |
|  | 36 | $\begin{array}{lll}58 & 22.51\end{array}$ | 69 | 77 | . 075 |
| 12 | 1 | $\begin{array}{llll}49 & 59 & 15.57\end{array}$ | 82 | $0.076 \quad 00$ | 40.062 |
|  | 12 | $\begin{array}{lll}50 & 00 & 08.63\end{array}$ | 95 | 22 | . 050 |
|  | 13 | 0101.70 | 0.19209 | 45 | . 037 |
|  | 24 | 0154.76 | 22 | 68 | . 025 |
|  | 25 | 0247.82 | 35 | 90 | . 013 |
|  | 36 | 0340.89 | 49 | 0.07713 | 40.000 |

TABLE IX - Continued

Latitude, with Logarithms of Secant and Tangent, etc.
First and Second Systems of Survey.

| Township | Section | Latitude L. |  | Log Sec L. |  | Log Tan L. |  | Quarter Section |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 13 |  | $\cdot$ 50 | " 33.95 | 0.192 |  | 0.077 |  | chains |
|  | 1 | $50 \quad 04$ | 33.95 |  |  |  |  | 39.988 |
|  | 12 | 05 | 27.01 |  | 75 |  | 59 | . 975 |
|  | 13 | 06 | 20.08 |  | 89 | 0.078 | 81 | . 963 |
|  | 24 | 07 | 13.14 | 0.193 | 02 |  | 04 | . 950 |
|  | 25 | 08 | 06.20 |  | 16 |  | 27 | . 938 |
|  | 36 | 08 | 59.26 |  | 29 |  | 49 | . 925 |
| 14 | 1 | 09 | 52.32 | 42566983960.194 |  | 0.079 | 72 | .913 |
|  | 12 |  | 45.39 |  |  | 95 | . 900 |
|  | 13 |  | 38.45 |  |  | 17 | . 888 |
|  | 24 | 12 | 31.51 |  |  | 40 | . 875 |
|  | 25 | 13 | 24.57 |  |  | 63 | . 863 |
|  | 36 | $14 \quad 17.63$ |  |  |  |  | $\left\{\begin{array}{l}39.850 \\ 40.152\end{array}\right.$ |
|  |  |  |  | 86 |  |  |
| 15 | 1 | $\begin{array}{lll}15 & 10.69\end{array}$ |  |  |  | 0.194 | 23 | 0.080 | 08 | . 139 |
|  | 12 | 16 | 03.76 |  | 36 | 31 | . 126 |  |
|  | 13 | 16 | 56.82 |  | 50 | 54 | . 114 |  |
|  | 24 |  | 49.88 |  | 63 |  | 77 | .101 |
|  | 25 | 18 | 42.94 |  | 77 |  | 99 | . 088 |
|  | 36 |  | 36.00 |  | 90 | 0.081 | 22 | .076 |
| 16 | 1 | 20 | 29.06 | 0.195 | 04 | 0.082 | 45 | . 063 |
|  | 12 | 21 | 22.12 |  | 17 |  | 67 | . 050 |
|  | 13 | 22 | 15.18 |  | 31 |  | 90 | . 038 |
|  | 24 | 23 | 08.24 |  | 44 |  | 13 | . 025 |
|  | 25 | 24 | 01.30 |  | 57 |  | 36 | . 013 |
|  | 36 |  | 54.36 |  | 71 |  | 58 | 40.000 |
| 17 | 1 | 25 | 47.42 | 0.196 | 85 | 0.083 | 81 | 39.987 |
|  | 12 | 26 | 40.48 |  | 98 |  | 04 | . 975 |
|  | 13 | 27 | 33.54 |  | 12 |  | 27 | . 962 |
|  | 24 | 28 | 26.60 . |  | 25 |  | 49 | . 949 |
|  | 25 | 29 | 19.66 |  | 39 |  | 72 | . 937 |
|  | 36 | 30 | 12.71 |  | 52 |  | 95 | . 924 |
| 18 | 1 | 31 | 05.77 | 0.197 | 66 | 0.084 | 18 | 39.911 |
|  | 12 | 31 | 58.83 |  | 79 |  | 41 | . 899 |
|  | 13 | 32 | 51.89 |  | 93 |  | 63 | . 886 |
|  | 24 | 33 | 44.95 |  | 06 | 0.085 | 86 | . 873 |
|  | 25 | 34 | 38.01 |  | 20 |  | 8632 | . 861 |
|  | 36 |  | 31.07 |  | 34 |  |  | (39.848 |
|  |  |  |  |  |  |  |  | \{40.153 |

TABLE IX - Continued

Latitude, with Logarithms of Secant and Tangent, etc.
First and Second Systems of Survey.


TABLE IX - Continued

Latitude, with Logarithms of Secant and Tangent, etc.
First and Second Systems of Survey.


TABLE IX - Continued

Latitude, with Logarithms of Secant and Tangent, etc.
First and Second Systems of Survey.


TABLE IX - Concluded

Latitude, with Logarithms of Secant and Tangent, etc.
First and Second Systems of Survey.

| Township | Section | Latitude L. | Log Sec L. | Log Tan L. | Quarter <br> Section |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 48 |  | 0 | 1 | 1 |  |  |  |
|  | 1 | 53 | 10 | 14.15 | 0.222 | 26 | 0.125 |
|  | 12 |  | 11 | 07.18 |  | 48 | chains |
|  | 13 | 12 | 00.21 |  | 51 |  | 81 |

## TABLE X

Latitude, etc., for the North Boundary of each Section. Third System of Survey.


TABLE X - Continued

Latitude, etc., for the North Boundary of each Section. Third System of Survey.


TABLE X - Continued
Latitude, etc., for the North Boundary of each Section. Third System of Survey.


TABLE X - Continued
Latitude, etc., for the North Boundary of each Section.
Third System of Survey.

| Township | Section | Latitude L. | Log Sec L. | Log Tan L. | Quarter <br> Section |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 19 |  | - 1" |  |  | chains |
|  | 1 | $\begin{array}{llll}50 & 35 & 13.49\end{array}$ | 0.19729 | $0.085 \quad 24$ | 40.138 |
|  | 12 | 3605.57 | 43 | 46 | . 125 |
|  | 13 | 3658.30 | 56 | 69 | . 113 |
|  | 24 | $37 \quad 50.38$ | 69 | 91 | . 100 |
|  | 25 | $\begin{array}{lll}38 & 43.12\end{array}$ | 83 | 0.08614 | . 088 |
|  | 36 | 3935.20 | 96 | 36 | . 075 |
| 20 | 1 | $40 \quad 27.93$ | 0.19810 | 59 | . 063 |
|  | 12 | $41 \quad 20.01$ | 23 | 81 | . 050 |
|  | 13 | $42 \quad 12.74$ | 37 | $0.087 \quad 04$ | . 038 |
|  | 24 | 4304.82 | 50 | 26 | . 025 |
|  | 25 | 4357.55 | 64 | 49 | . 013 |
|  | 36 | $44 \quad 49.63$ | 77 | 72 | 40.000 |
| 21 | 1 | $45 \quad 42.36$ | 91 | 94 | 39.987 |
|  | 12 | $46 \quad 34.44$ | 0.19904 | $0.088 \quad 17$ | . 975 |
|  | 13 | $47 \quad 27.18$ | 18 | 39 | . 962 |
|  | 24 | $48 \quad 19.26$ | 31 | 62 | . 950 |
|  | 25 | $49 \quad 11.99$ | 45 | 84 | . 937 |
|  | 36 | $50 \quad 04.07$ | 58 | $0.089 \quad 07$ | . 925 |
| 22 | 1 | $50 \quad 56.80$ | 72 | 29 | . 912 |
|  | 12 | 5148.87 | 85 | 52 | . 899 |
|  | 13 | 5241.60 | 99 | 74 | . 887 |
|  | 24 | 5333.68 | 0.20013 | 97 | . 874 |
|  | 25 | $54 \quad 26.41$ | 26 | $0.090 \quad 20$ | . 862 |
|  | 36 | $55 \quad 18.49$ | 40 | 42 | (39.849 |
|  |  |  |  |  | (40.152 |
| 23 | 1 | $56 \quad 11.22$ | 53 | 65 | . 140 |
|  | 12 | 5703.30 | 67 | 87 | . 127 |
|  | 13 | $57 \quad 56.03$ | 81 | 0.09110 | . 114 |
|  | 24 | 58 48.11 | 94 | 32 | . 102 |
|  | 25 | 5940.83 | 0.20108 | 55 | . 089 |
|  | 36 | $\begin{array}{llll}51 & 00 & 32.91\end{array}$ | 21 | 77 | . 076 |
| 24 |  | 0125.64 | 35 | 0.09200 | . 064 |
|  | 12 | 0217.72 | 49 | 22 | . 051 |
|  | 13 | 0310.45 | 62 | 45 | . 038 |
|  | 24 | 0402.52 | 76 | 68 | . 025 |
|  | 25 | 0455.25 | 90 | 90 | . 013 |
|  | 36 | 0547.33 | 0.20203 | 0.09313 | 40.000 |

TABLE X - Continued
Latitude, etc., for the North Boundary of each Section. Third System of Survey.

| Township | Section | Latitude L. | Log Sec L. | Log Tan L. | Quarter Section |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 25 |  | - ' ${ }^{\text {- }}$ |  |  | chains |
|  | 1 | 510640.06 | 0.20217 | 0.09335 | 39.987 |
|  | 12 | $07 \quad 32.13$ | 31 | 58 | . 975 |
|  | 13 | $08 \quad 24.86$ | 44 | 81 | . 962 |
|  | 24 | 0916.94 | 58 | 0.09403 | . 949 |
|  | 25 | $10 \quad 09.67$ | 72 | 26 | . 936 |
|  | 36 | 11101.74 | 85 | 48 | . 924 |
| 26 | 1 | 1154.47 | 99 | 71 | . 911 |
|  | 12 | 1246.54 | 0.20313 | 93 | . 898 |
|  | 13 | $13 \quad 39.27$ | 27 | 0.09516 | . 885 |
|  | 24 | 1431.35 | 40 | 39 | . 873 |
|  | 25 | $15 \quad 24.07$ | 54 | 61 | . 860 |
|  | 36 | $16 \quad 16.15$ | 68 | 84 | (39.847 |
|  |  |  |  |  | (40.154 |
| 27 | 1 | $17 \quad 08.87$ | 82 | 0.09607 | . 141 |
|  | 12 | $18 \quad 00.95$ | 95 | 29 | . 129 |
|  | 13 | 1853.68 | 0.20409 | 52 | . 116 |
|  | 24 | 1945.75 | 23 | 74 | . 103 |
|  | 25 | $20 \quad 38.48$ | 37 | 97 | . 090 |
|  | 36 | 2130.55 | 51 | 0.09719 | . 077 |
| 28 |  | $22 \quad 23.28$ |  | 42 | . 064 |
|  | 12 | 2315.35 | 78 | 65 | . 051 |
|  | 13 | 2408.08 | 92 | 87 | . 039 |
|  | 24 | 2500.15 | 0.20506 | 0.09810 | . 026 |
|  | 25 | $25 \quad 52.87$ | 20 | 33 | . 013 |
|  | 36 | $26 \quad 44.95$ | 33 | 55 | 40.000 |
| 29 | 1 | $27 \quad 37.67$ | 47 | 78 | 39.987 |
|  | 12 | $28 \quad 29.75$ | 61 | 0.09900 | . 974 |
|  | 13 | 2922.47 | 75 | 23 | . 961 |
|  | 24 | $30 \quad 14.54$ | 89 | 46 | . 948 |
|  | 25 | 3107.27 | 0.20603 | 69 | . 936 |
|  | 36 | $31 \quad 59.34$ | 17 | 91 | . 923 |
| 30 | 1 | 3252.07 | 31 | $0.100 \quad 14$ | . 910 |
|  | 12 | 3344.14 | 44 | 36 | . 897 |
|  | 13 | 3436.86 | 58 | 59 | . 884 |
|  | 24 | 3528.93 | 72 | 82 | . 871 |
|  | 25 | 3621.66 | 86 | 0.10105 | . 858 |
|  | 36 | $37 \quad 13.73$ | 0.20700 | 27 | $\left\{\begin{array}{l}39.845 \\ 40.156\end{array}\right.$ |

TABLE X - Continued

Latitude, etc., for the North Boundary of each Section. Third System of Survey.


TABLE X - Continued

Latitude, etc., for the North Boundary of each Section. Third System of Survey.

| Township | Section | Latitude L. | Log Sec L. | Log Tan L. | Quarter Section |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 37 |  | - 1 " |  |  | chains |
|  | 1 | $\begin{array}{lll}52 & 09 & 32.68\end{array}$ | $0.212 \quad 21$ | 0.10968 | 39.987 |
|  | 12 | $10 \quad 24.74$ | 35 | 90 | . 974 |
|  | 13 | 1117.46 | 49 | $0.110 \quad 13$ | . 960 |
|  | 24 | $12 \quad 09.53$ | 63 | 36 | . 947 |
|  | 25 | 1302.25 | 77 | 59 | . 934 |
|  | 36 | $13 \quad 54.31$ | 92 | 81 | . 921 |
| 38 | 1 | $14 \quad 47.03$ | 0.21306 | 0.11104 | . 907 |
|  | 12 | $15 \quad 39.10$ | 20 | 27 | . 894 |
|  | 13 | $16 \quad 31.81$ | 34 | 50 | . 881 |
|  | 24 | $17 \quad 23.88$ | 49 | 73 | . 868 |
|  | 25 | $18 \quad 16.60$ | 63 | 96 | . 855 |
|  | 36 | 1908.66 | 77 | 0.11218 | (39.841 |
|  |  |  |  |  | (40.160 |
| 39 | 1 | $20 \quad 01.38$ | 92 | 41 | .147 |
|  | 12 | $20 \quad 53.45$ | 0.21406 | 64 | . 134 |
|  | 13 | 2146.16 | 20 | 87 | .120 |
|  | 24 | 2238.23 | 34 | 0.11309 | . 107 |
|  | 25 | $\begin{array}{lll}23 & 30.94\end{array}$ | 49 | 32 | . 093 |
|  | 36 | $24 \quad 23.01$ | 63 | 55 | . 080 |
| 40 | 1 | $25 \quad 15.72$ | 77 | 78 | . 067 |
|  | 12 | $26 \quad 07.79$ | 92 | 0.11401 | . 053 |
|  | 13 | $27 \quad 00.51$ | 0.21506 | 24 | . 040 |
|  | 24 | $\begin{array}{ll}27 & 52.57\end{array}$ | 20 | 46 | . 027 |
|  | 25 | $28 \quad 45.29$ | 35 | 69 | . 013 |
|  | 36 | $29 \quad 37.35$ | 49 | 92 | 40.000 |
| 41 | 1 | $30 \quad 30.06$ | 64 | $0.115 \quad 15$ | 39.987 |
|  | 12 | $31 \quad 22.13$ | 78 | 38 | . 973 |
|  | 13 | $\begin{array}{ll}32 & 14.84\end{array}$ | 92 | 61 | . 960 |
|  | 24 | 3306.91 | $0.216 \quad 07$ | 83 | . 946 |
|  | 25 | $33 \quad 59.62$ | $21$ | 0.11606 | . 933 |
|  | 36 | $34 \quad 51.69$ | 35 | 29 | . 920 |
| 42 |  | $\begin{array}{ll}35 & 44.40\end{array}$ | 50 | 52 | . 906 |
|  | 12 | $36 \quad 36.46$ | 64 | 75 | . 893 |
|  | 13 | $\begin{array}{lll}37 & 29.18\end{array}$ | 79 | 98 | . 879 |
|  | 24 | $38 \quad 21.24$ | 93 | $0.117 \quad 21$ | . 866 |
|  | 25 | $\begin{array}{ll}39 & 13.95\end{array}$ | $0.217 \quad 08$ | 44 | . 853 |
|  | 36 | $40 \quad 06.02$ | 22 | 66 | (39.839 |
|  |  |  |  |  | (40.162 |

TABLE X - Continued

Latitude, etc., for the North Boundary of each Section.
Third System of Survey.


TABLE X - Continued

Latitude, etc., for the North Boundary of each Section. Third System of Survey.


TABLE X - Continued
Latitude, etc., for the North Boundary of each Section. Third System of Survey.

| Township | Section | Latitude L. | Log Sec L. | Log Tan L. | Quarter Section |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 55 |  | - 53 ' 13 |  |  | chains |
|  | 1 | $\begin{array}{lll}53 & 43 & 50.33\end{array}$ | 0.22799 | 0.13445 | 40.155 |
|  | 12 | $\begin{array}{ll}44 & 42.39\end{array}$ | 0.22813 | 68 | . 140 |
|  | 13 | $45 \quad 35.09$ | 29 | 91 | . 126 |
|  | 24 | $46 \quad 27.14$ | 44 | 0.13514 | . 112 |
|  | 25 | $47 \quad 19.85$ | 59 | 38 | . 098 |
|  | 36 | $48 \quad 11.90$ | 74 | 61 | . 084 |
| 56 | 1 | 4904.60 | 89 | 84 | . 070 |
|  | 12 | 4956.65 | 0.22904 | $0.136 \quad 07$ | . 056 |
|  | 13 | $50 \quad 49.36$ | 19 | 30 | . 042 |
|  | 24 | $\begin{array}{lll}51 & 41.41\end{array}$ | 34 | 53 | . 028 |
|  | 25 | 5234.11 | 49 | 77 | . 014 |
|  | 36 | $\begin{array}{lll}53 & 26.17\end{array}$ | 64 | 0.13700 | 40.000 |
| 57 | 1 | $\begin{array}{lll}54 & 18.87\end{array}$ | 79 | 23 | 39.986 |
|  | 12 | $\begin{array}{lll}55 & 10.92\end{array}$ | 95 | 46 | . 972 |
|  | 13 | $\begin{array}{lll}56 & 03.62\end{array}$ | $0.230 \quad 10$ | 69 | . 958 |
|  | 24 | $\begin{array}{ll}56 & 55.67\end{array}$ | 25 | 92 | . 944 |
|  | 25 | 5748.37 | 40 | 0.13816 | . 930 |
|  | 36 | $58 \quad 40.43$ | 55 | 39 | . 915 |
| 58 |  | $\begin{array}{lll}59 & 33.13\end{array}$ | 70 | 62 | . 901 |
|  | 12 | $\begin{array}{llll}54 & 00 & 25.18\end{array}$ | 85 | 85 | . 887 |
|  | 13 | $01 \quad 17.88$ | 0.23101 | 0.13908 | . 873 |
|  | 24 | 0209.93 | 16 | 31 | . 859 |
|  | 25 | 0302.63 | 31 | 55 | . 845 |
|  | 36 | 0354.68 | 46 | 78 | (39.831 |
|  |  |  |  |  | (40.171 |
| 59 |  | 0447.38 | 62 | 0.14001 | . 157 |
|  | 12 | $05 \quad 39.43$ | 77 | 24 | . 142 |
|  | 13 | $06 \quad 32.13$ | 92 | 48 | . 128 |
|  | 24 | $07 \quad 24.18$ | $0.232 \quad 07$ | 71 | . 114 |
|  | 25 | 0816.88 | 23 | 94 | . 100 |
|  | 36 | $09 \quad 08.93$ | 38 | $0.141 \quad 17$ | . 085 |
| 60 | 1 | $10 \quad 01.63$ | 53 | 41 | . 071 |
|  | 12 | $10 \quad 53.68$ | 68 | 64 | . 057 |
|  | 13 | 1146.38 | 84 | 87 | . 043 |
|  | 24 | 1238.43 | 99 | $0.142 \quad 10$ | . 028 |
|  | 25 | $13 \quad 31.13$ | 0.23314 | 34 | . 014 |
|  | 36 | $14 \quad 23.18$ | 29 | 57 | 40.000 |

TABLE X - Continued

Latitude, etc., for the North Boundary of each Section. Third System of Survey.

| Township | Section | Latitude L. | Log Sec L. | Log Tan L. | Quarter Section |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 61 |  | - ' $\quad 1$ |  |  | chains |
|  | 1 | $\begin{array}{llll}54 & 15 & 15.88\end{array}$ | 0.23345 | 0.14280 | 39.986 |
|  | 12 | $16 \quad 07.93$ | 60 | 0.14303 | . 971 |
|  | 13 | 1700.63 | 75 | 27 | . 957 |
|  | 24 | $17 \quad 52.68$ | 91 | 50 | . 943 |
|  | 25 | 1845.38 | 0.23406 | 73 | . 929 |
|  | 36 | 1937.42 | 21 | $96$ | . 914 |
| 62 | 1 | $20 \quad 30.12$ | 37 | $0.144 \quad 20$ | . 900 |
|  | 12 | $21 \quad 22.17$ | 52 | 43 | . 886 |
|  | 13 | $22 \quad 14.87$ | 68 | 66 | . 872 |
|  | 24 | $23 \quad 06.92$ | 83 | 89 | . 857 |
|  | 25 | $23 \quad 59.62$ | 98 | $0.145 \quad 13$ | . 843 |
|  | 36 | $24 \quad 51.66$ | $0.235 \quad 14$ | $36$ | $\{39.829$ |
|  |  |  |  |  | (40.173 |
| 63 | 1 | $25 \quad 44.36$ | 29 | 59 | . 159 |
|  | 12 | $26 \quad 36.41$ | 45 | 83 | . 144 |
|  | 13 | $27 \quad 29.10$ | 60 | $0.146 \quad 06$ | .130 |
|  | 24 | $28 \quad 21.15$ | 75 | 29 | . 115 |
|  | 25 | $\begin{array}{lll}29 & 13.85\end{array}$ | 91 | 53 | . 101 |
|  | 36 | $30 \quad 05.90$ | 0.23606 | 76 | . 086 |
| 64 | 1 | $30 \quad 58.59$ | 22 | 99 | . 072 |
|  | 12 | 3150.64 | 37 | $0.147 \quad 22$ | . 058 |
|  | 13 | 3243.34 | 53 | 46 | . 043 |
|  | 24 | $33 \quad 35.38$ | 68 | 69 | . 029 |
|  | 25 | $34 \quad 28.08$ | 84 | 93 | . 014 |
|  | 36 | $35 \quad 20.12$ | 99 | $0.148 \quad 16$ | 40.000 |
| 65 | 1 | $36 \quad 12.82$ | 0.23715 | 39 | 39.986 |
|  | 12 | $\begin{array}{lll}37 & 04.87\end{array}$ | 30 | 63 | . 971 |
|  | 13 | $37 \quad 57.56$ | 46 | 86 | . 957 |
|  | 24 | 3849.61 | 61 | $0.149 \quad 09$ | . 942 |
|  | 25 | $\begin{array}{ll}39 & 42.30\end{array}$ | 77 | 33 | . 928 |
|  | 36 | $40 \quad 34.35$ | 92 | 56 | . 913 |
| 66 | 1 | $41 \quad 27.04$ | 0.23808 | 80 | . 899 |
|  | 12 | $\begin{array}{lll}42 & 19.09\end{array}$ | 24 | $0.150 \quad 03$ | . 884 |
|  | 13 | $\begin{array}{lll}43 & 11.78\end{array}$ | 39 | 26 | . 870 |
|  | 24 | $44 \quad 03.83$ | 55 | 50 | . 856 |
|  | 25 | $44 \quad 56.52$ | 70 | 73 | . 841 |
|  | 36 | $45 \quad 48.57$ | 86 | 96 | \{39.827 |
|  |  |  |  |  | (40.175 |

TABLE X - Continued
Latitude, etc., for the North Boundary of each Section. Third System of Survey.

| Township | Section | Latitude L. | Log Sec L. | Log Tan L. | Quarter <br> Section |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 67 |  | - ' ${ }^{\prime}$ |  |  | chains |
|  | 1 | $\begin{array}{llll}54 & 46 & 41.26\end{array}$ | 0.23902 | 0.15120 | 40.161 |
|  | 12 | 4733.31 | 17 | 43 | . 146 |
|  | 13 | 4826.00 | 33 | 67 | . 131 |
|  | 24 | 4918.04 | 49 | 90 | . 117 |
|  | 25 | $50 \quad 10.74$ | 64 | 0.15214 | . 102 |
|  | 36 | $51 \quad 02.78$ | 80 | 37 | . 088 |
| 68 | 1 | 5155.48 | 96 | 60 | . 073 |
|  | 12 | 5247.52 | 0.24011 | 84 | . 058 |
|  | 13 | 5340.21 | 27 | $0.153 \quad 07$ | . 044 |
|  | 24 | $54 \quad 32.26$ | 43 | 31 | . 029 |
|  | 25 | $55 \quad 24.95$ | 58 | 54 | . 015 |
|  | 36 | $56 \quad 16.99$ | 74 | 77 | 40.000 |
| 69 | 1 | $\begin{array}{lll}57 & 09.69\end{array}$ | 90 | 0.15401 | 39.985 |
|  | 12 | $\begin{array}{lll}58 & 01.73\end{array}$ | 0.24105 | 24 | . 971 |
|  | 13 | 5854.42 | 21 | 48 | . 956 |
|  | 24 | 5946.46 | 37 | 71 | . 941 |
|  | 25 | $\begin{array}{llll}55 & 00 & 39.16\end{array}$ | 53 | 95 | . 927 |
|  | 36 | 0131.20 | 68 | $0.155 \quad 18$ | . 912 |
| 70 | 1 | 0223.89 | 84 | 42 | . 898 |
|  | 12 | $03 \quad 15.93$ | 0.24200 | 65 | . 883 |
|  | 13 | $04 \quad 08.62$ | 16 | 89 | . 868 |
|  | 24 | $05 \quad 00.67$ | 31 | $0.156 \quad 12$ | . 854 |
|  | 25 | $05 \quad 53.36$ | 47 | 36 | . 839 |
|  | 36 | 0645.40 | 63 | 59 |  |
|  |  |  |  |  | ${ }_{40.177}$ |
| 71 | 1 | $07 \quad 38.09$ | 79 | 83 | . 163 |
|  | 12 | $08 \quad 30.13$ | 95 | 0.15706 | . 148 |
|  | 13 | 0922.82 | 0.24311 | 30 | . 133 |
|  | 24 | 1014.86 | 26 | 53 | . 118 |
|  | 25 | 1107.56 | 42 | 77 | . 104 |
|  | 36 | 1159.60 | 58 | 0.15800 | . 089 |
| 72 | 1 | 1252.29 | 74 | 24 | . 074 |
|  | 12 | 1344.33 | 90 | 47 | . 059 |
|  | 13 | 1437.02 | 0.24406 | 71 | . 044 |
|  | 24 | $15 \quad 29.06$ | 22 | 94 | . 030 |
|  | 25 | $16 \quad 21.75$ | 38 | 0.15918 | . 015 |
|  | 36 | $17 \quad 13.79$ | 53 | 41 | 40.000 |

TABLE X - Continued

Latitude, etc., for the North Boundary of each Section.
Third System of Survey.

| Township | Section | Latitude L. | Log Sec L. | Log Tan L. | Quarter Section |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 73 |  | - 11 |  |  | chains |
|  | 1 | $\begin{array}{lll}55 & 18 & 06.48\end{array}$ | 0.24469 | 0.15965 | 39.985 |
|  | 12 | 1858.52 | 85 | 89 | . 970 |
|  | 13 | 1951.21 | 0.24501 | $0.160 \quad 12$ | . 956 |
|  | 24 | $20 \quad 43.25$ | 17 | 36 | . 941 |
|  | 25 | $21 \quad 35.94$ | 33 | 59 | . 926 |
|  | 36 | $22 \quad 27.98$ | 49 | 83 | . 911 |
| 74 | 1 | $23 \quad 20.67$ | 65 | $0.161 \quad 07$ | . 896 |
|  | 12 | $\begin{array}{lll}24 & 12.70\end{array}$ | 81 | 30 | . 881 |
|  | 13 | $25 \quad 05.39$ | 97 | 54 | . 867 |
|  | 24 | $25 \quad 57.43$ | $0.246 \quad 13$ | 77 | . 852 |
|  | 25 | $26 \quad 50.12$ | 29 | 0.16201 | . 837 |
|  | 36 | $27 \quad 42.16$ | 45 | 24 | $\left\{\begin{array}{l}39.822 \\ 40.180\end{array}\right.$ |
|  |  |  |  |  | (40.180 |
| 75 | 1 | $28 \quad 34.85$ | 61 | 48 | . 165 |
|  | 12 | $29 \quad 26.89$ | 77 | 72 | . 150 |
|  | 13 | 3019.57 | 93 | 95 | . 135 |
|  | 24 | $31 \quad 11.61$ | 0.24709 | 0.16319 | . 120 |
|  | 25 | 3204.30 | 25 | 43 | . 105 |
|  | 36 | 3256.34 | 41 | 66 | .090 |
| 76 | 1 | $33 \quad 49.02$ | 57 | 90 | . 075 |
|  | 12 | $34 \quad 41.06$ | 73 | $0.164 \quad 13$ | . 060 |
|  | 13 | $35 \quad 33.75$ | 90 | 37 | . 045 |
|  | 24 | $\begin{array}{lll}36 & 25.79\end{array}$ | 0.24806 | 61 | . 030 |
|  | 25 | $\begin{array}{lll}37 & 18.47\end{array}$ | 22 | 85 | . 015 |
|  | 36 | $\begin{array}{lll}38 & 10.51\end{array}$ | 38 | 0.16508 | 40.000 |
| 77 | 1 | 3903.20 | 54 | 32 | 39.985 |
|  | 12 | 3955.23 | 70 | 55 | . 970 |
|  | 13 | $40 \quad 47.92$ | 86 | 79 | . 955 |
|  | 24 | $41 \quad 39.96$ | 0.24902 | $0.166 \quad 03$ | . 940 |
|  | 25 | $42 \quad 32.64$ | 19 | 27 | . 925 |
|  | 36 | $43 \quad 24.68$ | 35 | 50 | . 910 |
| 78 | 1 | $44 \quad 17.37$ | 51 | 74 | . 895 |
|  | 12 | $\begin{array}{lll}45 & 09.40\end{array}$ | 67 | 97 | . 880 |
|  | 13 | $46 \quad 02.09$ | 83 | $0.167 \quad 21$ | . 865 |
|  | 24 | $46 \quad 54.12$ | 0.25000 | 45 | . 850 |
|  | 25 | $47 \quad 46.81$ | $16$ | 69 | . 835 |
|  | 36 | $48 \quad 38.85$ | 32 | 92 | (39.820 |
|  |  |  |  |  | $\{40.182$ |

78897-9

TABLE X - Continued
Latitude, etc., for the North Boundary of each Section. Third System of Survey.

| Township | Section | Latitude L. | Log Sec | L. | Log Ta | n L. | Quarter Section |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 79 | 1 | $55 \quad 4931.53$ | 0.250 | 48 | 0.168 |  | $\begin{aligned} & \text { chains } \\ & 40.167 \end{aligned}$ |
|  | 12 | 50 <br> 03.57 |  | 64 |  | 40 | . 152 |
|  | 13 | $\begin{array}{lll}51 & 16.25\end{array}$ |  | 81 |  | 64 | . 137 |
|  | 24 | 5208.29 | 0.251 | 97 | 0.169 | 87 | . 122 |
|  | 25 | 5300.97 |  | 13 |  | 11 | . 106 |
|  | 36 | 5353.01 |  | 30 |  | 35 | . 091 |
| 80 | 1 | 5445.69 |  | 46 | 0.170 | 59 | . 076 |
|  | 12 | $55 \quad 37.72$ |  | 62 |  | 82 | . 061 |
|  | 13 | 5630.41 |  | 79 |  | 06 | . 046 |
|  | 24 | 5722.44 |  | 95 |  | 30 | . 030 |
|  | 25 | $\begin{array}{lll}58 & 15.13\end{array}$ | 0.252 | 11 |  | 54 | . 015 |
|  | 36 | $59 \quad 07.16$ |  | 27 |  | 77 | 40.000 |
| 81 | 1 | 5959.84 | 0.253 | 44 | 0.171 | 01 | 39.985 |
|  | 12 | $\begin{array}{llll}56 & 00 & 51.88\end{array}$ |  | 60 |  | 25 | . 970 |
|  | 13 | 0144.56 |  | 77 |  | 49 | . 954 |
|  | 24 | 0236.60 |  | 93 |  | 72 | . 939 |
|  | 25 | 0329.28 |  | 09 |  | 96 | . 924 |
|  | 36 | 0421.31 |  | 26 | 0.172 | 20 | . 909 |
| 82 | 1 | 0514.00 | 0.254 | 42 | 0.173 | 44 | . 893 |
|  | 12 | 0606.03 |  | 58 |  | 68 | . 878 |
|  | 13 | 0658.71 |  | 75 |  | 92 | . 863 |
|  | 24 | $07 \quad 50.74$ |  | 91 |  | 15 | . 848 |
|  | 25 | 0843.43 |  | 08 |  | 39 | . 833 |
|  | 36 | 0935.46 |  | 24 |  | 63 | (39.817 |
|  |  |  |  |  |  |  | \{40.185 |
| 83 | 1 | $10 \quad 28.14$ | 0.255 | 41 | 0.174 | 87 | . 169 |
|  | 12 | 11120.17 |  | 57 |  | 11 | . 154 |
|  | 13 | 1212.86 |  | 74 |  | 35 | . 138 |
|  | 24 | 1304.89 |  | 90 |  | 58 | . 123 |
|  | 25 | $13 \quad 57.57$ |  | 06 |  | 82 | . 108 |
|  | 36 | 1449.60 |  | 23 | 0.175 | 06 | . 092 |
| 84 | 1 | 1542.28 | 0.256 | 39 | 0.176 | 30 | . 077 |
|  | 12 | 1634.31 |  | 56 |  | 54 | . 062 |
|  | 13 | $17 \quad 27.00$ |  | 72 |  | 78 | . 046 |
|  | 24 | 1819.03 |  | 89 |  | 01 | . 031 |
|  | 25 | $19 \quad 11.71$ |  | 06 |  | 26 | . 015 |
|  | 36 | $20 \quad 03.74$ |  | 22 |  | 49 | 40.000 |

TABLE X - Continued
Latitude, etc., for the North Boundary of each Section. Third System of Survey.

| Township | Section | Latitude L. | Log Sec L. | Log Tan L. | Quarter Section |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 85 |  | - " |  |  | chains |
|  | 1 | $\begin{array}{lll}56 & 20 & 56.42\end{array}$ | 0.25639 | 0.17673 | 39.985 |
|  | 12 | 2148.45 | 55 | 97 | . 969 |
|  | 13 | $22 \quad 41.13$ | 72 | $0.177 \quad 21$ | . 954 |
|  | 24 | $23 \quad 33.16$ | 88 | 45 | . 938 |
|  | 25 | $24 \quad 25.84$ | 0.25705 | 69 | . 923 |
|  | 36 | $\begin{array}{lll}25 & 17.87\end{array}$ | 21 | 93 | . 908 |
| 86 | 1 | $26 \quad 10.55$ | 38 | $0.178 \quad 17$ | . 892 |
|  | 12 | $27 \quad 02.58$ | 55 | 41 | . 877 |
|  | 13 | $27 \quad 55.26$ | 71 | 65 | . 861 |
|  | 24 | $28 \quad 47.29$ | 88 | 88 | . 846 |
|  | 25 | 2939.97 | 0.25805 | $0.179 \quad 13$ | . 830 |
|  | 36 | $30 \quad 32.00$ | 21 | 36 | $(39.815$ |
|  |  |  |  |  | (40.187 |
| 87 | 1 | 3124.68 | 38 | 60 | . 171 |
|  | 12 | 3216.71 | 55 | 84 | . 156 |
|  | 13 | 3309.39 | 71 | $0.180 \quad 08$ | . 140 |
|  | 24 | 3401.42 | 88 | 32 | . 125 |
|  | 25 | $34 \quad 54.10$ | 0.25905 | 56 | . 109 |
|  | 36 | 3546.12 | 21 | 80 | . 093 |
| 88 | 1 | 3638.80 | 38 | 0.18104 | . 078 |
|  | 12 | $37 \quad 30.83$ | 55 | 28 | . 062 |
|  | 13 | 3823.51 | 72 | 52 | . 047 |
|  | 24 | $39 \quad 15.54$ | 88 | 76 | . 031 |
|  | 25 | $40 \quad 08.21$ | 0.26005 | 0.18200 | . 015 |
|  | 36 | $41 \quad 00.24$ | 22 | 24 | 40.000 |
| 89 | 1 | 4152.92 | 39 | 48 | 39.984 |
|  | 12 | 4244.95 | 55 | 72 | . 969 |
|  | 13 | $43 \quad 37.63$ | 72 | 96 | . 953 |
|  | 24 | 4429.65 | 89 | 0.18320 | . 937 |
|  | 25 | $45 \quad 22.33$ | 0.26106 | 44 | . 922 |
|  | 36 | $46 \quad 14.36$ | 23 | 68 | . 906 |
| 90 | 1 | $47 \quad 07.03$ | 40 | 93 | . 891 |
|  | 12 | 4759.06 | 56 | 0.18416 | . 875 |
|  | 13 | $48 \quad 51.74$ | 73 | 41 | . 859 |
|  | 24 | 4943.76 | 90 | 65 | . 844 |
|  | 25 | $50 \quad 36.44$ | $0.262 \quad 07$ | 89 | . 828 |
|  | 36 | $51 \quad 28.47$ | 24 | $0.185 \quad 13$ | $\left\{\begin{array}{l}39.813 \\ 40.190\end{array}\right.$ |
|  |  |  |  |  | (40.190 |

78897-91

TABLE X - Continued

Latitude, etc., for the North Boundary of each Section. Third System of Survey.

| Township | Section | Latitude L. | Log Sec L. | Log Tan L. |  | Quarter Section |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 91 | 1 | $\begin{array}{ccc}\circ & \prime & \prime \prime \\ 56 & 52 & 21.14\end{array}$ | 0.26241 | 0.185 |  | chains $40.174$ |
|  | 12 | 53 13.17 | 0.263 |  | 61 | . 158 |
|  | 13 | $54 \quad 05.85$ |  | 0.186 | 85 | . 142 |
|  | 24 | $54 \quad 57.87$ |  |  | 0933 | . 126 |
|  | 25 | $55 \quad 50.55$ |  |  |  | . 111 |
|  | 36 | 5642.57 |  |  | 57 | . 095 |
| 92 | 1 | $\begin{array}{lll}57 & 35.25\end{array}$ | 0.264 | 0.187 | 82 | . 079 |
|  | 12 | $58 \quad 27.27$ |  |  | 06 | . 063 |
|  | 13 | $\begin{array}{lll}59 & 19.95\end{array}$ |  |  | 30 | . 047 |
|  | 24 | $\begin{array}{llll}57 & 00 & 11.98\end{array}$ |  |  | 54 | . 032 |
|  | 25 | 0104.65 |  |  | 78 | . 016 |
|  | 36 | 0156.68 |  | 0.188 | 02 | 40.000 |
| 93 | 1 | 0249.35 | 44 | 26 |  | 39.984 |
|  | 12 | 0341.38 | 61 | 50 |  | . 968 |
|  | 13 | 0434.05 | 78 | 75 |  | . 953 |
|  | 24 | $05 \quad 26.07$ | 95 $0.265 \quad 12$ | 0.189 | 99 | . 937 |
|  | 25 | 0618.75 | 0.265 |  | 2347 | . 921 |
|  | 36 | $07 \quad 10.77$ |  |  |  | . 905 |
| 94 | 1 | $08 \quad 03.45$ | 46 | 71 |  | . 889 |
|  | 12 | $08 \quad 55.47$ | 63 | 95 |  | . 873 |
|  | 13 | 0948.14 | 80 | 0.190 |  | . 858 |
|  | 24 | $10 \quad 40.17$ | $\begin{array}{ll} & 97 \\ 0.266 & 15 \\ & 32\end{array}$ | 44 |  | . 842 |
|  | 25 | $11 \quad 32.84$ |  | 68 |  | $\left\{\begin{array}{r}.826 \\ 39.810 \\ 40.192\end{array}\right.$ |
|  | 36 | $12 \quad 24.87$ |  |  | 92 |  |
|  |  |  |  |  |  |  |
| 95 | 1 | 1317.54 | 49 | 0.191 | 17 | .176 |
|  | 12 | $14 \quad 09.56$ | 66 | 41 |  | . 160 |
|  | 13 | $15 \quad 02.24$ | 0.2678 | 65 |  | . 144 |
|  | 24 | $15 \quad 54.26$ |  | 89 |  | . 128 |
|  | 25 | 1646.93 | 17 | 0.192 | 13 | . 112 |
|  | 36 | $17 \quad 38.95$ | 34 |  | 38 | . 096 |
| 96 | 112 | $18 \quad 31.63$ | 52 | 62 |  | . 080 |
|  |  | 1923.65 | 69 | 0.193 | 86 | . 064 |
|  | 13 | $20 \quad 16.32$ |  <br> 0.268 <br> 86 <br>  <br> 03 <br> 20 <br> 38 |  | 10 | . 048 |
|  | 24 | $21 \quad 08.34$ |  |  | 35 | . 032 |
|  | 25 | $\begin{array}{ll}22 & 01.02\end{array}$ |  |  | 5983 | . 016 |
|  | 36 | 2253.04 |  |  |  | 40.000 |

TABLE X - Continued
Latitude, etc., for the North Boundary of each Section. Third System of Survey.

| Township | Section | Latitude L. | Log Sec L. | Log Tan L. | Quarter Section |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 97 |  | - 1 |  |  | chains |
|  | 1 | $\begin{array}{lll}57 & 23 & 45.71\end{array}$ | 0.26855 | $0.194 \quad 07$ | 39.984 |
|  | 12 | $24 \quad 37.73$ | 72 | 32 | . 968 |
|  | 13 | $\begin{array}{ll}25 & 30.40\end{array}$ | 89 | 56 | . 952 |
|  | 24 | $26 \quad 22.42$ | 0.26907 | 80 | . 936 |
|  | 25 | $27 \quad 15.10$ | 24 | 0.19505 | . 920 |
|  | 36 | 2807.12 | 41 | $29$ | . 904 |
| 98 | 1 | $28 \quad 59.79$ | 58 | 53 | . 888 |
|  | 12 | $29 \quad 51.81$ | 76 | 77 | . 872 |
|  | 13 | $30 \quad 44.48$ | 93 | 0.19602 | . 856 |
|  | 24 | $31 \quad 36.50$ | $0.270 \quad 10$ | 26 | . 840 |
|  | 25 | $\begin{array}{lll}32 & 29.17\end{array}$ | 28 | 51 | . 824 |
|  | 36 | $33 \quad 21.19$ | 45 | 75 | $\{39.808$ |
|  |  |  |  |  | \{40.195 |
| 99 | 1 | $\begin{array}{ll}34 & 13.86\end{array}$ | 62 | 99 | . 178 |
|  | 12 | $\begin{array}{lll}35 & 05.88\end{array}$ | 80 | 0.19724 | . 162 |
|  | 13 | $35 \quad$ b8.55 | 97 | 48 | . 146 |
|  | 24 | $36 \quad 50.57$ | 0.27114 | 72 | . 130 |
|  | 25 | $37 \quad 43.24$ | 32 | 97 | . 114 |
|  | 36 | $38 \quad 35.26$ | 49 | $0.198 \quad 21$ | . 097 |
| 100 | 1 | $\begin{array}{lll}39 & 27.93\end{array}$ | 67 | 45 | . 081 |
|  | 12 | $\begin{array}{ll}40 & 19.95\end{array}$ | 84 | 70 | . 065 |
|  | 13 | $41 \quad 12.62$ | 0.27201 | 94 | . 049 |
|  | 24 | $42 \quad 04.64$ | 19 | 0.19919 | . 032 |
|  | 25 | $42 \quad 57.31$ | 36 | 43 | . 016 |
|  | 36 | $43 \quad 49.33$ | 54 | 67 | 40.000 |
| 101 | 1 | $44 \quad 42.00$ | 71 | 92 | 39.984 |
|  | 12 | $45 \quad 34.02$ | 89 | $0.200 \quad 16$ | . 968 |
|  | 13 | $46 \quad 26.69$ | 0.27306 | 41 | . 951 |
|  | 24 | $\begin{array}{lll}47 & 18.70\end{array}$ | $24$ | 65 | . 935 |
|  | 25 | $\begin{array}{lll}48 & 11.37\end{array}$ | 41 | 90 | . 919 |
|  | 36 | $49 \quad 03.39$ | 59 | 0.20114 | .902 |
| 102 | 1 | $49 \quad 56.06$ | 76 | 39 | . 886 |
|  | 12 | $\begin{array}{lll}50 & 48.08\end{array}$ | 94 | 63 | . 870 |
|  | 13 | $51 \quad 40.74$ | 0.27411 | 87 | . 854 |
|  | 24 | $52 \quad 32.76$ | 29 | 0.20212 | . 837 |
|  | 25 | $\begin{array}{lll}53 & 25.43\end{array}$ | 46 | 36 | . 821 |
|  | 36 | $54 \quad 17.45$ | 64 | 61 | (39.805 |
|  |  |  |  |  | $\{40.197$ |

TABLE X - Continued
Latitude, etc., for the North Boundary of each Section. Third System of Survey.


TABLE X - Continued

Latitude, etc., for the North Boundary of each Section.
Third System of Survey.


TABLE X - Continued

Latitude, etc., for the North Boundary of each Section.
Third System of Survey.


TABLE X - Continued

Latitude, etc., for the North Boundary of each Section.
Third System of Survey.

| Township | Section | Latitude L. | Log Sec L. | Log Tan L. | Quarter Section |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 121 |  | - 11 |  |  | chains |
|  | 1 | $\begin{array}{lll}59 & 29 & 22.39\end{array}$ | 0.29440 | 0.22967 | 39.983 |
|  | 12 | $30 \quad 14.39$ | 58 | 92 | . 965 |
|  | 13 | 3107.05 | 77 | 0.23017 | . 948 |
|  | 24 | $\begin{array}{lll}31 & 59.05\end{array}$ | 96 | 43 | . 930 |
|  | 25 | 3251.71 | 0.29515 | 68 | . 913 |
|  | 36 | $33 \quad 43.71$ | 33 | 93 | . 896 |
| 122 | 1 | $34 \quad 36.36$ | 52 | 0.23118 | . 878 |
|  | 12 | $\begin{array}{lll}35 & 28.37\end{array}$ | 71 | 43 | . 861 |
|  | 13 | $36 \quad 21.02$ | 90 | 69 | . 843 |
|  | 24 | $\begin{array}{ll}37 & 13.02\end{array}$ | 0.29608 | 94 | . 826 |
|  | 25 | $\begin{array}{lll}38 & 05.68\end{array}$ | 27 | 0.23219 | . 808 |
|  | 36 | $38 \quad 57.68$ | 46 | 44 | (39.791 |
|  |  |  |  |  | (40.212 |
| 123 | 1 | $39 \quad 50.33$ | 65 | 70 | . 194 |
|  | 12 | $40 \quad 42.34$ | 84 | 95 | . 176 |
|  | 13 | $41 \quad 34.99$ | 0.29702 | 0.23320 | . 159 |
|  | 24 | $42 \quad 26.99$ | 21 | 46 | . 141 |
|  | 25 | $\begin{array}{lll}43 & 19.65\end{array}$ | 40 | 71 | . 123 |
|  | 36 | $44 \quad 11.65$ | 59 | 96 | . 106 |
| 124 | 1 | $45 \quad 04.30$ | 78 | $0.234 \quad 22$ | . 088 |
|  | 12 | $45 \quad 56.30$ | 97 | 47 | . 071 |
|  | 13 | $46 \quad 48.96$ | 0.29816 | 72 | . 053 |
|  | 24 | $47 \quad 40.96$ | 35 | 97 | . 035 |
|  | 25 | $48 \quad 33.61$ | 54 | $0.235 \quad 23$ | . 018 |
|  | 36 | $49 \quad 25.61$ | 72 | 48 | 40.000 |
| 125 | 1 | $\begin{array}{lll}50 & 18.26\end{array}$ | 92 | 74 | 39.982 |
|  | 12 | $\begin{array}{lll}51 & 10.26\end{array}$ | 0.29910 | 99 | . 965 |
|  | 13 | $\begin{array}{lll}52 & 02.91\end{array}$ | 29 | $0.236 \quad 24$ | . 947 |
|  | 24 | 5254.92 | 48 | 50 | . 929 |
|  | 25 | $\begin{array}{lll}53 & 47.57\end{array}$ | 67 | 75 | .912 |
|  | 36 | $54 \quad 39.57$ | 86 | 0.23700 | . 894 |
| 126 | 1 | $55 \quad 32.22$ | $0.300 \quad 05$ | 26 | . 876 |
|  | 12 | $56 \quad 24.22$ | 24 | 51 | . 859 |
|  | 13 | $\begin{array}{lll}57 & 16.87\end{array}$ | 44 | 77 | . 841 |
|  | 24 | $\begin{array}{lll}58 & 08.87\end{array}$ | 63 | $0.238 \quad 02$ | . 824 |
|  | 25 | $59 \quad 01.52$ | 82 | 28 | . 80 t |
|  | 36 | $59 \quad 53.52$ | 0.30101 | 53 | (39.78) |
|  |  |  |  |  | \{40.2.4 |

TABLE X - Continued
Latitude, etc., for the North Boundary of each Section. Third System of Survey.

| Township | Section | Latitude L. | Log Sec L. | Log Tan L. | Quarter <br> Section |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 127 |  | - 1 |  |  | chains |
|  | 1 | $\begin{array}{lll}60 & 00 & 46.17\end{array}$ | 0.30120 | 0.23879 | 40.197 |
|  | 12 | 0138.17 | 39 | 0.23904 | . 179 |
|  | 13 | 0230.82 | 58 | 29 | . 161 |
|  | 24 | $03 \quad 22.82$ | 77 | 55 | . 143 |
|  | 25 | $\begin{array}{ll}04 & 15.47\end{array}$ | 96 | 80 | . 125 |
|  | 36 | $05 \quad 07.47$ | 0.30215 | $0.240 \quad 06$ | .107 |
| 128 | 1 | $06 \quad 00.12$ | 35 | 31 | . 089 |
|  | 12 | $06 \quad 52.12$ | 54 | 57 | . 072 |
|  | 13 | 0744.77 | 73 | 82 | . 054 |
|  | 24 | $08 \quad 36.77$ | 92 | 0.24108 | . 036 |
|  | 25 | 0929.42 | 0.30311 | 33 | . 018 |
|  | 36 | $10 \quad 21.42$ | 30 | 59 | 40.000 |
| 129 | 1 | 1114.07 | 50 | 84 | 39.982 |
|  | 12 | 1206.06 | 69 | 0.24210 | . 964 |
|  | 13 | $12 \quad 58.71$ | 88 | 36 | . 946 |
|  | 24 | 1350.71 | $0.304 \quad 07$ | 61 | . 928 |
|  | 25 | 1443.36 | 27 | 87 | . 910 |
|  | 36 | $15 \quad 35.36$ | 46 | 0.24312 | .893 |
| 130 | 1 | $16 \quad 28.01$ | 65 | 38 | . 875 |
|  | 12 | $17 \quad 20.00$ | 84 | 63 | . 857 |
|  | 13 | 1812.65 | $0.305 \quad 04$ | 89 | . 839 |
|  | 24 | 1904.65 | 23 | 0.24414 | . 821 |
|  | 25 | $19 \quad 57.30$ | 43 | 40 | . 803 |
|  | 36 | $20 \quad 49.30$ | 62 | 66 | (39.785 |
|  |  |  |  |  | $\{40.218$ |
| 131 | 1 | $21 \quad 41.94$ | 81 | 91 | . 199 |
|  | 12 | $22 \quad 33.94$ | 0.30601 | $0.245 \quad 17$ | . 181 |
|  | 13 | $23 \quad 26.59$ | 20 | 43 | . 163 |
|  | 24 | 2418.58 | 39 | 68 | . 145 |
|  | 25 | $25 \quad 11.23$ | 59 | 94 | . 127 |
|  | 36 | $26 \quad 03.23$ | 78 | $0.246 \quad 20$ | .109 |
| 132 | 1 | $26 \quad 55.88$ | 98 | 45 | . 091 |
|  | 12 | $27 \quad 47.87$ | 0.30717 | 71 | . 073 |
|  | 13 | $28 \quad 40.52$ | 37 | 97 | . 054 |
|  | 24 | $29 \quad 32.51$ | 56 | $0.247 \quad 22$ | . 036 |
|  | 25 | $30 \quad 25.16$ | 75 | 48 | . 018 |
|  | 36 | $31 \quad 17.16$ | 95 | 74 | 40.000 |

TABLE X - Continued

Latitude, etc., for the North Boundary of each Section.
Third System of Survey.


TABLE X - Continued
Latitude, etc., for the North Boundary of each Section.
Third System of Survey.

| Township | Section | Latitude L. | Log Sec | L. | Log Tan L. |  | Quarter Section |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 139 | 1 | $\begin{array}{lll} 61 & 03 & 33.28 \end{array}$ | 0.315 | 24 | 0.257 | 31 | chains40.205 |
|  |  |  |  |  |  |  |  |
|  | 12 | $04 \quad 25.28$ | 44 |  |  | 57 | . 187 |
|  | 13 | $05 \quad 17.92$ | 64 |  | 0.258 | 83 | . 168 |
|  | 24 | 0609.91 | 0.316 | 84 |  | 09 | . 149 |
|  | 2536 | $07 \quad 0255$ |  | 0424 |  | 35 | .131 |
|  |  | $07 \quad 54.54$ |  |  |  | 61 | . 112 |
| 140 | 1 | 0847.18 | 44 |  | 0.259 | 87 | . 093 |
|  | 12 | 0939.17 | 64 |  |  |  | . 075 |
|  | 13 | 1031.81 | 0.317 | 84 | 39 |  | . 056 |
|  | 24 | 1123.81 |  | 04 | 65 |  | . 037 |
|  | 25 | 1216.45 |  | 24 | 0.260 | 91 | . 019 |
|  | 36 | 1308.44 |  | 44 |  | 17 | 40.000 |
| 141 | 1 | 1401.08 | 64 |  | 44 |  | 39.981 |
|  | 12 | 1453.07 | $0.318{ }^{8}$ | 84 | 69 |  | . 963 |
|  | 13 | 1545.71 |  | 04 | 0.261 | 96 | . 944 |
|  | 24 | $16 \quad 37.70$ | 24 |  |  | 22 | . 925 |
|  | $\begin{aligned} & 25 \\ & 36 \end{aligned}$ | $17 \quad 30.34$ | 44 |  |  | 48 | . 906 |
|  |  | $18 \quad 22.33$ | 64 |  |  | 74 | . 888 |
| 142 | 1 | $19 \quad 14.97$ | 0.319 | 85 | 0.262 | 00 | . 869 |
|  | 12 | $20 \quad 06.96$ |  | 05 | 26 |  | . 851 |
|  | 13 | $20 \quad 59.60$ |  | 25 | 53 |  | . 832 |
|  | 24 | 2151.59 |  | 45 | 0.263 | 79 | . 813 |
|  | 25 | 2244.23 |  | 65 |  | 05 |  |
|  | 36 | $23 \quad 36.22$ |  | 85 |  | 31 | $\left\{\begin{array}{l} 39.776 \\ 40.227 \end{array}\right.$ |
|  |  |  |  |  |  |  |  |
| 143 | 1 | $24 \quad 28.85$ | 0.320 | 06 | 57 |  | . 208 |
|  | 12 | $25 \quad 20.84$ | 26 |  |  84 |  | .189.170 |
|  | 13 | $26 \quad 13.48$ | 46668686 |  |  |  |  |  |
|  | 24 | $27 \quad 05.47$ |  |  | 0.264 | 36 | .170 .151 |
|  | 25 | $27 \quad 58.11$ |  |  | 62 | .133.114 |  |
|  | 36 | $28 \quad 50.10$ |  |  | 88 |  |  |
| 144 | 11213242536 | 2942.74 | 27 |  |  | 0.265 | 1541 | . 095 |
|  |  | $30 \quad 34.73$ | 47 |  | . 076 |  |  |
|  |  | $31 \quad 27.36$ | 68 |  | 67 |  | . 057 |
|  |  | $\begin{array}{ll} 32 & 19.35 \\ 33 & 11.99 \end{array}$ | 0.322 | $\begin{aligned} & 88 \\ & 08 \\ & 28 \end{aligned}$ |  |  | .038.019 |
|  |  |  |  |  |  |  |  |  |
|  |  | $34 \quad 03.98$ |  |  |  |  | 40.000 |

TABLE X - Continued
Latitude, etc., for the North Boundary of each Section. Third System of Survey.

| Township | Section | Latitude L. | Log Sec L. |  | Log Tan L. |  | Quarter <br> Section |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 145 | 1 | $\begin{array}{lll} 61 & 34 & 56.63 \end{array}$ | 0.32249 |  | 0.266 | 73 | chains$39.981$ |
|  |  |  |  |  |  |  |  |
|  | 12 | 3548.61 | 6 | 69 |  | 99 | . 962 |
|  | 13 | $36 \quad 41.25$ | 0.323 | 90 | 0.267 | 25 | . 943 |
|  | 24 | $37 \quad 33.24$ |  | 10 |  | 51 | . 924 |
|  | 25 | $38 \quad 25.88$ |  | 30 | 0.268 | 78 | . 905 |
|  | 36 | $39 \quad 17.86$ |  | 51 |  | 04 | . 886 |
| 146 | 1 | $40 \quad 10.50$ | 779$0.324 \quad 1$ |  | 31 |  | . 867 |
|  | 12 | $41 \quad 02.49$ |  | 92 | 57 |  | . 848 |
|  | 13 | 4155.12 |  | 12 | 0.269 | 83 | . 829 |
|  | 24 | 4247.11 | 355 | 33 |  | 10 | . 810 |
|  | 25 | $43 \quad 39.75$ |  | 53 |  | 36 | . 791 |
|  | 36 | $44 \quad 31.73$ | 73 |  |  | 62 |  | $\left\{\begin{array}{l}39.772 \\ 40.230\end{array}\right.$ |
|  |  |  |  |  |  |  |  |  |  |
| 147 |  | $45 \quad 24.37$ | 0.325 | 94 | 0.270 |  | . 211 |  |
|  | 12 | $46 \quad 16.36$ |  | 14 |  | 89 15 | . 192 |  |
|  | 13 | $47 \quad 08.99$ |  | 35 |  | 42 | . 173 |  |
|  | 24 | $48 \quad 00.98$ |  | 56 |  | 68 | . 154 |  |
|  | 25 | $48 \quad 53.62$ |  | 76 | 0.271 | 95 | . 134 |  |
|  | 36 | 4945.60 |  | 97 |  | 21 | . 115 |  |
| 148 | 1 | $50 \quad 38.24$ | 0.326 | 17 | 48 |  | . 096 |  |
|  | 12 | 5130.22 |  | 38 |  | 74 | . 077 |  |
|  | 13 | $52 \quad 22.86$ |  | 59 | 0.272 | 01 | . 058 |  |
|  | 24 | 5314.84 |  | 79 |  | 27 | . 038 |  |
|  | 25 | $54 \quad 07.48$ | 0.327 | 00 |  | 54 | . 019 |  |
|  | 36 | $54 \quad 59.46$ |  | 20 |  | 80 | 40.000 |  |
| 149 | 1 | $55 \quad 52.10$ |  | 41 | 0.273 | 07 | 39.981 |  |
|  | 12 | 5644.08 |  | 62 |  | 33 | . 962 |  |
|  | 13 | $57 \quad 36.72$ |  | 82 |  | 60 | . 942 |  |
|  | 24 | $58 \quad 28.70$ | 0.328 | 03 |  | 86 | . 923 |  |
|  | 25 | $59 \quad 21.34$ |  | 24 | 0.274 | 13 | . 904 |  |
|  | 36 | $\begin{array}{llll}62 & 00 & 13.32\end{array}$ |  | 44 |  | 39 | . 885 |  |
| 150 | 1 | $01 \quad 05.96$ |  | 65 |  | 66 | . 865 |  |
|  | 12 | 0157.94 |  | 86 |  | 92 | . 846 |  |
|  | 13 | 0250.58 | 0.329 | 07 | 0.275 | 19 | . 827 |  |
|  | 24 | 0342.56 |  | 27 |  | 46 | . 808 |  |
|  | 25 | 0435.19 |  | 48 |  | 72 | . 788 |  |
|  | 36 | $05 \quad 27.18$ |  | 69 |  | 99 | (39.769 |  |
|  |  |  |  |  |  |  | \{40.234 |  |

TABLE X - Continued

Latitude, etc., for the North Boundary of each Section Third System of Survey.


TABLE X - Continued

Latitude, etc., for the North Boundary of each Section. Third System of Survey.

| Township | Section | Latitude L. | Log Sec L. | Log Tan L. | Quarter Section |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 157 | 1 | $\begin{array}{lll} 62 & 37 & 42.85 \end{array}$ | 0.33747 | 0.28591 | chains $39.980$ |
|  | 12 | $\begin{array}{ll}38 & 34.83\end{array}$ | 68 | $0.286 \quad 17$ | . 960 |
|  | 13 | $39 \quad 27.46$ | 90 | 45 | . 941 |
|  | 24 | $\begin{array}{ll}40 & 19.44\end{array}$ | 0.33811 | 71 | . 921 |
|  | 25 | $\begin{array}{lll}41 & 12.07\end{array}$ | 32 | 99 | . 901 |
|  | 36 | 4204.05 | 54 | $0.287 \quad 25$ | . 881 |
| 158 | 1 | $42 \quad 56.68$ | 75 | 53 | . 861 |
|  | 12 | $43 \quad 48.66$ | 96 | 80 | . 842 |
|  | 13 | $44 \quad 41.28$ | 0.33918 | 0.28807 | . 822 |
|  | 24 | $45 \quad 33.26$ | 39 | 34 | . 802 |
|  | 25 | $46 \quad 25.89$ | 61 | 61 | . 782 |
|  | 36 | $47 \quad 17.87$ | 82 | 88 | $\left\{\begin{array}{l}39.762\end{array}\right.$ |
|  |  |  |  |  | (40.241 |
| 159 | 1 | $48 \quad 10.50$ | $0.340 \quad 03$ | 0.28915 | . 221 |
|  | 12 | $49 \quad 02.48$ | 25 | 42 | . 201 |
|  | 13 | $49 \quad 55.11$ | 46 | 69 | . 181 |
|  | 24 | $50 \quad 47.08$ | 68 | 96 | . 161 |
|  | 25 | $\begin{array}{ll}51 & 39.71\end{array}$ | 89 | 0.29023 | . 141 |
|  | 36 | 5231.69 | 0.34111 | 50 | . 120 |
| 160 | 1 | $53 \quad 24.32$ | 32 | 78 | . 100 |
|  | 12 | $\begin{array}{lll}54 & 16.29\end{array}$ | 54 | 0.29105 | . 080 |
|  | 13 | 5508.92 | 75 | 32 | . 060 |
|  | 24 | 5600.90 | 97 | 59 | . 040 |
|  | 25 | $\begin{array}{lll}56 & 53.53\end{array}$ | $0.342 \quad 18$ | 86 | . 020 |
|  | 36 | $57 \quad 45.50$ | 40 | 0.29213 | 40.000 |
| 161 | 1 | $\begin{array}{lll}58 & 38.13\end{array}$ | 62 | 41 | 39.980 |
|  | 12 | $59 \quad 30.11$ | 83 | 68 | . 960 |
|  | 13 | $\begin{array}{llll}63 & 00 & 22.74\end{array}$ | 0.34305 | 95 | . 940 |
|  | 24 | $01 \quad 14.71$ | 26 | 0.29322 | . 920 |
|  | 25 | 0207.34 | 48 | 50 | . 899 |
|  | 36 | $02 \quad 59.32$ | 70 | 77 | . 879 |
| 162 | 1 | $03 \quad 51.94$ | 91 | $0.294 \quad 04$ | . 859 |
|  | 12 | 0443.92 | 0.34413 | 31 | . 839 |
|  | 13 | $05 \quad 36.54$ | 35 | 59 | . 819 |
|  | 24 | $06 \quad 28.52$ | 56 | 86 | . 799 |
|  | 25 | $07 \quad 21.15$ | 78 | 0.29513 | . 779 |
|  | 36 | $08 \quad 13.12$ | 0.34500 | 41 | (39.759 |
|  |  |  |  |  | (40.245 |

TABLE X - Continued

Latitude, etc., for the North Boundary of each Section. Third System of Survey.

| Township | Section | Latitude L. | Log Sec | L. | Log Ta | L. | Quarter <br> Section |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 163 | 1 | - 11 | 0.345 | 22 | 0.295 |  | chains |
|  |  | $\begin{array}{lll}63 & 09 & 05.75\end{array}$ |  |  |  |  | 40.224 |
|  | 12 | 0957.72 |  | 43 |  | 95 | . 204 |
|  | 13 | $10 \quad 50.35$ |  | 65 | 0.296 | 23 | . 183 |
|  | 24 | $11 \quad 42.32$ |  | 87 |  | 50 | . 163 |
|  | 25 | 1234.95 | 0.346 | 09 |  | 77 | .143 |
|  | 36 | $13 \quad 26.93$ |  | 30 | 0.297 | 05 | .122 |
| 164 | 1 | $\begin{array}{lll}14 & 19.55\end{array}$ | 52 |  | 32 |  | . 102 |
|  | 12 | 1511.53 | 74 |  | 59 |  | . 082 |
|  | 13 | 1604.15 | 0.347 | 96 | 0.298 | 87 | . 061 |
|  | 24 | $16 \quad 56.13$ |  | 1840 |  | 1442 | . 041 |
|  | 25 | 1748.75 |  |  |  |  | . 020 |
|  | 36 | 1840.72 | 62 |  |  | 69 | 40.000 |
| 165 | 1 | 1933.35 | 0.348 | 84 | 0.299 | 97 | 39.980 |
|  | 12 | $20 \quad 25.32$ |  | 05 |  | 24 | . 959 |
|  | 13 | $21 \quad 17.95$ |  | 27 |  | 52 | . 939 |
|  | 24 | $22 \quad 09.92$ |  | 49 | 0.300 | 79 | . 918 |
|  | 25 | $23 \quad 02.55$ |  | 71 |  | 07 | . 898 |
|  | 36 | 2354.52 |  | 93 |  | 34 | . 878 |
| 166 | 1 | $24 \quad 47.14$ | 0.349 | 15 | 62 |  | . 857 |
|  | 12 | $25 \quad 39.12$ | 37 |  | 0.301 | 89 | . 837 |
|  | 13 | $26 \quad 31.74$ | 59 |  |  | 17 | . 816 |
|  | 24 | $27 \quad 23.71$ | 81 |  | 44 |  | .796.77539.75540.248 |
|  | 25 | $28 \quad 16.34$ | 0.350 | 03 | 7299 |  |  |
|  | 36 | $29 \quad 08.31$ |  | 25 |  |  |  |  |
| 167 | 1 | $30 \quad 00.93$ | 48 |  | 0.302 |  | .228 |
|  | 12 | $30 \quad 52.91$ | 70 |  | 0.303 | 54 | .207 |
|  | 13 | $\begin{array}{ll}30 & 52.91 \\ 31 & 45.53\end{array}$ | 92 |  |  | 82 | . 186 |
|  | 24 | $32 \quad 37.50$ | 0.351 | 1436 |  | 820937 | . 166 |
|  | 25 | $33 \quad 30.13$ |  |  |  |  | $\begin{aligned} & .145 \\ & .124 \end{aligned}$ |
|  | 36 | $34 \quad 22.10$ | 58 |  |  | 65 |  |
| 168 | 1 | $35 \quad 14.72$ | 0.352 | 80 | 0.304 | 9320 | . 103 |
|  | 12 | $36 \quad 06.69$ |  | 02 |  |  | . 083 |
|  | 13 | $36 \quad 59.31$ |  | 25 |  | 48 | . 062 |
|  | 24 | $37 \quad 51.29$ |  | 47 | 0.305 | 75 | . 041 |
|  | 25 | $\begin{array}{ll}38 & 43.91\end{array}$ |  | 6991 |  | 03 | . 021 |
|  | 36 | $39 \quad 35.88$ |  |  |  | 31 | 40.000 |

TABLE X - Continued

Latitude, etc., for the North Boundary of each Section. Third System of Survey.

| Township | Section | Latitude L. | Log Sec L. | Log Tan L. | Quarter Section |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 169 |  | - 11 |  |  | chains |
|  | 1 | $\begin{array}{lll}63 & 40 & 28.50\end{array}$ | 0.35314 | $0.305 \quad 59$ | 39.979 |
|  | 12 | $41 \quad 20.48$ | 36 | 86 | . 959 |
|  | 13 | $\begin{array}{ll}42 & 13.10\end{array}$ | 58 | $0.306 \quad 14$ | . 938 |
|  | 24 | $43 \quad 05.07$ | 80 | 42 | . 917 |
|  | 25 | $43 \quad 57.69$ | 0.35403 | 69 | . 896 |
|  | 36 | $44 \quad 49.66$ | 25 | 97 | . 876 |
| 170 | 1 | $45 \quad 42.28$ | 47 | 0.30725 | . 855 |
|  | 12 | $46 \quad 34.25$ | 70 | 53 | . 834 |
|  | 13 | $47 \quad 26.87$ | 92 | 80 | . 813 |
|  | 24 | $\begin{array}{lll}48 & 18.85\end{array}$ | $0.355 \quad 14$ | 0.30808 | . 793 |
|  | 25 | $\begin{array}{lll}49 & 11.47\end{array}$ | 37 | 36 | . 772 |
|  | 36 | $50 \quad 03.44$ | 59 | 64 | (39.751 |
|  |  |  |  |  | $\{40.252$ |
| 171 | 1 | $50 \quad 56.06$ | 82 | 92 | . 231 |
|  | 12 | 5148.03 | 0.35604 | 0.30919 | . 210 |
|  | 13 | 5240.65 | 27 | 47 | . 189 |
|  | 24 | $\begin{array}{lll}53 & 32.62\end{array}$ | 49 | 75 | . 168 |
|  | 25 | $54 \quad 25.24$ | 72 | $0.310 \quad 03$ | . 147 |
|  | 36 | $55 \quad 17.21$ | 94 | 31 | . 126 |
| 172 | 1 | $\begin{array}{lll}56 & 09.83\end{array}$ | 0.35717 | 59 | . 105 |
|  | 12 | $57 \quad 01.80$ | 39 | 87 | . 084 |
|  | 13 | $57 \quad 54.42$ | 62 | 0.31115 | . 063 |
|  | 24 | - 5846.39 | 84 | 42 | . 042 |
|  | 25 | [ 5939.01 | $0.358 \quad 07$ | 71 | . 021 |
|  | 36 | $\begin{array}{lll}64 & 00 & 30.98\end{array}$ | 29 | 98 | 40.000 |
| 173 | 1 | $01 \quad 23.60$ | 52 | $0.312 \quad 27$ | 39.979 |
|  | 12 | $\begin{array}{lll}02 & 15.57\end{array}$ | 74 | 54 | . 958 |
|  | 13 | $03 \quad 08.18$ | 97 | 82 | . 937 |
|  | - 24 | $04 \quad 00.15$ | 0.35920 | 0.31310 | . 916 |
|  | 25 | $04 \quad 52.77$ | 42 | 38 | . 895 |
|  | 36 | $05 \quad 44.74$ | 65 | 66 | . 874 |
| 174 | 1 | $06 \quad 37.36$ | 88 | 94 | . 853 |
|  | 12 | $07 \quad 29.33$ | $0.360 \quad 10$ | 0.31422 | . 832 |
|  | 13 | $08 \quad 21.95$ | 33 | 51 | . 811 |
|  | 24 | 0913.92 | 56 | 78 | . 790 |
|  | 25 | $10 \quad 06.53$ | 79 | 0.31507 | . 768 |
|  | 36 | $10 \quad 58.50$ | 0.36101 | 35 | $\{39.747$ |
|  |  |  |  |  | (40.256 |

TABLE X - Continued

Latitude, etc., for the North Boundary of each Section.
Third System of Survey.

| Township | Section | Latitude L. | Log Sec | L. | Log Tan L. |  | Quarter Section |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 175 | 1 | $\begin{array}{lll} 64 & 11 & 51.12 \end{array}$ | 0.361 |  | 0.315 |  | chains$40.235$ |
|  |  |  |  |  |  |  |  |
|  | 12 | $12 \quad 43.09$ |  | 47 | 91 |  | . 213 |
|  | 13 | $13 \quad 35.70$ | 70 |  | 0.316 |  | . 192 |
|  | 24 | $14 \quad 27.67$ | 92 |  | 47 |  | . 171 |
|  | 25 | $15 \quad 20.29$ | 0.362 |  | 0.317 | 75 | . 149 |
|  | 36 | $16 \quad 12.26$ |  | 38 |  |  | . 128 |
| 176 | 1 | $17 \quad 04.87$ | 0.363 | 61 | 0.318 | 32 | . 107 |
|  | 12 | $17 \quad 56.84$ |  | 84 |  | 60 | . 085 |
|  | 13 | $18 \quad 49.46$ |  | 0730 |  | 88 | . 064 |
|  | 24 | $19 \quad 41.43$ |  |  |  | 16 | . 043 |
|  | 25 | $20 \quad 34.04$ |  | 53 |  | 44 | . 021 |
|  | 36 | $21 \quad 26.01$ |  | 75 |  | 72 | 40.100 |
| 177 | 1 | $22 \quad 18.63$ | 98 |  | 0.319 | 01 | 39.979 |
|  | 12 | $23 \quad 10.59$ | 0.364 |  |  | 29 | . 957 |
|  | 13 | $24 \quad 03.21$ |  | 44 | 57 |  | . 936 |
|  | 24 | $24 \quad 55.18$ | 67 |  | 85 |  | . 914 |
|  | 25 | $25 \quad 47.79$ | 90 |  | 0.320 | 14 | . 893 |
|  | 36 | $26 \quad 39.76$ | 0.365 | 13 |  | 42 | . 872 |
| 178 | 1 | $27 \quad 32.38$ | 0.366 | 36 |  |  | . 850 |
|  | 12 | $28 \quad 24.34$ |  | 59 | 7099 |  | . 829 |
|  | 13 | $29 \quad 16.96$ |  | 8306 | 0.321 | 99 27 | . 808 |
|  | 24 | $30 \quad 08.92$ |  |  |  | 55 | . 786 |
|  | 25 | 3101.54 |  | 29 |  | 84 | . 765 |
|  | 36 | 3153.50 |  | 52 | 0.322 | 12 | $\left\{\begin{array}{l}39.743 \\ 40.260\end{array}\right.$ |
|  |  |  |  |  |  |  |  |
| 179 | 1 | $32 \quad 46.12$ | 0.367 | 75 | 40 |  | . 238 |
|  | 12 | $33 \quad 38.09$ |  | 98 | 69 |  | . 217 |
|  | 13 | $34 \quad 30.70$ |  | 21 | 97$0.323 ~$ |  | . 195 |
|  | 24 | $35 \quad 22.67$ |  | 44 | 0.323 | $25$ | . 173 |
|  | 25 | $\begin{array}{lll}36 & 15.28\end{array}$ |  | 68 |  | 54 | . 152 |
|  | 36 | $37 \quad 07.25$ |  | 91 | 82 |  | . 130 |
| 180 | 1 | $37 \quad 59.86$ | 0.368 | 1437 | 0.324 | 11 | . 108 |
|  | 12 | $38 \quad 51.83$ |  |  |  | 39 | . 087 |
|  | 13 | 3944.44 |  | 60 | $\begin{array}{r} \\ 0.325 \\ \hline 96 \\ \\ \hline\end{array}$ |  | . 065 |
|  | 24 | $40 \quad 36.41$ | 0.369 | 84 |  |  | . 043 |
|  | 25 | $\begin{array}{lll}41 & 29.02\end{array}$ |  | $\begin{aligned} & 07 \\ & 30 \end{aligned}$ |  |  | . 022 |
|  | 36 | $42 \quad 20.98$ |  |  |  |  | 40.000 |

TABLE X - Continued
Latitude, etc. for the North Boundary of each Section.
Third System of Survey.


TABLE X - Continued

Latitude, etc., for the North Boundary of each Section. Third System of Survey.


TABLE X - Continued

Latitude, etc., for the North Boundary of each Section.
Third System of Survey.


TABLE X - Continued

Latitude, etc., for the North Boundary of each Section. Third System of Survey.


TABLE X - Continued

Latitude, etc., for the North Boundary of each Section.
Third System of Survey.

| Township | Section | Latitude L. | Log Sec L. | Log Tan L. | Quarter Section |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 205 |  | - 11 |  |  | chains |
|  | 1 | $\begin{array}{lll}66 & 48 & 42.18\end{array}$ | 0.40478 | $0.368 \quad 19$ | 40.976 |
|  | 12 | 4934.13 | 0.40503 | 49 | . 952 |
|  | 13 | $50 \quad 26.73$ | 29 | 80 | . 928 |
|  | 24 | $\begin{array}{lll}51 & 18.68\end{array}$ | 55 | $0.369 \quad 10$ | . 904 |
|  | 25 | $\begin{array}{lll}52 & 11.28\end{array}$ | 80 | 41 | . 880 |
|  | 36 | 5303.23 | 0.40606 | $71$ | . 856 |
| 206 | 1 | $\begin{array}{lll}53 & 55.82\end{array}$ | 32 | $0.370 \quad 02$ | . 832 |
|  | 12 | $54 \quad 47.77$ | 58 | 32 | . 808 |
|  | 13 | $\begin{array}{lll}55 & 40.37\end{array}$ | 84 | 63 | . 785 |
|  | 24 | 56 32.32 | $0.407 \quad 09$ | 93 | . 761 |
|  | 25 | $57 \quad 24.92$ | 35 | 0.37124 | . 737 |
|  | 36 | $58 \quad 16.87$ | 61 | 54 | $\{39.713$ |
|  |  |  |  |  | (40.292 |
| 207 | 1 | $59 \quad 09.47$ | 87 | 85 | . 267 |
|  | 12 | $67 \quad 0001.42$ | $\begin{array}{ll}0.408 & 13\end{array}$ | 0.37216 | . 243 |
|  | 13 | $00 \quad 54.01$ | 39 | 46 | . 219 |
|  | 24 | 0145.96 | 65 | 77 | . 195 |
|  | 25 | 0238.56 | 91 | 0.37308 | . 170 |
|  | 36 | 0330.51 | 0.40917 | 38 | . 146 |
| 208 | 1 | $04 \quad 23.11$ | 43 | 69 | . 122 |
|  | 12 | $05 \quad 15.05$ | 69 | $0.374 \quad 00$ | . 097 |
|  | 13 | 0607.65 | 95 | 30 | . 073 |
|  | 24 | $06 \quad 59.60$ | $0.410 \quad 21$ | 61 | . 049 |
|  | 25 | $07 \quad 52.20$ | $47$ | 92 | . 024 |
|  | 36 | $08 \quad 44.14$ | 73 | $0.375 \quad 22$ | 40.000 |
| 209 | 1 | $09 \quad 36.74$ | 99 | 53 | 39.976 |
|  | 12 | $10 \quad 28.69$ | 0.41125 | 84 | . 951 |
|  | 13 | $11 \quad 21.28$ | 52 | $0.376 \quad 15$ | . 927 |
|  | 24 | $12 \quad 13.23$ | $78$ | 46 | . 903 |
|  | 25 | $13 \quad 05.83$ | 0.41204 | $77$ | . 878 |
|  | 36 | $13 \quad 57.78$ | $30$ | $0.377 \quad 07$ | . 854 |
| 210 | 1 | $\begin{array}{ll}14 & 50.37\end{array}$ | 57 | 38 | . 830 |
|  | 12 | 1542.32 | 83 | 69 | . 805 |
|  | 13 | $16 \quad 34.92$ | 0.41309 | 0.37800 | . 781 |
|  | 24 | $17 \quad 26.86$ | 35 | 31 | . 757 |
|  | $25$ | $18 \quad 19.46$ | $62$ | 62 | $.732$ |
|  | 36 | 1911.40 | 88 | 93 | $\left\{\begin{array}{l}39.708 \\ 40.297\end{array}\right.$ |

TABLE X - Continued
Latitude, etc., for the North Boundary of each Section.
Third System of Survey.


TABLE X - Continued

Latitude, etc., for the North Boundary of each Section. Third System of Survey.


TABLEX - Continued
Latitude, etc., for the North Boundary of each Section.
Third System of Survey.

| Township | Section | Latitude L. | Log Sec L. | Log Tan L. | Quarter <br> Section |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 223 |  | - 11 |  |  | chains |
|  | 1 | $\begin{array}{lll}68 & 22 & 47.27\end{array}$ | 0.43362 | 0.40194 | 40.287 |
|  | 12 | $\begin{array}{lll}23 & 39.21\end{array}$ | 89 | 0.40226 | . 261 |
|  | 13 | $24 \quad 31.80$ | $0.434 \quad 17$ | 58 | . 235 |
|  | 24 | $\begin{array}{lll}25 & 23.74\end{array}$ | 45 | 90 | . 209 |
|  | 25 | $\begin{array}{ll}26 & 16.33\end{array}$ | 73 | 0.40322 | . 182 |
|  | 36 | $27 \quad 08.27$ | 0.43501 | 54 | . 156 |
| 224 | 1 | $28 \quad 00.86$ | 29 | 87 | . 130 |
|  | 12 | $28 \quad 52.80$ | 57 | 0.40419 | . 104 |
|  | 13 | $\begin{array}{ll}29 & 45.39\end{array}$ | 85 | 51 | . 078 |
|  | 24 | $\begin{array}{ll}30 & 37.32\end{array}$ | 0.43612 | 83 | . 052 |
|  | 25 | $31 \quad 29.91$ | 41 | 0.40516 | . 026 |
|  | 36 | $32 \quad 21.85$ | 68 | 48 | 40.000 |
| 225 | 1 | 3314.44 | 97 | 80 | 39.974 |
|  | 12 | 3406.38 | 0.43724 | $0.406 \quad 13$ | . 948 |
|  | 13 | $\begin{array}{lll}34 & 58.97\end{array}$ | 53 | 45 | . 922 |
|  | 24 | $35 \quad 50.90$ | 80 | 77 | . 896 |
|  | 25 | $36 \quad 43.49$ | 0.43809 | 0.40710 | . 869 |
|  | 36 | $37 \quad 35.43$ | 37 | 42 | . 843 |
| 226 | 1 | $\begin{array}{lll}38 & 28.02\end{array}$ | 65 | 75 | . 817 |
|  | 12 | $\begin{array}{lll}39 & 19.95\end{array}$ | 93 | $0.408 \quad 07$ | . 791 |
|  | 13 | $40 \quad 12.54$ | 0.43921 | 40 | . 765 |
|  | 24 | 4104.48 | 49 | 72 | . 739 |
|  | 25 | $41 \quad 57.07$ | 78 | 0.40905 | . 713 |
|  | 36 | $42 \quad 49.00$ | $0.440 \quad 06$ | 37 | (39.687 |
|  |  |  |  |  | (40.318 |
| 227 | 1 | $43 \quad 41.59$ | 34 | 70 | .292 |
|  | 12 | 4433.53 | 62 | $0.410 \quad 02$ | . 265 |
|  | 13 | $45 \quad 26.11$ | 91 | 35 | . 239 |
|  | 24 | $\begin{array}{ll}46 & 18.05\end{array}$ | 0.44119 | 67 | .212 |
|  | 25 | $47 \quad 10.64$ | 47 | 0.41100 | . 186 |
|  | 36 | $48 \quad 02.58$ | 76 | 32 | . 159 |
| 228 | 1 | $48 \quad 55.16$ | $0.442 \quad 04$ | 65 | . 133 |
|  | 12 | $49 \quad 47.10$ | 32 | 98 | . 106 |
|  | 13 | $\begin{array}{lll}50 & 39.68\end{array}$ | 61 | 0.41231 | . 080 |
|  | 24 | $51 \quad 31.62$ | 89 | 63 | . 053 |
|  | 25 | $52 \quad 24.21$ | 0.44318 | 96 | . 026 |
|  | 36 | 5316.14 | 46 | $0.413 \quad 29$ | 40.000 |

TABLE X - Continued

Latitude, etc., for the North Boundary of each Section. Third System of Survey.


TABLE X - Continued
Latitude, etc., for the North Boundary of each Section.
Third System of Survey.

| Township | Section | Latitude L. | Log Sec L. |  | Log Tan L. |  | Quarter Section |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 235 |  | - ' " | 0.454 |  | 0.425 | 53 | chains <br> 40.303 |
|  | 1 | $\begin{array}{llll}69 & 25 \quad 30.07\end{array}$ |  |  |  |  |  |
|  | 12 | $26 \quad 22.00$ |  | 45 |  | 86 | . 275 |
|  | 13 | $\begin{array}{ll}27 & 14.58\end{array}$ |  | 74 | 0.426 |  | . 248 |
|  | 24 | $28 \quad 06.51$ | 0.455 | 04 |  | 53 | . 220 |
|  | 25 | $28 \quad 59.10$ |  | 33 | 0.427 | 87 | . 193 |
|  | 36 | 2951.03 |  | 62 |  | 20 | . 165 |
| 236 | 1 | $\begin{array}{lll}30 & 43.61\end{array}$ | 0.456 | 92 | 5488 |  | $\begin{aligned} & .138 \\ & .110 \end{aligned}$ |
|  | 12 | $31 \quad 35.54$ |  | 21 |  |  |  |  |
|  | 13 | $32 \quad 28.13$ |  | 51 | 0.428 | 21 | . 083 |
|  | 24 | 3320.06 | 0.457 | 80 | 5589$0.429 \quad 22$ |  | . 055 |
|  | 25 | $\begin{array}{lll}34 & 12.64\end{array}$ |  | 10 |  |  | . 027 |
|  | 36 | 3504.57 |  | 39 |  |  | 40.000 |
| 237 | 1 | $35 \quad 57.15$ | 0.458 | 69 | 5689 |  | 39.972 |
|  | 12 | 3649.08 |  | 99 |  |  | . 945 |
|  | 13 | $37 \quad 41.67$ |  | 28 | 0.430 |  |  |
|  | 24 | $38 \quad 33.60$ |  | 58 |  | 23 57 | $.890$ |
|  | 25 | $\begin{array}{ll}39 & 26.18\end{array}$ | 0.459 | 88 |   <br> 0.431 91 <br> 04  |  | $\begin{aligned} & .862 \\ & .835 \end{aligned}$ |
|  | 36 | $40 \quad 18.11$ |  | 17 |  |  |  |  |
| 238 |  | $41 \quad 10.69$ | 0.460 | 47 | 5892 |  | . 807 |
|  | 12 | $42 \quad 02.62$ |  | 77 |  |  | . 780 |
|  | 13 | 4255.20 |  | 07 | 0.432 | 26 | . 752 |
|  | 24 | $43 \quad 47.13$ |  | 36 |  | 60 | . 724 |
|  | 25 | $44 \quad 39.72$ |  | 66 | 0.433 | $\begin{aligned} & 94 \\ & 27 \end{aligned}$ | $\begin{array}{r} .697 \\ 39.669 \\ 40.337 \end{array}$ |
|  | 36 | $45 \quad 31.65$ |  | 96 |  |  |  |
|  |  |  |  |  |  |  |  |
| 239 |  | $46 \quad 24.23$ | 0.461 |  | 6195 |  | . 309 |
|  | 12 | $47 \quad 16.16$ |  | 55 |  |  | . 280 |
|  | 13 | $48 \quad 08.74$ | 0.462 | 86 | 0.434 | 29 | . 252 |
|  | 24 | $49 \quad 00.67$ |  | 15 |  | 63 | . 224 |
|  | 25 | 4953.25 |  | 45 | 0.435 | 97 | . 196 |
|  | 36 | $50 \quad 45.18$ |  | 75 |  | 31 | . 168 |
| 240 | 1 | 5137.76 | 0.463 | 05 |  | 65 | .140.112 |
|  | 12 | 5229.69 |  | 35 | 99 |  |  |
|  | 13 | $53 \quad 22.27$ |  | 65 | 0.436 | 33 | . 084 |
|  | 24 | $\begin{array}{ll}54 & 14.20\end{array}$ |  | 95 |  | 67 | $\begin{aligned} & .056 \\ & .028 \end{aligned}$ |
|  | 25 | $\begin{array}{lll}55 & 06.78\end{array}$ | 0.4642 | 26 | 0.437 | 02 |  |
|  | 36 | $55 \quad 58.71$ |  | 55 |  | 36 | 40.000 |

TABLE X - Concluded
Latitude, etc., for the North Boundary of each Section. Third System of Survey.

| Township | Section | Latitude L. | Log Sec L. |  | Log Tan L. |  | Quarter <br> Section |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 241 |  | - ' |  |  |  |  | chains |
|  | 1 | $69 \quad 5651.29$ | 0.464 | 86 | 0.437 | 70 | 39.972 |
|  | 12 | 5743.22 | 0.465 | 16 | 0.438 | 04 | . 944 |
|  | 13 | 5835.80 |  | 46 |  | 38 | . 916 |
|  | 24 | 5927.73 |  | 76 |  | 72 | . 888 |
|  | 25 | $\begin{array}{lll}70 & 00 & 20.31\end{array}$ | 0.466 | 07 | 0.439 | 07 | . 860 |
|  | 36 | $01 \quad 12.24$ |  | 37 |  | 41 | . 832 |
| 242 | 1 | 0204.81 |  | 67 | 0.440 | 75 | . 803 |
|  | 12 | 0256.74 | 0.467 | 97 |  | 09 | . 775 |
|  | 13 | 0349.32 |  | 28 |  | 44 | . 747 |
|  | 24 | 0441.25 |  | 58 |  | 78 | . 719 |
|  | 25 | $05 \quad 33.83$ |  | 88 | 0.441 | 13 | . 691 |
|  | 36 | $06 \quad 25.76$ | 0.468 | 19 |  | 47 | $(39.663$ |
|  |  |  |  |  |  |  | (40.343 |
| 243 | 1 | $07 \quad 18.34$ |  | 49 |  | 81 | . 314 |
|  | 12 | $08 \quad 10.27$ | 0.469 | 80 | 0.442 |  | . 286 |
|  | 13 | 0902.84 |  | 10 |  | 50 | . 257 |
|  | 24 | 0954.77 |  | 40 |  | 84 | . 229 |
|  | 25 | 1047.35 |  | 71 | 0.443 |  | . 200 |
|  | 36 | 1139.28 | 0.470 | 02 |  | 53 | . 172 |
| 244 | 1 | 1231.86 | 0.471 | 32 | 0.444 | 88 | . 143 |
|  | 12 | $13 \quad 23.78$ |  | 63 |  | 22 | . 114 |
|  | 13 | $14 \quad 16.36$ |  | 93 |  | 57 | . 086 |
|  | 24 | $15 \quad 08.29$ |  | 24 | 0.445 | 92 | . 057 |
|  | 25 | $16 \quad 00.87$ |  | 55 |  | 26 | . 029 |
|  | 36 | 1652.80 |  | 85 |  | 61 | 40.000 |

TABLE XI

Showing the difference of Latitude between Township Corners and Section and Quarter Section Posts on a Township Chord.

| Number of Line | dL for $1 / 2 \mathrm{Sec}$. from Corner | dL <br> for <br> 1 Sec . <br> from <br> Corner | dL for <br> $11 / 2$ Secs. from Corner | dL for 2 Secs. from Corner | dL for $21 / 2$ Secs. from Corner | $\begin{gathered} \text { dL } \\ \text { for } \\ 3 \text { Secs. } \\ \text { from } \\ \text { Corner } \end{gathered}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| lst Base | 0.02 | 0.04 | 9.05 | 0.06 | 0.07 | 0.07 |
|  | lks. | lks. | lks. | lks. | lks. | lks. |
| do | 3.2 | 5.9 | 7.9 | 9.4 | 10.3 | 10.6 |
| 1lth Base | 0.02 | 0.04 | 0.06 | 0.07 | 0.08 | 0.08 |
|  | lks. | 1ks. | lks. | lks. | 1 ks . | lks. |
| do | 3.7 | 6.7 | 9.0 | 10.6 | 11.6 | 12.0 |
| 21 st Base | 0.103 | 0.05 | 0.07 | 0.08 | 0.09 | 0.09 |
|  | lks. | lks. | 1 ks . | lks. | lks. | lks. |
| do | 4.2 | 7.6 | 10.2 | 12.1 | 13.2 | 13.6 |
| 31 st Base | 0.03 | 0.06 | 0.08 | 0.09 | 0.10 | 0.10 |
|  | lks. | lks. | lks. | lks. | lks. | lks. |
| do | 4.8 | 8.7 | 11.7 | 13.9 | 15.2 | 15.6 |
| 41 st Base | 0.04 | 0.06 | 0.09 | 0.10 | 0.111 | 0.12 |
|  | lks. | lks. | lks. | lks. | lks. | lks. |
| do | 5.5 | 10.0 | 13.5 | 16.0 | 17.5 | 18.0 |
| 5lst Base | 0.04 | 0.08 | 0.10 | ${ }^{11} 12$ | 0.13 | 0.14 |
|  | lks. | lks. | lks. | lks. | lks. | lks. |
| do | 6.4 | 11.7 | 15.8 | 18.7 | 20.5 | 21.1 |
| $61 s t$ Base | 0.05 | 0.09 | 0.12 | 0.14 | 0.16 | 0.16 |
|  | lks. | lks. | lks. | lks. | lks. | lks. |
| do | 7.7 | 14.0 | 18.9 | 22.4 | 24.5 | 25.2 |

TABLE XII

For Converting Logarithmic Tangents of Small Arcs into
Logarithms of Seconds of Arc

| Log Tan | Log $T$ | Log Tan | $\log T$ | Log Tan | Log T |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | 8.49305 |  | 8.64361 |  |
| 7.92263 | 5.31442 |  | 5.31428 | 65116 |  |
|  | 41 |  | 27 |  | 13 |
| 8.07156 |  | . 52200 |  | .65849 |  |
|  | 40 |  | 26 |  | 12 |
| . 15924 |  | .53516 | . | .66562 |  |
|  | 39 |  | 25 |  | 11 |
| .22142 |  | .54753 |  | .67253 |  |
|  | 38 |  | 24 |  | 10 |
| .26973 |  | .55938 |  | .67921 |  |
|  | 37 |  | 23 |  | 09 |
| . 30930 |  | .57046 |  | .68570 |  |
|  | 36 |  | 22 |  | 08 |
| . 34270 |  | . 58099 |  | .69201 |  |
|  | 35 |  | 21 |  | 07 |
| .37167 |  | .59105 |  | .69814 |  |
|  | 34 |  | 20 |  | 06 |
| .39713 |  | .60073 |  | . 70410 |  |
|  | 33 |  | 19 |  | 05 |
| . 41999 |  | .61009 |  | .70991 |  |
|  | 32 |  | 18 |  | 04 |
| .44072 |  | .61872 |  | .71555 |  |
|  | 31 |  | 17 |  | 03 |
| .45955 |  | .62745 |  | .72104 |  |
|  | 30 |  | 16 |  | 02 |
| .47697 |  | .63567 |  | .72639 |  |
|  | 29 |  | 15 |  | 01 |

TABLE Xlll
$\log \frac{1}{1-m}$
m positive
that is, when ties between $0^{\mathrm{h}}$ and $6^{\mathrm{h}}$, or $18^{\mathrm{h}}$ and $24^{\mathrm{h}}$.

| Log m | 0 |  | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 5. | 0.00 | 000 | 001 | 001 | 001 | 001 | 001 | 002 | 002 | 003 | 003 |
| 6.0 |  | 004 | 004 | 005 | 005 | 005 | 005 | 005 | 005 | 005 | 005 |
| 1 |  | 006 | 006 | 006 | 006 | 006 | 006 | 006 | 006 | 007 | 007 |
| 2 |  | 007 | 007 | 007 | 007 | 008 | 008 | 008 | 008 | 008 | 009 |
| 3 |  | 009 | 009 | 009 | 009 | 010 | 010 | 010 | 010 | 010 | 011 |
| 4 |  | 011 | 011 | 011 | 012 | 012 | 012 | 013 | 013 | 013 | 013 |
| 5 |  | 014 | 014 | 014 | 015 | 015 | 015 | 016 | 016 | 017 | 017 |
| 6 |  | 017 | 018 | 018 | 019 | 019 | 019 | 020 | 020 | 021 | 021 |
| 7 |  | 022 | 022 | 023 | 023 | 024 | 024 | 025 | 026 | 026 | 027 |
| 8 |  | 027 | 028 | 029 | 029 | 030 | 031 | 032 | 032 | 033 | 034 |
| 9 |  | 035 | 035 | 036 | 037 | 038 | 039 | 040 | 041 | 042 | 043 |
| 7.0 |  | 044 | 045 | 046 | 047 | 048 | 049 | 050 | 051 | 052 | 054 |
| 1 |  | 055 | 056 | 057 | 059 | 060 | 061 | 063 | 064 | 066 | 067 |
| 2 |  | 069 | 071 | 072 | 074 | 076 | 077 | 079 | 081 | 083 | 085 |
| 3 |  | 087 | 089 | 091 | 093 | 095 | 097 | 100 | 102 | 104 | 107 |
| 4 |  | 109 | 112 | 114 | 117 | 120 | 123 | 125 | 128 | 131 | 134 |
| 5 |  | 138 | 141 | 144 | 147 | 151 | 154 | 158 | 162 | 165 | 169 |
| 6 |  | 173 | 177 | 181 | 186 | 190 | 194 | 199 | 204 | 208 | 213 |
| 7 |  | 218 | 223 | 229 | 234 | 239 | 245 | 251 | 257 | 263 | 269 |
| 8 |  | 275 | 281 | 288 | 295 | 302 | 309 | 316 | 323 | 331 | 338 |
| 9 |  | 346 | 355 | 363 | 371 | 380 | 389 | 398 | 407 | 417 | 427 |
| 8.00 |  | 437 | 438 | 439 | 440 | 441 | 442 | 443 | 444 | 445 | 446 |
| 01 |  | 447 | 448 | 449 | 450 | 451 | 452 | 453 | 454 | 455 | 456 |
| 02 |  | 457 | 458 | 459 | 460 | 461 | 463 | 464 | 465 | 466 | 467 |
| 03 |  | 468 | 469 | 470 | 471 | 472 | 473 | 474 | 476 | 477 | 478 |
| 04 |  | 479 | 480 | 481 | 482 | 483 | 484 | 486 | 487 | 488 | 489 |
| 05 |  | 490 | 491 | 492 | 494 | 495 | 496 | 497 | 498 | 499 | 500 |
| 06 |  | 502 | 503 | 504 | 505 | 506 | 507 | 509 | 510 | 511 | 512 |
| 07 |  | 513 | 515 | 516 | 517 | 518 | 519 | 521 | 522 | 5ここ | 524 |
| 08 |  | 525 | 527 | 528 | 529 | 530 | 531 | 533 | 534 | 535 | 536 |
| 09 |  | 538 | 539 | 540 | 541 | 543 | 544 | 545 | 546 | 548 | 549 |
| 8.10 | 0.00 | 550 | 552 | 553 | 554 | 555 | 557 | 558 | 559 | 561 | 562 |

TABLE XIII. --Continued.
$\log \frac{1}{1-m}$
m negative
that is, when $t$ lies between $6^{\mathrm{h}}$ and $18^{\mathrm{h}}$.

| Log m | 0 |  | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 5. n | $\begin{array}{r} 10.00 \\ 9.99 \end{array}$ | 000 | 999 | 999 | 999 | 999 | 999 | 998 | 998 | 997 | 997 |
| 6.0 n | 9.99 | 996 | 996 | 996 | 995 | 995 | 995 | 995 | 995 | 995 | 995 |
| 1 n |  | 995 | 994 | 994 | 994 | 994 | 994 | 994 | 994 | 993 | 993 |
| 2 n |  | 993 | 993 | 993 | 993 | 993 | 992 | 992 | 992 | 992 | 992 |
| 3 n |  | 991 | 991 | 991 | 991 | 991 | 990 | 990 | 990 | 990 | 989 |
| 4 n |  | 989 | 989 | 989 | 988 | 988 | 988 | 988 | 987 | 987 | 987 |
| 5 n |  | 986 | 986 | 986 | 985 | 985 | 985 | 984 | 984 | 984 | 983 |
| 6 n |  | 983 | 982 | 982 | 982 | 981 | 981 | 980 | 980 | 979 | 979 |
| 7 n |  | 978 | 978 | 977 | 977 | 976 | 976 | 975 | 974 | 974 | 973 |
| 8 n |  | 973 | 972 | 971 | 971 | 970 | 969 | 969 | 968 | 967 | 966 |
| 9 n |  | 966 | 965 | 964 | 963 | 962 | 961 | 960 | 960 | 959 | 958 |
| 7.0 n |  | 957 | 956 | 955 | 954 | 952 | 951 | 950 | 949 | 948 | 947 |
| 1 n |  | 945 | 944 | 943 | 942 | 940 | 939 | 937 | 936 | 934 | 933 |
| 2 n |  | 931 | 930 | 928 | 926 | 925 | 923 | 921 | 919 | 917 | 915 |
| 3 n |  | 913 | 911 | 909 | 907 | 905 | 903 | 901 | 898 | 896 | 894 |
| 4 n |  | 891 | 889 | 886 | 883 | 881 | 878 | 875 | 872 | 869 | 866 |
| 5 n |  | 863 | 860 | 856 | 853 | 850 | 846 | 843 | 839 | 835 | 831 |
| 6 n |  | 827 | 823 | 819 | 815 | 811 | 806 | 802 | 797 | 793 | 788 |
| 7 n |  | 783 | 778 | 773 | 767 | 762 | 757 | 751 | 745 | 739 | 733 |
| 8 n |  | 727 | 721 | 714 | 707 | 701 | 694 | 687 | 679 | 672 | 664 |
| 9 n |  | 656 | 648 | 640 | 632 | 623 | 615 | 606 | 597 | 587 | 578 |
| 8.00 n |  | 568 | 567 | 566 | 565 | 564 | 563 | 562 | 561 | 560 | 559 |
| 01 n |  | 558 | 557 | 556 | 555 | 554 | 553 | 552 | 551 | 550 | 549 |
| 02n |  | 548 | 547 | 546 | 545 | 543 | 542 | 541 | 540 | 539 | 538 |
| 03n |  | 537 | 536 | 535 | 534 | 533 | 532 | 531 | 530 | 529 | 528 |
| 04n |  | 526 | 525 | 524 | 523 | 522 | 521 | 520 | 519 | 518 | 517 |
| 05n |  | 515 | 514 | 513 | 512 | 511 | 510 | 509 | 508 | 507 | 505 |
| $06 n$ |  | 504 | 503 | 502 | 501 | 500 | 499 | 497 | 496 | 495 | 494 |
| 07n |  | 493 | 492 | 490 | 489 | 488 | 487 | 486 | 485 | 483 | 482 |
| 08n |  | 481 | 480 | 479 | 477 | 476 | 475 | 474 | 473 | 471 | 470 |
| 09n |  | 469 | 468 | 467 | 465 | 464 | 463 | 462 | 460 | 459 | 458 |
| 8.10 n | 9.99 | 457 | 455 | 454 | 453 | 452 | 450 | 449 | 448 | 447 | 445 |

TABLE XIII.--Continued.
$\log \frac{1}{1-m}$
m positive
that is, when ties between $0^{\mathrm{h}}$ and $6^{\mathrm{h}}$, or $18^{\mathrm{h}}$ and $24^{\mathrm{h}}$.

| Log m | 0 |  | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 8.10 | 0.00 | 550 | 552 | 553 | 554 | 555 | 557 | 558 | 559 | 561 | 562 |
| 11 |  | 563 | 564 | 566 | 567 | 568 | 570 | 571 | 572 | 574 | 575 |
| 12 |  | 576 | 578 | 579 | 580 | 582 | 583 | 584 | 586 | 587 | 589 |
| 13 |  | 590 | 591 | 593 | 594 | 595 | 597 | 598 | 600 | 601 | 602 |
| 14 |  | 604 | 605 | 607 | 608 | 609 | 611 | 612 | 614 | 615 | 616 |
| 15 |  | 618 | 619 | 621 | 622 | 624 | 625 | 627 | 628 | 629 | 631 |
| 16 |  | 632 | 634 | 635 | 637 | 638 | 640 | 641 | 643 | 644 | 646 |
| 17 |  | 647 | 649 | 650 | 652 | 653 | 655 | 656 | 658 | 659 | 661 |
| 18 |  | 662 | 664 | 665 | 667 | 669 | 670 | 672 | 673 | 675 | 676 |
| 19 |  | 678 | 680 | 681 | 683 | 684 | 686 | 687 | 689 | 691 | 692 |
| 8.20 |  | 694 | 695 | 697 | 699 | 700 | 702 | 704 | 705 | 707 | 709 |
| 21 |  | 710 | 712 | 713 | 715 | 717 | 718 | 720 | 722 | 723 | 725 |
| 22 |  | 727 | 729 | 730 | 732 | 734 | 735 | 737 | 739 | 740 | 742 |
| 23 |  | 744 | 746 | 747 | 749 | 751 | 753 | 754 | 756 | 758 | 760 |
| 24 |  | 761 | 763 | 765 | 767 | 769 | 770 | 772 | 774 | 776 | 777 |
| 25 |  | 779 | 781 | 783 | 785 | 787 | 788 | 790 | 792 | 794 | 796 |
| 26 |  | 798 | 799 | 801 | 803 | 805 | 807 | 809 | 811 | 813 | 814 |
| 27 |  | 816 | 818 | 820 | 822 | 824 | 826 | 828 | 830 | 832 | 834 |
| 28 |  | 836 | 838 | 839 | 841 | 843 | 845 | 847 | 849 | 851 | 853 |
| 29 |  | 855 | 857 | 859 | 861 | 863 | 865 | 867 | 869 | 871 | 873 |
| 8.30 |  | 875 | 877 | 879 | 881 | 884 | 886 | 888 | 890 | 892 | 894 |
| 31 |  | 896 | 898 | 900 | 902 | 904 | 906 | 909 | 911 | 913 | 915 |
| 32 |  | 917 | 919 | 921 | 923 | 926 | 928 | 930 | 932 | 934 | 936 |
| 33 |  | 939 | 941 | 943 | 945 | 947 | 950 | 952 | 954 | 956 | 958 |
| 34 |  | 961 | 963 | 965 | 967 | 970 | 972 | 974 | 977 | 979 | 981 |
| 35 |  | 983 | 986 | 988 | 990 | 993 | 995 | 997 | 000 | 002 | 004 |
| 36 | 0.01 | 007 | 009 | 011 | 014 | 016 | 018 | 021 | 023 | 025 | 028 |
| 37 |  | 030 | 033 | 035 | 037 | 040 | 042 | 045 | 047 | 050 | 052 |
| 38 |  | 055 | 057 | 059 | 062 | 064 | 067 | 069 | 072 | 074 | 077 |
| 39 |  | 079 | 082 | 084 | 087 | 090 | 092 | 095 | 097 | 100 | 102 |
| 8.40 | 0.01 | 105 | 107 | 110 | 113 | 115 | 118 | 120 | 123 | 126 | 128 |

TABLE XIII. --Continued.
$\log \frac{1}{1-m}$
$m$ negative
that is, when $t$ lies between $6^{\mathrm{h}}$ and $18^{\mathrm{h}}$.

| Log m | 0 |  | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 8.10n | 9.99 | 457 | 455 | 454 | 453 | 452 | 450 | 449 | 448 | 447 | 445 |
| $11 n$ |  | 444 | 443 | 442 | 440 | 439 | 438 | 436 | 435 | 434 | 433 |
| 12 n |  | 431 | 430 | 429 | 427 | 426 | 425 | 423 | 422 | 421 | 419 |
| $13 n$ |  | 418 | 417 | 415 | 414 | 413 | 411 | 410 | 409 | 407 | 406 |
| $14 n$ |  | 405 | 403 | 402 | 401 | 399 | 398 | 396 | 395 | 394 | 392 |
| $15 n$ |  | 391 | 389 | 388 | 387 | 385 | 384 | 382 | 381 | 380 | 378 |
| 16 n |  | 377 | 375 | 374 | 373 | 371 | 370 | 368 | 367 | 365 | 364 |
| 17 n |  | 362 | 361 | 359 | 358 | 357 | 355 | 354 | 352 | 351 | 349 |
| 18 n |  | 348 | 346 | 345 | 343 | 342 | 340 | 339 | 337 | 336 | 334 |
| $19 n$ |  | 333 | 331 | 330 | 328 | 326 | 325 | 323 | 322 | 320 | 319 |
| 8.20n |  | 317 | 316 | 314 | 312 | 311 | 309 | 308 | 306 | 305 | 303 |
| $21 n$ |  | 301 | 300 | 298 | 297 | 295 | 293 | 292 | 290 | 288 | 287 |
| 22 n |  | 285 | 284 | 282 | 280 | 279 | 277 | 275 | 274 | 272 | 270 |
| 23n |  | 269 | 267 | 265 | 264 | 262 | 260 | 259 | 257 | 255 | 254 |
| $24 n$ |  | 252 | 250 | 248 | 247 | 245 | 243 | 241 | 240 | 238 | 236 |
| 25n |  | 235 | 233 | 231 | 229 | 228 | 226 | 224 | 222 | 220 | 219 |
| 26 n |  | 217 | 215 | 213 | 211 | 210 | 208 | 206 | 204 | 202 | 201 |
| 27 n |  | 199 | 197 | 195 | 193 | 191 | 190 | 188 | 186 | 184 | 182 |
| 28 n |  | 180 | 178 | 177 | 175 | 173 | 171 | 169 | 167 | 165 | 163 |
| $29 n$ |  | 161 | 159 | 158 | 156 | 154 | 152 | 150 | 148 | 146 | 144 |
| $8.30 n$ |  | 142 | 140 | 138 | 136 | 134 | 132 | 130 | 128 | 126 | 124 |
| 31 n |  | 122 | 120 | 118 | 116 | 114 | 112 | 110 | 108 | 106 | 104 |
| 32 n |  | 102 | 100 | 098 | 096 | 094 | 092 | 090 | 088 | 086 | 083 |
| $33 n$ |  | 081 | 079 | 077 | 075 | 073 | 071 | 069 | 066 | 064 | 062 |
| $34 n$ |  | 060 | 058 | 056 | 054 | 052 | 049 | 047 | 045 | 043 | 041 |
| 35n |  | 039 | 036 | 034 | 032 | 030 | 027 | 025 | 023 | 021 | 019 |
| 36 n |  | 016 | 014 | 012 | 010 | 007 | 005 | 003 | 001 | 998 | 996 |
| 37 n | 9.98 | 994 | 991 | 989 | 987 | 985 | 982 | 980 | 978 | 975 | 973 |
| 38 n |  | 971 | 968 | 966 | 963 | 961 | 959 | 956 | 954 | 952 | 949 |
| $39 n$ |  | 947 | 944 | 942 | 940 | 937 | 935 | 932 | 930 | 928 | 925 |
| 8.40 n | 9.98 | 923 | 920 | 918 | 915 | 913 | 910 | 908 | 905 | 903 | 900 |

TABLE XIIl. --Continued.
$\log \frac{1}{1-\mathrm{m}}$
m positive
that is, when ties between $0^{\mathrm{h}}$ and $6^{\mathrm{h}}$, or $18^{\mathrm{h}}$ and $24^{\mathrm{h}}$.

| Log m | 0 |  | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 8.40 | 0.01 | 105 | 107 | 110 | 113 | 115 | 118 | 120 | 123 | 126 | 128 |
| 41 |  | 131 | 134 | 136 | 139 | 142 | 144 | 147 | 150 | 152 | 155 |
| 42 |  | 158 | 160 | 163 | 166 | 169 | 171 | 174 | 177 | 179 | 182 |
| 43 |  | 185 | 188 | 191 | 193 | 196 | 199 | 202 | 205 | 207 | 210 |
| 44 |  | 213 | 216 | 219 | 222 | 224 | 227 | 230 | 233 | 236 | 239 |
| 45 |  | 242 | 245 | 247 | 250 | 253 | 256 | 259 | 262 | 265 | 268 |
| 46 |  | 271 | 274 | 277 | 280 | 283 | 286 | 289 | 292 | 295 | 298 |
| 47 |  | 301 | 304 | 307 | 310 | 313 | 316 | 319 | 323 | 326 | 329 |
| 48 |  | 332 | 335 | 338 | 341 | 344 | 347 | 351 | 354 | 357 | 360 |
| 49 |  | 363 | 367 | 370 | 373 | 376 | 379 | 383 | 386 | 389 | 392 |
| 8.50 |  | 396 | 399 | 402 | 405 | 409 | 412 | 415 | 419 | 422 | 425 |
| 51 |  | 429 | 432 | 435 | 439 | 442 | 445 | 449 | 452 | 456 | 459 |
| 52 |  | 462 | 466 | 469 | 473 | 476 | 480 | 483 | 487 | 490 | 494 |
| 53 |  | 497 | 501 | 504 | 508 | 511 | 515 | 518 | 522 | 525 | 529 |
| 54 |  | 533 | 536 | 540 | 543 | 547 | 551 | 554 | 558 | 562 | 565 |
| 55 |  | 569 | 573 | 576 | 580 | 584 | 587 | 591 | 595 | 599 | 602 |
| 56 |  | 606 | 610 | 614 | 618 | 621 | 625 | 629 | 633 | 637 | 640 |
| 57 |  | 644 | 648 | 652 | 656 | 660 | 664 | 668 | 672 | 676 | 679 |
| 58 |  | 683 | 687 | 691 | 695 | 699 | 703 | 707 | 711 | 715 | 719 |
| 59 |  | 723 | 727 | 732 | 736 | 740 | 744 | 748 | 752 | 756 | 760 |
| 8.60 |  | 764 | 768 | 773 | 777 | 781 | 785 | 789 | 794 | 798 | 802 |
| 61 |  | 806 | 811 | 815 | 819 | 823 | 828 | 832 | 836 | 841 | 845 |
| 62 |  | 849 | 854 | 858 | 862 | 867 | 871 | 876 | 880 | 884 | 889 |
| 63 |  | 893 | 898 | 902 | 907 | 911 | 916 | 920 | 925 | 929 | 934 |
| 64 |  | 938 | 943 | 948 | 952 | 957 | 961 | 966 | 971 | 975 | 980 |
| 65 |  | 985 | 989 | 994 | 999 | 003 | 008 | 013 | 018 | 022 | 027 |
| 66 | 0.02 | 032 | 037 | 042 | 046 | 051 | 056 | 061 | 066 | 071 | 075 |
| 67 |  | 080 | 085 | 090 | 095 | 100 | 105 | 110 | 115 | 120 | 125 |
| 68 |  | 130 | 135 | 140 | 145 | 150 | 155 | 160 | 166 | 171 | 176 |
| 69 |  | 181 | 186 | 191 | 196 | 202 | 207 | 212 | 217 | 223 | 228 |
| 8.70 | 0.02 | 233 | 238 | 244 | 249 | 254 | 260 | 265 | 270 | 276 | 281 |

TABLE XIII. --Concluded.
$\log \frac{1}{1-m}$
$m$ negative
that is, when $t$ lies between $6^{h}$ and $18^{h}$.

| Log m | 0 |  | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $8.40 n$ | 9.98 | 923 | 920 | 918 | 915 | 913 | 910 | ソ08 | 905 | 903 | 900 |
| $41 n$ |  | 898 | 895 | 893 | 890 | 888 | 885 | 883 | 880 | 878 | 875 |
| 42 n |  | 873 | 870 | 867 | 865 | 862 | 860 | 857 | 854 | 852 | 849 |
| 43n |  | 847 | 844 | 841 | 839 | 836 | 833 | 831 | 828 | 825 | 823 |
| 44 n |  | 820 | 817 | 815 | 812 | 809 | 807 | 804 | 801 | 798 | 796 |
| $45 n$ |  | 793 | 790 | 787 | 785 | 782 | 779 | 776 | 774 | 771 | 768 |
| 46 n |  | 765 | 762 | 760 | 757 | 754 | 751 | 748 | 745 | 743 | 740 |
| 47 n |  | 737 | 734 | 731 | 728 | 725 | 722 | 720 | 717 | 714 | 711 |
| 48n |  | 708 | 705 | 702 | 699 | 696 | 693 | 690 | 687 | 684 | 681 |
| $49 n$ |  | 678 | 675 | 672 | 669 | 666 | 663 | 660 | 657 | 654 | 651 |
| 8.50 n |  | 648 | 645 | 642 | 639 | 636 | 633 | 629 | 626 | 623 | 620 |
| $5 \ln$ |  | 617 | 614 | 611 | 608 | 604 | 601 | 598 | 595 | 592 | 588 |
| 52 n |  | 585 | 582 | 579 | 576 | 572 | 569 | 566 | 563 | 559 | 556 |
| 53 n |  | 553 | 550 | 546 | 543 | 540 | 536 | 533 | 530 | 526 | 523 |
| 54 n |  | 520 | 516 | 513 | 510 | 506 | 503 | 499 | 496 | 493 | 489 |
| $55 n$ |  | 486 | 482 | 479 | 476 | 472 | 469 | 465 | 462 | 458 | 455 |
| $56 n$ |  | 451 | 448 | 444 | 441 | 437 | 434 | 430 | 426 | 423 | 419 |
| 57 n |  | 416 | 412 | 409 | 405 | 401 | 398 | 394 | 390 | 387 | 383 |
| 58 n |  | 380 | 376 | 372 | 368 | 365 | 361 | 357 | 354 | 350 | 346 |
| $59 n$ |  | 342 | 339 | 335 | 331 | 327 | 324 | 320 | 316 | 312 | 308 |
| 8.60 n |  | 305 | 301 | 297 | 293 | 289 | 285 | 281 | 278 | 274 | 270 |
| 61 n |  | 266 | 262 | 258 | 254 | 250 | 246 | 242 | 238 | 234 | 230 |
| 62 n |  | 226 | 222 | 218 | 214 | 210 | 206 | 202 | 198 | 194 | 190 |
| $63 n$ |  | 186 | 182 | 178 | 173 | 169 | 165 | 161 | 157 | 153 | 149 |
| $64 n$ |  | 144 | 140 | 136 | 132 | 128 | 123 | 119 | 115 | 111 | 106 |
| $65 n$ |  | 102 | 098 | 094 | 089 | 085 | 081 | 076 | 072 | 068 | 063 |
| 66 n |  | 059 | 055 | 050 | 046 | 041 | 037 | 033 | 028 | 024 | 019 |
| 67 n |  | 015 | 010 | 006 | 001 | 997 | 992 | 988 | 983 | 979 | 974 |
| 68 n | 9.97 | 970 | 965 | 960 | 956 | 951 | 947 | 942 | 937 | 933 | 928 |
| 69 n |  | 923 | 919 | 914 | 909 | 905 | 900 | 895 | 890 | 886 | 881 |
| $8.70 n$ | 9.97 | 876 | 871 | 867 | 862 | 857 | 852 | 847 | 842 | 838 | 833 |

TABLE XIV
Deflection of a Trial Line for Deviations from 1 to 149 Links

| $\sigma$ | $=\underset{\sim}{\sigma} \underset{\sim}{\infty} \underset{\sim}{m}$ が <br>  |
| :---: | :---: |
| $\infty$ |  <br>  |
| $\cdots$ |  <br>  |
| $\bigcirc$ |  |
| in |  －NoOザ |
| ＋ | ＝N゙ <br>  |
| m |  <br>  |
| $\sim$ |  <br>  |
| － |  <br> － $0+\infty$ m |
| － |  |
| n |  |

TABLE XV
Corrections in Links to Slope Measurements.


TABLE XV - Continued
Corrections in Links to Slope Measurements.


TABLE XV - Continued
Corrections in Links to Slope Measurements.

| Slope |  | $\begin{gathered} 1 \\ \mathrm{ch} \end{gathered}$ | $\stackrel{2}{\text { chs. }}$ | $\begin{gathered} 3 \\ \text { chs. } \end{gathered}$ | $\begin{gathered} 4 \\ \text { chs. } \end{gathered}$ | $\begin{array}{r} 5 \\ \text { chs. } \end{array}$ | $\begin{array}{r} 6 \\ \text { chs. } \end{array}$ | $\begin{gathered} 7 \\ \text { chs. } \end{gathered}$ | $\begin{gathered} 8 \\ \text { chs. } \end{gathered}$ | $\stackrel{9}{\text { chs. }}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $23$ |  |  |  |  |  |  |  |  |  |  |
|  | 04 | 8.0 | 16.0 | 24.0 | 32.0 | 40.0 | 48.0 | 56.0 | 64.0 | 72.0 |
|  | 13 | 8.1 | 16.2 | 24.3 | 32.4 | 40.5 | 48.6 | 56.7 | 64.8 | 72.9 |
|  | 22 | 8.2 | 16.4 | 24.6 | 32.8 | 41.0 | 49.2 | 57.4 | 65.6 | 73.8 |
|  | 31 | 8.3 | 16.6 | 24.9 | 33.2 | 41.5 | 49.8 | 58.1 | 66.4 | 74.7 |
|  | 39 | 8.4 | 16.8 | 25.2 | 33.6 | 42.0 | 50.4 | 58.8 | 67.2 | 75.6 |
|  | 48 | 8.5 | 17.0 | 25.5 | 34.0 | 42.5 | 51.0 | 59.5 | 68.0 | 76.5 |
|  | 56 | 8.6 | 17.2 | 25.8 | 34.4 | 43.0 | 51.6 | 60.2 | 68.8 | 77.4 |
| 24 | 05 | 8.7 | 17.4 | 26.1 | 34.8 | 43.5 | 52.2 | 60.9 | 69.6 | 78.3 |
|  | 13 | 8.8 | 17.6 | 26.4 | 35.2 | 44.0 | 52.8 | 61.6 | 70.4 | 79.2 |
|  | 21 | 8.9 | 17.8 | 26.7 | 35.6 | 44.5 | 53.4 | 62.3 | 71.2 | 80.1 |
| 25 | 30 | 9.0 | 18.0 | 27.0 | 36.0 | 45.0 | 54.0 | 63.0 | 72.0 | 81.0 |
|  | 38 | 9.1 | 18.2 | 27.3 | 36.4 | 45.5 | 54.6 | 63.7 | 72.8 | 81.9 |
|  | 46 | 9.2 | 18.4 | 27.6 | 36.8 | 46.0 | 55.2 | 64.4 | 73.6 | 82.8 |
|  | 54 | 9.3 | 18.6 | 27.9 | 37.2 | 46.5 | 55.8 | 65.1 | 74.4 | 83.7 |
|  | 03 | 9.4 | 18.8 | 28.2 | 37.6 | 47.0 | 56.4 | 65.8 | 75.2 | 84.6 |
|  | 11 | 9.5 | 19.0 | 28.5 | 38.0 | 47.5 | 57.0 | 66.5 | 76.0 | 85.5 |
|  | 19 | 9.6 | 19.2 | 28.8 | 38.4 | 48.0 | 57.6 | 67.2 | 76.8 | 86.4 |
|  | 27 | 9.7 | 19.4 | 29.1 | 38.8 | 48.5 | 58.2 | 67.9 | 77.6 | 87.3 |
|  | 35 | 9.8 | 19.6 | 29.4 | 39.2 | 49.0 | 58.8 | 68.6 | 78.4 | 88.2 |
|  | 43 | 9.9 | 19.8 | 29.7 | 39.6 | 49.5 | 59.4 | 69.3 | 79.2 | 89.1 |
| 26 | 51 | 10.0 | 20.0 | 30.0 | 40.0 | 50.0 | 60.0 | 70.0 | 80.0 | 90.0 |
|  | 58 | 10.1 | 20.2 | 30.3 | 40.4 | 50.5 | 60.6 | 70.7 | 80.8 | 90.9 |
|  | 06 | 10.2 | 20.4 | 30.6 | 40.8 | 51.0 | 61.2 | 71.4 | 81.6 | 91.8 |
|  | 14 | 10.3 | 20.6 | 30.9 | 41.2 | 51.5 | 61.8 | 72.1 | 82.4 | 92.7 |
|  | 22 | 10.4 | 20.8 | 31.2 | 41.6 | 52.0 | 62.4 | 72.8 | 83.2 | 93.6 |
| 27 | 29 | 10.5 | 21.0 | 31.5 | 42.0 | 52.5 | 63.0 | 73.5 | 84.0 | 94.5 |
|  | 37 | 10.6 | 21.2 | 31.8 | 42.4 | 53.0 | 63.6 | 74.2 | 84.8 | 95.4 |
|  | 45 | 10.7 | 21.4 | 32.1 | 42.8 | 53.5 | 64.2 | 74.9 | 85.6 | 96.3 |
|  | 52 | 10.8 | 21.6 | 32.4 | 43.2 | 54.0 | 64.8 | 75.6 | 86.4 | 97.2 |
|  | 00 | 10.9 | 21.8 | 32.7 | 43.6 | 54.5 | 65.4 | 76.3 | 87.2 | 98.1 |
|  | 08 | 11.0 | 22.0 | 33.0 | 44.0 | 55.0 | 66.0 | 77.0 | 88.0 | 99.0 |
|  | 15 | 11.1 | 22.2 | 33.3 | 44.4 | 55.5 | 66.6 | 77.7 | 88.8 | 99.9 |
|  | 23 | 11.2 | 22.4 | 33.6 | 44.8 | 56.0 | 67.2 | 78.4 | 89.6 | 100.8 |
|  | 30 | 11.3 | 22.6 | 33.9 | 45.2 | 56.5 | 67.8 | 79.1 | 90.4 | 101.7 |
|  | 38 | 11.4 | 22.8 | 34.2 | 45.6 | 57.0 | 68.4 | 79.8 | 91.2 | 102.6 |
| 28 | 45 | 11.5 | 23.0 | 34.5 | 46.0 | 57.5 | 69.0 | 80.5 | 92.0 | 103.5 |
|  | 52 | 11.6 | 23.2 | 34.8 | 46.4 | 58.0 | 69.6 | 81.2 | 92.8 | 104.4 |
|  | 00 | 11.7 | 23.4 | 35.1 | 46.8 | 58.5 | 70.2 | 81.9 | 93.6 | 105.3 |
|  | 07 | 11.8 | 23.6 | 35.4 | 47.2 | 59.0 | 70.8 | 82.6 | 94.4 | 106.2 |
|  | 14 | 11.9 | 23.8 | 35.7 | 47.6 | 59.5 | 71.4 | 83.3 | 95.2 | 107.1 |

TABLE XV-Continued.
Corrections in Links to Slope Measurements.

| Slope |  | $\begin{array}{r} 1 \\ \mathrm{ch} \end{array}$ | $\begin{gathered} 2 \\ \text { chs. } \end{gathered}$ | $\begin{gathered} 3 \\ \text { chs. } \end{gathered}$ | $\begin{gathered} 4 \\ \text { chs. } \end{gathered}$ | $\begin{gathered} 5 \\ \text { chs. } \end{gathered}$ | $\begin{gathered} 6 \\ \text { chs. } \end{gathered}$ | $\begin{gathered} 7 \\ \text { chs. } \end{gathered}$ | $\begin{gathered} 8 \\ \text { chs. } \end{gathered}$ | $\begin{gathered} 9 \\ \text { chs. } \end{gathered}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |  |
|  | 21 | 12.0 | 24.0 | 36.0 | 48.0 | 60.0 | 72.0 | 84.0 | 96.0 | 108.0 |
|  | 29 | 12.1 | 24.2 | 36.3 | 48.4 | 60.5 | 72.6 | 84.7 | 96.8 | 108.9 |
|  | 36 | 12.2 | 24.4 | 36.6 | 48.8 | 61.0 | 73.2 | 85.4 | 97.6 | 109.8 |
|  | 43 | 12.3 | 24.6 | 36.9 | 49.2 | 61.5 | 73.8 | 86.1 | 98.4 | 110.7 |
|  | 50 | 12.4 | 24.8 | 37.2 | 49.6 | 62.0 | 74.4 | 86.8 | 99.2 | 111.6 |
|  | 57 | 12.5 | 25.0 | 37.5 | 50.0 | 62.5 | 75.0 | 87.5 | 100.0 | 112.5 |
| 29 | 04 | 12.6 | 25.2 | 37.8 | 50.4 | 63.0 | 75.6 | 88.2 | 100.8 | 113.4 |
|  | 11 | 12.7 | 25.4 | 38.1 | 50.8 | 63.5 | 76.2 | 88.9 | 101.6 | 114.3 |
|  | 18 | 12.8 | 25.6 | 38.4 | 51.2 | 64.0 | 76.8 | 89.6 | 102.4 | 115.2 |
|  | 25 | 12.9 | 25.8 | 38.7 | 51.6 | 64.5 | 77.4 | 90.3 | 103.2 | 116.1 |
|  | 32 | 13.0 | 26.0 | 39.0 | 52.0 | 65.0 | 78.0 | 91.0 | 104.0 | 117.0 |
|  | 39 | 13.1 | 26.2 | 39.3 | 52.4 | 65.5 | 78.6 | 91.7 | 104.8 | 117.9 |
|  | 46 | 13.2 | 26.4 | 39.6 | 52.8 | 66.0 | 79.2 | 92.4 | 105.6 | 118.8 |
|  | 53 | 13.3 | 26.6 | 39.9 | 53.2 | 66.5 | 79.8 | 93.1 | 106.4 | 119.7 |
| 30 | 00 | 13.4 | 26.8 | 40.2 | 53.6 | 67.0 | 80.4 | 93.8 | 107.2 | 120.6 |
|  | 07 | 13.5 | 27.0 | 40.5 | 54.0 | 67.5 | 81.0 | 94.5 | 108.0 | 121.5 |
|  | 14 | 13.6 | 27.2 | 40.8 | 54.4 | 68.0 | 81.6 | 95.2 | 108.8 | 122.4 |
|  | 21 | 13.7 | 27.4 | 41.1 | 54.8 | 68.5 | 82.2 | 95.9 | 109.6 | 123.3 |
|  | 27 | 13.8 | 27.6 | 41.4 | 55.2 | 69.0 | 82.8 | 96.6 | 110.4 | 124.2 |
|  | 34 | 13.9 | 27.8 | 41.7 | 55.6 | 69.5 | 83.4 | 97.3 | 111.2 | 125.1 |
|  | 41 | 14.0 | 28.0 | 42.0 | 56.0 | 70.0 | 84.0 | 98.0 | 112.0 | 126.0 |
|  | 48 | 14.1 | 28.2 | 42.3 | 56.4 | 70.5 | 84.6 | 98.7 | 112.8 | 126.9 |
|  | 54 | 14.2 | 28.4 | 42.6 | 56.8 | 71.0 | 85.2 | 99.4 | 113.6 | 127.8 |
| 31 | 01 | 14.3 | 28.6 | 42.9 | 57.2 | 71.5 | 85.8 | 100.1 | 114.4 | 128.7 |
|  | 08 | 14.4 | 28.8 | 43.2 | 57.6 | 72.0 | 86.4 | 100.8 | 115.2 | 129.6 |
|  | 14 | 14.5 | 29.0 | 43.5 | 58.0 | 72.5 | 87.0 | 101.5 | 116.0 | 130.5 |
|  | 21 | 14.6 | 29.2 | 43.8 | 58.4 | 73.0 | 87.6 | 102.2 | 116.8 | 131.4 |
|  | 28 | 14.7 | 29.4 | 44.1 | 58.8 | 73.5 | 88.2 | 102.9 | 117.6 | 132.3 |
|  | 34 | 14.8 | 29.6 | 44.4 | 59.2 | 74.0 | 88.8 | 103.6 | 118.4 | 133.2 |
|  | 41 | 14.9 | 29.8 | 44.7 | 59.6 | 74.5 | 89.4 | 104.3 | 119.2 | 134.1 |
|  | 47 | 15.0 | 30.0 | 45.0 | 60.0 | 75.0 | 90.0 | 105.0 | 120.0 | 135.0 |
|  | 54 | 15.1 | 30.2 | 45.3 | 60.4 | 75.5 | 90.6 | 105.7 | 120.8 | 135.9 |
| 32 | 00 | 15.2 | 30.4 | 45.6 | 60.8 | 76.0 | 91.2 | 106.4 | 121.6 | 136.8 |
|  | 07 | 15.3 | 30.6 | 45.9 | 61.2 | 76.5 | 91.8 | 107.1 | 122.4 | 137.7 |
|  | 13 | 15.4 | 30.8 | 46.2 | 61.6 | 77.0 | 92.4 | 107.8 | 123.2 | 138.6 |
|  | 20 | 15.5 | 31.0 | 46.5 | 62.0 | 77.5 | 93.0 | 108.5 | 124.0 | 139.5 |
|  | 26 | 15.6 | 31.2 | 46.8 | 62.4 | 78.0 | 93.6 | 109.2 | 124.8 | 140.4 |
|  | 33 | 15.7 | 31.4 | 47.1 | 62.8 | 78.5 | 94.2 | 109.9 | 125.6 | 141.3 |
|  | 39 | 15.8 | 31.6 | 47.4 | 63.2 | 79.0 | 94.8 | 110.6 | 126.4 | 142.2 |
|  | 45 | 15.9 | 31.8 | 47.7 | 63.6 | 79.5 | 95.4 | 111.3 | 127.2 | 143.1 |

TABLE XV - Concluded.
Corrections in Links to Slope Measurements.

| Slope |  | $\begin{gathered} 1 \\ \mathrm{ch} \end{gathered}$ | $\begin{gathered} 2 \\ \text { chs. } \end{gathered}$ | $\begin{gathered} 3 \\ \text { chs. } \end{gathered}$ | $\begin{gathered} 4 \\ \text { chs. } \end{gathered}$ | $\begin{gathered} 5 \\ \text { chs. } \end{gathered}$ | $\begin{array}{r} 6 \\ \text { chs. } \end{array}$ | $\stackrel{7}{\text { chs. }}$ | $\begin{gathered} 8 \\ \text { chs. } \end{gathered}$ | $\stackrel{9}{\text { chs. }}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $32$ |  |  |  |  |  |  |  |  |  |  |
|  | 52 | 16.0 | 32.0 | 48.0 | 64.0 | 80.0 | 96.0 | 112.0 | 128.0 | 144.0 |
|  | 58 | 16.1 | 32.2 | 48.3 | 64.4 | 80.5 | 96.6 | 112.7 | 128.8 | 144.9 |
| 33 | 04 | 16.2 | 32.4 | 48.6 | 64.8 | 81.0 | 97.2 | 113.4 | 129.6 | 145.8 |
|  | 11 | 16.3 | 32.6 | 48.9 | 65.2 | 81.5 | 97.8 | 114.1 | 130.4 | 146.7 |
|  | 17 | 16.4 | 32.8 | 49.2 | 65.6 | 82.0 | 98.4 | 114.8 | 131.2 | 147.6 |
|  | 23 | 16.5 | 33.0 | 49.5 | 66.0 | 82.5 | 99.0 | 115.5 | 132.0 | 148.5 |
|  | 29 | 16.6 | 33.2 | 49.8 | 66.4 | 83.0 | 99.6 | 116.2 | 132.8 | 149.4 |
|  | 36 | 16.7 | 33.4 | 50.1 | 66.8 | 83.5 | 100.2 | 116.9 | 133.6 | 150.3 |
|  | 42 | 16.8 | 33.6 | 50.4 | 67.2 | 84.0 | 100.8 | 117.6 | 134.4 | 151.2 |
|  | 48 | 16.9 | 33.8 | 50.7 | 67.6 | 84.5 | 101.4 | 118.3 | 135.2 | 152.1 |
|  | 54 | 17.0 | 34.0 | 51.0 | 68.0 | 85.0 | 102.0 | 119.0 | 136.0 | 153.0 |
| 34 | 00 | 17.1 | 34.2 | 51.3 | 68.4 | 85.5 | 102.6 | 119.7 | 136.8 | 153.9 |
|  | 06 | 17.2 | 34.4 | 51.6 | 68.8 | 86.0 | 103.2 | 120.4 | 137.6 | 154.8 |
|  | 12 | 17.3 | 34.6 | 51.9 | 69.2 | 86.5 | 103.8 | 121.1 | 138.4 | 155.7 |
|  | 19 | 17.4 | 34.8 | 52.2 | 69.6 | 87.0 | 104.4 | 121.8 | 139.2 | 156.6 |
|  | 25 | 17.5 | 35.0 | 52.5 | 70.0 | 87.5 | 105.0 | 122.5 | 140.0 | 157.5 |
|  | 31 | 17.6 | 35.2 | 52.8 | 70.4 | 88.0 | 105.6 | 123.2 | 140.8 | 158.4 |
|  | 37 | 17.7 | 35.4 | 53.1 | 70.8 | 88.5 | 106.2 | 123.9 | 141.6 | 159.3 |
|  | 43 | 17.8 | 35.6 | 53.4 | 71.2 | 89.0 | 106.8 | 124.6 | 142.4 | 160.2 |
|  | 49 | 17.9 | 35.8 | 53.7 | 71.6 | 89.5 | 107.4 | 125.3 | 143.2 | 161.1 |
|  | 55 | 18.0 | 36.0 | 54.0 | 72.0 | 90.0 | 108.0 | 126.0 | 144.0 | 162.0 |
| 35 | 01 | 18.1 | 36.2 | 54.3 | 72.4 | 90.5 | 108.6 | 126.7 | 144.8 | 162.9 |
|  | 07 | 18.2 | 36.4 | 54.6 | 72.8 | 91.0 | 109.2 | 127.4 | 145.6 | 163.8 |
|  | 13 | 18.3 | 36.6 | 54.9 | 73.2 | 91.5 | 109.8 | 128.1 | 146.4 | 164.7 |
|  | 19 | 18.4 | 36.8 | 55.2 | 73.6 | 92.0 | 110.4 | 128.8 | 147.2 | 165.6 |
|  | 25 | 18.5 | 37.0 | 55.5 | 74.0 | 92.5 | 111.0 | 129.5 | 148.0 | 166.5 |
|  | 31 | 18.6 | 37.2 | 55.8 | 74.4 | 93.0 | 111.6 | 130.2 | 148.8 | 167.4 |
|  | 37 | 18.7 | 37.4 | 56.1 | 74.8 | 93.5 | 112.2 | 130.9 | 149.6 | 168.3 |
|  | 42 | 18.8 | 37.6 | 56.4 | 75.2 | 94.0 | 112.8 | 131.6 | 150.4 | 169.2 |
|  | 48 | 18.9 | 37.8 | 56.7 | 75.6 | 94.5 | 113.4 | 132.3 | 151.2 | 170.1 |
|  | 54 | 19.0 | 38.0 | 57.0 | 76.0 | 95.0 | 114.0 | 133.0 | 152.0 | 171.0 |
| 36 | 00 | 19.1 | 38.2 | 57.3 | 76.4 | 95.5 | 114.6 | 133.7 | 152.8 | 171.9 |
|  | 06 | 19.2 | 38.4 | 57.6 | 76.8 | 96.0 | 115.2 | 134.4 | 153.6 | 172.8 |
|  | 12 | 19.3 | 38.6 | 57.9 | 77.2 | 96.5 | 115.8 | 135.1 | 154.4 | 173.7 |
|  | 18 | 19.4 | 38.8 | 58.2 | 77.6 | 97.0 | 116.4 | 135.8 | 155.2 | 174.6 |
|  | 23 | 19.5 | 39.0 | 58.5 | 78.0 | 97.5 | 117.0 | 136.5 | 156.0 | 175.5 |
|  | 29 | 19.6 | 39.2 | 58.8 | 78.4 | 98.0 | 117.6 | 137.2 | 156.8 | 176.4 |
|  | 35 | 19.7 | 39.4 | 59.1 | 78.8 | 98.5 | 118.2 | 137.9 | 157.6 | 177.3 |
|  | 41 | 19.8 | 39.6 | 59.4 | 79.2 | 99.0 | 118.8 | 138.6 | 158.4 | 178.2 |
|  | 46 | 19.9 | 39.8 | 59.7 | 79.6 | 99.5 | 119.4 | 139.3 | 159.2 | 179.1 |
|  | 52 | 20.0 | 40.0 | 60.0 | 80.0 | 100.0 | 120.0 | 140.0 | 160.0 | 180.0 |

TABLE XVI

Table for Laying Out Roads One Chain Wide.

| Difference of Bearing |  |  |  | 告 | Difference of Bearing |  |  |  | 告 | Difference of Bearing |  |  |  | $\xrightarrow{\frac{n}{c}}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $360$ | $\dot{1}^{\prime} 0$ | 0 |  | 100 | $275$ | $35$ |  |  | 13 |  |  |  |  |  |
| 343 | 52 | 16 |  | 101 | 274 | 40 | 85 |  | 136 | 251 | 35 | 108 |  | 1 |
| 337 | 16 | 22 | 44 | 102 | 273 | 46 | 86 |  | 137 | 251 | 06 | 108 | 54 | 172 |
| 332 | 16 | 27 | 44 | 103 | 272 | 53 | 87 | 07 | 138 | 250 | 38 | 109 | 22 | 173 |
| 328 | 07 | 31 | 53 | 104 | 272 | 01 | 87 |  | 139 | 250 | 10 | 109 |  | 174 |
| 324 | 30 | 35 | 30 | 105 | 271 | 10 | 88 |  | 140 | 249 | 42 | 110 | 18 | 175 |
| 321 | 16 | 38 | 44 | 106 | 270 | 21 | 89 | 39 | 141 | 249 | 15 | 110 | 45 | 176 |
| 318 | 19 | 41 | 41 | 107 | 269 | 32 | 90 | 28 | 142 | 248 | 48 | 111 | 12 | 177 |
| 315 | 37 | 44 | 23 | 108 | 268 | 44 | 91 | 16 | 143 | 248 | 22 | 111 | 38 | 178 |
| 313 | 06 | 46 | 54 | 109 | 267 | 58 | 92 | 02 | 144 | 247 | 56 | 112 | 04 | 179 |
| 310 | 46 | 49 | 14 | 110 | 267 | 12 | 92 | 48 | 145 | 247 | 30 | 112 | 30 | 180 |
| 308 | 33 | 51 | 27 | 111 | 266 | 28 | 93 | 32 | 146 | 247 | 05 | 112 | 55 | 181 |
| 306 | 28 | 53 | 32 | 112 | 265 | 44 | 94 | 16 | 147 | 246 | 40 | 113 | 20 | 182 |
| 304 | 30 | 55 | 30 | 113 | 265 | 01 | 94 | 59 | 148 | 246 | 15 | 113 | 45 | 183 |
| 302 | 37 | 57 | 23 | 114 | 264 | 19 | 95 | 41 | 149 | 245 | 50 | 114 | 10 | 184 |
| 300 | 49 | 59 | 11 | 115 | 263 | 37 | 96 | 23 | 150 | 245 | 26 | 114 | 34 | 185 |
| 299 | 06 | 60 | 54 | 116 | 262 | 57 | 97 | 03 | 151 | 245 | 03 | 114 | 57 | 186 |
| 297 | 27 | 62 | 33 | 117 | 262 | 17 | 97 | 43 | 152 | 244 | 39 | 115 | 21 | 187 |
| 295 | 52 | 64 | 08 | 118 | 261 | 38 | 98 | 22 | 153 | 244 | 16 | 115 | 44 | 188 |
| 294 | 21 | 65 | 39 | 119 | 260 | 59 | 99 | 01 | 154 | 243 | 53 | 116 | 07 | 189 |
| 292 | 53 | 67 | 07 | 120 | 260 | 21 | 99 | 39 | 155 | 243 | 31 | 116 | 29 | 190 |
| 291 | 28 | 68 | 32 | 121 | 259 | 44 | 100 | 16 | 156 | 243 | 09 | 116 | 51 | 191 |
| 290 | 06 | 69 | 54 | 122 | 259 | 08 | 100 | 52 | 157 | 242 | 47 | 117 | 13 | 192 |
| 288 | 47 | 71 | 13 | 123 | 258 | 32 | 101 | 28 | 158 | 242 | 25 | 117 | 35 | 193 |
| 287 | 30 | 72 | 30 | 124 | 257 | 57 | 102 | 03 | 159 | 242 | 03 | 117 | 57 | 194 |
| 286 | 16 | 73 | 44 | 125 | 257 | 22 | 102 | 38 | 160 | 241 | 42 | 118 | 18 | 195 |
| 285 | 03 | 74 | 57 | 126 | 256 | 48 | 103 | 12 | 161 | 241 | 21 | 118 | 39 | 196 |
| 283 | 53 | 76 | 07 | 127 | 256 | 14 | 103 | 46 | 162 | 241 | 01 | 118 | 59 | 197 |
| 282 | 45 | 77 | 15 | 128 | 255 | 41 | 104 | 19 | 163 | 240 | 40 | 119 | 20 | 198 |
| 281 | 39 | 78 | 21 | 129 | 255 | 09 | 104 | 51 | 164 | 240 | 20 | 119 | 40 | 199 |
| 280 | 34 | 79 | 26 | 130 | 254 | 37 | 105 | 23 | 165 | 240 | 00 | 120 | 00 | 200 |
| 279 | 31 | 80 | 29 | 131 | 254 | 05 | 105 | 55 | 166 |  |  |  |  |  |
| 278 | 30 | 81 | 30 | 132 | 253 | 34 | 106 | 26 | 167 |  |  |  |  |  |
| 277 | 30 | 82 | 30 | 133 | 253 | 04 | 100 | 56 | 168 |  |  |  |  |  |
| 276 | 32 | 83 | 28 | 134 | 252 | 34 | 107 | 26 | 169 |  |  |  |  |  |

To Convert Time Into Arc

Hours of Time into Arc

| Time | Arc | Time | Arc | Time | Arc | Time | Arc | Time | Arc | Time | Arc |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| hrs. | 0 | hrs. | 0 | hrs. | 0 | hrs. | 0 | hrs. | 0 | hrs. | $\circ$ |
| 1 | 15 | 5 | 75 | 9 | 135 | 13 | 195 | 17 | 255 | 21 | 315 |
| 2 | 30 | 6 | 90 | 10 | 150 | 14 | 210 | 18 | 270 | 22 | 330 |
| 3 | 45 | 7 | 105 | 11 | 165 | 15 | 225 | 19 | 285 | 23 | 345 |
| 4 | 60 | 8 | 120 | 12 | 180 | 16 | 240 | 20 | 300 | 24 | 360 |

Minutes of Time into Arc
Seconds of Time into Arc

| m. | - ' | m. |  | m. | - 1 | s. | 1 " | s. | ' ${ }^{\prime}$ | s. | 1 ' |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | $\begin{array}{ll}0 & 15\end{array}$ | 21 | 515 | 41 | $10 \quad 15$ | 1 | 015 | 21 | 515 | 41 | 1015 |
| 2 | 030 | 22 | 530 | 42 | $10 \quad 30$ | 2 | 030 | 22 | 530 | 42 | 1030 |
| 3 | 045 | 23 | 545 | 43 | 1045 | 3 | 045 | 23 | 545 | 43 | 1045 |
| 4 |  | 24 |  | 44 | 110 | 4 | 10 | 24 | 6 | 44 | 11 |
| 5 | 115 | 25 | 615 | 45 | 1115 | 5 | 115 | 25 | 615 | 45 | 1115 |
| 6 | 130 | 26 | 630 | 46 | 1130 | 6 | 130 | 26 | 630 | 46 | 1130 |
| 7 | 145 | 27 | 645 | 47 | 1145 | 7 | 145 | 27 | 645 | 47 | 1145 |
| 8 | 20 | 28 | 70 | 48 | 120 | 8 | 20 | 28 | 70 | 48 | 12 |
| 9 | 215 | 29 | $7 \quad 15$ | 49 | 1215 | 9 | 215 | 29 | 715 | 49 | 12. 15 |
| 10 | 230 | 30 | 730 | 50 | 1230 | 10 | 230 | 30 | 30 | 50 | 1230 |
| 11 | 245 | 31 | 745 | 51 | 1245 | 11 | 245 | 31 | 745 | 51 | 1245 |
| 12 | $3 \begin{array}{ll}3 & 0\end{array}$ | 32 | 80 | 52 | 130 | 12 | 30 | 32 | 80 | 52 | 13 |
| 13 | $\begin{array}{lll}3 & 15\end{array}$ | 33 | 815 | 53 | 1315 | 13 | $\begin{array}{ll}3 & 15\end{array}$ | 33 | 815 | 53 | 1315 |
| 14 | 330 | 34 | 830 | 54 | 1330 | 14 | 330 | 34 | 830 | 54 | 1330 |
| 15 | 345 | 35 | 845 | 55 | 1345 | 15 | 345 | 35 | 845 | 55 | 1345 |
| 16 |  | 36 | 90 | 56 | 140 | 16 | 40 | 36 | 90 | 56 | 14 |
| 17 | 415 | 37 | 915 | 57 | 1415 | 17 |  | 37 | 915 | 57 | 14 |
| 18 | 430 | 38 | 930 | 58 | 1430 | 18 | 430 | 38 | 930 | 58 | 1430 |
| 19 | 445 | 39 | 945 | 59 | 1445 | 19 | 445 | 39 | 945 | 59 | 1445 |
| 20 |  | 40 | $10 \quad 0$ | 60 | 150 | 20 | 50 | 40 | $10 \quad 0$ | 60 | 15 |

TABLE XVIII

Conversion of Mean Time Interval To The Equivalent Siderial Time Interval
(Add listed correction to Mean Time Interval)

| Days | Add <br> m. | s. | Hours | Add <br> m. | s. |  | Minutes |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: | | Add |
| :---: |
|  |



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622
C353
1913
Supp 7 .

Canada. Topographical Survey Manual of instructions for the survey of dominion lands 8th ed.

Forestry

PLEASE DO NOT REMOVE
CARDS OR SLIPS FROM THIS POCKET


[^0]:    * The formula may be deduced as follows:

